



ALFIX MODUL MULTI

[Seal: Deutsches Institut für Bautechnik]

Notification

of supplement the national technical approval / general construction technique permit of 15 October 2021 An institution under public law jointly funded by the German Federation and the federal states (Länder)

Approval Body for Construction Products & Techniques

 Date:
 Reference number:

 31 January 2023
 | 37.1-1.8.22-56/22

Approval number: **Z-8.22-906**

Period of validity: from: **31 January 2023** to: **15 October 2026**

Applicant: Alfix GmbH Langhennersdorfer Straße 15 09603 Großschirma (Germany)

Subject of approval: **Scaffolding components of the "ALFIX MODUL MULTI" modular scaffolding system**

This notification supplements the national technical approval / general construction technique permit no. Z-8.22-906 of 15 October 2021.

This national technical approval includes two pages and an annex.

It shall only be valid in connection with above mentioned national technical approval / general construction technique permit and shall only be used in conjuction with it.

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II SPECIAL PROVISIONS

The special provisions of the national technical approval / general construction technique permit are supplemented as follows:

a) Table 1 is supplemented as follow:

Table 1: Scaffolding components for the modular scaffolding system "ALFIX MODUL MULTI"

Designation	Annex B, page	Details / components in accordance with Annex B, page
Lift-off preventer 0.37 – 3.07 m, steel	160	

REGARDING ANNEX B:

b) In Annex B, page 160 will be added.

REGARDING ANNEX C:

c) Table C.1 is supplemented as follow:

 Table C.1:
 Components of the standard assembly configuration SW06 / LC3

Designation	Annex B, page
Lift-off preventer 0.37 – 0.73 m, steel	160

REGARDING ANNEX E:

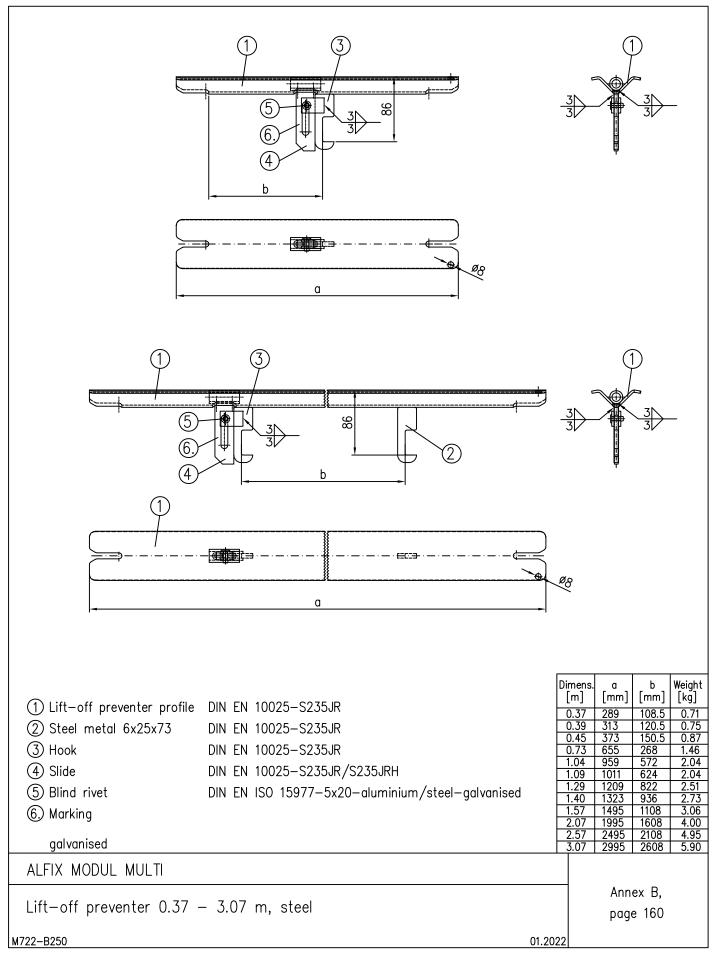
d) Table E.1 is supplemented as follow:

 Table E.1:
 Components of the standard assembly configuration SW09 / LC4

Designation	Annex B, page
Lift-off preventer 0.37 – 1.09 m, steel	160

Andreas Schult Head of Division

Certified Dr.-Ing. Gilow-Schiller



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National technical approval / general construction technique permit

An institution under public law jointly funded by the German Federation and the federal states (Länder)

Approval Body for Construction Products & Techniques

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Approval number: **Z-8.22-906**

Period of validity: from: **15 October 2021** to: **15 October 2026**

Applicant: Alfix GmbH Langhennersdorfer Straße 15 09603 Großschirma (Germany)

Subject of approval: Scaffolding components of the "ALFIX MODUL MULTI" modular scaffolding system

The above-mentioned subject is hereby granted general construction technique permit. This decision comprises 33 pages as well as Annex A (pages 1 to 3), Annex B (pages 1 to 159), Annex C (pages 1 to 5), Annex D (pages 1 to 8), Annex E (pages 1 to 5), and Annex F (pages 1 to 8).

The subject was first granted general construction technique permit on 21 January 2006.

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I GENERAL PROVISIONS

- 1 This general construction technique permit serves to demonstrate the usability or the applicability of the subject matter of the permit as defined by the Building Codes of the federal states.
- 2 This decision does not replace statutory approvals, authorisations and certifications specified for carrying out construction works.
- 3 This decision is granted without prejudice to the rights of third parties, especially private property rights.
- 4 The user of the subject matter of the decision must, without prejudice to further regulations laid out under "Special Provisions", make copies of this decision available to the persons using or applying the subject matter of the decision. The user of the subject matter shall also be informed that the decision must be present at the place of use. Upon request, copies must be provided to the relevant authorities.
- 5 This decision may only be reproduced in its entirety. Publication of the approval in excerpts requires the prior consent of the Deutsches Institut für Bautechnik (DIBt). Text and drawings of promotional material must be consistent with this decision. Translations must include the following note: "Translation of the original German version not reviewed by Deutsches Institut für Bautechnik".
- 6 This decision is issued in a revocable manner. The provisions of this decision may be amended or modified at a later time, particularly if new technical knowledge requires this.
- 7 This decision relates to the information on the subject matter of the permit made available by the applicant during the approval process and the documents submitted. Any change made to these permit bases is not covered by this decision and must be disclosed to the Deutsches Institut für Bautechnik without delay.

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SPECIAL PROVISONS Ш

1 Subject matter of the permit and scope of application

1.1 Subject matter of the approval and scope of application

Subject matter of the approval are prefabricated scaffolding components in accordance with Table 1 as well as scaffolding components using components according to Table 2 for use in the modular scaffolding system "ALFIX MODUL MULTI".

1.2 Subject matter of the permit and scope of application

Subject matter of the permit are the planning, dimensioning and execution of the modular scaffolding system "ALFIX MODUL MULTI", which consists of

- scaffolding components according to Table 1
- scaffolding components according to Table 4 and
- scaffolding components in accordance with MVV TB (Model Administrative Provisions -Technical Building Rules), Part C 2.16 according to the respective scope of application.

Other scaffolding components, that are constructed in accordance with Section 2.1.3 of this decision using components according to Table 2, may be used in the modular scaffolding system as well.

The modular scaffolding system is erected using standards, ledgers, diagonal braces and decks as basic components as well as scaffolding spindles (base jacks), scaffold retainers / wall ties, system components for the side protection, access components and supplementary components. The standards, ledgers and diagonal braces are connected to each other by means of special scaffolding (module) nodes. Depending on the scaffolding structure, there are multiple scaffolding (module) nodes which can be combined with each other in two different load-bearing groups for the ledger connection according to Table 5.

The scaffolding nodes are constructed using a connector disc (rosette) welded onto a standard tube and connector heads which are welded onto a U- or tube-ledger or which are connected to vertical diagonal braces by means of a simple joint. The connector heads enclose the connector disc and are wedged to the connector disc by driving the wedge into its housing with a hammer blow in a way that the connector heads are pressed against the standard tube. The horizontal diagonal braces are connected to the connector disc by fitting a bolt into one of its openings.

Each connector disc (rosette) has 8 openings, allowing to connect up to 8 members.

The modular scaffolding system ALFIX MODUL MULTI may be used as service and working scaffolding according to DIN EN 12811-1:2004-03 in connection with the "Application Guideline for working scaffolds" in accordance with DIN EN 12811-1"1 and DIN 4420-1:2004-03, as supporting framework in accordance with DIN EN 12812:2008-12 in connection with the "Application Guideline for supporting frameworks" in accordance with DIN EN 12812"² or as any other temporary structure.

2 Provisions for scaffolding components

2.1 **Properties**

2.1.1 **General Provisions**

The scaffolding components according to Table 1 must comply with the provisions of Annex B, the provisions in the documents filed at the Deutsches Institut für Bautechnik (DIBt) as well as the regulations of the sections below.

1 see DIBt-Mitteilungen (notifications of the DIBt), issue 2/2006, p. 61 et seq. 2

see DIBt-Mitteilungen (notifications of the DIBt), issue 6/2009, p. 227 et seq.

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Designation	Annex B, page	Details / components in accordance with Annxe B, page
Vertical diagonal braces	8	3, 6
Horizontal diagonal braces	9	7
Vertical starter piece	10	2
Standard with tube connector 200	11	2
Standard with screwed-in tube connector 520	12	2
Standard with screwed-in tube connector 500	13	2
Standard with screwed-in tube connector 520, s=4.05 mm	14	2, 12
Vertical starter standard	15	2
Top standard	16	2
U-head jack	21	
Spindle coupler	22	
Suspended scaffold connector	23	3, 4, 152
Locking device for base jack	24	3, 4, 152
Tube ledger	25	3, 4
Horizontal diagonal ledger	26	3, 4
Tube ledger, reinforced	27	3, 4
Double tube ledger 1.57 m	28	3, 4
Double tube ledger 2.07 m	29	3, 4, 28
Double tube ledger 2.57 m	30	3, 4, 28
Double tube ledger 3.07 m	31	3, 4, 28
U-ledger 0.37 m; 0.39 m; 0.45 m; 0.73 m	32	3, 5, 153
Support ledger with tube fixture	40	3
Support ledger	43	3, 32
U-transom lattice girder 0.73 m/ 1.09 m V	44	27, 32, 127
Tube-transom lattice girder 0.73 m/ 1.09 m V	45	27, 144
MODUL lattice girder 6.14 m	46	3, 4, 152
MODUL lattice girder 4.14 m/ 5.14 m	47	3, 4, 46, 152
MODUL lattice girder with tube fixture 6.14 m	48	3, 4, 46, 152
MODUL lattice girder with tube fixture 4.14 m/ 5.14 m	49	3, 4, 46, 48, 152
MODUL lift-off preventer	50	
Aluminium frame platform with tube fixture 1.57 m; 2.07 m	51	53
Aluminium frame platform with tube fixture 2.57 m; 3.07 m	52	53
Aluminium access frame platform with tube fixture 3.07 m	54	53, 56, 60
Aluminium access frame platform with tube fixture 2.57 m	55	53, 56, 60
Aluminium access frame platform with tube fixture 1.57 m – 3.07 m without ladder	57	53, 56
Aluminium access frame platform with tube fixture 2.57 m; 3.07 m with aluminium chequer plate	58	59, 60
Steel deck AF with tube fixture 0.32 m	61	
Steel deck AF with tube fixture 0.30 m; 0.34 m	62	

Table 1: Scaffolding components for the modular scaffolding system "ALFIX MODUL MULTI"

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(continued)

		Details /
Designation	Annex B,	components in
Dooighadon	page	accordance with
		Annxe B, page
Intermediate deck AF with tube fixture 0.16 m; 0.19 m	63	
Intermediate deck with tube fixture	65	
MODUL gap cover	94	
MODUL gap cover with tube fixture	95	
Stair guardrail 2.57 m, 3.07 m	98	3
MODUL stair guardrail holder	101	3, 4, 152
MODUL swing gate	102	3, 139
Bracket 0.39 with tube fixture	103	3, 4, 152
MODUL bracket 0.39 m	104	3, 5, 32, 153
MODUL bracket 0.73 m	105	3, 5, 32, 153
Bracket 0.50 m with tube fixture	106	3, 4, 152
MODUL toeboard	107	
MODUL toeboard 4.14 m	108	
MODUL toeboard, aluminium	109	
MODUL guard net system	116	3, 4, 25, 152, 156
MODUL double end guardrail	117	3, 4, 152
Wedge head coupler, swivel base	122	3, 139
MODUL U-tube connector	123	
MODUL tube connector	124	3
Wedge head coupler, rigid	125	3, 4, 152
Support ledger	126	3, 4, 152
MODUL advanced guardrail post	137	
AB U-head jack	140	
		3, 4, 5, 32, 147,
MODUL U-lattice girder 6.14 m, 7.71 m	141	152, 153
MODUL U-lattice girder 4.14 m; 5.14 m	142	3, 4, 5, 32, 141, 147, 152, 153
Claw coupler	143	
Tube ledger, reinforced, 1.09 m; 1.29 m; 1.40 m	144	3, 4, 152
Tube ledger, reinforced, 1.57 m; 2.07 m	145	3, 4, 144, 152
Tube ledger, reinforced, 2.57 m, 3.07 m	146	3, 4, 144, 152
U-ledger 1.04 m, 1.09 m, 1.29 m	147	3, 5, 153
U-ledger with integrated main beam, 1.40 m-2.07 m	148	3, 5, 153
U-ledger 1.40 m-2.57 m, reinforced	149	3, 5, 144, 147, 153
U-ledger 3.07 m, reinforced	150	3, 5, 144, 147, 149, 153
Standard 4.0	154	2
Vertical starter piece 4.0	155	2
Tube ledger 4.0	156	3, 152
Horizontal diagonal ledger 4.0	157	3, 152
MODUL gap cover, T-shaped and claw coupler, universal design	158	
Standard with tube connector 200 45/ 5	159	2
Standard With tube connector 200 45/ 5	109	۷.

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2.1.2 Components of the scaffolding nodes

The components of the scaffolding nodes according to Table 2 used for some of the scaffolding components must comply with the provisions of Annex B, the provisions in the documents filed at the Deutsches Institut für Bautechnik (DIBt) as well as the regulations of the Sections below.

Table 2: Components of the scaffolding nodes

Designation	Annex B, page
Connector disc	2
Wedge	3
Connection of tube ledger	4
Connection of U-ledger	5
Connection of V-diagonal brace	6
Connection of H-diagonal brace	7
U-ledger head PLUS, new connection	139
Connection of tube ledger 4.0	152
Connection of U-ledger 4.0	153

2.1.3 Other scaffolding components that are constructed using components according to Table 2

Other scaffolding components that are constructed using members according to Table 2 in accordance with Section 2.2.1.2 according to this decision must comply with the following sections of this decision. With the exception of the connection between the individual members, these components must fully comply with the Technical Building Regulations (Technische Baubestimmungen) and must meet all other requirements in accordance with the "Approval principles for working and service scaffolds, requirements, calculation, tests and proof of conformity"³.

2.1.4 Materials

2.1.4.1 Metals

Metal materials must comply with the technical rules according to Table 3. Their properties must be confirmed by means of a material test certificate in accordance with Table 3. Material test certificates for aluminium alloys must contain at least information on the chemical composition, tensile strength R_m , yield point a $R_{p0,2}$ and strain A bzw. A_{50mm} .

Components for which the material specifications are stored at the Deutsches Institut für Bautechnik, the properties shall be confirmed by means of the following material test certificates:

- For structural steel with an increased yield strength and with a defined minimum yield stength of ≤ 275 N/mm² a test report 2.2 issued by the factory is sufficient.
- For all other metal materials, an inspection report 3.1 is mandatory.

Table 3:Technical regulations and material test certificates for the metal materials of
the individual and scaffolding components

Material	Material number	Designation	Technical regulation	Material test certificate according to DIN 10204:2005-01
Scaffolding nodes	filed at the Deutsches Institut für Bautechnik (DIBt)			3.1
	1.0039	S235JRH *)		2.2 ^{*)}
Structural steel	1.0576	S355J2H	DIN EN 10219-1: 2006-07	3.1
	1.8849	S460MH	2000-07	J. I

3

To be obtained from the Deutsches Institut für Bautechnik (the German technical authority and service provider for the construction sector).

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<u>Table 3:</u>	(continued)
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Material	Material number	Designation	Technical regulation	Material test certificate according to DIN 10204:2005-01
	1.0038	S235JR		2.2
Structural steel	1.0122	S235JRC+C	DIN EN 10025-2: 2019-10	2.2
	1.0577	S355J2	2013-10	
Bright steel	1.0122	S235JRC+C	DIN EN 10277-2: 2008-06	
Precision steel tube	1.0308	E235+C	DIN EN 10305-3: 2016-08	
	1.0332	DD11 **)	DIN EN 10111:	
Strip and sheet	1.0398	DD12 **)	2008-06	
metal	1.0917	DX51D	DIN EN 10346: 2015-10	
	5.4201 (EN-JM1020)	EN-GJMW- 360-12		
Malleable cast iron	5.4202 (EN-JM1030)	EN-GJMW- 400-5	DIN EN 1562: 2019-06	
	5.4205 (EN-JM1140)	EN-GJMB- 450-6		
Cast iron	5.3106 (EN-JS1030)	EN-GJS- 400-15	DIN EN 1563: 2019-04	3.1
Cast steel	1.6220	G20Mn5	DIN EN 10293:	
Casi sieei	1.0446	GE240+N	2015-04	
Flats	1.0976	S355MC	DIN EN 10149-2:	
1 1013	1.0982	S460MC	2013-12	
	EN AW-5083 H114 / H224	EN AW-AI Mg4,5Mn0,7	DIN EN 1386:	
Aluminium alloy	EN AW-5754 H111 / H114	EN AW- AlMg3	2008-05	
	EN AW-6060 T66	EN AW- AlMgSi	DIN EN 755-2:	
*)	EN AW-6063 T66	EN AW- AlMg0,7Si	2016-10	

*) For some scaffolding components, a higher yield strength ReH ≥ 280N/mm² or ReH ≥ 320 N/mm² has been determined. These components have been marked accordingly in the drawings in Annex A. The proportional strain at fracture A may not be lower than 15 %. For a wall thickness of < 3 mm, the proportional strain at fracture of A80mm shall be determined. The conversion of A80mm to A shall be done in accordance with DIN EN ISO 2566-1. The values of the yield strength, the strain at fracture and the tensile strength shall be confirmed by means of an inspection certificate 3.1 in accordance with DIN EN 10204:2005-01. The purchase requisition regarding the increased yield strength shall be indicated in the inspection certificate 3.1 as a desired value.</p>

**) R_{eH} and R_m in accordance with Annex B

2.1.4.2 Extruded sections

Extruded sections shall meet the requirements of the EN 755 standards.

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2.1.4.3 Solid wood

The solid wood of the toeboards shall meet at least the requirements of visual strength grade S10 in accordance with DIN 4074-1:2012-06 or the minimum strength of strength class C24 in accordance with DIN EN 338:2016-07.

2.1.4.4 Constructional veneer plywood

Constructional veneer plywood shall meet at least the requirements of the "Approval Principles for the use of constructional veneer plywood in scaffolding"⁴ as well as the provisions in the drawings of Annex B.

2.1.5 Corrosion protection

The technical building regulations shall apply.

2.1.6 Couplers

Class B halfcouplers in accordance with DIN EN 74-2:2009-01 shall are used as halfcouplers that are attached to various components.

2.2 Manufacturing and marking

2.2.1 Manufacturing

2.2.1.1 Manufacturer's qualifications

Companies that manufacture welded scaffolding components in accordance with the present National Technical Approval shall demonstrate that they are qualified for this task.

For steel components, this proof shall be considered to be furnished, if welding procedures and welding personnel are qualified in accordance with DIN EN 1090-2:2018-09 and the company holds a welding certificate of at least exectution class 2 (EXC 2) in accordance with DIN EN 1090-1:2012-02.

For aluminium components, this proof shall be considered to be furnished, if welding procedures and welding personnel are qualified in accordance with DIN EN 1090-2:2019-07 and the company holds a welding certificate of at least execution class 2 (EXC 2) in accordance with DIN EN 1090-1:2012-02.

2.2.1.2 Manufacturing further scaffolding components using components according to Table 2

Any other scaffolding components manufactured using components according to Table 2 shall be manufactured as follows:

- Connector discs (rosettes) according to Annex B, page 2 shall be welded to scaffold tubes Ø 48.3x3.2 mm of steel grade S235JRH with ReH ≥ 320 N/mm² according to DIN EN 10219-1:2006-07 with a welding seam in accordance with the specifications documented at DIBt.
- Connector heads for tube ledgers according to Annex B, page 4 shall be welded to scaffold tubes Ø 48.3 x 3.2 mm of steel grade S235JRH with ReH ≥ 320 N/mm² according to DIN EN 10219-1:2006-07 with a welding seam in accordance with the specifications documented at DIBt.
- Connector heads for U-ledgers according to Annex B, page 5 shall be welded to U-profiles *48x52x2.5 mm* of steel grade S235JR according to DIN EN 10025-2:2019-10 with a welding seam in accordance with the specifications documented at DIBt.
- Connector heads for tube ledgers 4.0 according to Annex B, page 152 shall be welded to scaffold tubes Ø 48.3 x 2.7 mm of steel grade S460MH according to DIN EN 10219-1:2006-07 with a welding seam in accordance with the specifications documented at DIBt.
- Connector heads for U-ledgers 4.0 according to Annex B, page 153 shall be welded to U-profiles *48x52x2.5 mm* of steel grade S460MC according to DIN EN 10149-2:2013-12 with a welding seam in accordance with the specifications documented at DIBt.

4

Please also refer to the DIBt notifications "Mitteilungen, Deutsches Institut für Bautechnik", issue 3/1999, p. 122 et seq.

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2.2.2 Marking

The delivery notes for scaffolding components according to Section 2.1 shall be marked in accordance with the regulations for the mark of conformity of the federal states (Länder). In addition, scaffolding components shall be permanently and easily recognisably marked with

- the uppercase letter "Ü",
- at least the abbreviated approval number "906",
- the identifying mark (logo) of the manufacturer concerned,
- and the last two numbers of the year of manufacture.

Alternatively, a coded identifying mark in accordance with Annex B, page 151, may be used. These identifying marks may only be applied if the requirements under Section 2.3 are fulfilled.

2.3 Attestation of conformity

2.3.1 General Provisions

Attestation of conformity of the scaffolding components according to Section 2.1 with the provisions of the National Technical Approval must be provided for each production site by a declaration of conformity on the basis of factory production controls and a certificate of conformity issued by a recognised certification body as well as regular external supervision, including a product test of scaffolding components and their components in accordance with the provisions below by a recognised inspection body.

The manufacturer of the scaffolding components must involve a recognised certification body as well as a recognised inspection body to obtain a certificate of conformity and to carry out the external supervision, including the product tests.

The declaration that a certificate of conformity has been issued must be indicated by the manufacturer by marking the scaffolding components with the mark of conformity (Ü mark) with reference to the intended use.

The certifying body shall provide Deutsches Institut für Bautechnik (DIBt) with a copy of the certificate of conformity issued by the former, and the supervisory body shall provide it with a copy of the supervision report upon request.

DIBt shall be given a copy of the initial test report upon request of the same.

2.3.2 Factory production control

A factory production control system must be set up and operated at each production site. Factory production control is to be understood as a continuous monitoring of production to be carried out by the manufacturer, by means of which the manufacturer ensures that the components and scaffolding components manufactured are in compliance with the provisions of this National Technical Approval.

The factory production control must include at least the following measures:

Components in accordance with Table 2:

- In the case of template or automatic production of scaffolding components, the respective templates and / or machine settings shall be checked and documented before commissioning.

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- Component and material checks and inspections:
 - It shall be checked whether inspection certificates as per Section 2.1.2 are available for the materials and that the attested inspection results meet the requirements.
 - By examining at least 10 individual parts per production batch, comprising at least 1 individual part from every 10,000 parts of the components according to Table 2, conformity of the basic dimensions and angles with the documentation available at DIBt must be checked. The actual dimensions shall be documented.
 - The connecting heads are to be checked for cracks.
 - The scaffold tubes Ø 48.3 x 2.9 mm and Ø 48.3 x 2.7 mm made of the material S460MH are to be checked for the stricter requirements compared to DIN EN 10219-1:2006-07 in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt).
- Checks to be conducted on the scaffolding nodes:
 - Scaffolding connectors shall be checked according to the inspection plan filed at DIBt.

Scaffolding components according to Table 1 and scaffolding components according to Section 2.1.3:

- Checks and inspections on the starting material:
 - It shall be checked whether inspection certificates as per Section 2.1.4 are available for the materials and that the attested inspection results meet the requirements.
 - At least 1‰ of the components shall be checked for conformity with dimensions and tolerances as specified in the design drawings.
- Checks and inspections on scaffolding components:
 - At least 1‰ of the scaffolding components shall be checked for conformity with dimensions and tolerances and, if necessary, welding seams and corrosion protection, as specified in the design drawings.
 - On at least 0,1‰ of the pressed-in tube connectors of the standards according to Annex B, pages 11 and 15, however at least once every production week, a tensile test on non-galvanized members is to be conducted. The breaking load value (BL) must not be lower than 13.75 kN.
 - Inspections are to be carried out on at least 0.1 ‰ of the integrated tube connectors of the standards according to Annex B, pages 154, 155 and 159, however at least once every production week, in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt).
 - Inspections are to be carried out on at least 0.1 ‰ of the riveted connector heads of the vertical diagonal braces in accordance with the documents filed with the Deutsches Institut f
 ür Bautechnik (DIBt).

Documentation:

The results of the factory production control shall be recorded and evaluated. The records must contain at least the following information:

- Designation of the scaffolding components
- Type of inspection
- Date of manufacturing and inspection of the scaffolding components
- Result of the production controls and inspections and comparison with requirements
- Signature of the person responsible for the factory production controls.

The records shall be kept for at least five years and shall be made available to the external supervisory body in charge of the external supervision. Upon request, these records must be presented to DIBt and to the competent superior building inspection authority.

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Measures in case the inspection results are unsatisfactory:

If inspection results are unsatisfactory, the manufacturer must immediately take corrective actions. Scaffolding components or components that do not meet the requirements must be handled in such a way that they cannot become confused with conforming parts. After the corrective actions, the inspection/test concerned must be repeated immediately, provided this is technically possible and necessary to prove that the defect has been rectified.

2.3.3 External supervision

At each manufacturing site, the factory production control shall be inspected by an external supervisory body on a regular basis: at least twice a year for the components in accordance with Table 2, and including the riveting of the connections of the diagonal braces and the integrated tube connectors according to Annex B, pages 154, 155 and 159 as well as once every five years for the scaffolding components in accordance with Table 1. External supervision includes an inspection of the factory and the factory production control system, including a product inspection. Sampling and inspections/tests shall be the responsibility of the recognised body.

The initial inspection of scaffolding components according to section 2.1.3 may be carried out by the manufacturer, when the scaffold components belong to a product group, for which the initial inspection was carried out by a recognized body.

At least the following inspections/tests are to be carried out:

- Inspection of the requirements in terms of personnel and equipment for proper manufacturing of the scaffolding components and components
- Inspection of the factory production control system
- Checks on random samples for conformity of scaffolding components and components with the provisions of the approval in terms of:
- construction type, form and dimensions
- Corrosion protection
- Marking
- Verification of the required welding certificate
- For each inspection, the scaffold tubes Ø 48.3 x 2.9 mm and Ø 48.3 x 2.7 mm made of the material S460MH are to be checked for the stricter requirements compared to DIN EN 10219-1:2006-07 in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt).
- Conformity with the dimensions and angles of at least five individual parts each of the scaffolding connector filed in the documents with DIBt are to be inspected and compared with the allowed tolerances.
- The scaffolding connectors shall be checked according to the inspection plan filed at DIBt.
- Per inspection, at least 5 pressed-in tube connector tests are to be carried out accordance with the provisions of Section 2.3.2.
- External supervision shall comprise inspections of at least 5 integrated tube connectors of the standards according to Annex B, pages 154, 155 and 159 in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt).
- The riveted connector heads of the vertical diagonal braces according to Annex B, page 8 are to be inspected by the external supervisory body in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt).

The scaffolding components and components shall be drawn from current production.

The results of the certification and external supervision shall be kept for at least five years. Upon request, they must be presented to DIBt and to the competent superior building inspection authority by the certification body and/or supervisory body.

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3 Provisions for planning, dimensioning and execution

3.1 Planning

3.1.1 General Provisions

The Technical Building Rules [Technische Baubestimmungen], in particular DIN EN 12812:2008-12 as well as the provisions stated hereinafter shall apply for the planning of scaffoldings using components of the modular scaffolding system "ALFIX MODUL MULTI". The scaffolds shall be planned in accordance with engineering standards. Verifiable calculations shall be made in accordance with the technical regulations and the construction drawings.

The modular scaffolding system "ALFIX MODUL MULTI" consists of scaffolding components in accordance with section 1. Scaffolding components in accordance with Table 4, which refer to provisions for manufacturing, marking and mark of conformity set out in this decision, are no longer manufactured and are therefore only approved for continued use.

Any differences in the design of the individual ledger connection variants can be taken

- from Annex B, pages 3 to 5 for the former design and
- Annex B, pages 152 and 153 for design 4.0

The modular disc (rosette) according to Annex B, page 2 and the diagonal brace connections according to Annex B, pages 6 and 7 are only available as shown in the respective pages of the Annex.

Depending on the configuration used, different load-bearing groups for the ledger connection according to Table 5 apply.

Table 4:Further scaffolding components for use in the modular scaffolding system "ALFIX
MODUL MULTI"

Designation	Annex B, page	Details / components in accordance with Annex B, page	Regulations for manufacturing, marking and certificate of conformity
Base jack	17		
AB Base jack	18		according to
Base jack, with swivel base	19		Z-8.1-862
Base jack with swivel base	20		
U-ledger 1.09 m, 1.40 m, reinforced	33	3, 5, 27, 32	
U-2-deck bearer 1.57 m	34	3, 5, 32	according to Z-8.22-906
U-2-deck bearer 2.07 m	35	3, 5, 32, 34	No longer
U-2-deck bearer 2.57 m	36	3, 5, 32, 34	manufactured.
U-2-deck bearer 3.07 m	37	3, 5, 32, 34	
Internal ladder	60		according to Z-8.1-862
Steel deck with tube fixture	64		
Aluminium frame platform with plywood 0.50 m - 2.07 m	66	68	
Aluminium frame platform with plywood 2.57 m; 3.07m	67	68	according to Z-8.22-906
Aluminium frame platform with internal hatch 2.57m; 3.07m	69	60, 68, 71	No longer manufactured.
Aluminium frame platform with internal hatch 1.09 m – 3.07 m without ladder	70	68, 71	

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Table 4: (continued)

Designation	Annex B, page	Details / components in accordance with Annex B, page	Regulations for manufacturing, marking and certificate of conformity
Aluminium deck with plywood 2.57 m, 3.07 m	72	74	
Aluminium deck with plywood 1.57 m; 2.07 m	73	74	
Aluminium access deck with ladder 3.07 m	75	60, 74, 77	
Aluminium access deck with ladder 2.57 m	76	60, 74, 77	
Aluminium deck with plywood 3.07 m	78	80	
Aluminium deck with plywood 1.57 m; 2.07 m; 2.57 m	79	80	
Aluminium access deck with ladder 3.07 m	81	60, 80, 83	
Aluminium access deck with ladder 2.57 m	82	60, 80, 83	
Steel deck AF 0.32 m	84		
Steel deck	85		
Steel deck AF 0.30 m, 0.34 m	86		
Steel plank 0.30 m	87		according to
Intermediate deck AF 0.16 m, 0.19 m	88		Z-8.1-862
Intermediate deck	89		
Lightweight aluminium deck 0.60 m	90		
Solid wood deck 48	91		
Solid wood deck 45	92		
Wooden deck	93		
Gap cover	96		
Aluminium stairway AF-0.62 m, 2.57 m, 3.07 m	97		
Inner guardrail for aluminium stairway 2.00 m	99		
Stair stringer fall protection 1.00 x 0.5 m	100		
Toeboard; End toeboard AF	110		
Toeboard AF 4.14 m	111		
Toeboard; End toeboard	112		
Toeboard 4.14 m	113		
Aluminium toeboard; Aluminium end toeboard AF	114		
Aluminium toeboard; Aluminium end toeboard	115		
Storey ladder 2.00 x 0.40 m, steel	118		according to
Storey ladder 2.00 x 0.40 m, aluminium	119		Z-8.1-847
Scaffold retainer / wall tie	120		
Quick-release wall tie	121		
Transom 0.73 m, 1.09 m	127	32	according to
Guardrail coupler AF	128		Z-8.1-862
Toeboard coupler; Halfcoupler with hook	129		

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Table 4: ((continued)
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Designation	Annex B, page	Details / components in accordance with Annex B, page	Regulations for manufacturing, marking and certificate of conformity
Squared timber coupler	130		
Toeboard support	131	3	
Putlog coupler	133		
Diagonal cross brace	134		according to
Advanced guardrail post 2.00 m	135		Z-8.1-862
Telescopic guardrail 2.00 - 3.07 m	136		
Advanced end guardrail / Aluminium telescopic guardrail	138		

3.1.2 Standard Assembly Configuration

The use of the scaffold components in facade scaffolding is described in the provisions of the standard assembly configuration. Proof of structural stability for said fully erected scaffolds in their standard assembly configuration has been furnished. Assembly configurations of facade scaffolds are considered to be standard assembly variations when they comply with the provisions of Annex C and D or Annex E and F. Any assembly configurations that deviate from the standard assembly configuration shall be assessable and verified in each individual case.

The standard assembly configuration applies to facade scaffolds with a structural height that does not exceed 24 m, not including the spindle extension length above the ground. In its standard assembly configuration, the scaffolding system may be used

- with system width b = 0.73 m, with bay widths $l = \le 3.07 m$ and load classes ≤ 3 in accordance with Annex C and D or
- with system width b = 1.09 m, with bay widths $\ell = \le 2.57 m$ and load classes ≤ 4 in accordance with Annex E and F.

for working scaffolds in accordance with DIN EN 12811-1:2004-03, as protection scaffold and roof edge protection scaffold with a class FL1 fall arrest level and as a protection scaffold and roof edge protection scaffold with protective walls of class SWD 1 in accordance with DIN 4420-1:2004-03.

3.1.3 Deviations from the Standard Assembly Configurations

If assembly configurations deviate from the standard assembly configurations in accordance with Annex C and D or Annex E and F, proof of structural stability of the scaffoldings shall be provided for each individual configuration or by means of a structural design calculation in accordance with the Technical Building Rules [Technische Baubestimmungen] and the provisions of this decision. The characteristic properties to be used for the stability verification are specified in this decision.

Other anchoring configurations are possible and other nettings may be used as scaffold covering. In a scaffolding, any increased stresses / loads (e.g. from higher dead weights and wind loads or from increased live loads) must be verified up to the anchors and the supporting surface (ground). The impact of building hoists or other lifting equipment must also be taken into account if they are not operated independently of the scaffold.

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3.2 Dimensioning

3.2.1 General provisions and system assumptions

For the design and calculation of scaffoldings erected using the modular scaffolding system, unless otherwise specified in this decision, particular attention shall be paid to the Technical Building Rules [Technische Baubestimmungen], especially those for working and service scaffolds according to DIN EN 12811-1:2004-03 in conjunction with the "Application Guideline for Working Scaffolds according to DIN EN12811-1"1, DIN 4420-1:2004-03, the "Approval Principles for Working and Service Scaffolds - Requirements, Calculation Assumptions, Tests, Certificate of Conformity"⁵, and for falsework DIN EN 12812:2008-12 in consideration of the "Application Guideline for Falsework Scaffolding in accordance with DIN EN 12812"². ⁶

In case alternative component designs are offered, any proof of structural stability of the scaffolding shall assume the least favourable variant.

Depending on the scaffolding structure used, the different load-bearing groups "A" and "B" for the ledger connection according to Table 5 apply, see Table 5.

If it is not possible to ensure that only components of load-bearing group "A" shall be used, the load-bearing capacities and stiffness values of load-bearing group "B" shall be assumed for the proof of structural stability of the scaffolding for the ledger connections.

The provisions of the following sections apply to the node connection and the connection of the connecting heads to the standard, ledger and diagonal brace tubes listed in the Annexes.

	Ledger design				
	Ledger	Ledger 4.0		Ledger	
Design of the standards	with ledger head according to Annex B,		with ledger head according to Annex B,		
	page 152 page 153		page 4	page 5	
	(Tube ledger)	(U-ledger)	(Tube ledger)	(U-ledger)	
Connector disc (rosette) attached to standard 4.0 circular hollow section Ø 48.3x2.9 mm – S460MH	Load- bearing group "A" = BG "A"		Load- bearing group "B" = BG "B"		
Connector disc (rosette) attached to standard circular hollow section $\emptyset 48.3x3.2 mm - 235JRH$ with $R_{\rm eH} \ge 320 N/mm^2$	Load- bearing group "B" = BG "B"		Loa bea grouj = BC	ring p "B"	

Table 5:Load-bearing groups for the ledger connection

The structural systems for the calculation are to be modelled in accordance with Annex A, page 3. The short members specified there from the standard tube axis to the connectors may be assumed to be rigid. The indices specified in the following sections refer to a local coordinate system, in which the x-axis represents the ledger axis, and the z-axis the axis of the standards (cf. Annex A, page 3).

In the connection of a ledger, transmission of axial forces as well as bending moments and shear forces in the plane between the standard and the ledger and in the plane at a right angle thereto – for which load-bearing capacities are specified in Table 6 – is allowed. In load-bearing group "A", tube ledgers 4.0 in accordance with Annex B, page 156 may additionally also transfer torsional moments when connected to the standard 4.0.

To be obtained from the Deutsches Institut f
ür Bautechnik (the German technical authority and service provider for the construction sector).
 Blasse clea take into construction the advisory results of the "Seaffeld Expert Committee" the construction sector).

Please also take into consideration the advisory results of the "Scaffold Expert Committee", the so-called "SVA Gerüste", available on the DIBt homepage.

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When using short ledgers with L < 0.60 m, connections shall be assumed as articulated; the only allowable transmission is that of axial forces and shear forces.

When providing proof of stability of the scaffolding system, it is to be borne in mind that the bending moment at the joint between the ledger and the standard is taken with reference to the outer edge of the standard, and that the vertical components of the vertical diagonal connection must take into account an eccentricity in the connection corresponding to the data given in Annex A, page 3. The torsional moment resulting from the horizontal component at the vertical diagonal connection around the axis of the standard is transmitted by the connector and must be verified in the ledgers.

Only the transmission of normal forces at the connections of the diagonals is allowed.

The data for the stiffness and load bearing capacity of the connections applies to connections made in the "small" and the "large" openings of the connector disc (rosette).

In all equations in the following sections, the internal forces N and V must be given in [kN], while bending moments M must be quoted in [kNcm].

3.2.2 Ledger connection

3.2.2.1 Load-deformation behaviour

- 3.2.2.1.1 Bending in the standard/ledger plane (vertical plane)
 - For proof of stability of a scaffolding, ledger connections in the plane formed by the standard and the ledger (vertical plane) with torsion spring fixation according to the moment/rotation angle relationship M_y/ϕ
 - in accordance with Annex A, Figure 1 for load-bearing group "A", and
 - in accordance with Annex A, Figure 2 for load-bearing group "B"

shall be taken into account, when an articulated connection is not assumed.

- 3.2.2.1.2 Bending in the plane at a right angle to the standard tube / ledger plane (horizontal plane)
 - When no articulated connection is assumed, proof of stability of the ledger under bending stresses in the plane at a right angle to the plane of the standard tube/ledger (horizontal plane) in the ledger connection for both load-bearing group "A" and "B", shall be calculated assuming a torsion spring fixation that corresponds to the moment/rotation angle relationship Mz/ϕ according to Annex A, Figure 3.
- 3.2.2.1.3 Torsion at the tube ledger of load-bearing group "A"

When no articulated connection is assumed, proof of stability of the O-ledger of load-bearing group "A" under torsion stresses in the ledger connection shall be calculated assuming a spring fixation that corresponds to the moment/rotation angle relationship (MT/ ϕ) in accordance with Annex A, Figure 4. As specified, torsion may not be transmitted in the connection of the U-ledgers.

3.2.2.1.4 Vertical load at right angles to the axis of the ledger

For ledger lengths > 0.7 m in conjunction with vertical shear forces $Vd \le 10 \text{ kN}$, additional floating bearing forces applied in the direction of the shearing load may be disregarded. Otherwise, an additional floating bearing force in the direction of the shear force of

 $f0 = 0.175 \, cm$ must be applied.

3.2.2.2 Proof of structural stability

3.2.2.2.1 General Proofs

For the connection of a ledger, proof shall be provided that the loads shall not exceed the load-bearing capacities given in Table 6. With the exception of the torsion in load-bearing group "A", the load-bearing capacities may be assumed for both the tube ledgers and U-ledgers.

The values for load-bearing group "A" may only be used for the calculation in case standards 4.0 <u>and</u> ledgers 4.0 are used.

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Internal forces and moments of connection			Load-bea	aring capacity
Internal forces and moments of connection		Load-bearing group "A"	Load-bearing group "B"	
Bending moment M _{y,Rd}	[kNcm]		± 120.0	± 104.0
Vertical shear force $V_{z,Rd}$	[kN]		± 39.9	± 35.0
Bending moment $M_{z,Rd}$	ending moment <i>M</i> _{z,Rd} [kNcm]		± 50.0	
Horizontal shear force V _{y,Rd}	Horizontal shear force V _{y,Rd} [kN]		± 16.0	
Torsional moment <i>M</i> _{T.Rd}	[kNcm] Tube ledger 4.0 U-ledger		± 64.0	
Normal force M [kN]	Connection using the large opening in the connector disc		± 39.6	1 26 0
Normal force <i>N_{Rd}</i> [kN]		tion using the small in the connector disc	± 46.6	± 36.0

Load-bearing capacities of a ledger connection Table 6:

3.2.2.2.2 Interaction standard / ledger connection

For connector discs on which stresses act, the interaction relationship below has to be fulfilled depending on the assembly configuration:

Load-bearing group "A":	$I_{\rm S}+0.324\cdot I_{\rm A}\leq 1.0$	(equation 1)
Load-bearing group "B":	$I_{\rm S}+0.326\cdot I_{\rm A}\leq 1.0$	(equation 2)
Where:		

where:

$$I_{A} = \frac{M_{y,\text{Ed}}}{M_{y,\text{Rd}}}$$
 (equation 3)

$M_{y,Ed}$	Bending stresses in the ledger connection
$M_{y,Rd}$	Load-bearing capacity with regard to bending in the ledger connection
	according to Table 6 depending on the assembly configuration
Is	Vectorial coefficient of utilization in standard at loaded connector discs

- For $v_{act} \leq 1/3$ the following applies:

$$I_{\rm S} = \frac{a}{b}$$

(equation 4)

see Figure 1 a, b

For $1/3 < v_{act} \le 0.9$ the vectorial coefficient of utilization must be determined under _ consideration of the interaction relationship as shown in the left side of the equation, column 4 of Table 7, DIN 4420-1:1990-12.

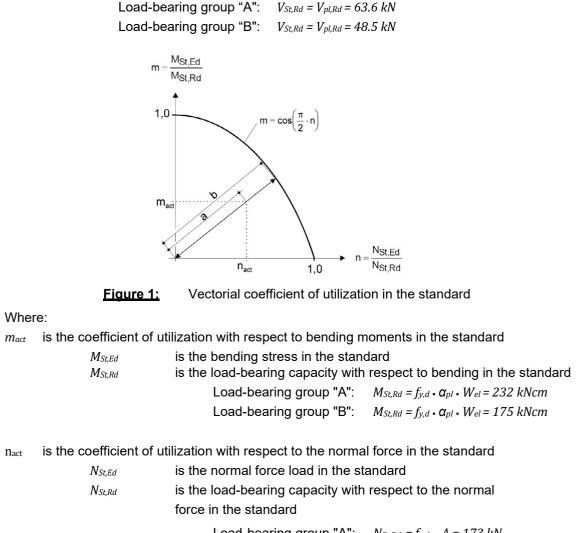
Where:

coefficient of utilization with respect to the shear force in the standard Vact

$$v_{\rm act} = \frac{V_{\rm St,Ed}}{V_{\rm St,Rd}}$$
 (equation 5)

- V_{St,Ed} shear force in the standard
- load-bearing capacity with respect to shear force in the standard VSt,Rd

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Load-bearing group "A": $N_{St,Rd} = f_{y,d} \cdot A = 173 \ kN$ Load-bearing group "B": $N_{St,Rd} = f_{y,d} \cdot A = 132 \ kN$

3.2.2.3 Combinations of internal forces and moments

For combined internal forces and moments in a ledger connection, the following requirements are to be met, whereby the torsion term with Mx in load-bearing group "B" and for all U-ledger connections is not taken into account:

$\frac{N_{Ed}^{(+)}}{N} + max$	$\left(\mid M_{y,Ed} \mid \right)$	Vz,Ed	V _{y,Ed}	M _{x,Ed}	$\left M_{z,Ed} \right < 1$	
N _{Rd} + max	(M _{y,Rd}	$V_{z,Rd}$ +	$V_{y,Rd}$	M _{x,Rd}	$M_{z,Rd} \ge 1$	

(equation 6)

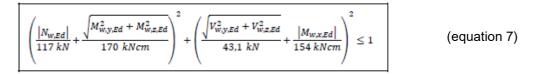
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Where:
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$N_E^{(+)}$				is the tensile normal force load acting in the ledger connection
$M_{\rm y,Ed}$,	$V_{\rm z,Ed}$,	$M_{\rm z,Ed}$,	$V_{\rm y,Ed}$	are forces acting in the ledger connection
N _{Rd}				is the load-bearing capacity with resp. to the tensile normal force according to Table 6
$M_{\rm y,Rd}$,	$V_{\rm z,Rd}$,	$M_{\rm z,Rd}$,	$V_{\rm y,Rd}$	Load-bearing capacities in accordance with Table 6

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Additional proof of stability of the welding connections between U-ledgers and ledger head is not required.

In case $V_{z,W,Ed} \leq 30.5 \ kN$, additional proof of stability of the welding connections between tube ledger 4.0 and the corresponding ledger head 4.0 is not required. In all other cases, the following proof of stability shall be provided:



Where:

 $N_{\mathrm{w.Ed}}, M_{\mathrm{w,x,Ed}}, M_{\mathrm{w,y,Ed}}, M_{\mathrm{w,z,Ed}}, V_{\mathrm{w,y,Ed}}, V_{\mathrm{w,z,Ed}}$

Load-bearing capacities of the weld

3.2.3 Connections of the diagonal braces

- 3.2.3.1 Connection to vertical diagonal brace
- 3.2.3.1.1 Load-deformation behaviour

In the entire system vertical diagonal braces including their connections for load-bearing group "A" and load-bearing group "B" must be taken into account subject to the load direction (compressive or tensile force) and the diagonal brace length with the equivalent stiffness

(E_d· A_{eff}) according to Table 7 as well as the looseness in diagonal direction of $f_0 = 0.7 \text{ cm}$ (cf. Annex A, p. 3).

The deformation components of the standard and ledger due to the eccentricity ey (cf. Annex A, page 3) are included in the data. Consequently, only ex shall be taken into account on a level structural model. Proof is to be provided that the node moments Mk are absorbed by the longitudinal ledgers and transoms fitted to the nodes in accordance with Annex A, page 3.

3.2.3.1.2 Proof of structural stability

NV,Ed

NV,Rd

Depending on the load direction, the following proof of structural stability is to be provided for the vertical diagonal braces:

$$rac{N_{
m V,Ed}}{N_{
m V,Rd}}~\leq~1$$

(equation 8)

Where:

tensile or compressive forces acting in the vertical diagonal braces

Load-bearing capacity of the vertical diagonal braces with wedge-head in relation to tensile and/or compressive force in accordance with Table 7

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Bay length L [m]	Bay height	Compression load		Tensile load		
	H [m]	$E_{ m d} \cdot A_{ m eff}$ [kN]	<i>N</i> ⁽⁻⁾ _{V,Rd} [kN]	$E_{ m d} \cdot A_{ m eff}$ [kN]	N ⁽⁺⁾ _{V,Rd} [kN]	
3.07		2800	10.5	9940	22.9	
2.57		2610	12.8	8040	23.5	
2.07		2380	15.5	6390	24.3	
1.57	2.0	2820	18.5	5270	23.7	
1.40	2.0	3390	18.6	4920	23.4	
1.29		4000	18.4	4700	23.2	
1.09		4920	18.1	4340	22.9	
0.73		4850	17.1	3890	21.6	
3.07		1940	11.9	11120	22.0	
2.57		1680	14.9	8790	22.6	
2.07		1540	18.3	6640	23.3	
1.57	1.5	1660	19.5	4880	24.3	
1.40		2020	19.2	4470	24.0	
1.29		2330	19.0	4200	23.8	
1.09		3170	18.6	3780	23.4	
0.73		4400	17.6	3150	22.2	
3.07		1540	13.1	10050	20.7	
2.57		1250	16.2	8920	21.2	
2.07		1160	16.9	7840	22.1	
1.57		1160	17.9	5240	23.0	
1.40	1.0	1160	18.4	4490	23.4	
1.29		1160	18.8	4030	23.7	
1.09		1210	19.4	3340	24.2	
0.73		2580	18.5	2500	23.3	
3.07		1330	14.0	9390	19.9	
2.57		1170	15.3	8090	20.0	
2.07		1010	15.4	6760	20.2	
1.57	0.5	800	15.8	5430	20.8	
1.40		730	16.1	4990	21.1	
1.29		680	16.3	4700	21.4	
1.09		610	16.9	4130	22.1	
0.73		600	18.7	2180	23.6	

Table 7: Load-bearing capacities of the vertical diagonal braces according to Annex B, page 8

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3.2.3.2 Horizontal diagonal brace connection

3.2.3.2.1 Load-deformation behaviour

In the entire system horizontal diagonal braces according to Annex B, page 9 including their connections taking into consideration the length of the diagonal cross braces must be taken into account irrespective of the load direction (compression or tensile load) with the equivalent stiffness (E_{d} · A_{eff}) according to Table 8 as well as the looseness in diagonal direction of $f_0 = 0.12 \text{ cm}$.

3.2.3.2.2 Proof of structural stability

For the horizontal diagonal braces the following proof of stability is to be provided:

$$\frac{N_{\rm H.Ed}}{N_{\rm H,Rd}} \leq 1$$

(equation 9)

Where:

*N*_{*H,Ed} tensile or compressive forces acting on the horizontal diagonal braces*</sub>

N_{H,Rd} load-bearing capacity of the horizontal diagonal ledgers according to Table 8

Table 8: Load-bearing capacities of the horizontal diagonal braces according to Table B, page 9

Bay length L [m]	Bay width [m]	N _{H,Rd} [kN]	$E_{ m d} \cdot A_{ m eff}$ [kN]
0.73	0.73	3.10	2760
1.09	1.09	3.07	2970
1.57	1.57	3.03	2780
2.07	2.07	2.98	2240
2.57	2.57	2.91	1530
3.07	3.07	2.81	830
1.09		3.08	3160
1.40		3.07	3210
1.57	0.72	3.06	3200
2.07	- 0.73 -	3.03	3070
2.57	Π Γ	3.00	2850
3.07	Π Γ	2.96	2530
1.40		3.06	3210
1.57		3.05	3190
2.07	1.09	3.03	3040
2.57	Π Γ	2.99	2790
3.07	Π Γ	2.95	2460
1.40		3.04	3140
2.07		3.01	2910
2.57	- 1.57 -	2.98	2650
3.07	Π Γ	2.93	2330
1.40		3.02	2970
2.57	2.07	2.95	2450
3.07	1 [2.90	2130
1.40	0.57	2.99	2900
3.07	- 2.57 -	2.86	1880
1.40	3.07	2.94	2380

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3.2.3.3 Diagonal cross brace

For the proof of structural stability of the entire system, the diagonal cross braces according to Annex B, page 134 must be taken into account in connection with the equivalent stiffness and load-bearing capacities given in Table 9.

Table 9: Load-bearing capacities of the diagonal cross brace

Scaffolding width [m]	System length [m]	Load	Stiffness <i>E_d</i> • <i>A_{eff}</i> [kN]	Load-bearing capacity <i>N_{Rd}</i> [kN]
0.732	1.05	Compression	2730	-10.2
1.088	1.95	Tension	2890	+10.2
0.732	4 77	Compression	2570	-10.2
1.088	1.77	Tension	2670	+10.2
when $E_d = (21.000 / 1.1) kN/cm^2$				

3.2.4 Connector disc

3.2.4.1 Connection through adjacent opening in connector disc

When two ledgers or one ledger and one vertical diagonal brace or one ledger and one horizontal diagonal brace are connected in adjacent openings, the following proof of structural stability has to be provided:

$$(n^{A} + n^{B})^{2} + (v^{A} + v^{B})^{2} \leq 1$$
 (

(equation 10)

where:

n, *v* is the interaction ratio according to Table 10

A ledger A

a ledger a or vertical or horizontal diagonal brace

When 3 ledgers or vertical diagonal braces are connected in adjacent holes or two ledgers are connected at an angle of 90°, the proof of structural stability below must additionally be provided, if

$v^{A} > 0.814$	or	$v^{\scriptscriptstyle B} > 0.814$	applies.	
	$0.55 \cdot (v^{A} +$	$-v^{a}+v^{B}) \leq 1$		(equation 11)

where:

V	is the interaction ratio according to Table 10
Α	ledger A
В	ledger B at an angle smaller than 90° in relation to A
а	ledger or vertical diagonal brace between A and B according to Figure 2

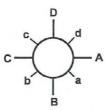


Figure 2: Connections to the connector disc (rosette)

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Table 10: Ir	nteraction ratios		
Interaction ratio	Connection ledger A/ ledger a	Connection ledger A/ ledger B/ vdiagonal brace a	Connection ledger A / horizontal diagonal brace a
$n^{ m A}$		$\frac{\frac{N^{\rm A(+)}}{\rm Ed} + M^{\rm A}_{y,\rm Ed} /e}{N_{\rm Rd}}$	
nª	$\frac{\frac{N^{\mathrm{a}(+)} + M^{\mathrm{a}}_{y,\mathrm{Ed}} /e}{N^{*}_{\mathrm{Rd}}}$	$\frac{0,707 \cdot N^{(+)}_{V,Ed} \cdot \sin \alpha + 1,883 \cdot N_{V,Ed} \cdot \cos \alpha}{1,29 \cdot N^*_{Rd}}$	$\frac{N_{\rm H,Ed}^{(+)}}{N_{\rm Rd}^{*}}$
$v^{ m A}$		$V^{\mathrm{A}}_{\mathrm{z,E}}{}^{\mathrm{d}}_{\mathrm{V}_{\mathrm{z,Rd}}}$	
$v^{ m B}$		$V^{ m B}_{{ m z,E}{ m d}} onumber V^{ m R}_{ m z,Rd}$	
v^{a}	V ^a zE ^d V _{z,Rd}	$\frac{ N_{\rm V,Ed} \cdot \cos \alpha}{V_{\rm z,Rd}}$	

Where:

$N_{Es}^{A(+)}; N_{Ed}^{a(+)}$	Normal force (only consider tensile forces) in the ledger connection
	(ledger A bzw. ledger a)
$M^A_{y,Ed}$; $M^a_{y,Ed}$	Vertical lateral load in ledger connection (ledger A, ledger B, vertical diagonal a)
$V^A_{z,Ed}$; $V^a_{z,Ed}$, $V^B_{z,Ed}$	Bending in the ledger connection (ledger A bzw. ledger a)
N _{V,Ed}	Normal force in the vertical diagonal brace
$N_{V,Ed}^{(+)}$	Tensile force in the vertical diagonal brace
$N_{H,Ed}^{(+)}$	Tensile force in the horizontal diagonal brace
е	Load-bearing group "A": Lever arm tube ledger connection <i>e</i> = 3.7 <i>cm</i>
	Load-bearing group "B": Lever arm tube ledger connection e = 3.3 cm
N _{Rd}	Load-bearing group "A": <i>N_{Rd}</i> = 47.9 <i>kN</i>
	Load-bearing group "B": Load-bearing capacities in accordance with Table 6
N_{Rd}^*	Load-bearing group "A": <i>N_{Rd}</i> = 40.7 <i>kN</i>
	Load-bearing group "B": Load-bearing capacities in accordance with Table 6 $N_{Rd}^* = N_{Rd}$
$V_{z,Rd}$,	Load-bearing group "A": <i>N_{Rd}</i> = 44.6 <i>kN</i>
	Load-bearing group "B": Load-bearing capacities in accordance with Table 6
Droof of stability	shall be previded in pairs around the nodes

Proof of stability shall be provided in pairs around the nodes.

3.2.4.2 Connection of ledgers and/or diagonal braces in any openings of the connector disc (rosette)

$\frac{\sum V_{z,Ed}}{\sum V_{z,Rd}} \leq$	≤	1
--	---	---

(equation 12)

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Where:	
$\sum V_{ m z,Ed}$	is the sum of all vertical shear forces acting on the connector disc (including vertical component of vertical diagonal braces)
$\sum V_{ m z,Rd}$	is the load-bearing capacity of the connector discs in relation to vertical sh

- $V_{z,Rd}$ is the load-bearing capacity of the connector discs in relation to vertical shear forces depending on the load-bearing group
 - Load-bearing group "A": $\sum V_{z,Rd} = 173.0 \ kN$
 - Load-bearing group "B": $\sum V_{z,Rd} = 127.0 \ kN$
- 3.2.4.3 Interaction in case of adjacent ledger connections for connections to standard 4.0 *circular hollow* section Ø 48.3 x 2.9 mm S460MH

For connection moments of opposite ledger connections A and C, see Figure 2, with the same sign, the following conditions are to be met:

 $\frac{M_{\rm y,Ed}^{\rm A} + 0.226 \cdot M_{\rm y,Ed}^{\rm C}}{M_{\rm y,Rd}} \le 1$

where $|M_{y,Ed}^{A}| \ge |M_{y,Ed}^{C}|$ (equation 13)

3.2.5 Configuration and proof of structural stability of the standard joints

3.2.5.1 General Provisions

Unless otherwise specified below, the joints of the standards in the modular scaffolding system "ALFIX MODUL MULTI" are to be constructed and proof of structural stability is to be provided in accordance with the current Technical Building Rules. Please also refer to the "Calculation of standard joints with one-sided, centrally fixed joint pins for working and service scaffolding, and for supporting framework made of steel"⁷.

The decision comprises 4 configurations of these detail. Table 11 summarizes these configurations and lists the most important characteristic values. If it is not possible to ensure which standard joint configuration shall be used, any proof of structural stability shall assume the least favourable variant.

		Standard		with tub	e connector
Тур	Designation	Annex B, page	Tube (D _i at the base) / yield stress	Design	Tube / yield stress
1	Standard 4.0	154, 155, 159	Ø48.3x2.9 mm (40.9) / 460 N/mm²	integrated	Ø39.0x3.5 mm/ 460 N/mm ²
2	Standard with tube connector 200	11, 15, 159	Ø48.3x3.2 mm (41.9) / 320 N/mm²	plugged in, pressed	Ø38.0x3.6 mm / 320N/mm²
3	Standard with	12, 13			
4	screwed-in tube connector	14	Ø48.3x4.05 mm (40.2) / 320 N/mm²	plugged-in, screwed	Ø38.0x4.0 mm / 320N/mm²

Table 11:Standard and tube connector configurations

3.2.5.2 Load-bearing model "lap joint"

In accordance with the "Calculation notes for standard joints with centrally fixed joint pins on one side for working and service scaffolds, and for supporting framework made of steel"1, the standard joint parameters as per Table 12 shall be taken into account for the structural analysis of type 1 standard joints with standard tubes Ø 48.3 x 2.9 mm made of S460MH steel with integrated tube connectors in the load-bearing model "lap joint".

7 see DIBt Newsletter 4/2017

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	Loud boaring	g oupdotties and load		pe i
Internal forces and moments	Tube connector	Load deformation behaviour		aviour
Bending moment	Тур 1	$M_{ m Rd} = 122$ $kNcm^{*)}$	Stiffness behaviour: $ \varphi_{d} = \frac{M}{18900 - 49 \cdot M } $	<i>M</i> in [kNcm]
*) Separate proof of stability of the net section at the tube connector is not mandatory.				

Table 12: Load-bearing capacities and load-deformation behaviour for type 1

3.2.5.3 Structural behaviour under tensile stress

If tensile forces must be transmitted via a standard joint, the tubes shall be connected by means of bolt connections with a short thread whereby the thread may not be positioned in the shear loaded joint and whereby the clearances at the connecting members with short thread are to be taken into consideration. Connecting members are to be passed through the openings in the joint area intended for this use and secured against unintended lift-off (e.g. by means of a screw joint tightened by hand). Depending on the connecting members used and the standard joint variant, loads can be transmitted in accordance with Table 13 or 14 depending on the screws used.

For standard joints with standard tubes \emptyset 48.3 x 2.9 mm made of S460MH steel with integrated tube connectors \emptyset 39 x 3.5 mm according to Annex B, pages 154, 155 and 159, proof of structural stability (eq. 14) for the deformation range must additionally be met.

$$\frac{|M_{Ed}|}{M_{Rd} \cdot cos \ \left(\frac{\pi}{2} \cdot \frac{Z_{Ed}}{72, 1 \ kN}\right)} \leq 1$$

(equation 14)

Where:

$M_{ m Ed}$	Bending stress
$M_{ m Rd}$	Bending capacity according to Table 12
$Z_{ m Ed}$	Tensile force load

<u> Table 13:</u>	Tensile stress resistance of the standards in connection with M12-8.8
-------------------	---

Tensile stress resis	stance Z _{Rd} [kN] wh	nen using M	12-8.8 scre	WS		
Tube II	connection by means of		Typ * ⁾ Tube II			
	one screw		1	2	3	4
		1	43.7	33	.5	42.4
	Тур * ⁾	2		10		
	Tube I	3	00.0	29.2		29.2
L I Tube I	4	4	28.8			30.4
Tuball	connection by means of		Typ * ⁾ Tube II			
Tube II two screws						
	two screws		1	2	3	4
	two screws	1	1 72.1	2 67	-	4 72.1
		1 2	=	_	.0	-
	two screws Typ *) Tube I	-	72.1	67 10	.0	-
Tube I	Typ *)	2	=	67	.0	72.1

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Tensile stress resis	stance Z _{Rd} [kN] wh	nen using M	10-8.8 scre	WS		
Tube II	connection by means of one screw		Typ * ⁾ Tube II			
			1	2	3	4
		1	30.1	27	7.9	32.9
	Тур * ⁾	2		10).0	
Tubol	Tube I 3	3	17.0	17	7.0	17.2
I I Tube I		4	17.0	17.2		17.9
	connection by means of					
Tube II	connection by	means of		Typ *)	Tube II	
Tube II	connection by two screws	means of	1	Typ *) 2	Tube II 3	4
Tube II	-	means of 1	1 60.2	2		4 65.8
Tube II	-		•	2	3	-
	two screws	1	60.2	2 55 10	3 5.8 0.0	-
Tube II Tube I	two screws Typ *)	1 2	•	2 55 10	3 5.8	65.8

Table 14:Tensile stress resistance of the standards in connection with M10-8.8

3.2.5.4 Structural behaviour under compressive stress

Please refer to Table 15 for the compressive stress resistance of the standard joints for the respective assembly variant.

Proof of interaction for the lap joint supporting structure model is not mandatory in case standard joints are simultaneously subject to compressive and bending stress.

Standard joint		Compressive stress resistance D _{Rd} [kN]		Typ * ⁾ Tube II				
Tube II				2	3	4		
		1	173	13	32	164		
ע נ	Тур *)	2	87.6	80.9		115		
	Tube I	3	39.6	36	.9	67.2		
i i Tube I		4	72.4	67	.2	107		
*) Tube types according to Table 11								

3.2.6 Wedge-head couplers

Wedge-head couplers (rigid or rotatable) according to Annex B, pages 122 and 125 are only to be used for connecting "free" scaffolding tubes \emptyset 48.3 x 3.2 mm to the standards of the scaffolding system in conjunction with the roof edge protection (e.g. see Annex D, p. 7).

3.2.7 Scaffolding components that are designed using components of the scaffolding nodes

Proof of structural stability for the node connections of the scaffolding components that are designed in accordance with Section 2.1.3 is to be provided according to Sections 3.2.2 and 3.2.4. Any other proof of structural stability shall be provided in accordance with the Technical Building Regulations (Technische Baubestimmungen).

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3.2.8 Proof of structural stability of the entire scaffolding system

- 3.2.8.1 Cross-section properties of the U-profiles
- 3.2.8.1.1 U-profile U48 x 52 x 2.5 (without holes)

Proof of structural stability of the U-profile U 48 x 52 x 2.5 (without holes) in accordance with Annex B, pages 32, 127, 147 and 149 is to be provided based on the properties given in Figure 3. t_z

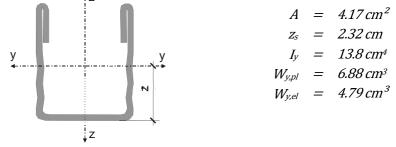


Figure 3: Properties of the U-profile U 48 x 52 x 2.5 without holes

3.2.8.1.2 U-profile U 48 x 52 x 2.5 with 15 mm opening width

Proof of structural stability of the U-profile U 48 x 52 x 2.5 with a 15 mm opening in accordance with Annex B, pages 32, 147 and 149 is to be provided based on the properties given in Figure 4.

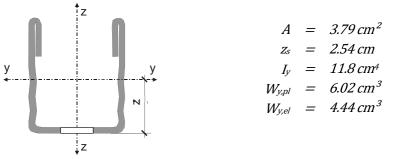
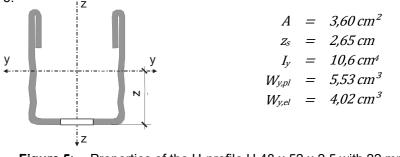
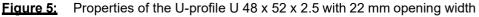


Figure 4: Properties of the U-profile U 48 x 52 x 2.5 with 15 mm opening width

3.2.8.1.3 U-profile U 48 x 52 x 2.5 with 22 mm opening width

Proof of structural stability of the U-profile U 48 x 52 x 2.5 with 22 mm opening in accordance with Annex B, page 127 is to be provided based on the properties given in Figure 5. \ddagger_z





3.2.8.2 Vertical load-bearing capacity of decks

Proof of structural stability for the decks of the "ALFIX MODUL MULTI" modular scaffolding system is provided in accordance with Table 16 for live loads of the scaffold load classes / service classes according to EN 12811-1:2004-03, Table 3 and for use in protection scaffold and roof edge protection scaffold with fall heights of up to 2 m according to DIN 4420-1:2004-03 (class D according to DIN EN 12810-1:2004-03).

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Designation	Annex B, page	Bay width ℓ [m]	Use in load class (service class)	
Aluminium frame platform with tube fixture	51 and 52	≤ 3.07	≤ 3	
Aluminium access frame platform with tube fixture	54 and 55	≤ 3 .07	≤ 3	
Aluminium access frame platform with tube fixture 1.57 m – 3.07 m without ladder	57	≤ 3.07	≤ 3	
Aluminium access frame platform with tube fixture 2.57 m – 3.07 m with aluminium chequer plate	58	≤ 3.07	≤ 3	
		4.14	≤ 3	
Steel deck AF with tube fixture	61 and 62	3.07	≤ 4	
Intermediate deck AF with tube fixture 0.19 m	63	2.57	≤ 5	
		≤ 2.07	≤ 6	
Intermediate deck AF with tube		3.07	≤ 4	
fixture 0.16 m Steel deck AF with tube fixture	63	2.57	≤ 5	
Intermediate deck with tube fixture	64 65	≤ 2.07	≤ 6	
Aluminium frame platform with plywood	66 and 67	≤ 3.07	≤ 3	
Aluminium frame platform with internal hatch	69 and 70	≤ 3.07	≤ 3	
Aluminium deck with plywood	72, 73, 78 and 79	≤ 3 .07	≤ 3	
Aluminium access deck with ladder	75, 76, 81 and 82	≤ 3 .07	≤ 3	
Steel deck	85	3.07	≤ 4	
Intermediate deck AF 0.16 m	88	2.57	≤ 5	
Intermediate deck	89	≤ 2.07	≤ 6	
		4.14	≤ 3	
Steel deck AF	84 and 86	3.07	≤ 4	
Intermediate deck AF 0.19 m	88	2.57	≤ 5	
		≤ 2.07	≤ 6	
Steel plank 0.30 m	87	1.45 to 1.85	≤ 3	
		≤ 1.45	≤ 4	
Lightweight aluminium deck 0.60 m	90	3.07	≤ 3	
	90	≤ 2.57	≤ 4	
		3.07	≤ 3	
Solid wood deck 48	91	2.57	≤ 4	
Wooden deck	93	2.07	≤ 5	
		≤ 1.57	≤ 6	
		2.57	≤ 3	
Solid wood deck 45	92	2.07	≤ 4	
		≤ 1.57	≤ 5	

Table 16: Assignment of decking to scaffold load classes (service classes)

3.2.8.3 Elastic support of the vertical frame sections

Non-anchored nodes of vertical frame sections on the frame level (in case of facade scaffolding perpendicular to the facade) may be assumed to be elastically supported by the horizontal level (decking), provided that the neighbouring horizontal nodes are anchored. This elastic support may be taken into account by assuming a travel spring in connection with the design values given in Table 17.

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	ч [m] q		Bay width ℓ [m]	لرح ا	bearing capacity Clearance $f_{\perp,o}$ [cm]	Stiffness <i>c</i> _{⊥,d} [kN/cm]			city
Deck	in accordance with Annex B, page	in accordance with Annex B, page Scaffolding width b [m]		Maximum load- bearing capacity		$0 < F_{\perp} \leq F_{\perp 1,2}$ [kN]	$F_{\perp 1,2} {<} F_{\perp} {\leq} F_{\perp Rd}$ [kN]	$F_{\perp 1,2}$ [kN]	Load-bearing capacity of the spring load $F_{\perp,Rd}[{ m kN}]$
Aluminium frame platform with tube fixture	51, 52			LC 3	3.40	0.78	0.78	1.50	1.71
Aluminium frame platform with plywood	66, 67		3 ≤ 3.07			•			
Aluminium deck with plywood	72, 73, 78, 79	0.73							
Steel deck AF with tube fixture 0.32 m	61								
Steel deck with tube fixture	64			LC 4	3.96	0.58	0.46	1.50	3.00
Steel deck AF 0.32 m	84								
Steel deck	85								
Lightweight aluminium deck 0.60 m	90			LC 3	3.50	1.20	0.48	2.00	2.80
Steel deck AF with tube fixture 0.32 m	61								
Steel deck with tube fixture	64	1.09	≤ 3.07	LC 4	4.39	0.79	0.79	1.50	2.46
Steel deck AF 0.32 m	84								
Steel deck	85								

 Table 17:
 Design values of the horizontal travel springs

3.2.8.4 Elastic coupling of the vertical levels

The inner and outer vertical level of a scaffolding may be assumed to be elastically coupled to each other by the decking in the direction of these levels (in the case of facade scaffolding parallel to the facade). This elastic coupling may be taken into account by assuming coupling springs in connection with the design values given in Table 18, irrespective of the bay width.

Table 18:

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		[m] o		aring	Clearance $f_{l\!/o}$ [cm]	Stiffness <i>c</i> //, <i>d</i> [kN/cm]			oity
Deck	in accordance with Annex B, page	Scaffolding width b [m]	Bay width ℓ [m]	Maximum load-bearing capacity		$0 < F_{l/l} \le F_{l/1,2}$ [KN]	$F_{l/1,2}\!<\!F_{l/S}\!\!\leq\!\!F_{l/Rd}$ [kN]	<i>F</i> // <i>1.</i> 2 [KN]	Load-bearing capacity of the spring load $F_{l/,Rd}$ [kN]
Aluminium frame platform with tube fixture	51, 52								
Aluminium frame platform with plywood	66, 67		≤ 3.07	LC 3	0.50	2.65	2.22	3.0	3.86
Aluminium deck with plywood	72, 73, 78, 79								
Steel deck AF with tube fixture 0.32 m	61	0.73		LC 4	1.40	2.58	3.46	3.0	4.50
Steel deck with tube fixture	64								
Steel deck AF 0.32 m	84								
Steel deck	85								
Lightweight aluminium deck 0.60 m	90			LC 3	0.40	6.90	2.47	3.75	5.75
Steel deck AF with tube fixture 0.32 m	61								
Steel deck with tube fixture	64		< 2.07	LC 4	1.95	1.67	1.67	3.0	3.94
Steel deck AF 0.32 m	84		≤ 3.07						
Steel deck	85	1 00							
Steel deck AF with tube fixture 0.32 m	61	1.09	≤ 2.57	LC 4	1.95	1.39	1.39	3.0	
Steel deck with tube fixture	64								3.28
Steel deck AF 0.32 m	84				1.95	1.59	1.59	3.0	5.20
Steel deck	85								

Design values of the horizontal coupling springs

3.2.8.5 Material parameters

For components made of S235JRH steel with an increased yield point ($R_{eff} \ge 320 \ N/mm^2$) - any such components are marked accordingly in the drawings of Annex B - the design value of the yield point $f_{y,d} = 291 \ N/mm^2$ may be used for the calculation. All other parameters are to be applied according to the basic material S235JRH.

3.2.8.6 Tubes Ø 48.3 mm made from S460MH steel

Scaffold tubes Ø 48.3 x 2.9 mm and Ø 48.3 x 2.7 mm made from S460MH may be assigned to buckling curve "a".

For the proof of structural stability, the plastic shape coefficient shall be limited to 1.25. If for the tubes \emptyset 48.3 mm made from S460MH a calculation according to the second order theory of elasticity is carried out, the next value may be assumed as the design value of the initial bow imperfection:

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The cosine interaction may be used to provide proof of interaction pressure with bending.

3.2.8.7 Scaffolding spindles / base jacks

The substitute section properties for the stress and / or interaction analyses and calculations of deformation according to DIN 4425:2017-04 (Annex B of DIN EN 12811-1:2004-03) shall be assumed as follows:

- for the scaffolding spindles / base jacks according to Annex B, pages 17, 20, 21 and 22:

$A = A_S =$	$3.52 cm^2$
I =	4.00 cm ⁴
W_{el} =	2.68 cm ³
$W_{pl} =$	$1.25 \cdot 2.68 = 3.35 \mathrm{cm}^3$

- for the scaffolding spindles / base jacks according to Annex B, pages 18, 19 and 140:

A = A	s =	$3.85 cm^2$
Ι	=	4.27 cm ⁴
Wel	=	2.83 cm ³
W_{pl}	=	$1.25 \cdot 2.83 = 3.54 \mathrm{cm}^3$

The cosine interaction in accordance with DIN 4420- 1:1990-12, Table 7 may be used to provide proof of stability of the load-bearing capacity of the scaffolding spindles / base jacks.

3.2.8.8 Halfcouplers

For the proof of stability of the halfcouplers attached to the various components, the loadbearing capacities and stiffness values for halfcouplers of class B shall be applied in accordance with the data provided in DIN EN 74-2:2009-01.

For halfcouplers of class B manufactured before January 2009, for which proof is provided that they comply with the "Zulassungsgrundsätze für den Verwendbarkeitsnachweis von Halbkupplungen an Stahl- and Aluminiumrohren" (Approval principles for the proof of applicability of halfcouplers on steel and aluminium tubes), the resistances in accordance with the approval principles may be assumed, in deviation from DIN EN 74-2:2009-01.⁸

If it is not clear, which components are used, the load-bearing capacities and stiffness values for halfcouplers of class B in accordance with the DIN EN 74-2:2009-01 shall be applied for the verification of the scaffolding.

3.3 **Provisions for execution**

3.3.1 General Provisions

The assembly, alteration and dismantling of the scaffolding must be carried out in compliance with the Instructions for Assembly and Use⁹ which are not the subject of this decision.

3.3.2 Condition of components

All components shall be inspected for proper condition prior to assembly. Damaged components may not be used.

⁸ To be obtained from the Deutsches Institut für Bautechnik (the German technical authority and service provider for the construction sector).

⁹ The "Instructions for Assembly and Use" must comply with the requirements of the "Application guideline for working scaffolds according to DIN EN 12811-1", see DIBt-Mitteilungen (notifications of the DIBt) issue 2/2006.

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3.3.3 Structural design

3.3.3.1 General Provisions

In a working scaffold, the standards in accordance with Annex A, page 159 may not be mixed with other designs of vertical standards.

The following applies when using scaffolding nodes:

- Each connector disc (rosette) has a maximum of 8 openings, allowing to connect 8 members.
- The wedges of the connector heads are to be fixed by driving the wedge from top to bottom to the end-stop with a 0.5 kg hammer.

3.3.3.2 Base area

The lower standards or vertical starter pieces are to be positioned and adjusted on the base jacks in a way that the working levels are horizontal. It must be ensured that the base plates of the base jacks are horizontal and supported over the entire area to absorb and transmit the forces resulting from the scaffolding in the supporting surface.

3.3.3.3 Scaffolding decks

Scaffolding decks must be secured to prevent them from accidental lift-off.

3.3.3.4 Side protection

The provisions of DIN EN 12811-1:2004-03 apply to the side protection. Primarily use components intended for this use and only exceptionally use components such as steel tubes and couplers according to DIN EN 12811-1:2004-03 as well as scaffold decks and planks according to DIN 4420-1:2004-03.

3.3.3.5 Bracing

Scaffolds must be braced.

The vertical planes are to be braced by means of longitudinal ledgers or longitudinal ledgers in conjunction with vertical diagonal braces. System decks in conjunction with transoms can also be used as longitudinal ledgers for the structural analysis.

Horizontal scaffolding levels are to be braced by means of ledgers and horizontal diagonal braces or by means of system decks in conjunction with transoms in accordance with Sections 3.2.8.3 and 3.2.8.4.

The individual bracing levels are to be erected and positioned in accordance with the structural analysis.

3.3.3.6 Anchoring

Please refer to the structural analysis for anchor forces and the anchor configuration.

The anchorage of scaffold retainers/wall ties to the facade or to other parts of the building is not covered by this decision. The user must ensure that the respective forces can be securely absorbed and transmitted from scaffold retainers/wall ties. Vertical forces must not be transmitted in this process.

3.3.3.7 Couplers

Couplers with screwed connectors must be tightened with a torque of 50 Nm when connecting to the standards; tolerances of \pm 10 % are permitted. According to the manufacturer's instructions for use, bolts/screws must be easy to reposition.

3.3.3.8 Standard joints

To secure against uplifting forces in accordance with the structural analysis, the standard joint shall be installed in accordance with the Instructions for Assembly and Use. If tensile stresses in accordance with Section 3.2.5.3 are included in the calculation, all screws in the required quality grades with short thread, the thread section of which may not be in the shear loaded joint, must be used to secure against tensile forces.

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3.3.4 Attestation of conformity

The building contractor shall submit a declaration of conformity in accordance with §§ 16 a (5) in conjunction with 21 (2) Model Building Regulation (MBO) in order to confirm the conformity of the erected scaffold with the General construction technique permit (aBG) covered by this decision.

4 Provisions for use, maintenance, and inspection

4.1 General Provisions

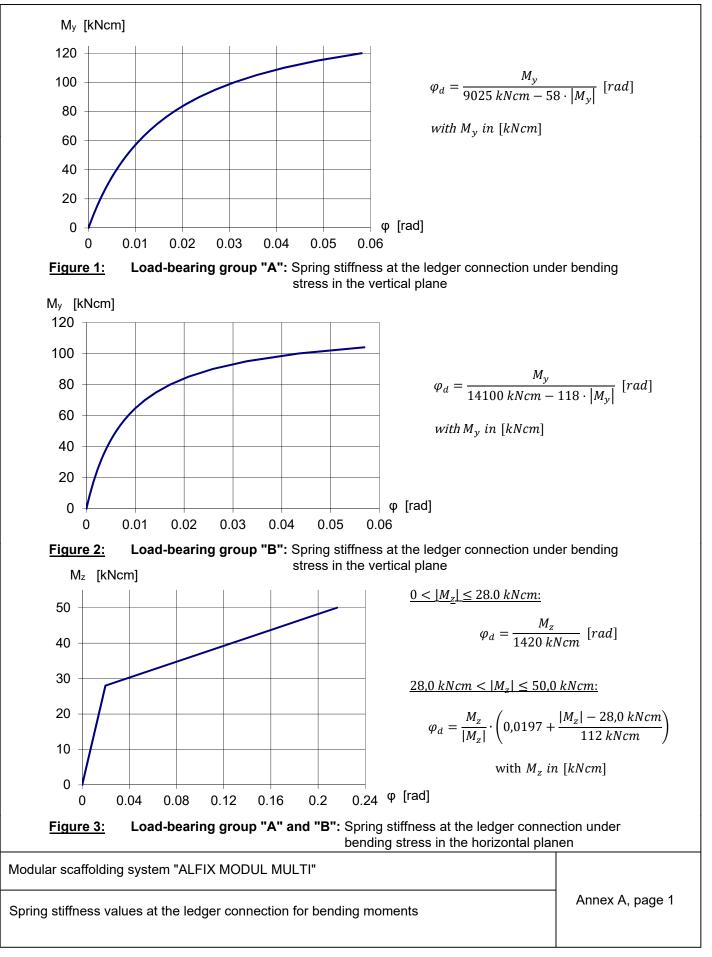
The use of the scaffolding is not covered by this approval.

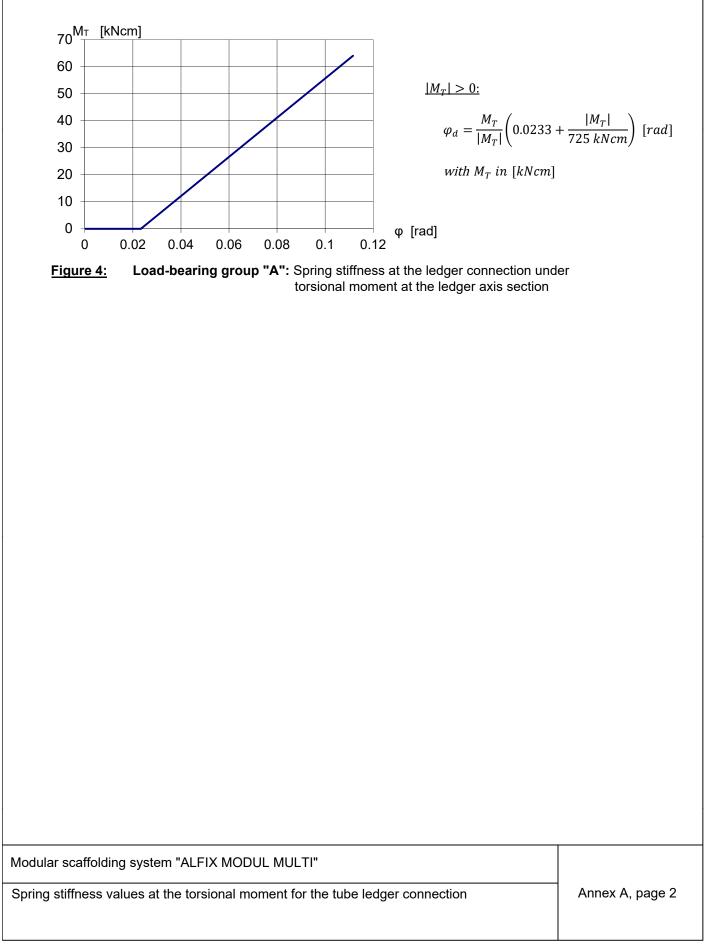
4.2 Wooden scaffolding components

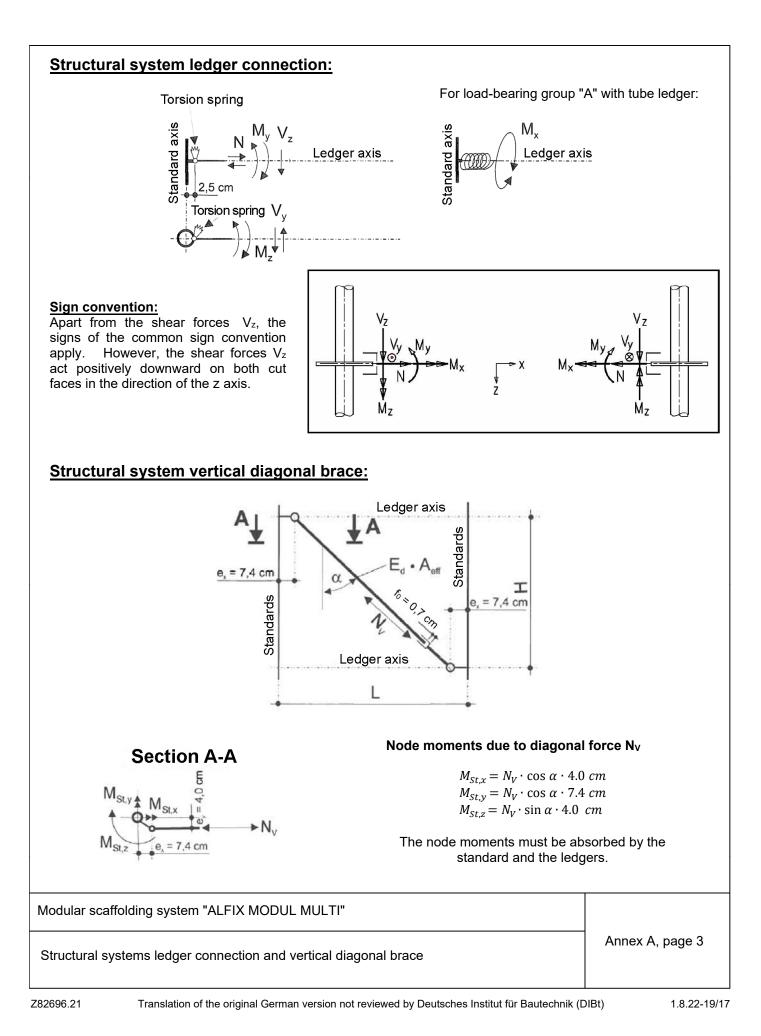
In order to prevent damage caused by moisture to wooden scaffolding components, they must be stored in a dry place, off the ground, and providing adequate ventilation.

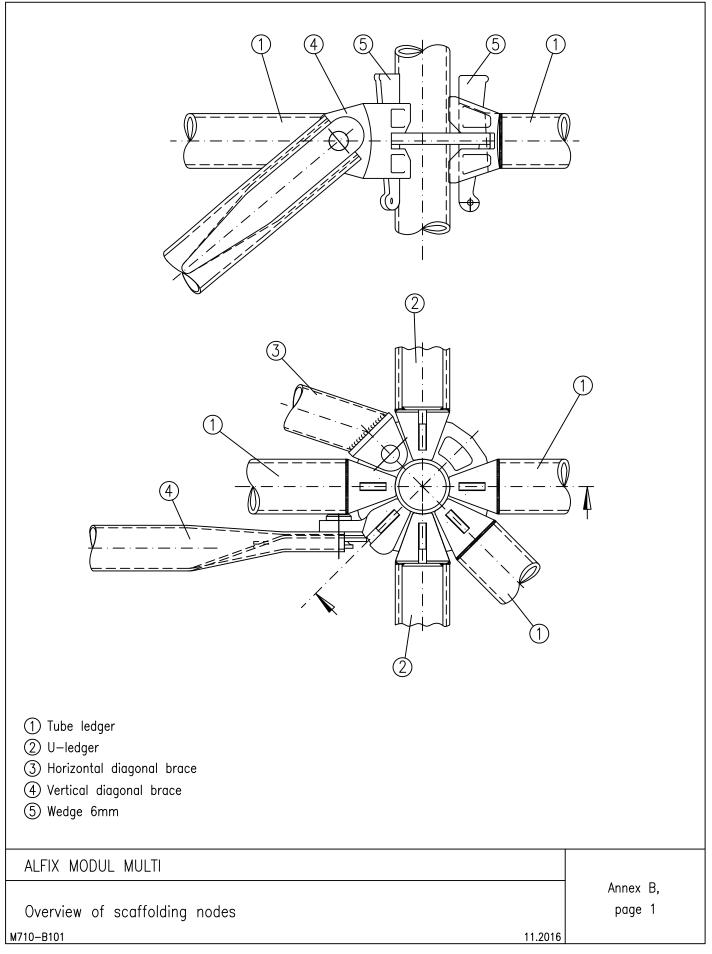
Head of Division

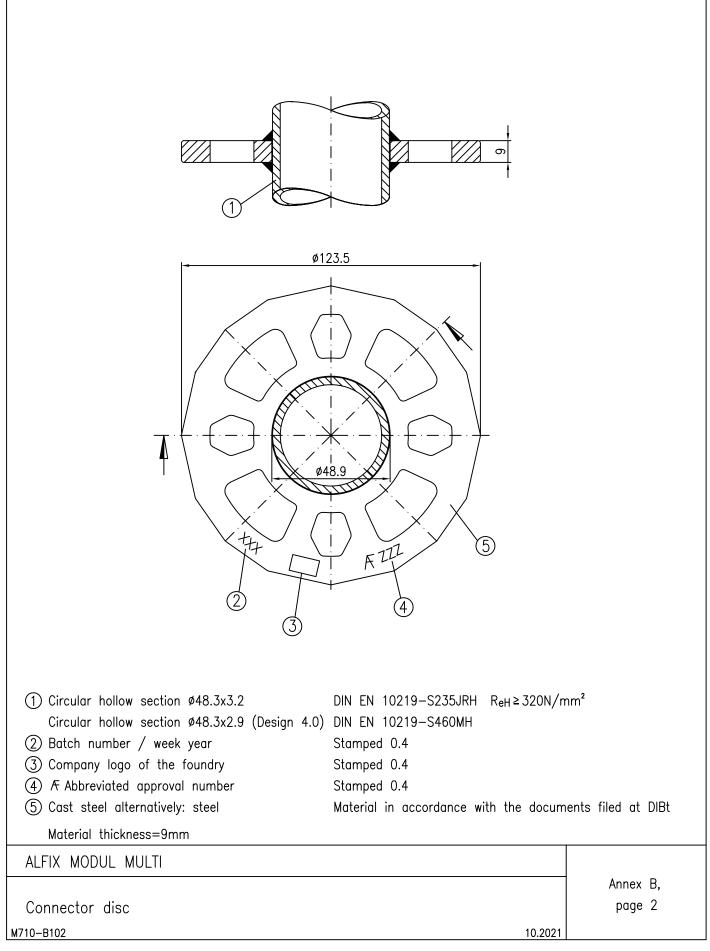
Certified

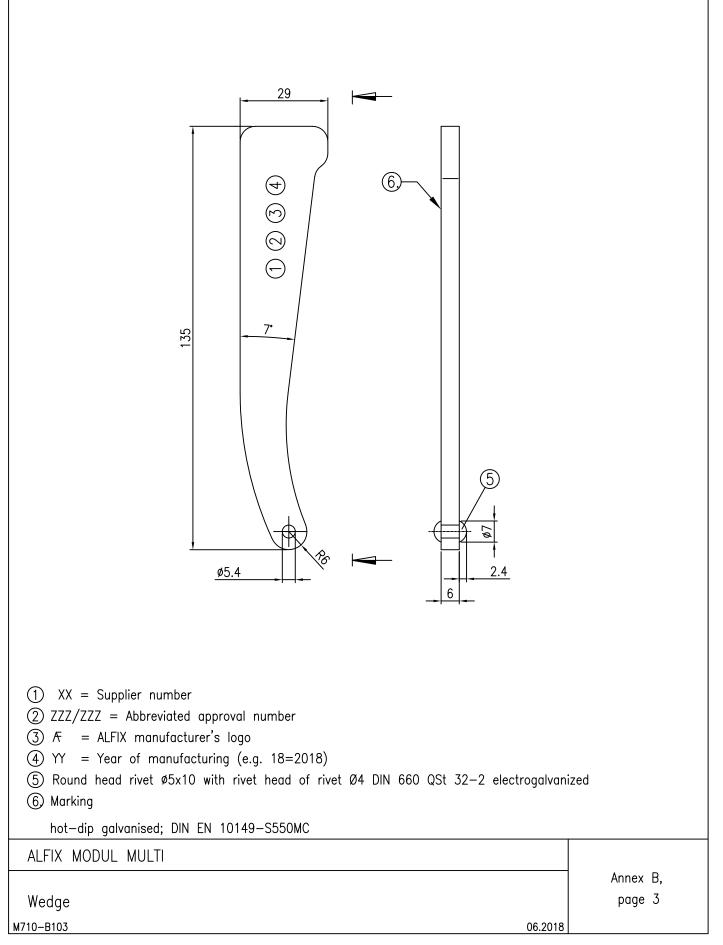


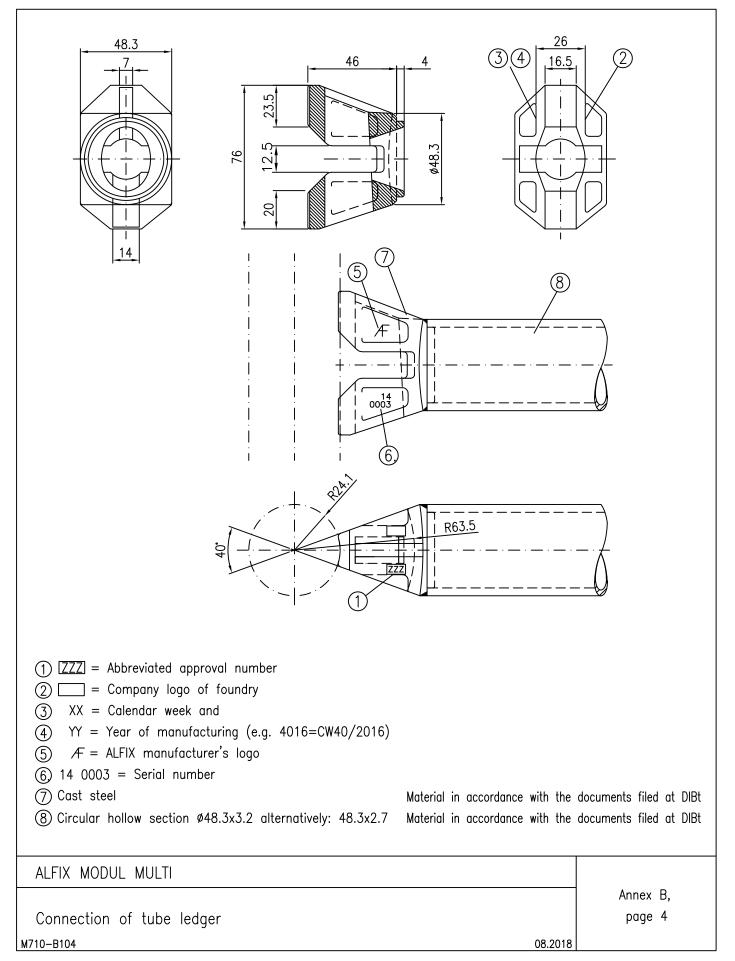


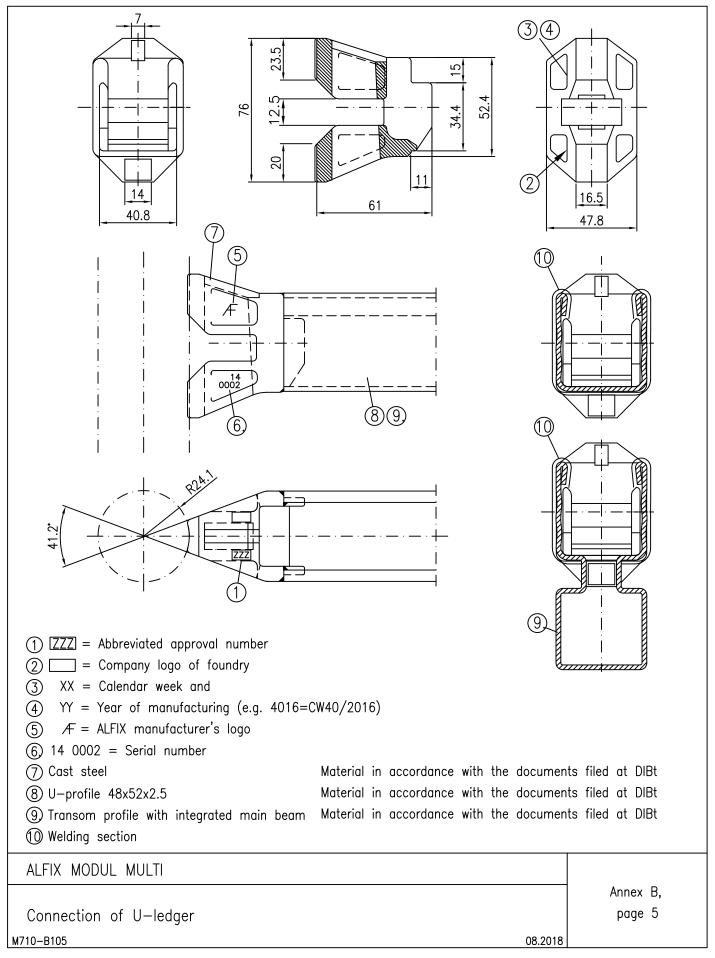


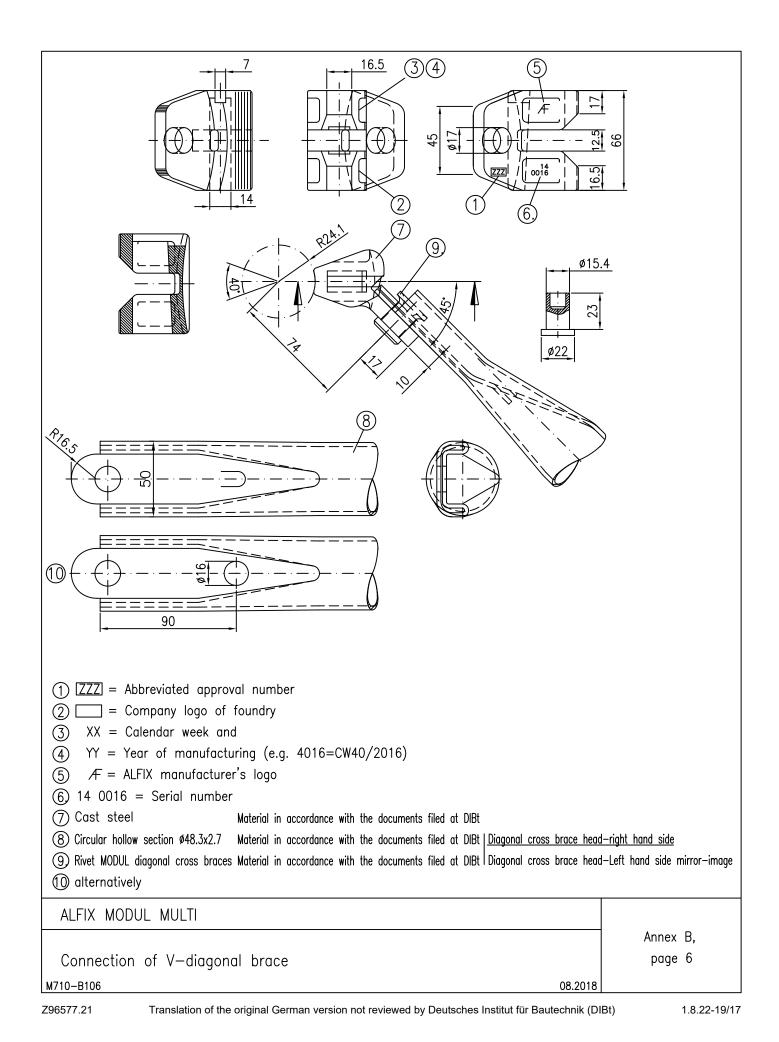


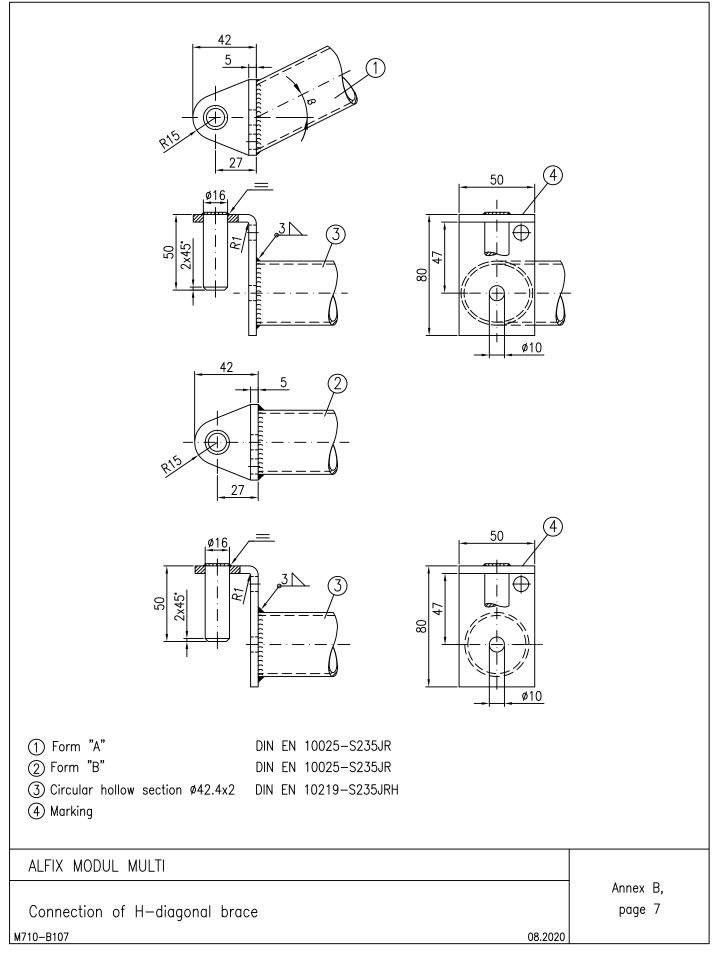




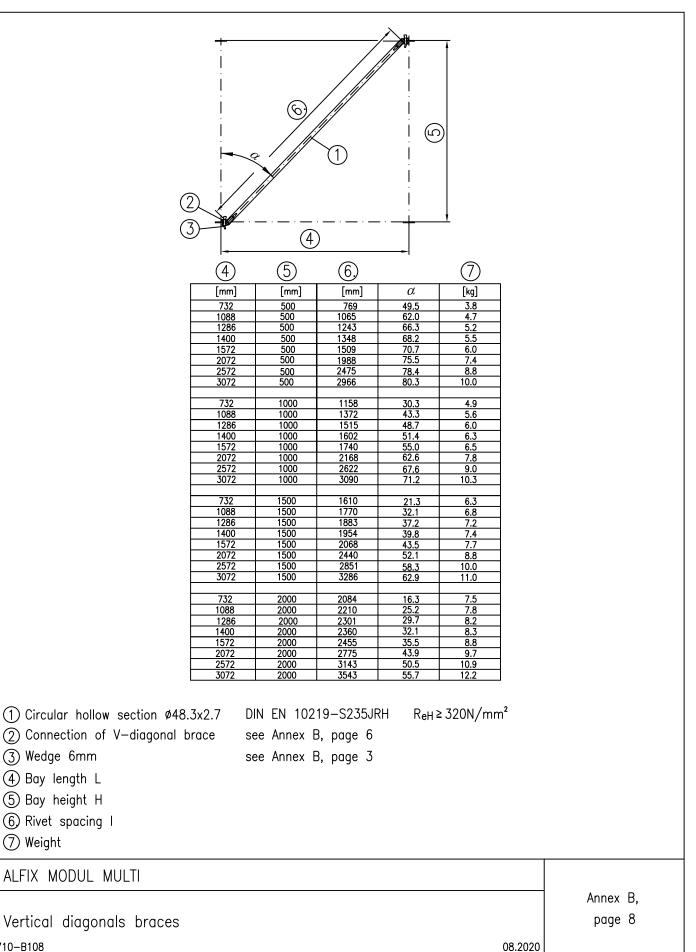




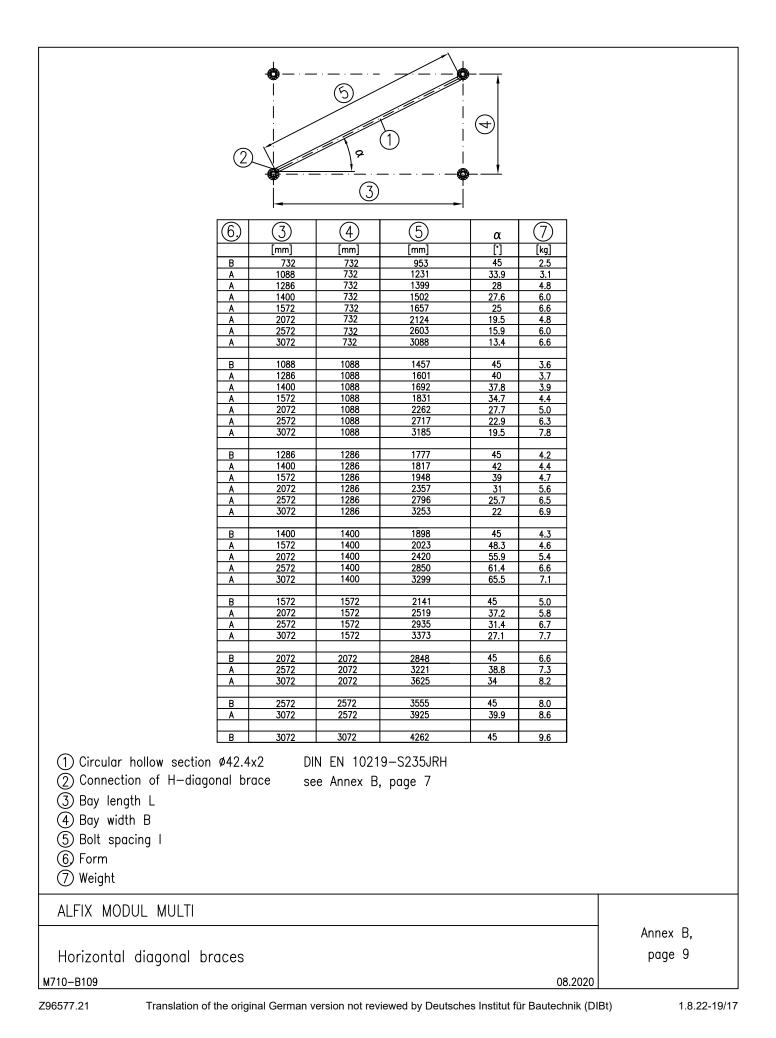


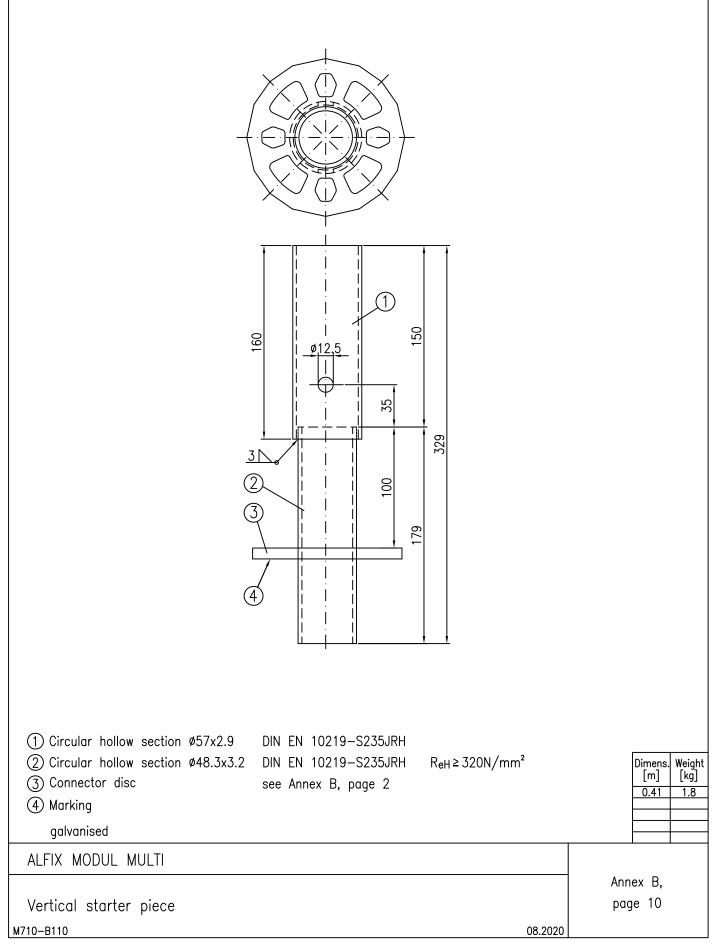


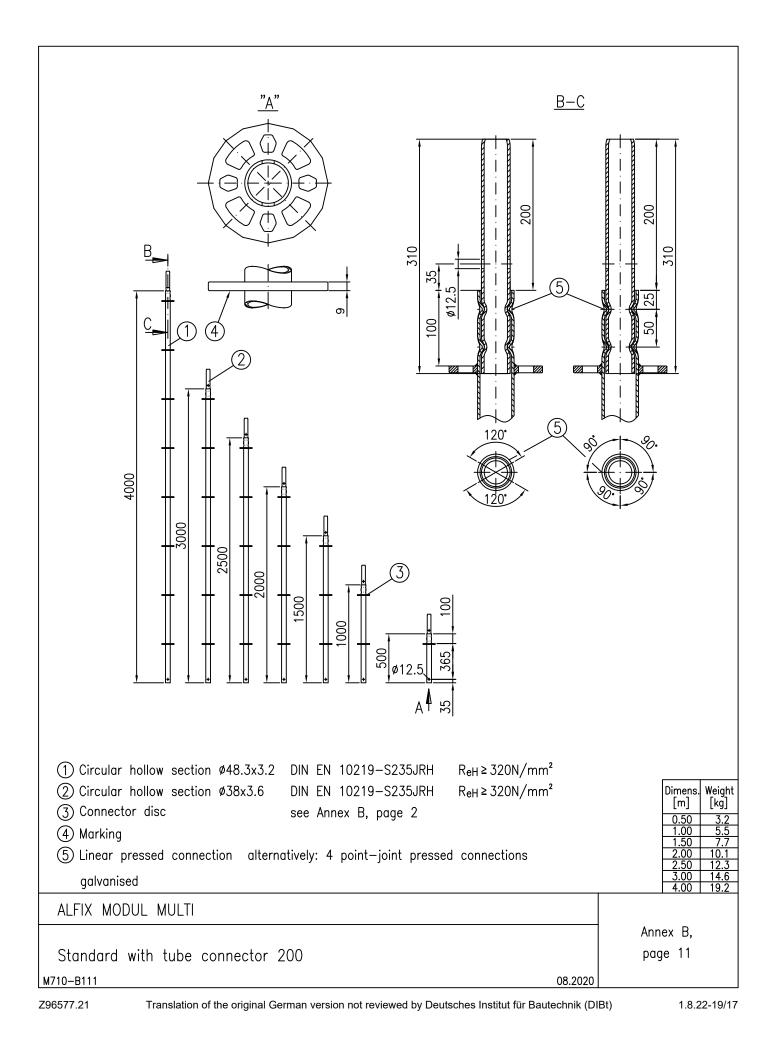
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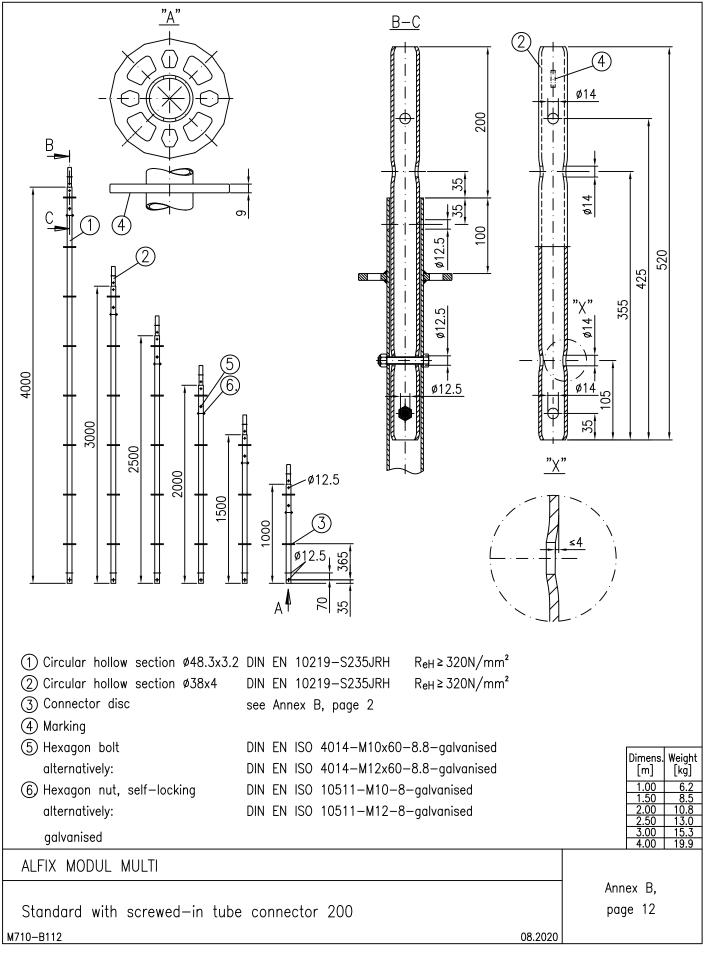


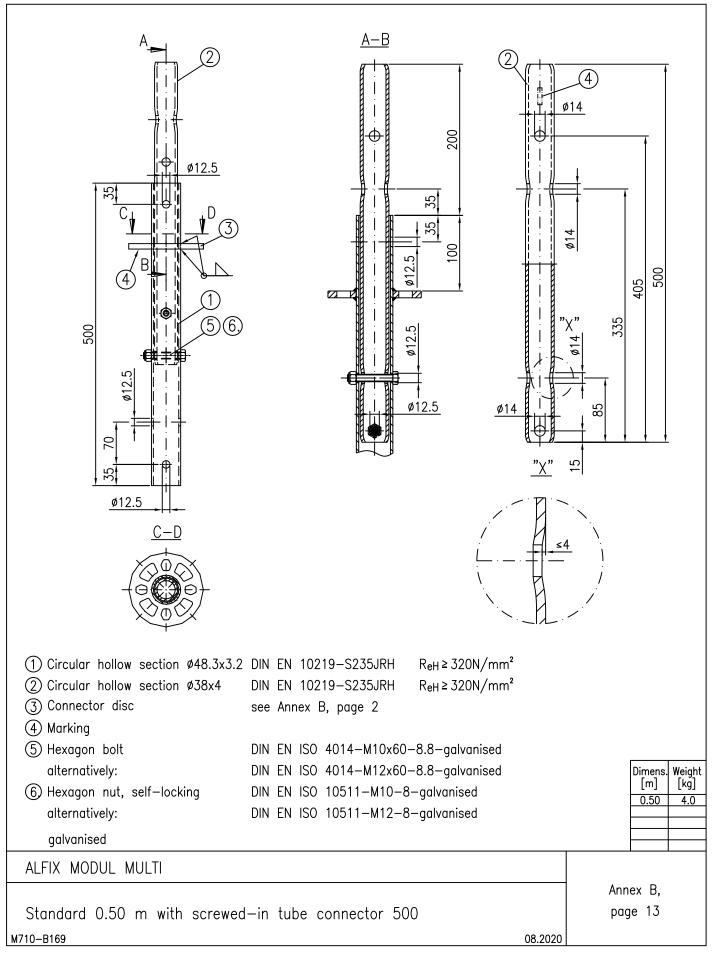
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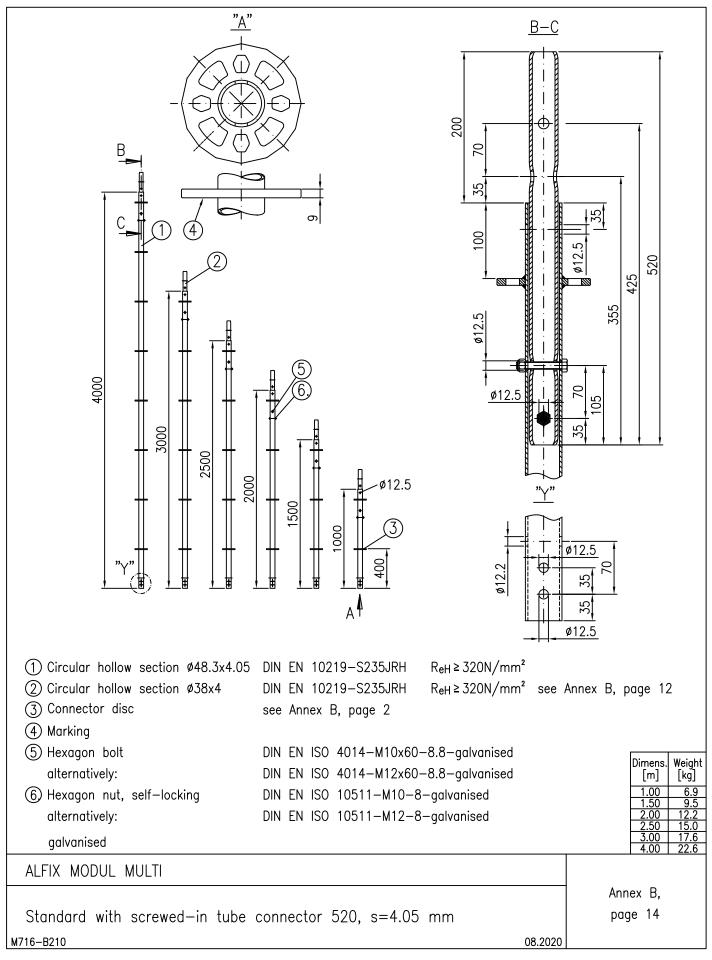


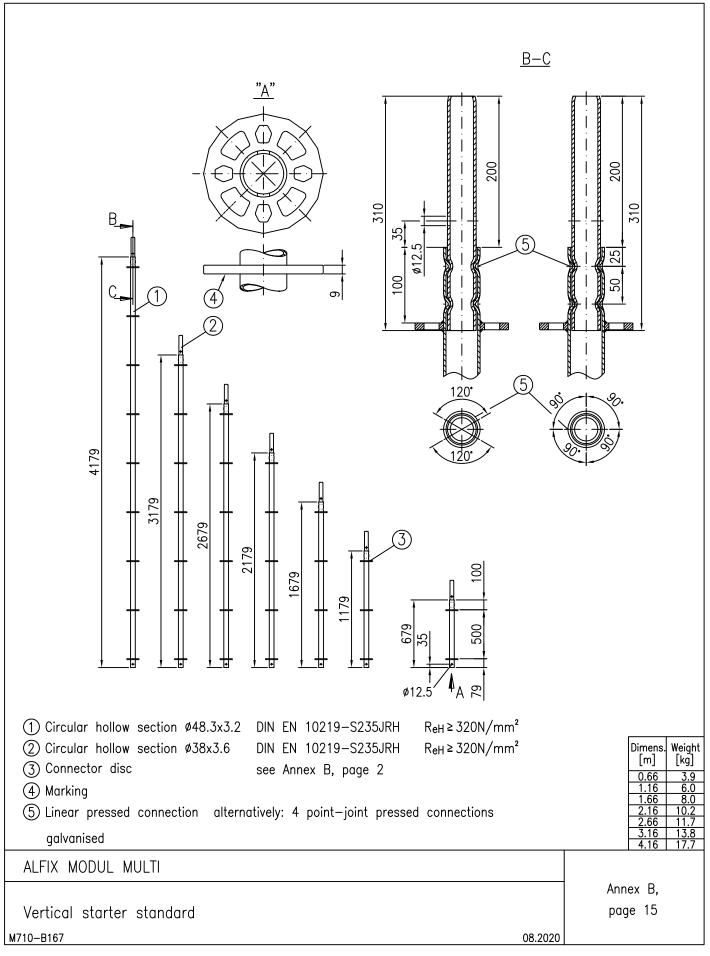


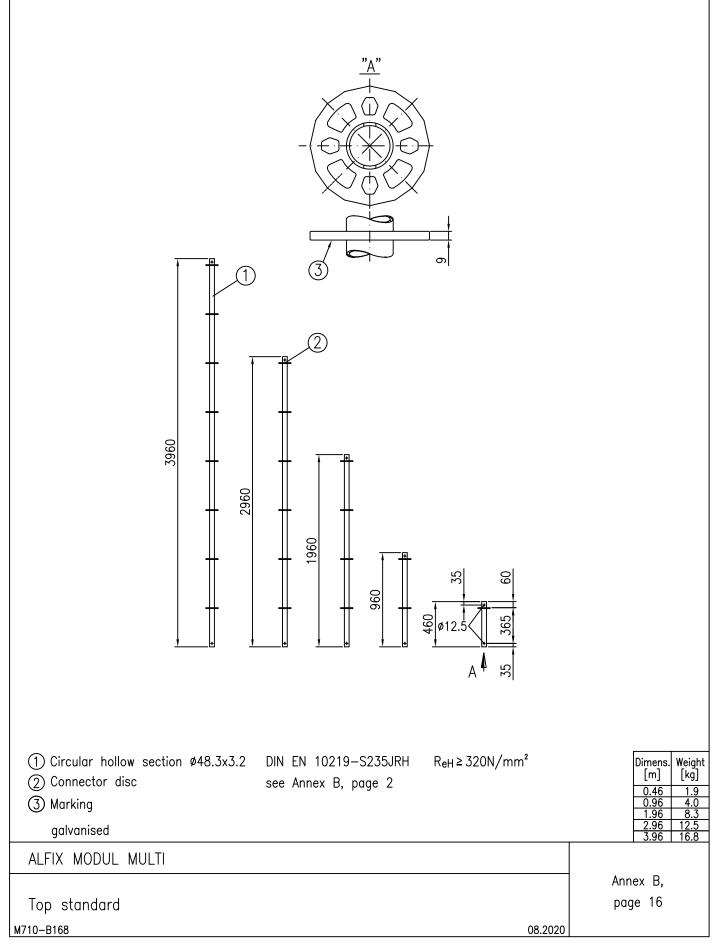


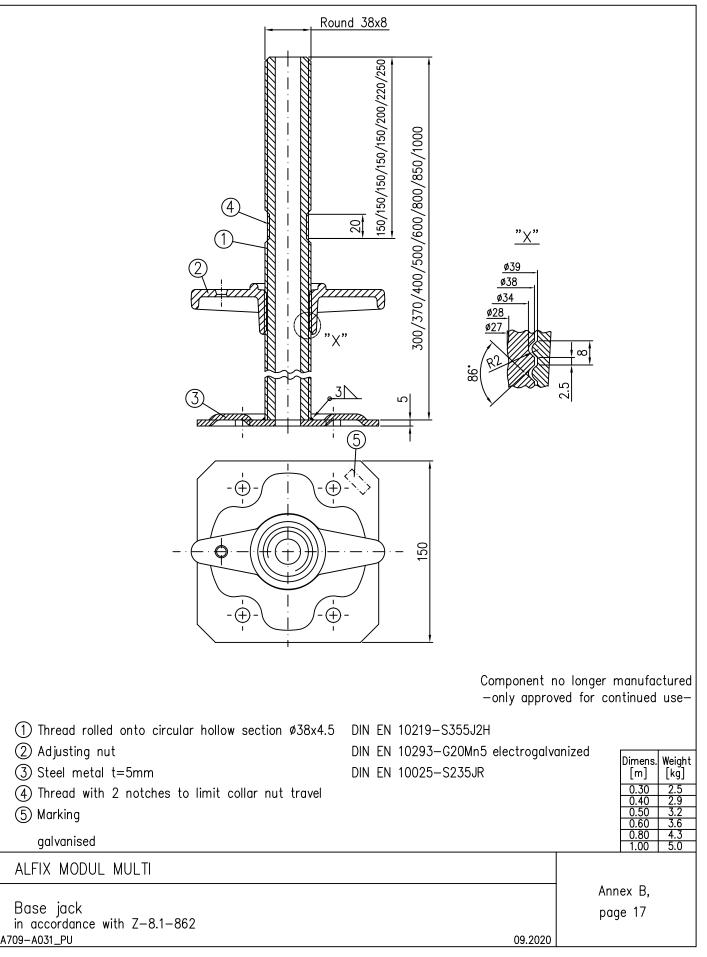


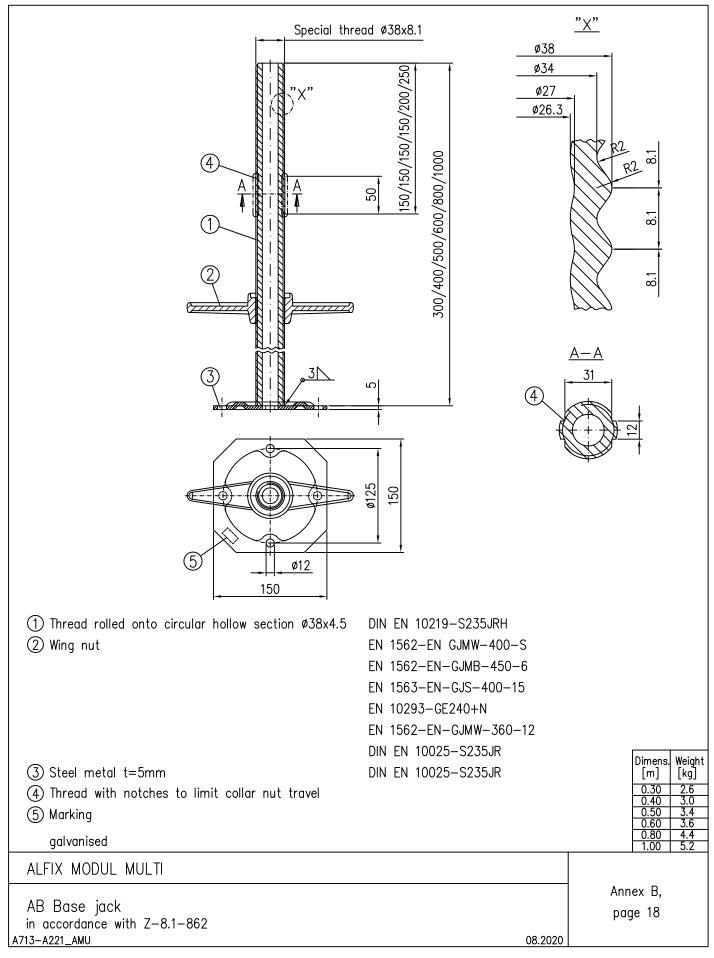


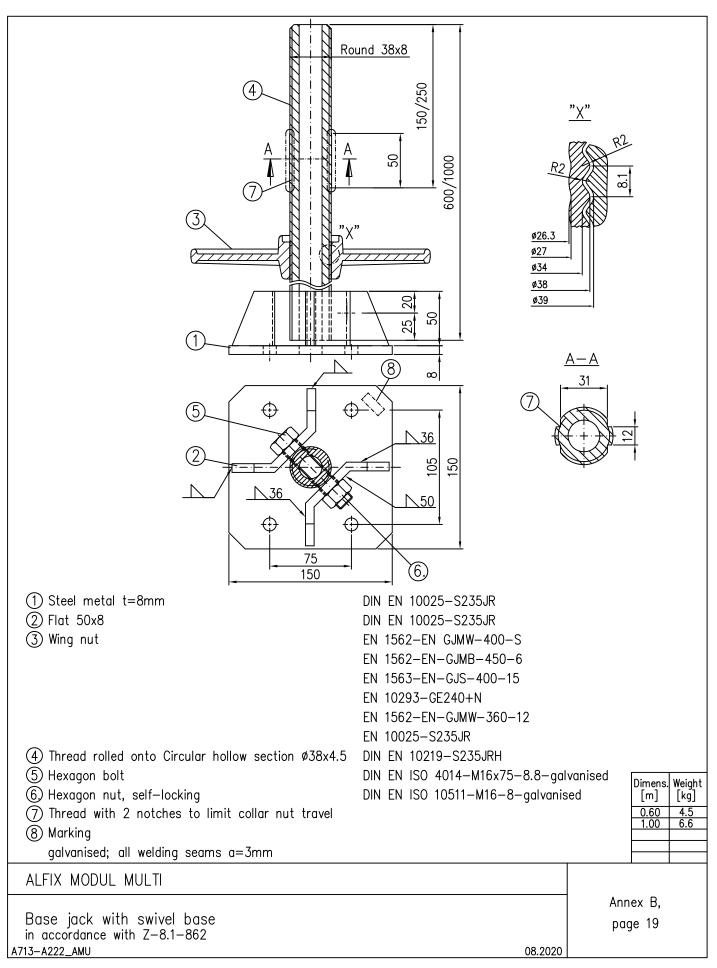


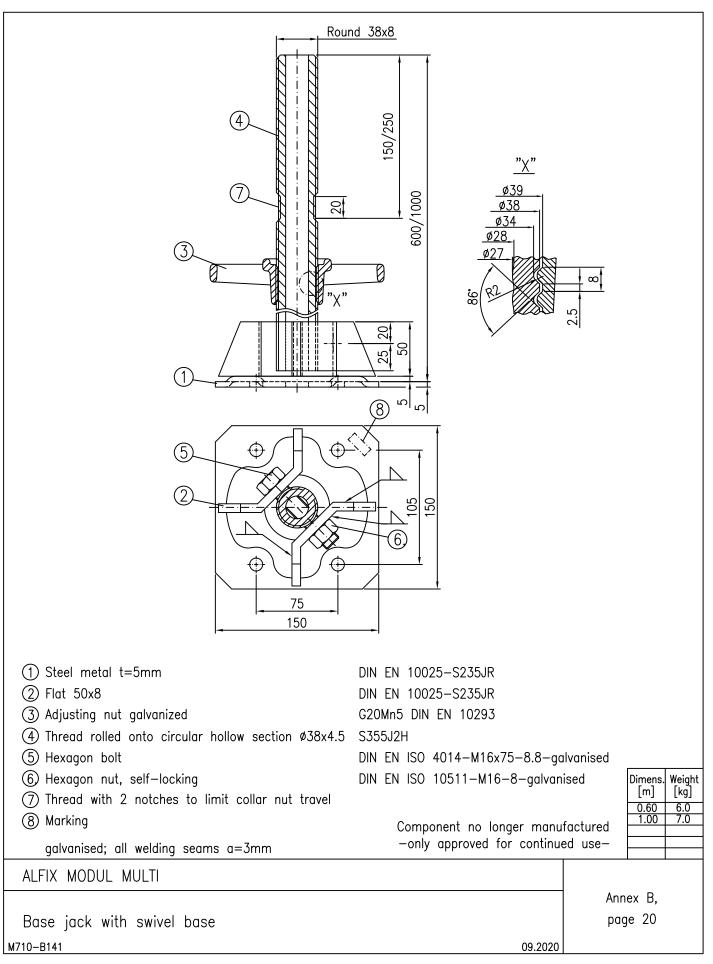


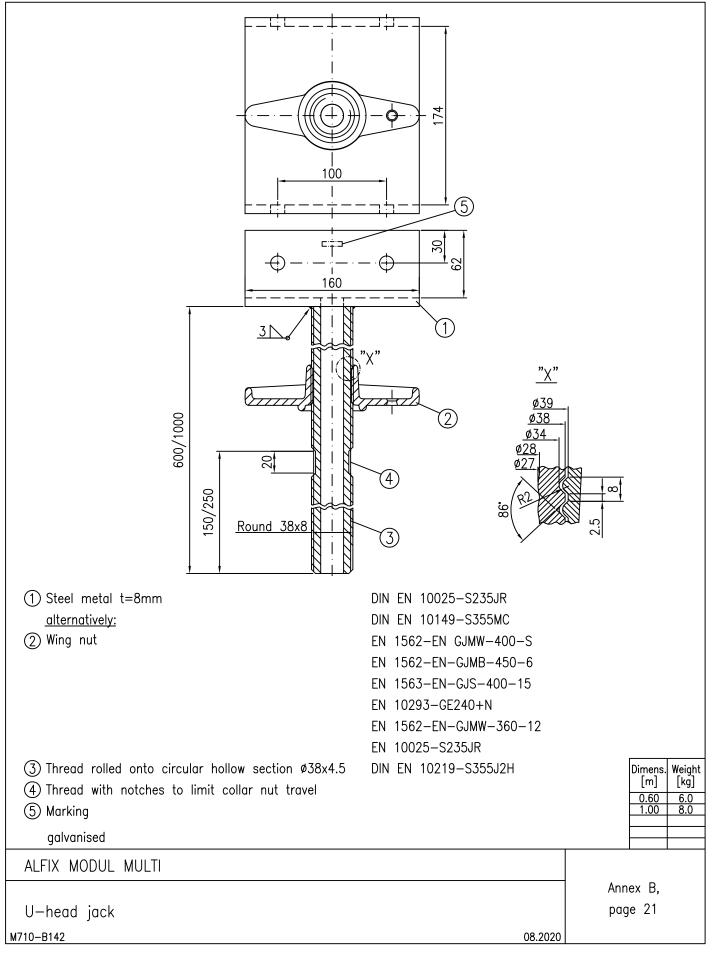


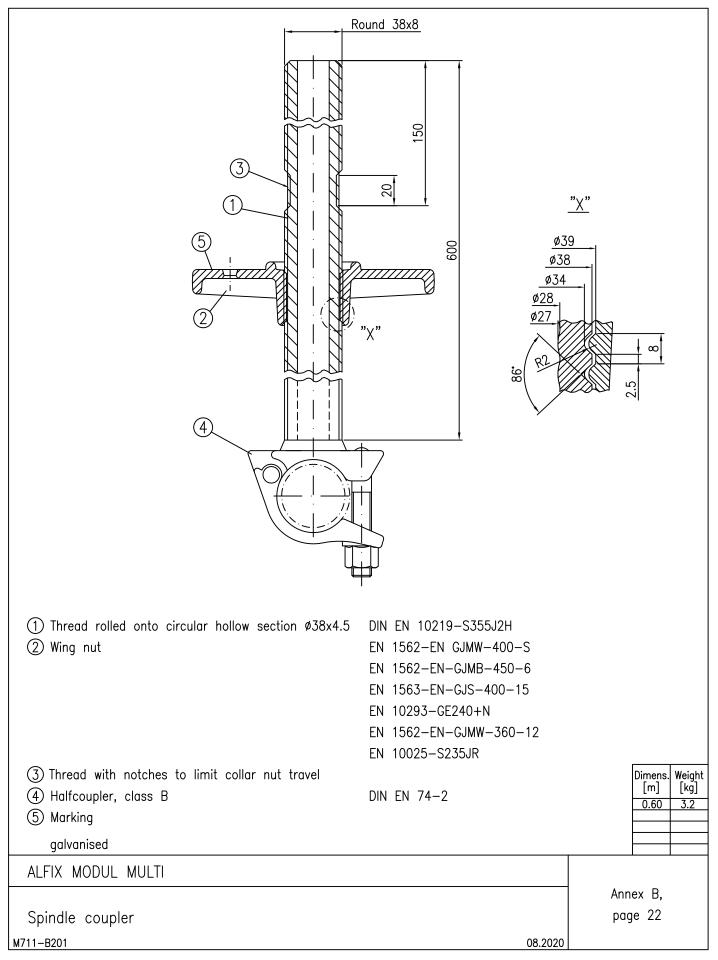


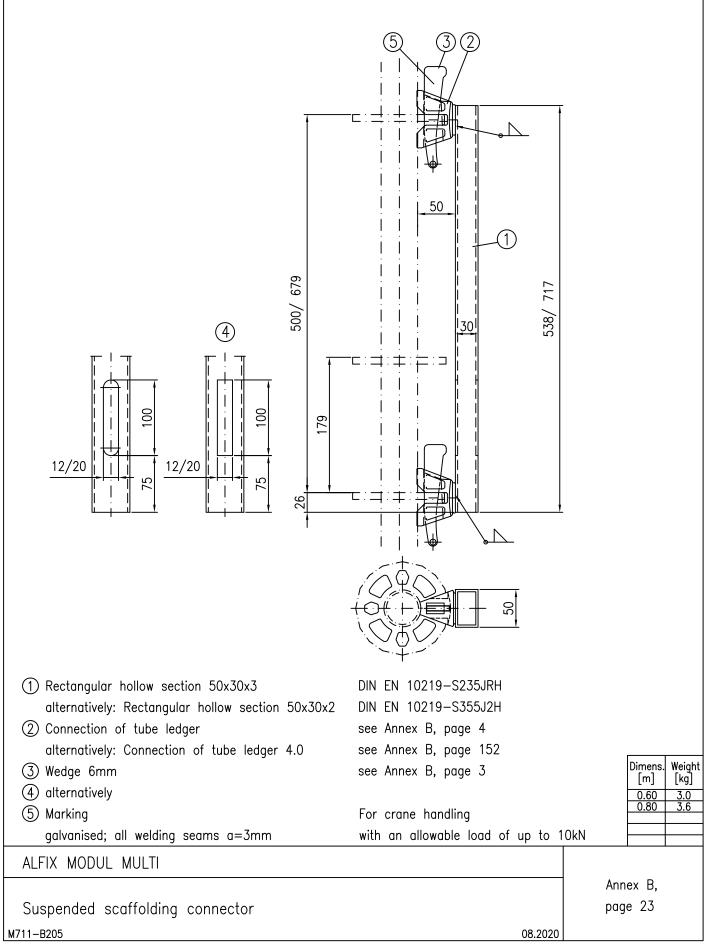


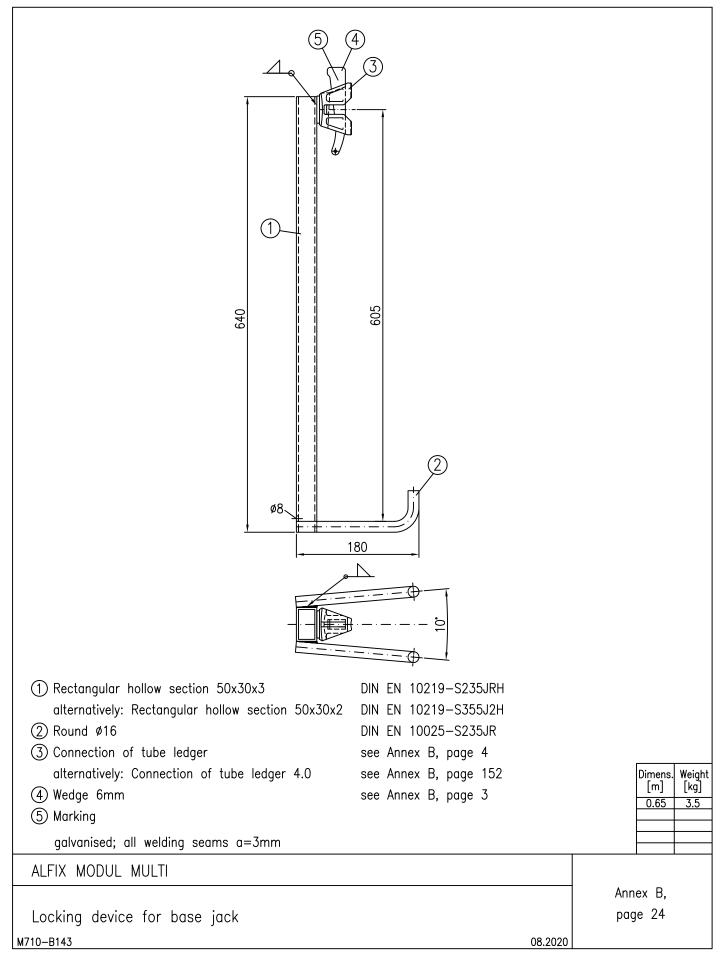


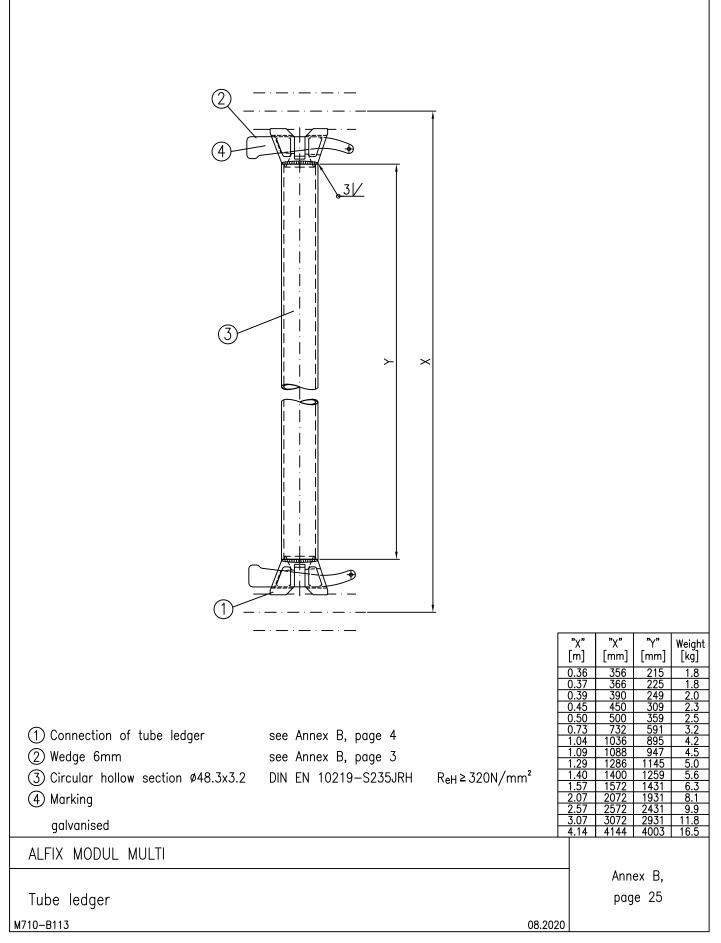


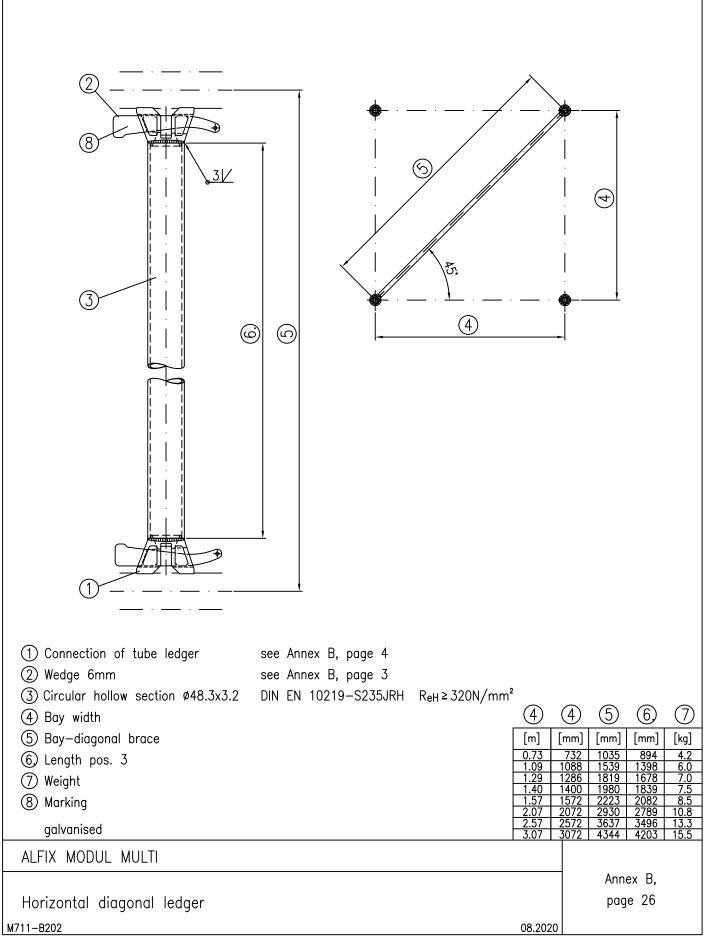


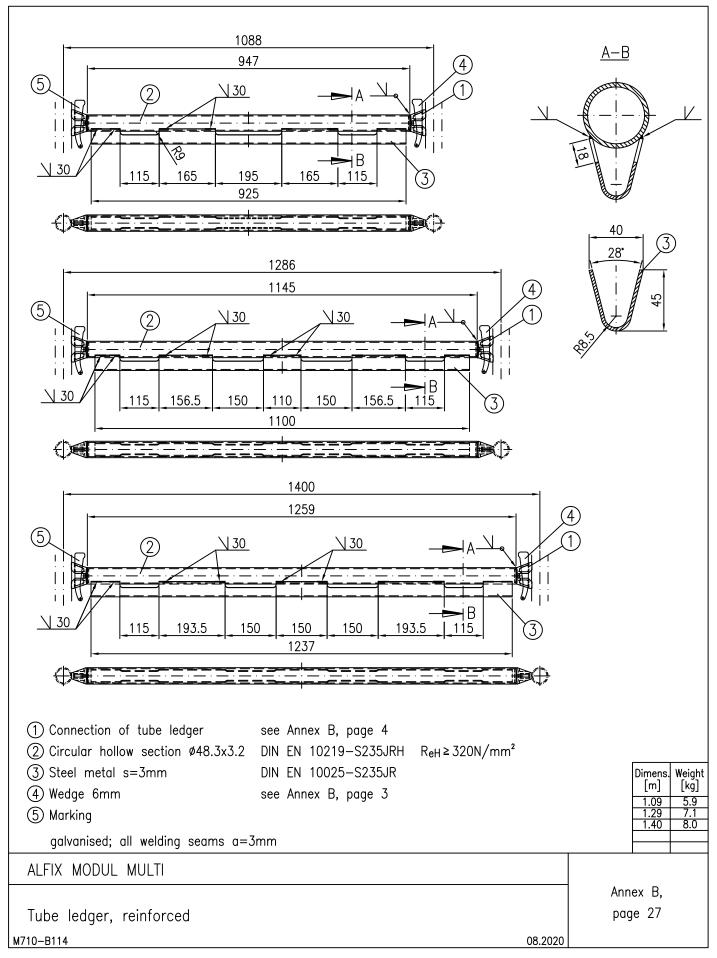


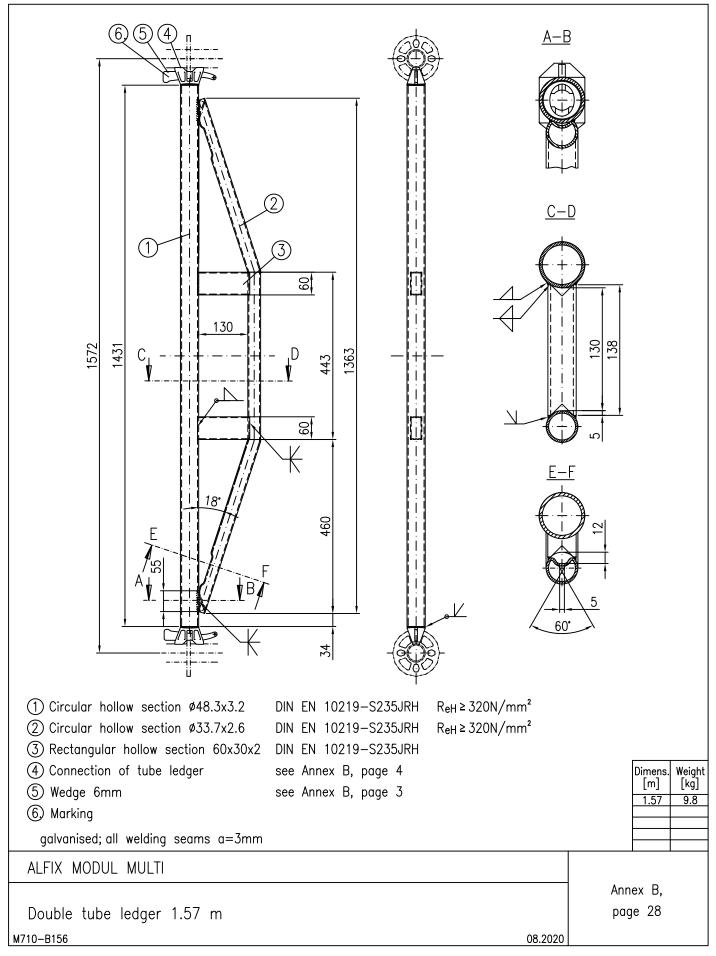


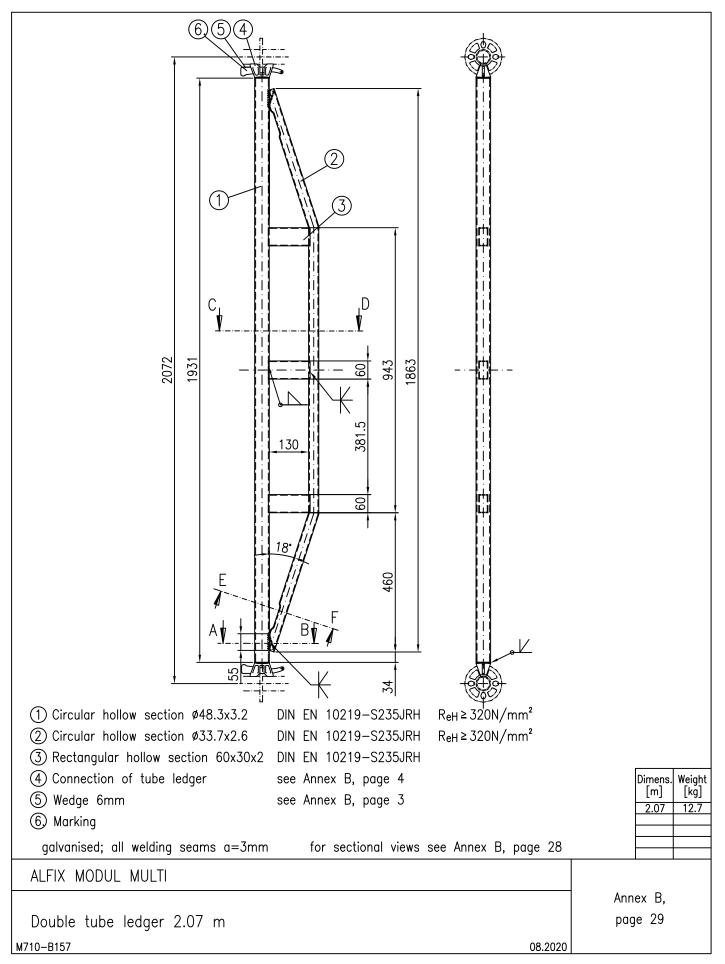


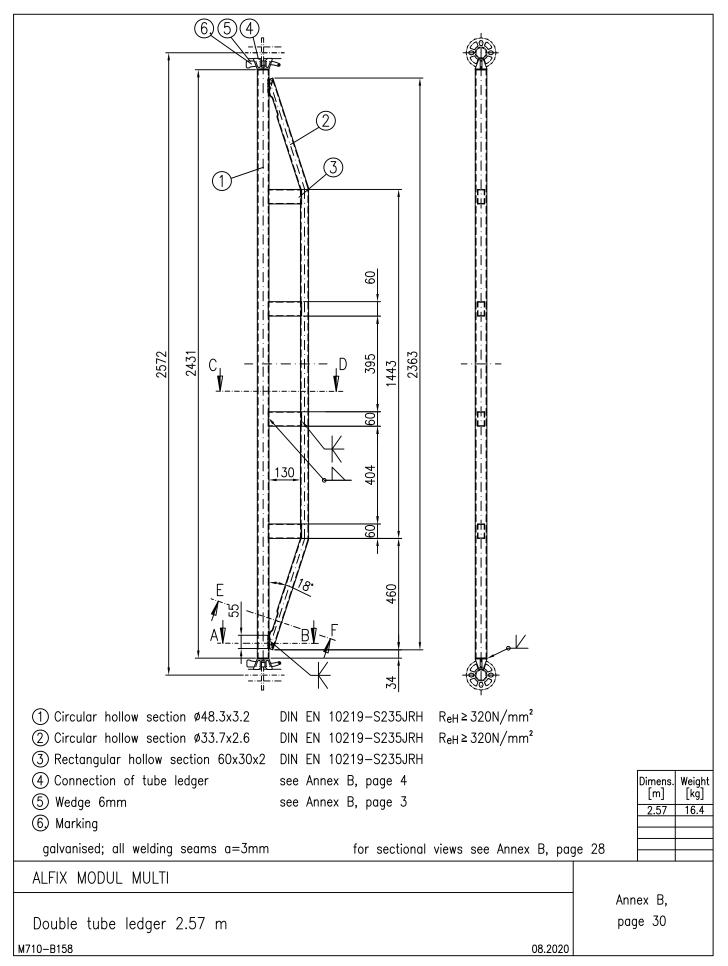


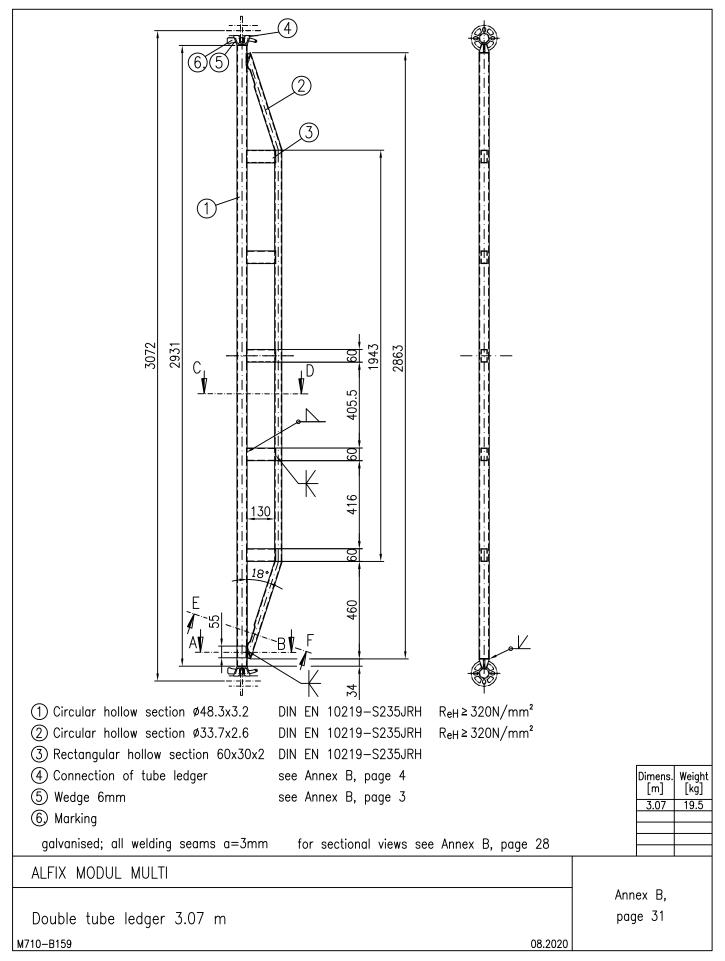


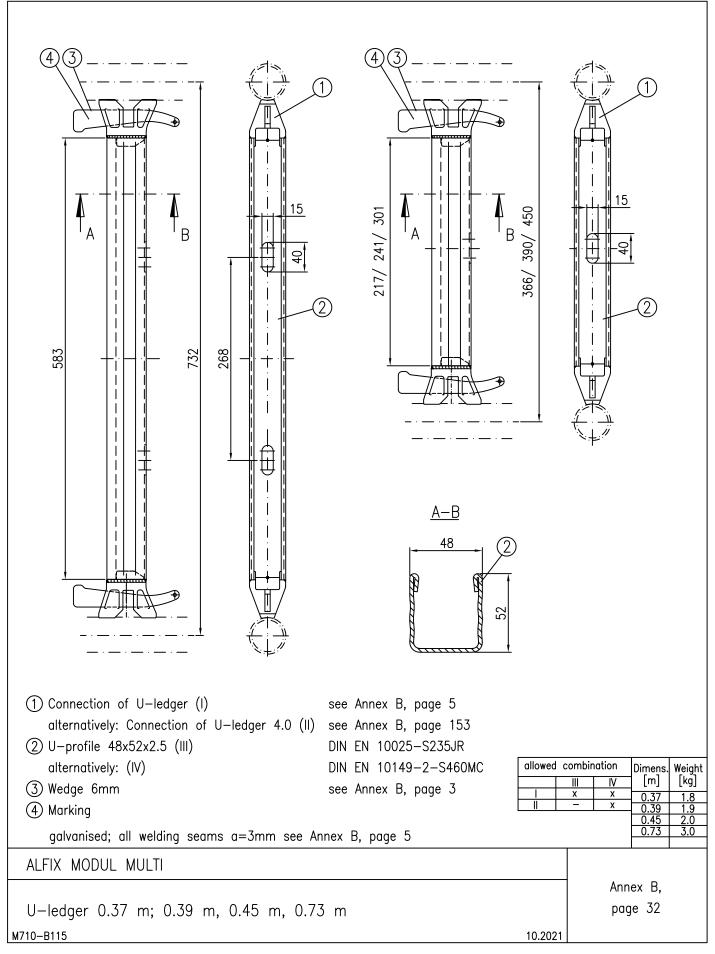


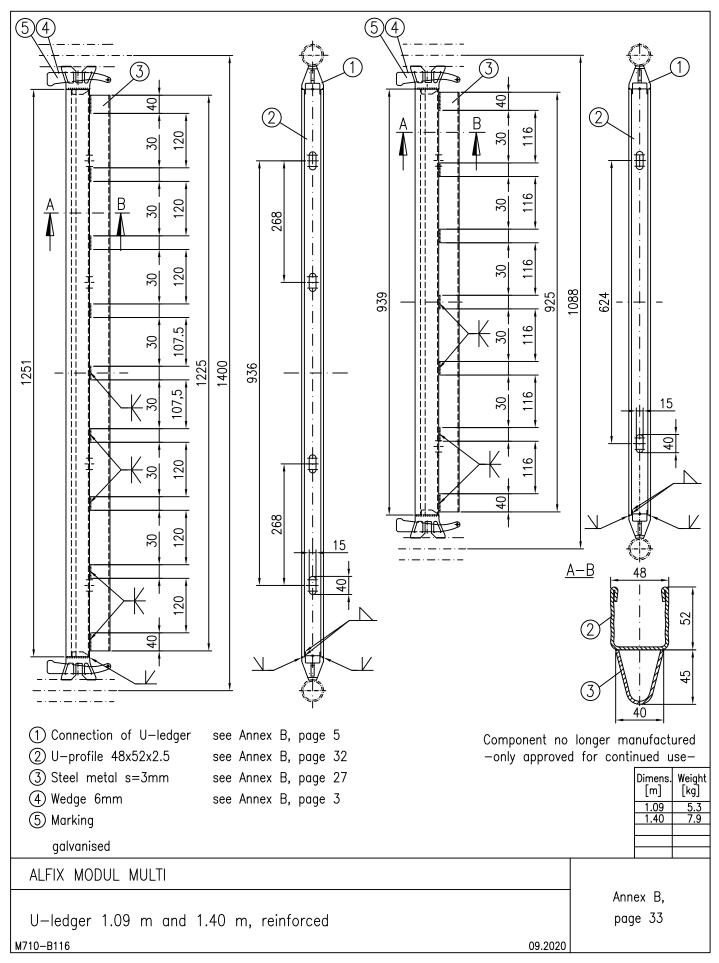


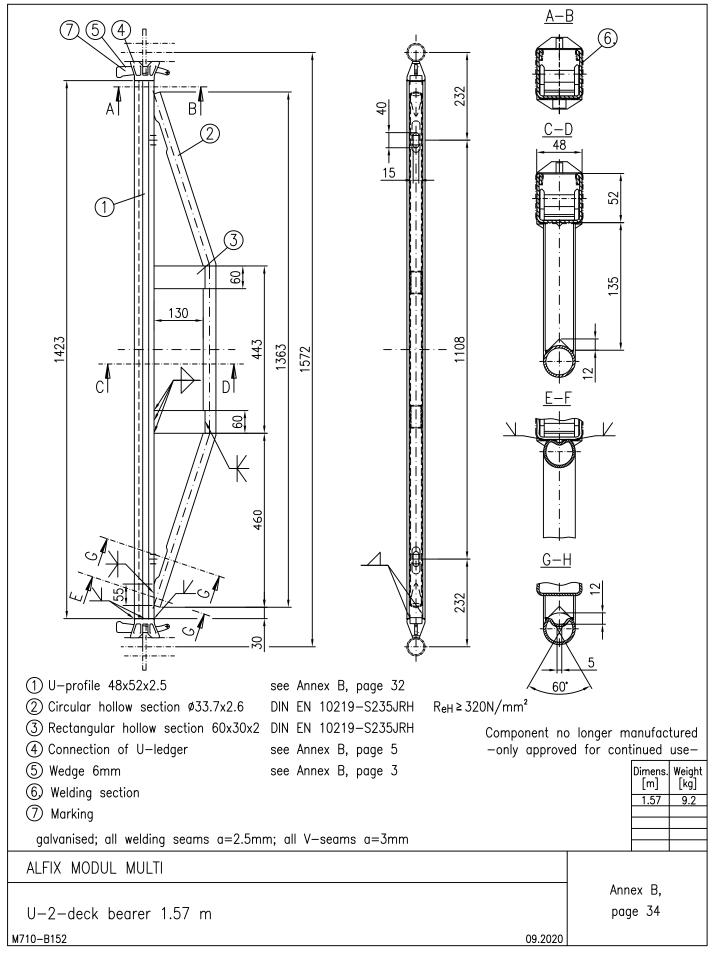


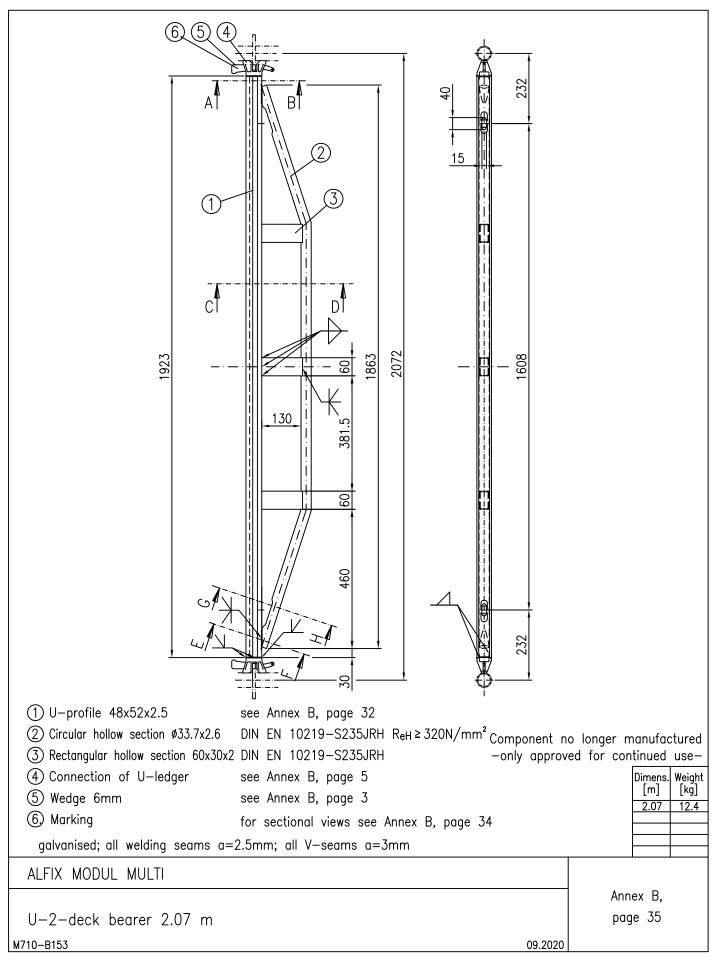


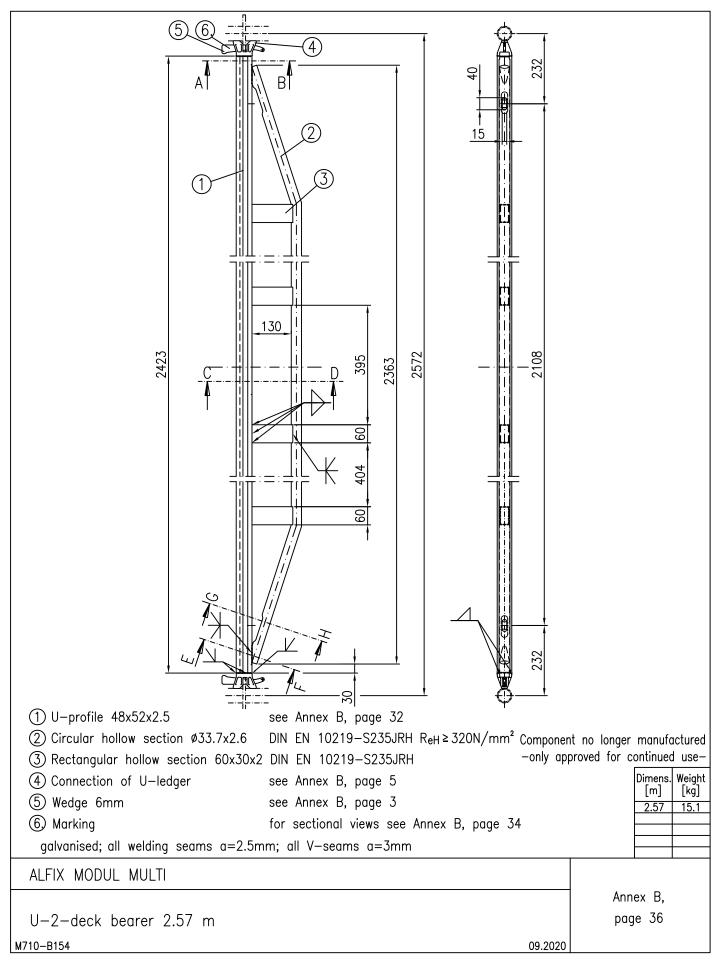


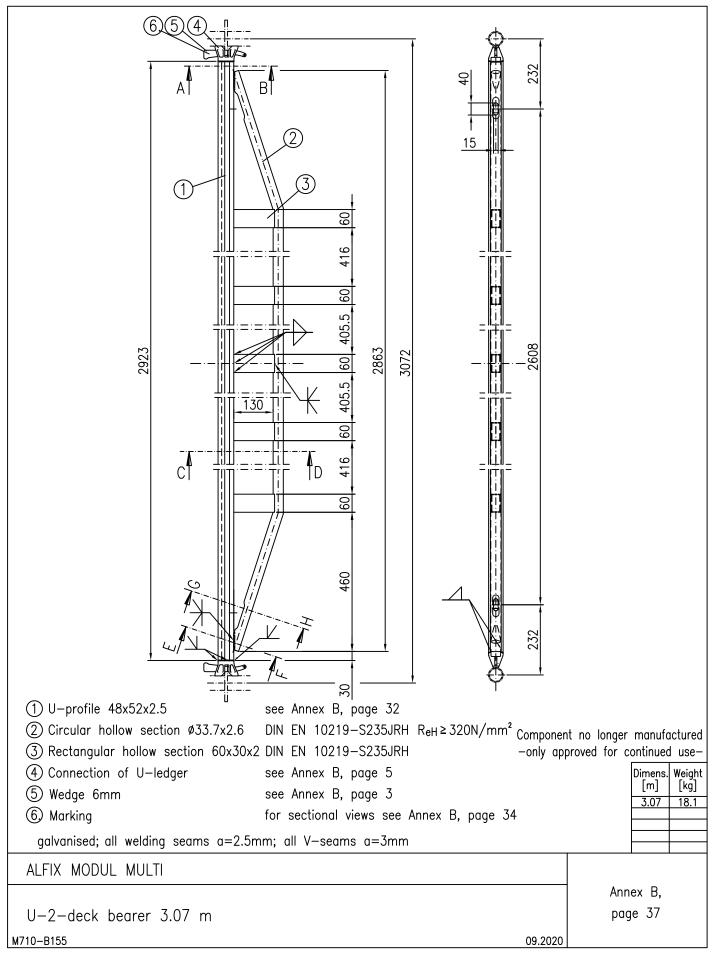






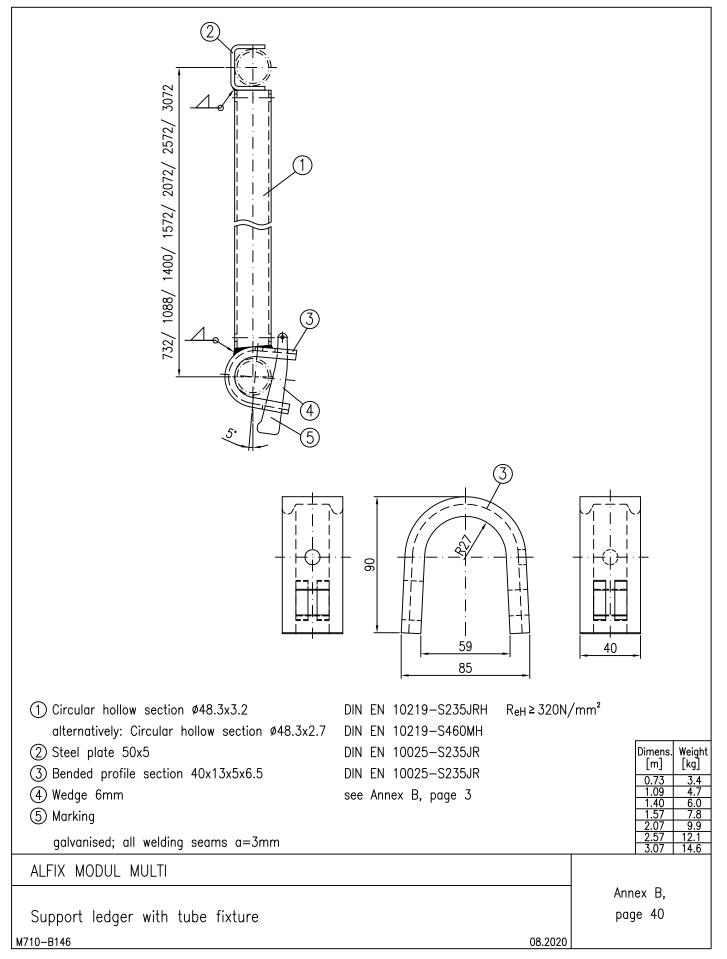






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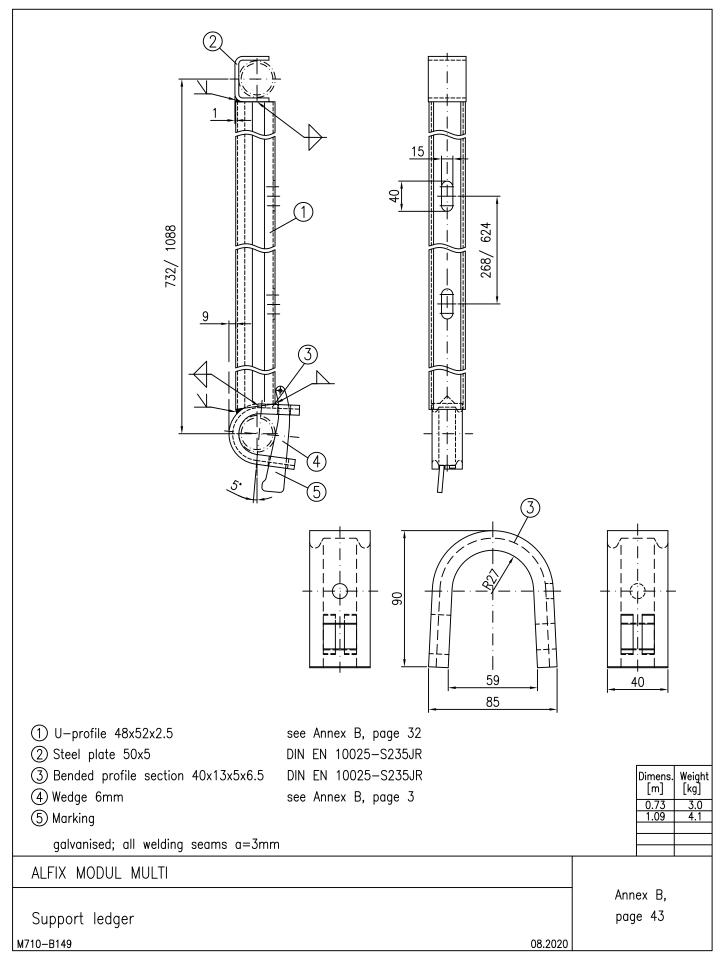


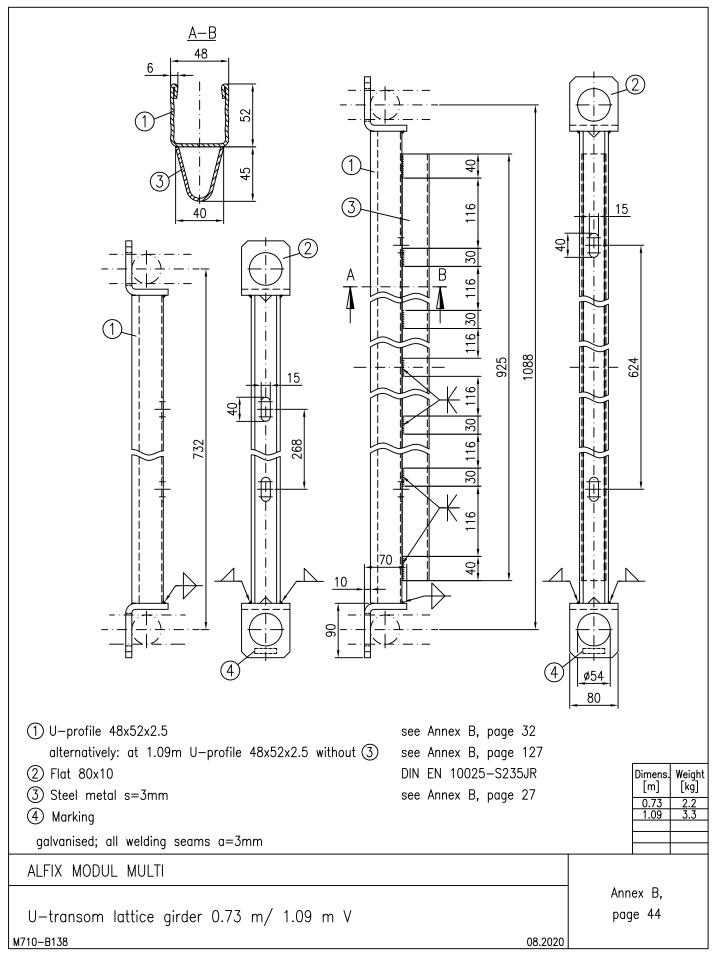
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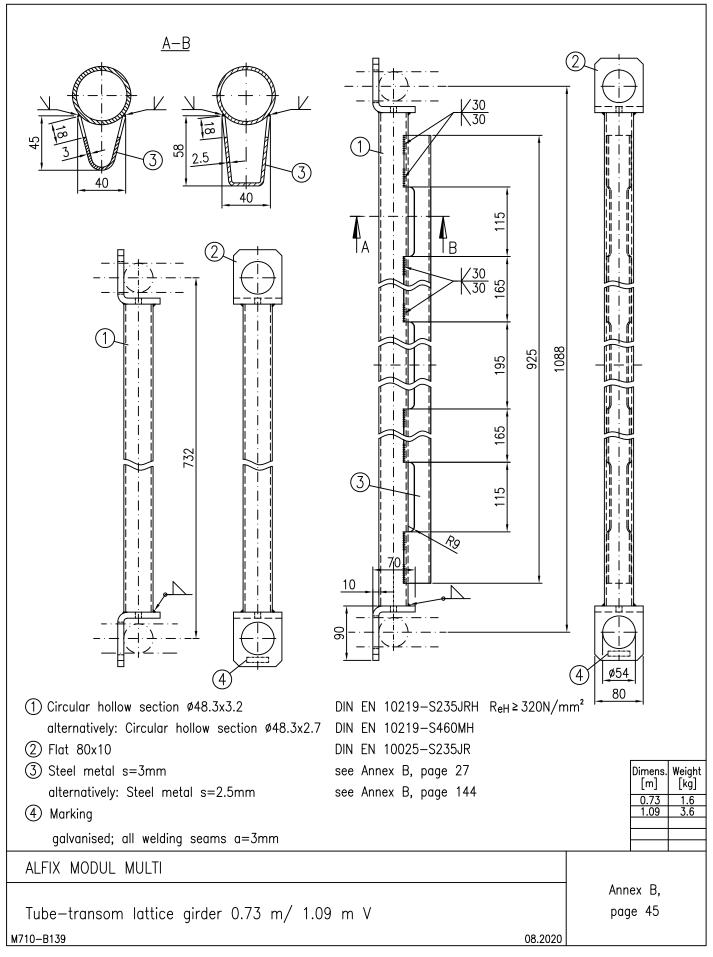
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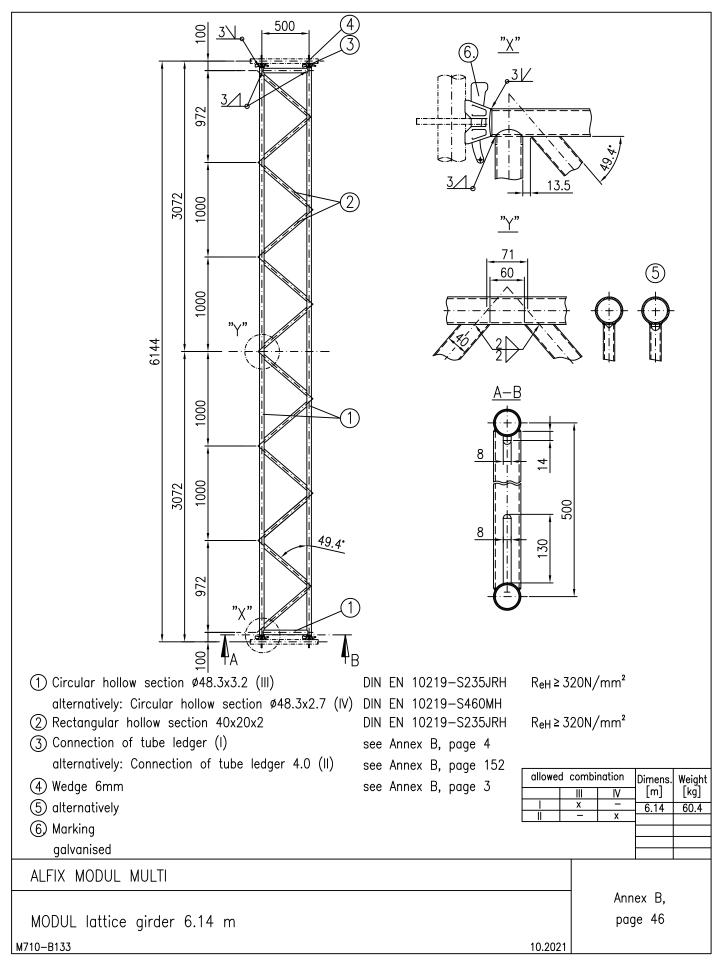
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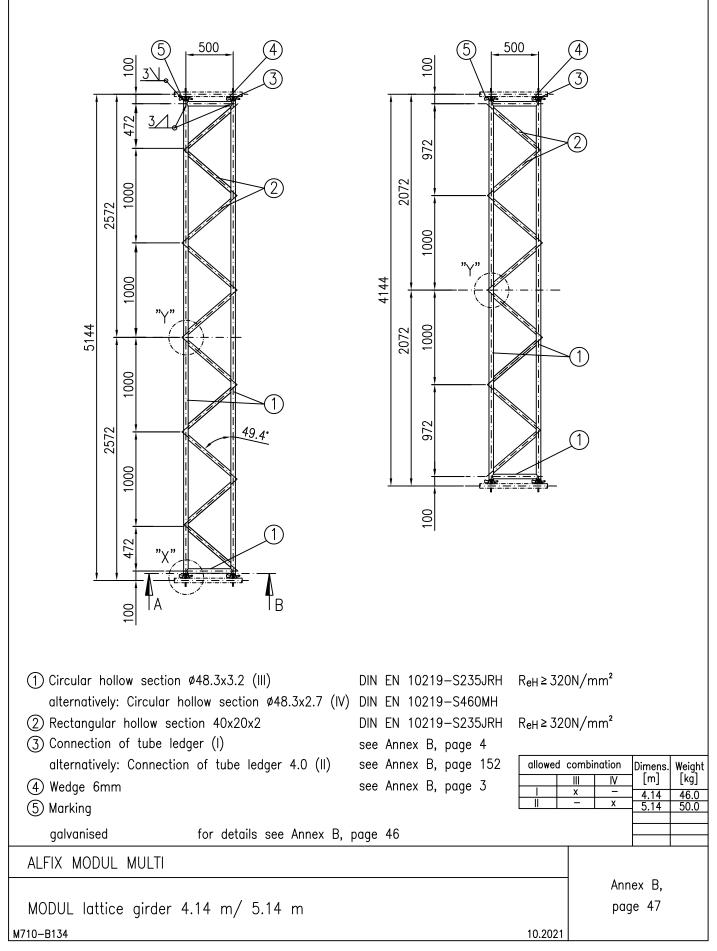
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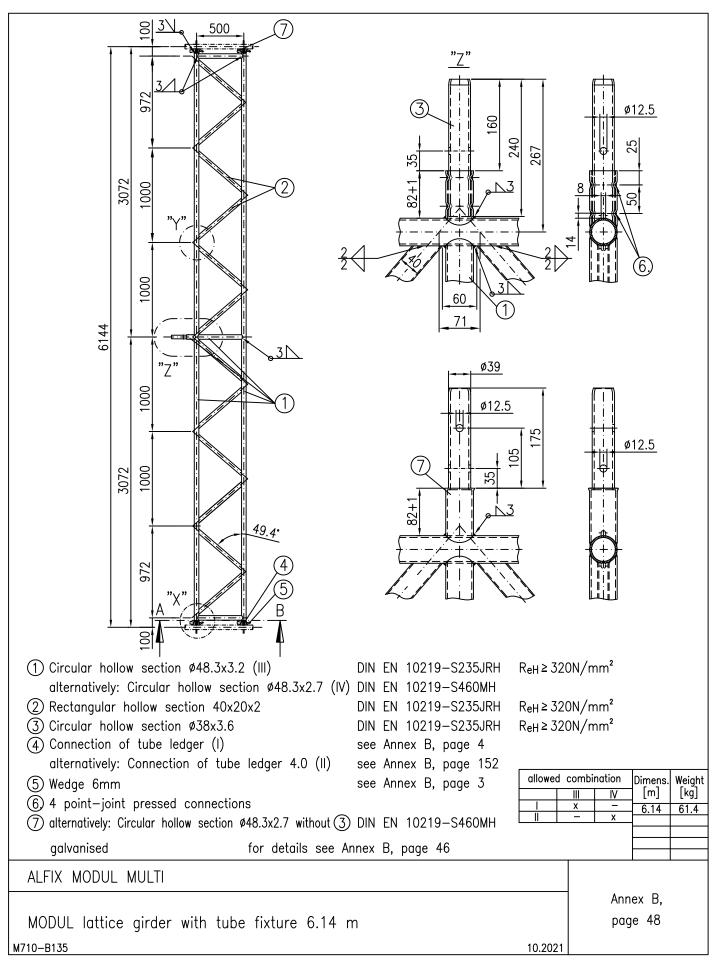


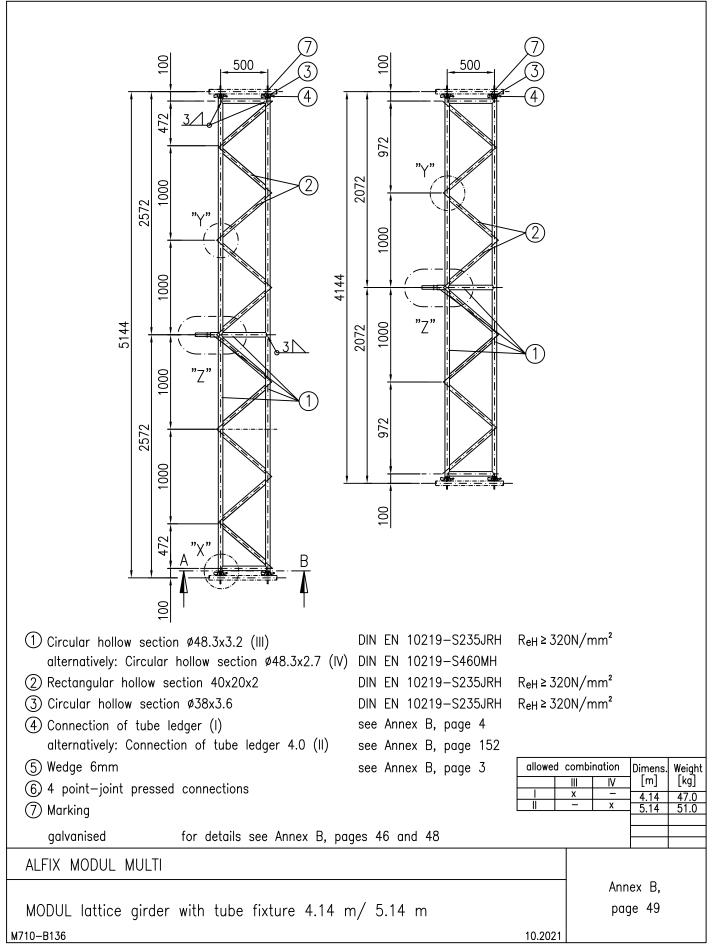


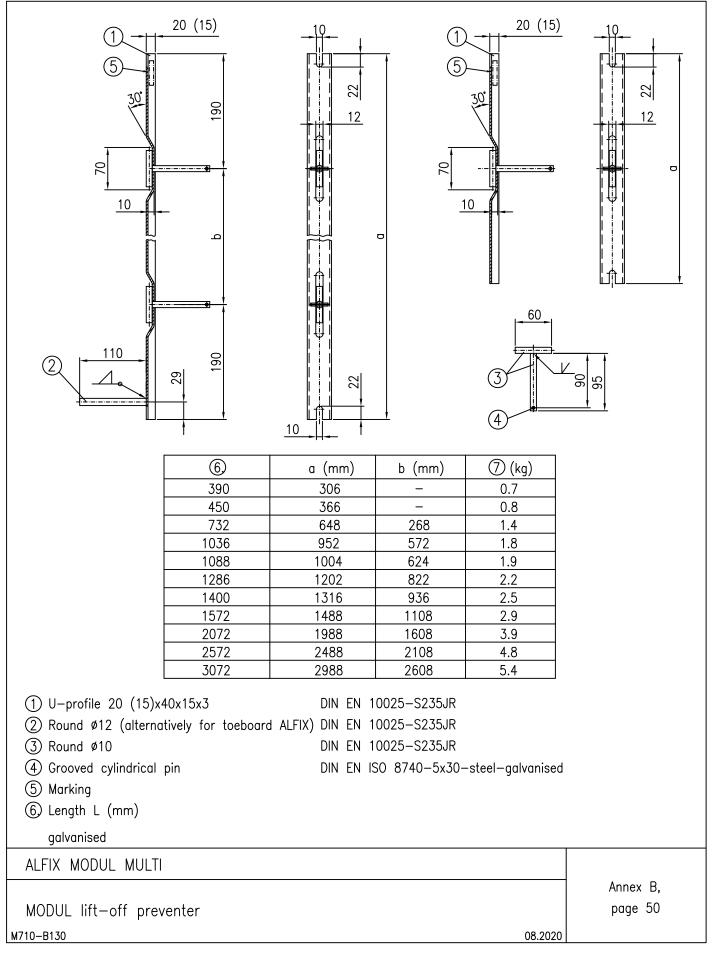


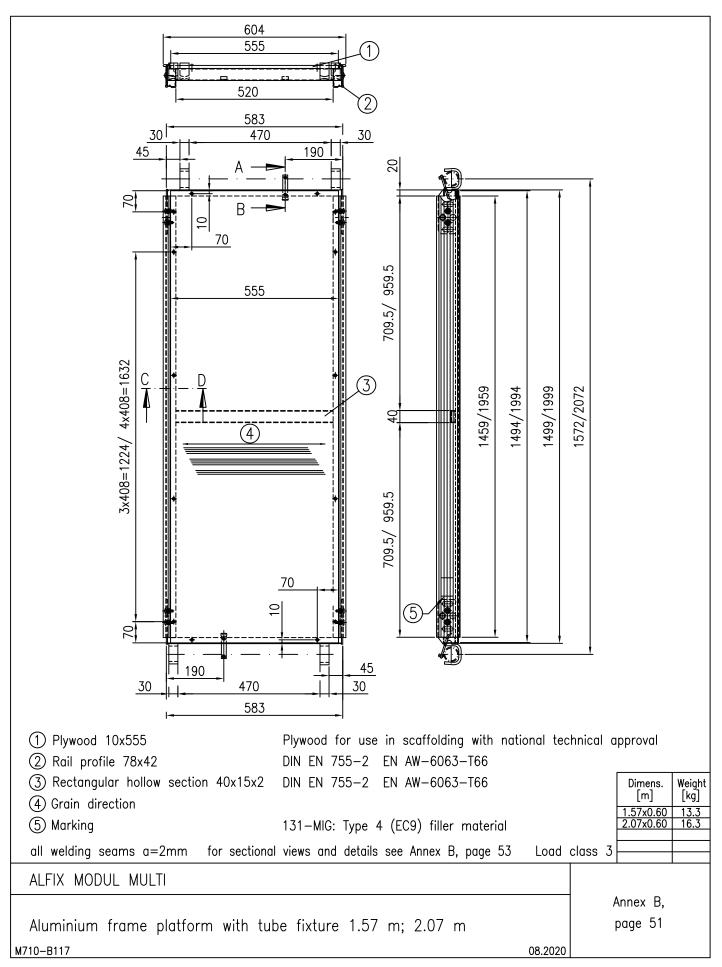


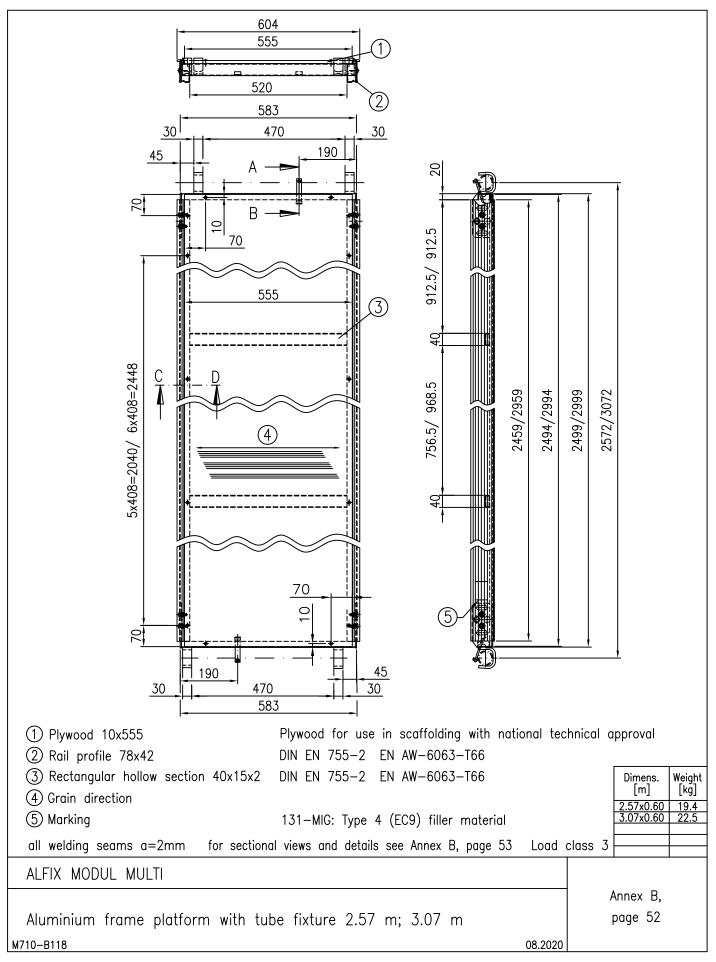


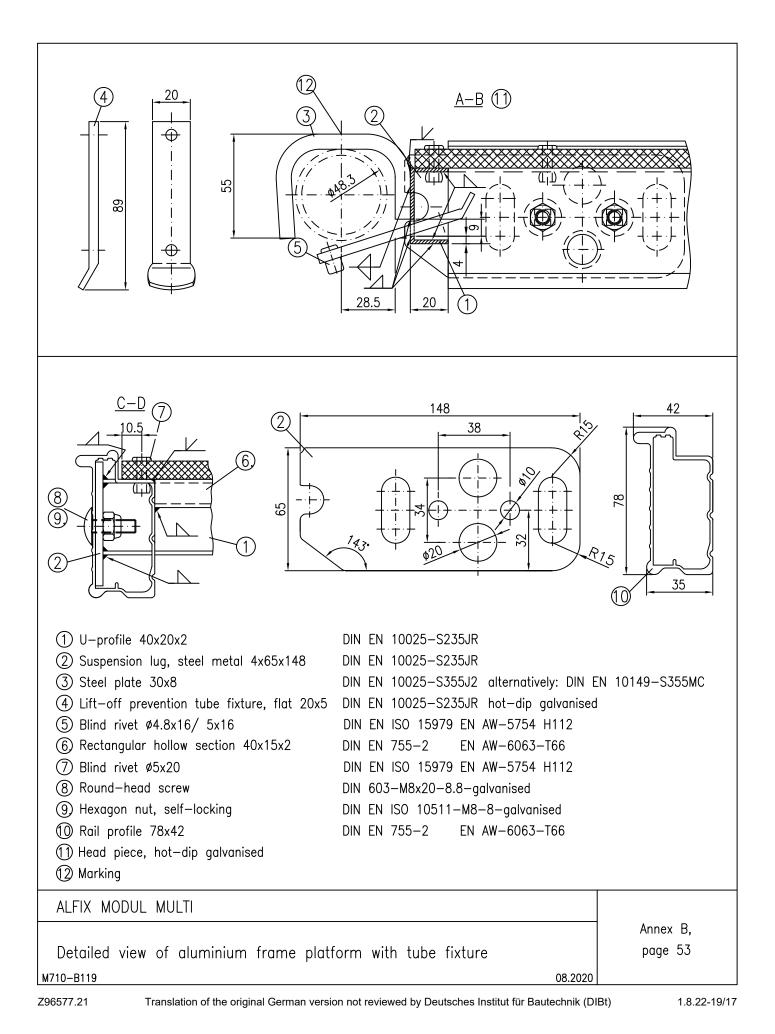


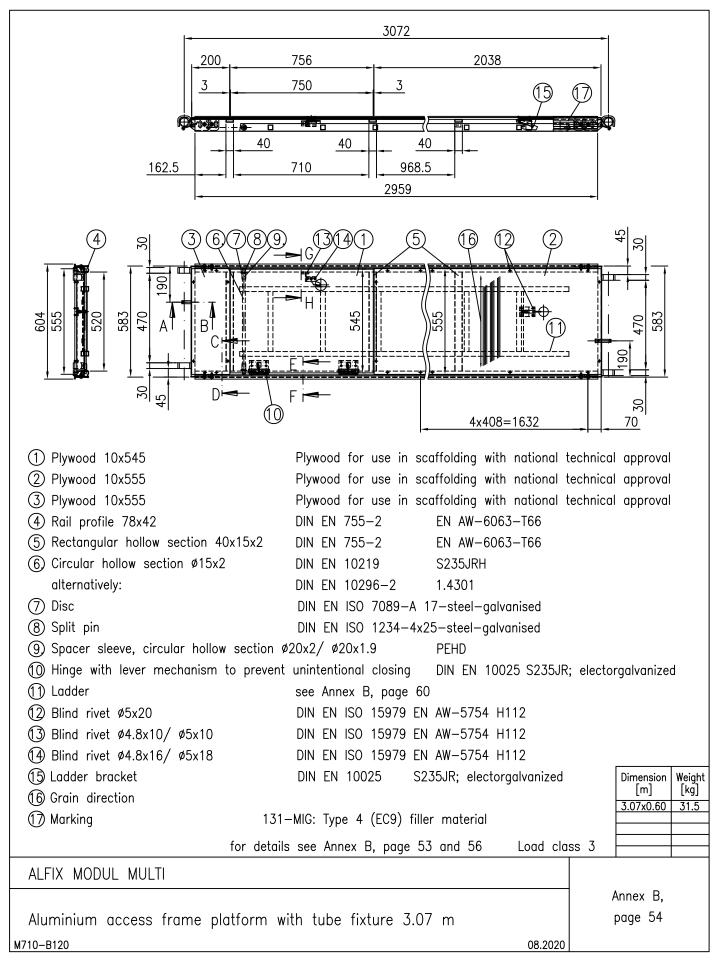


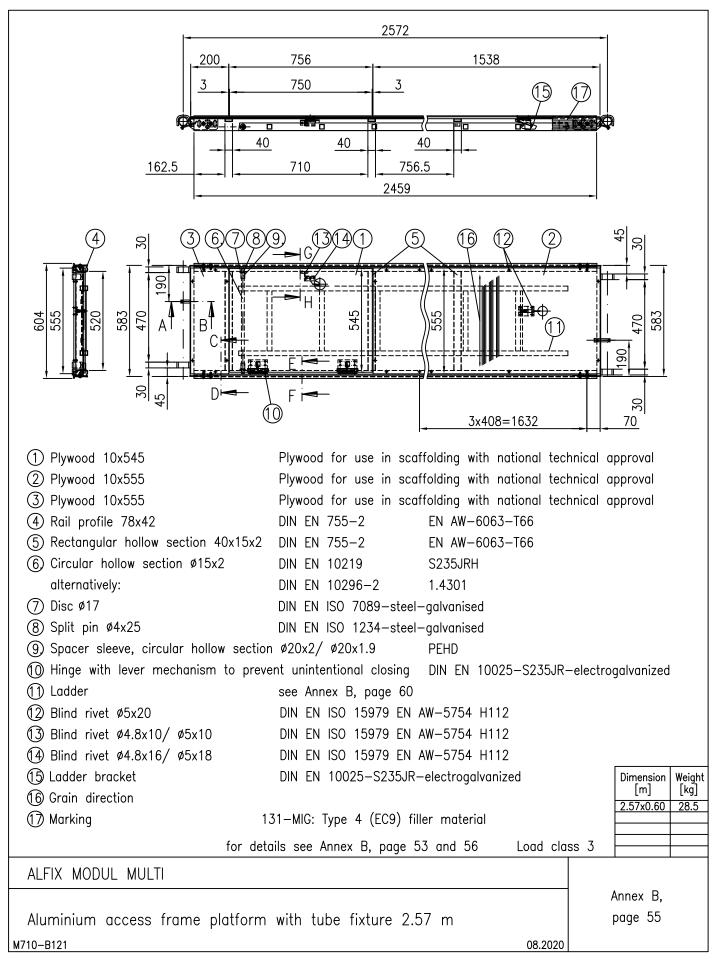


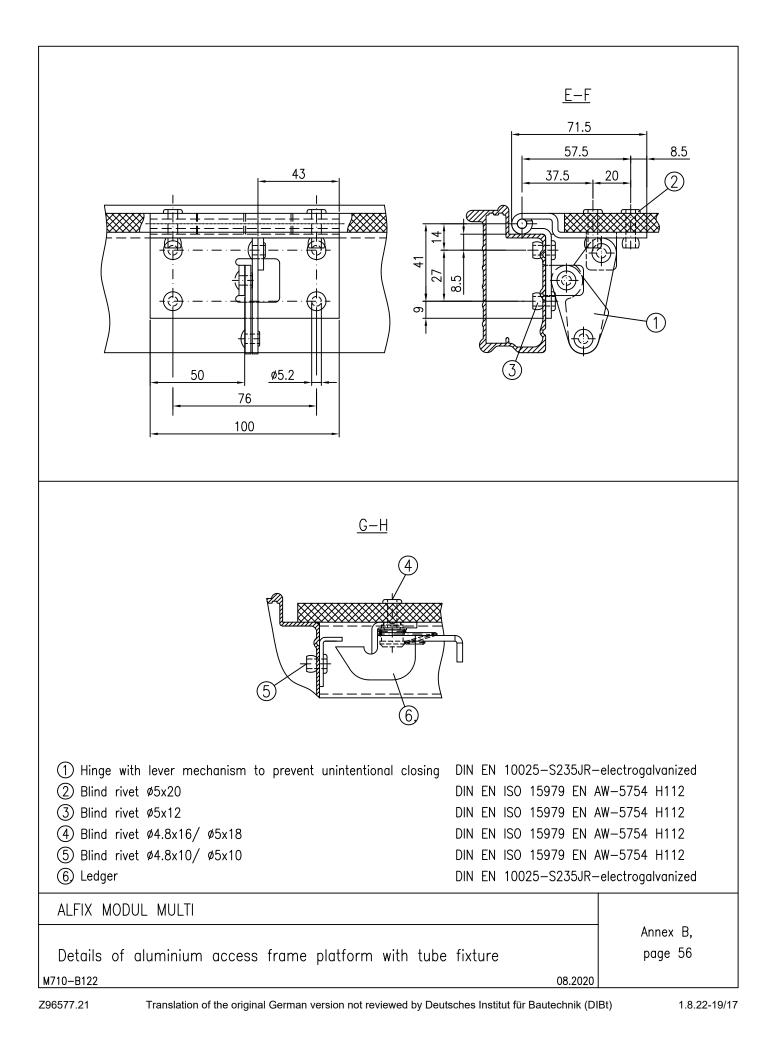


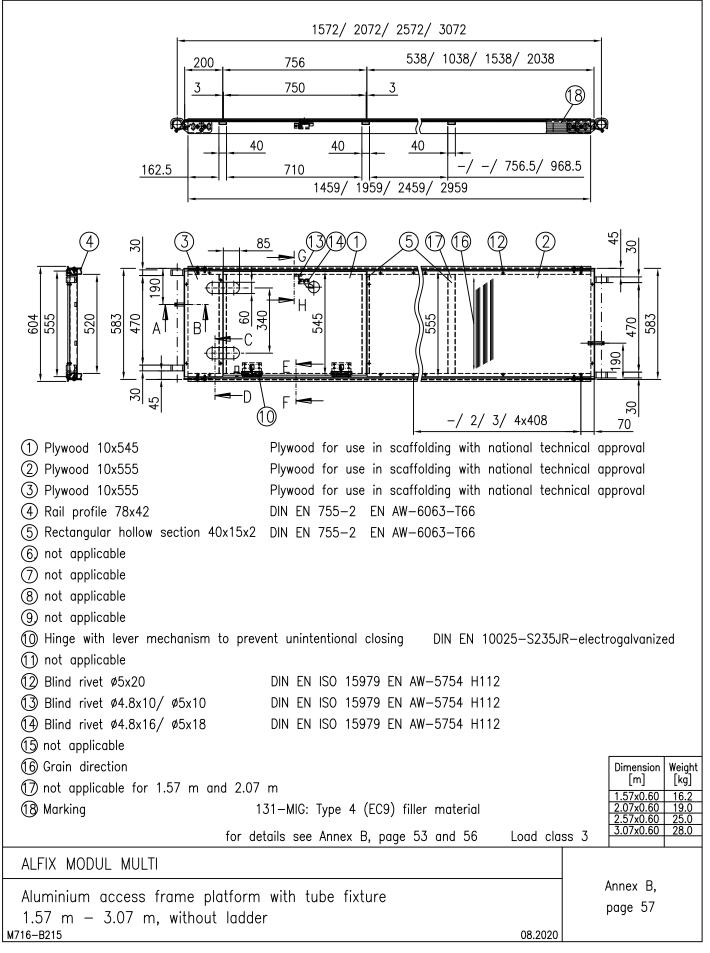


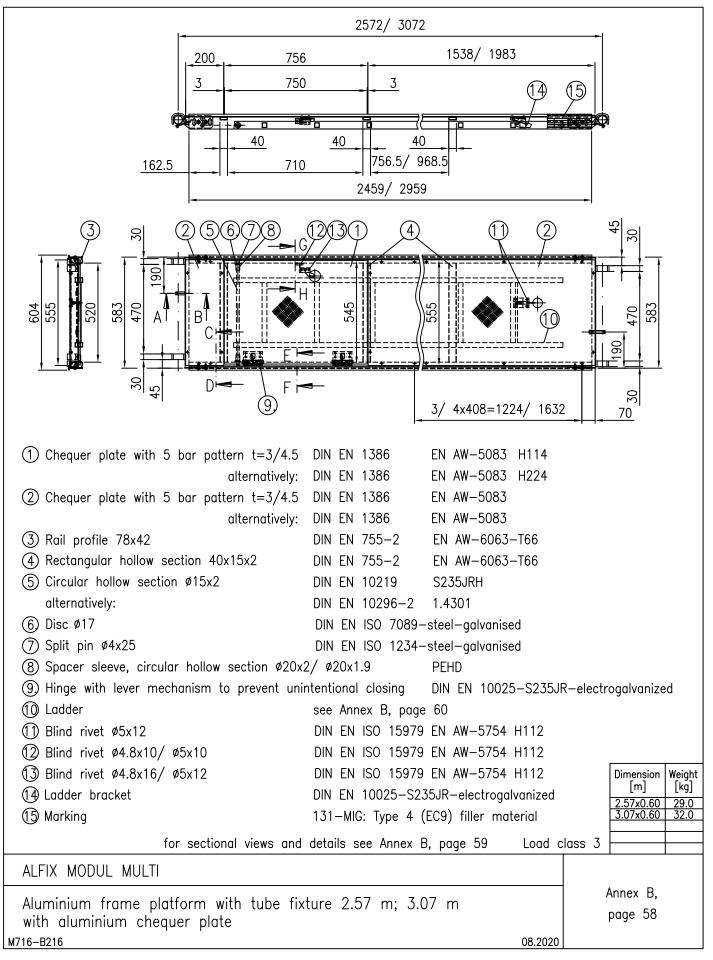


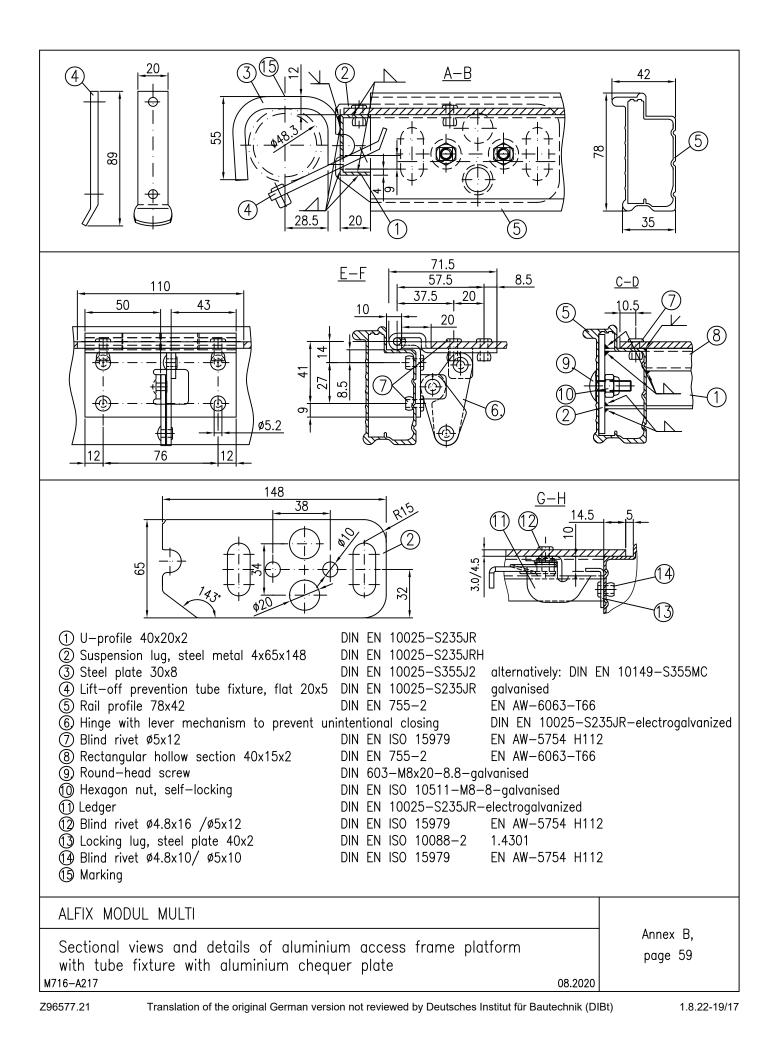


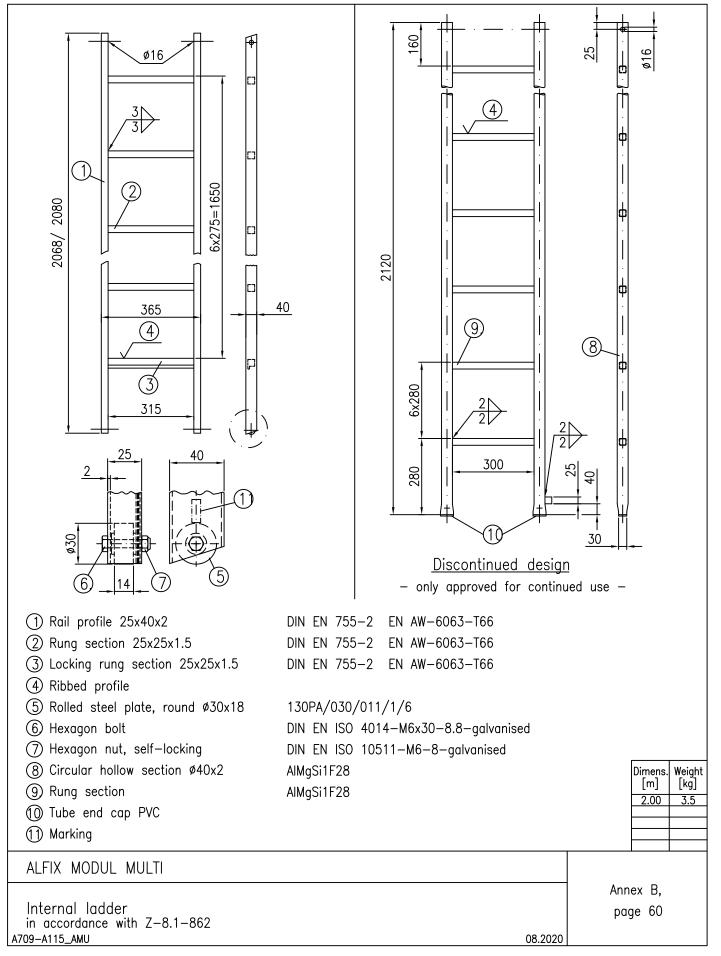


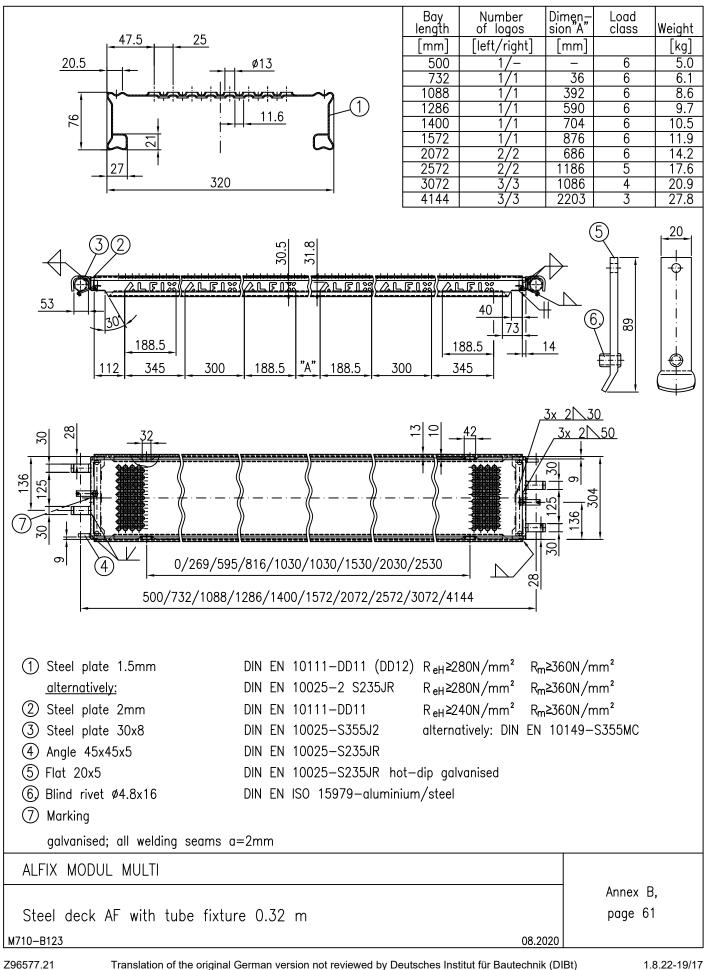


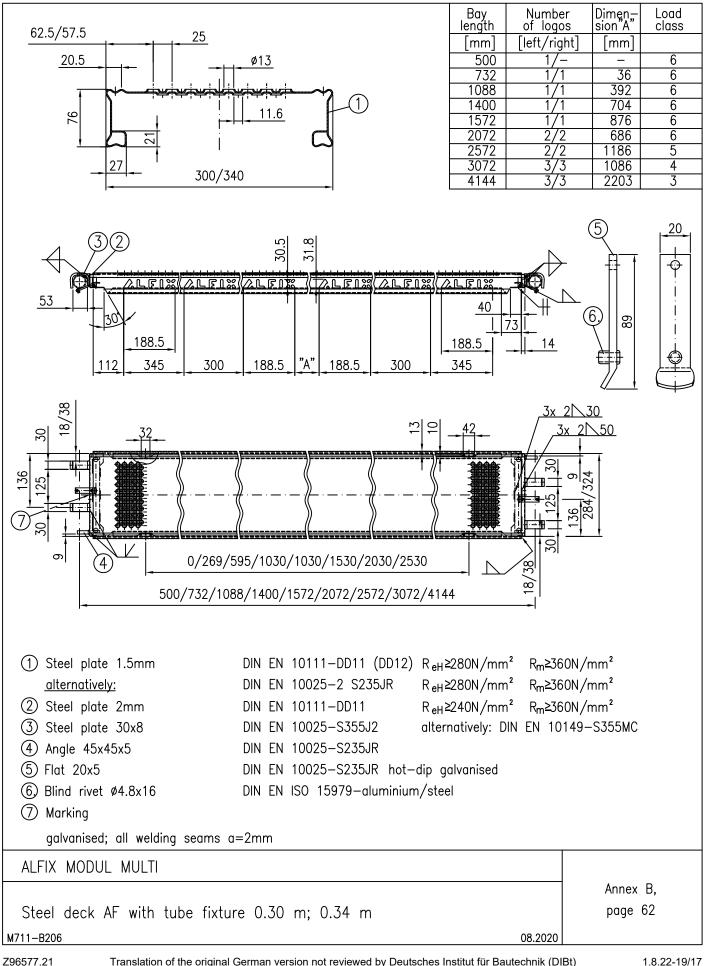


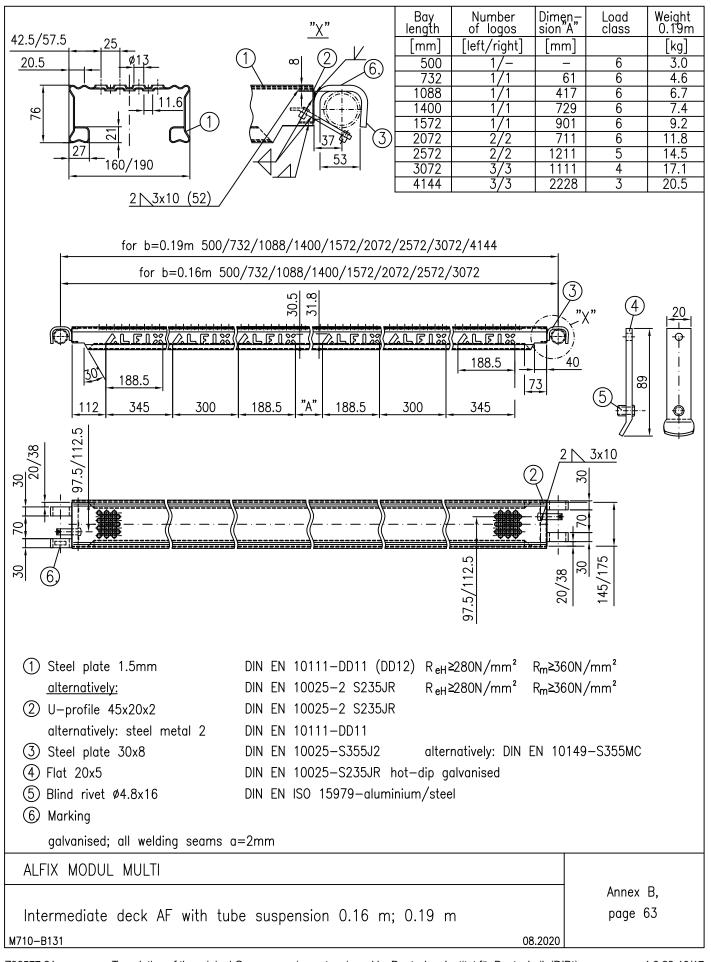


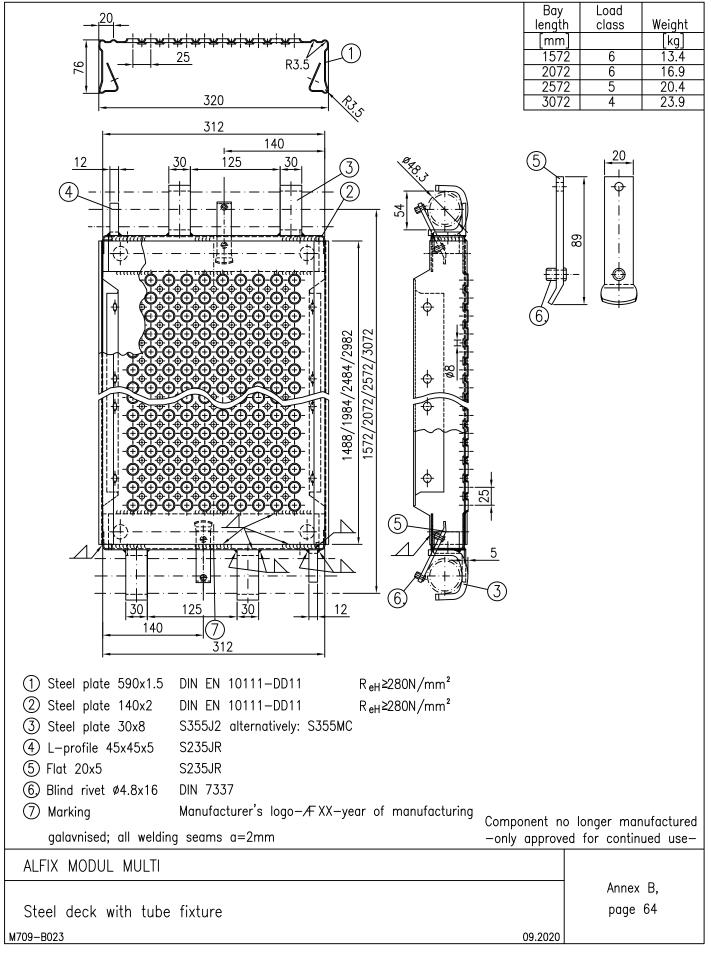


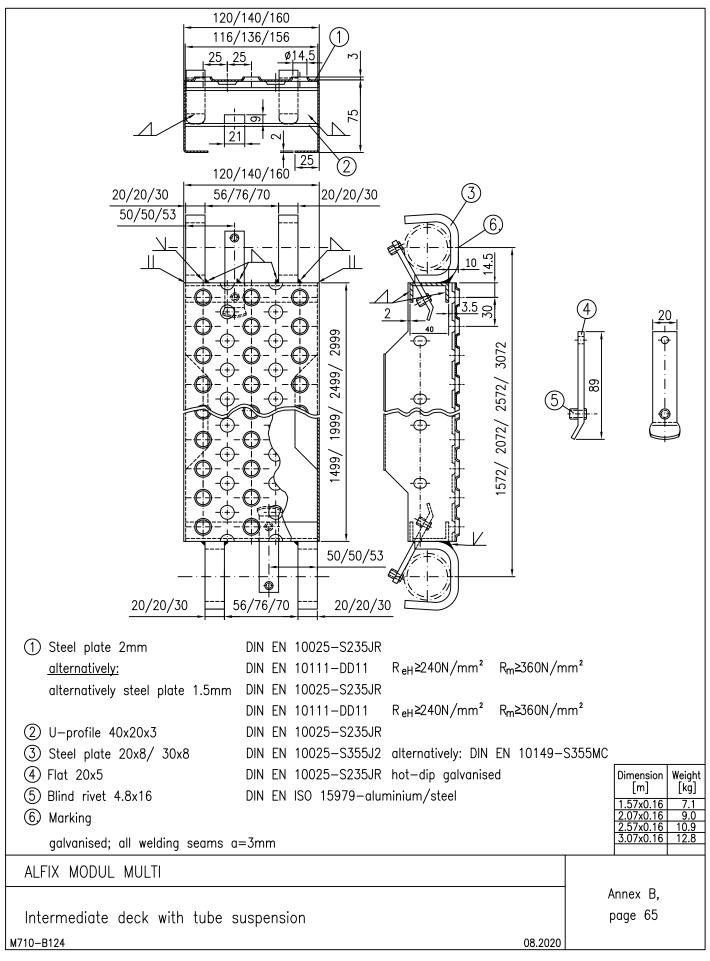


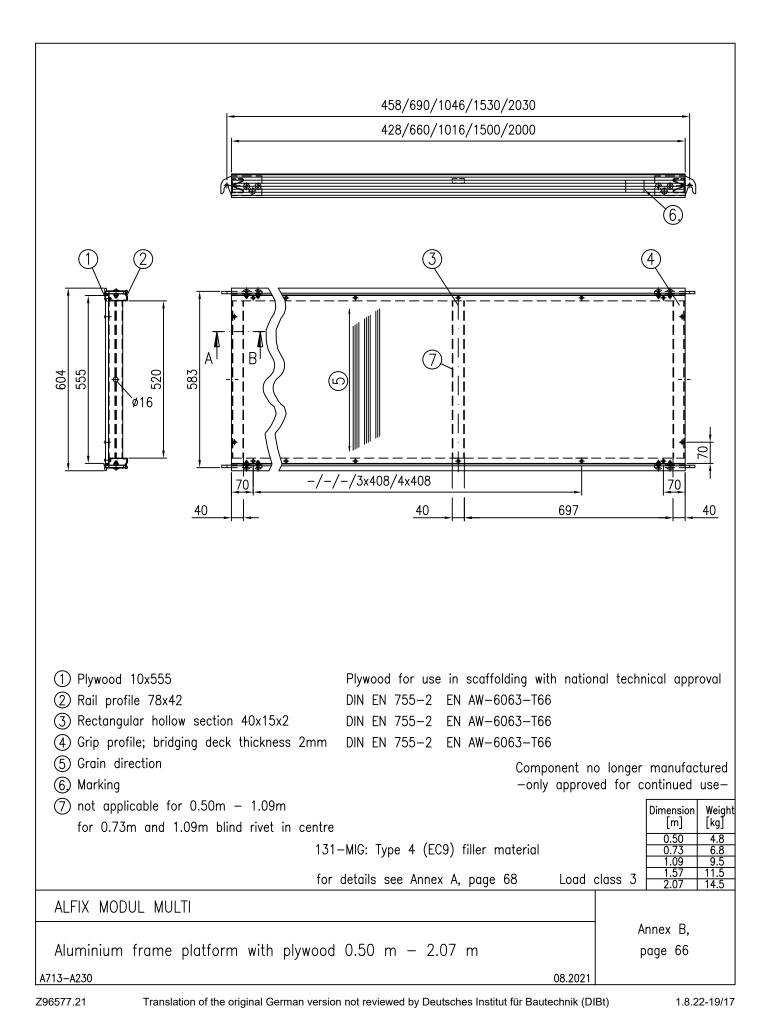


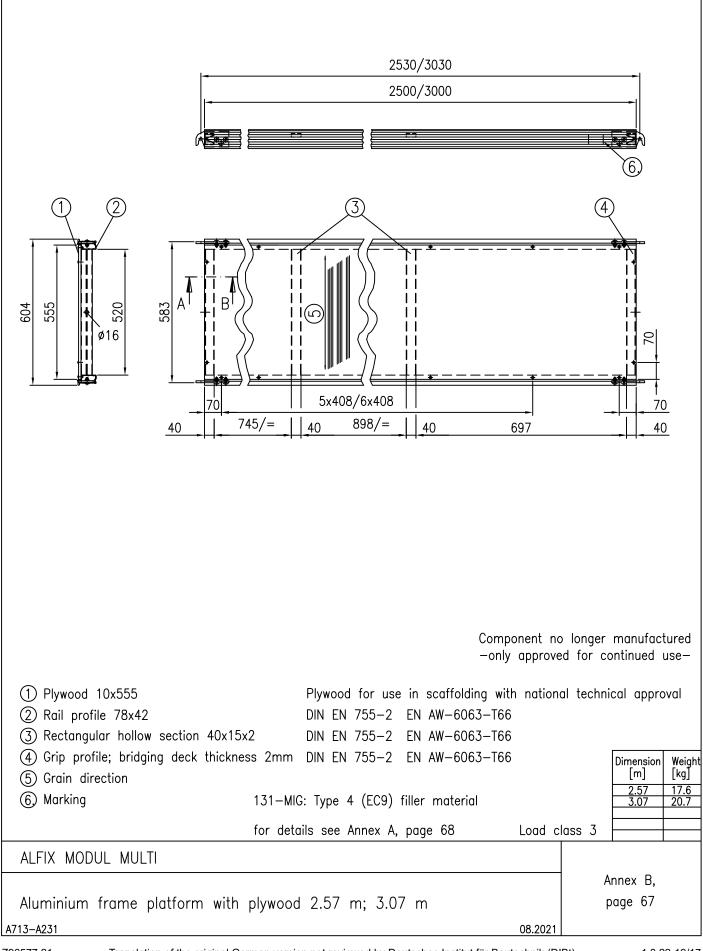


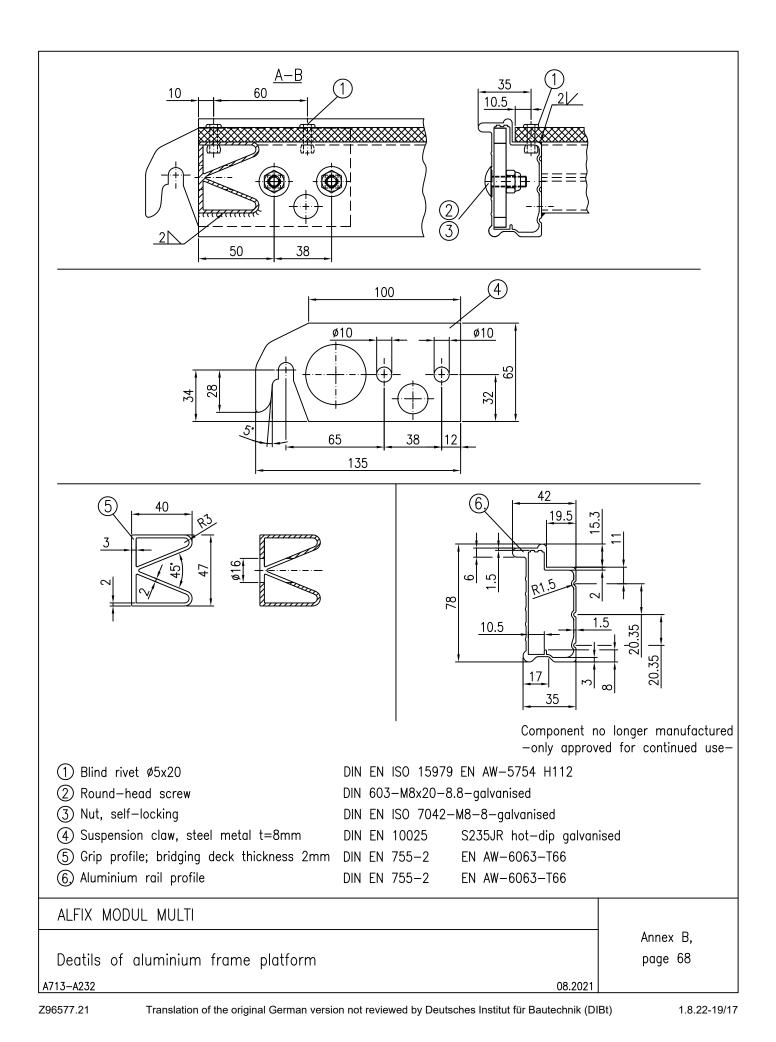


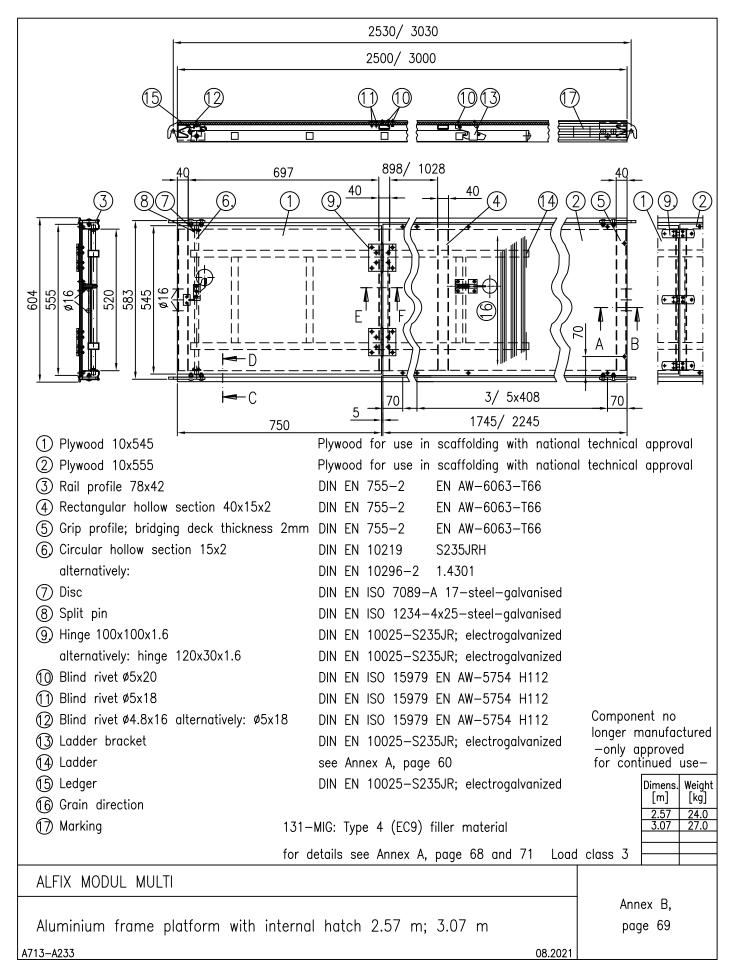


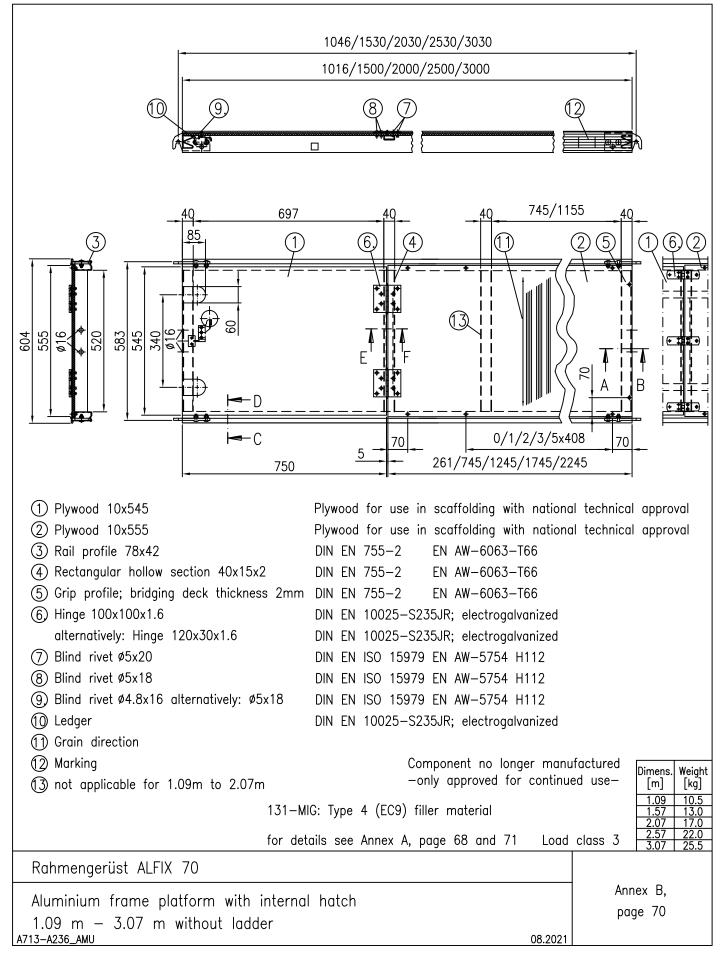


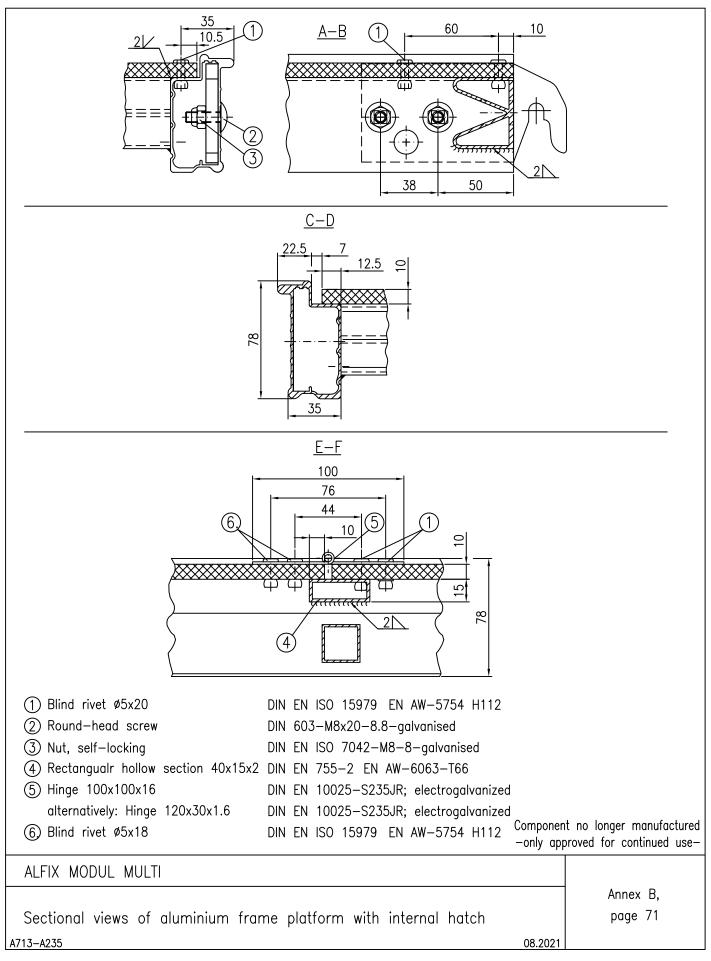


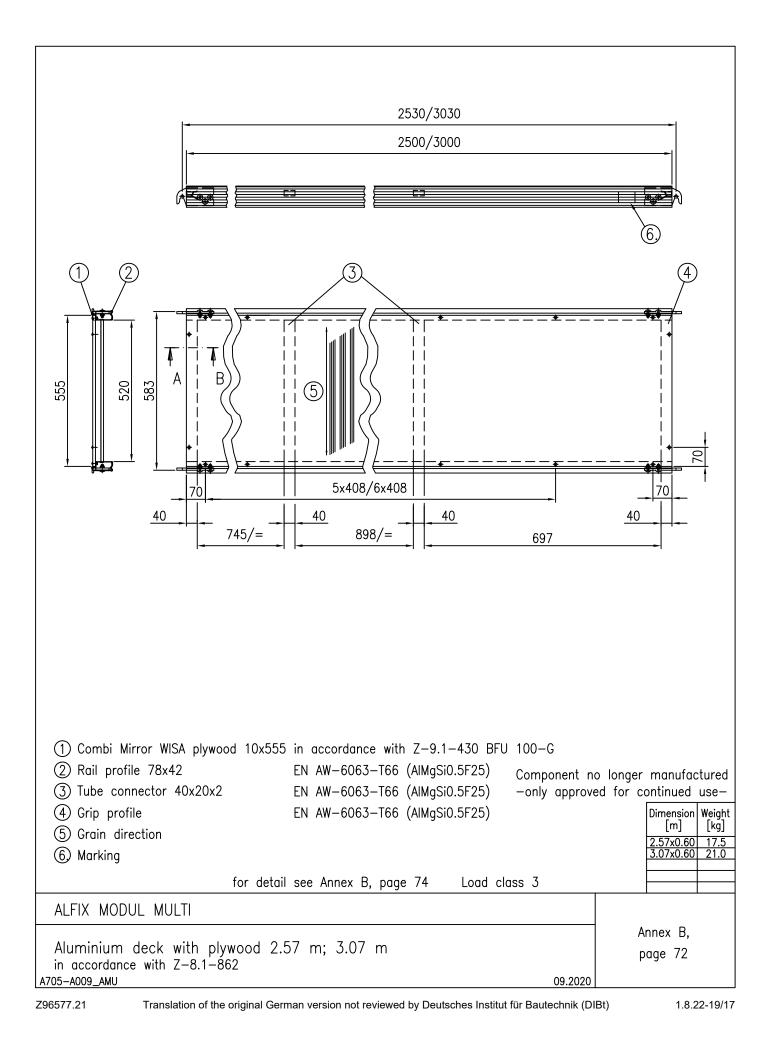


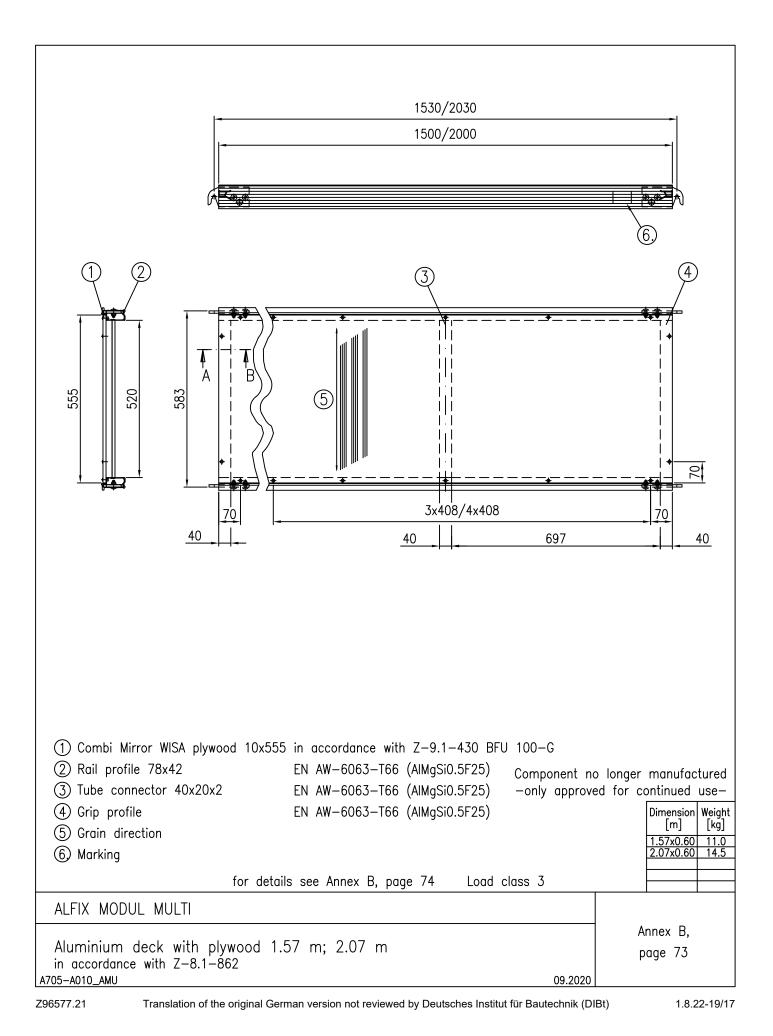


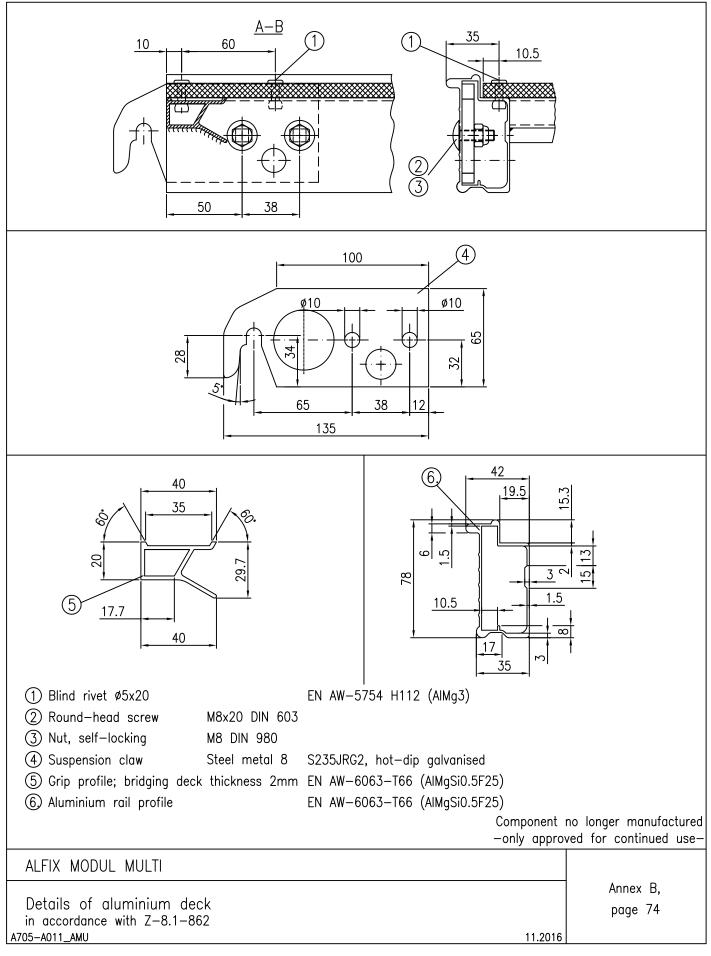


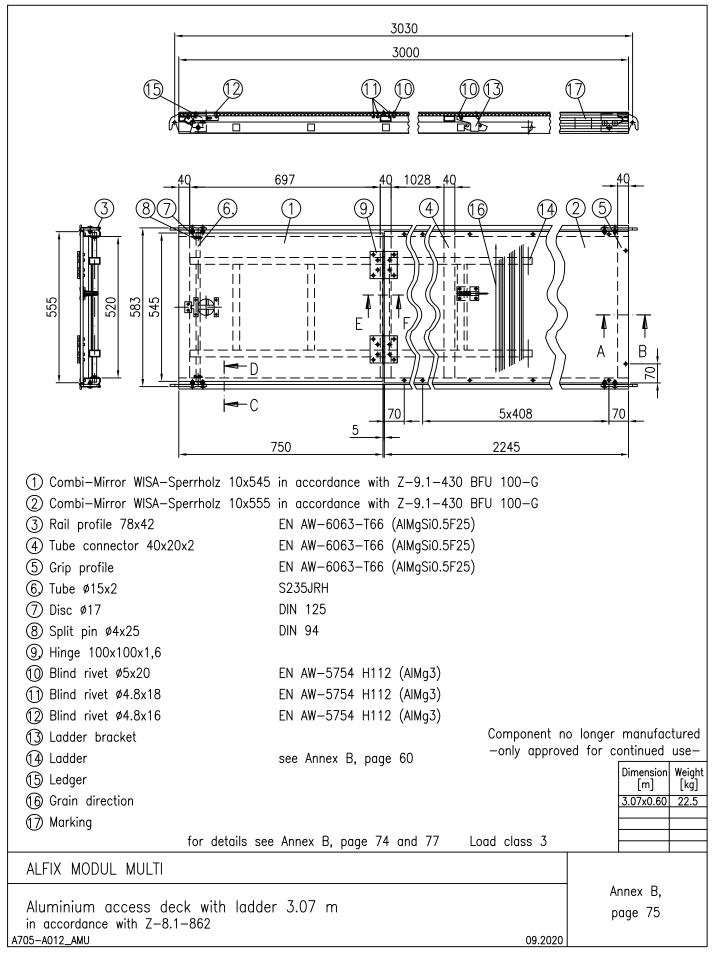


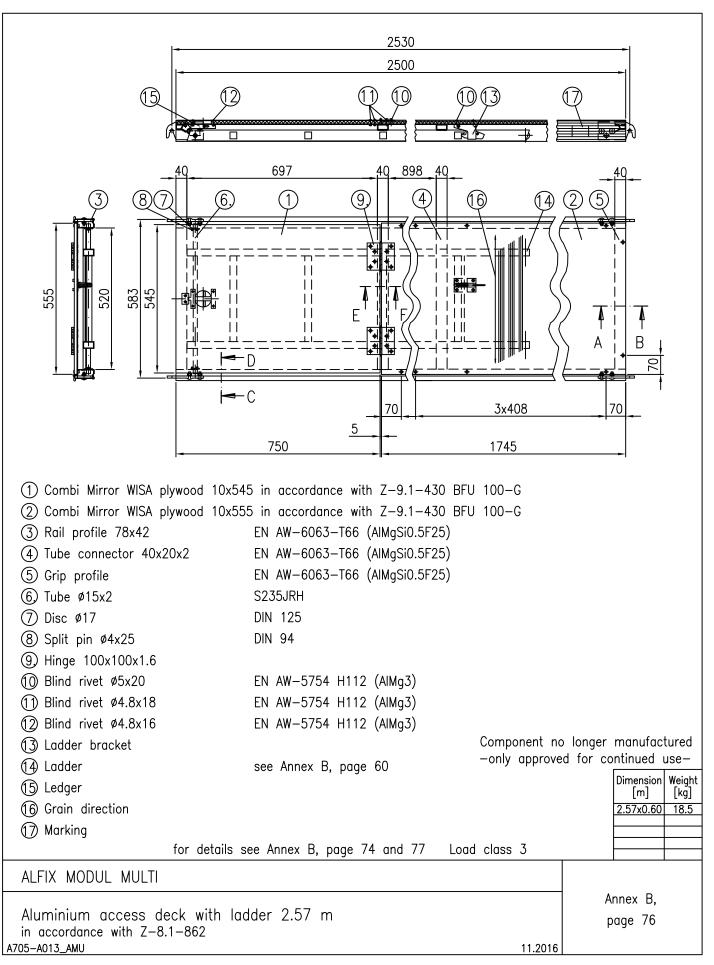


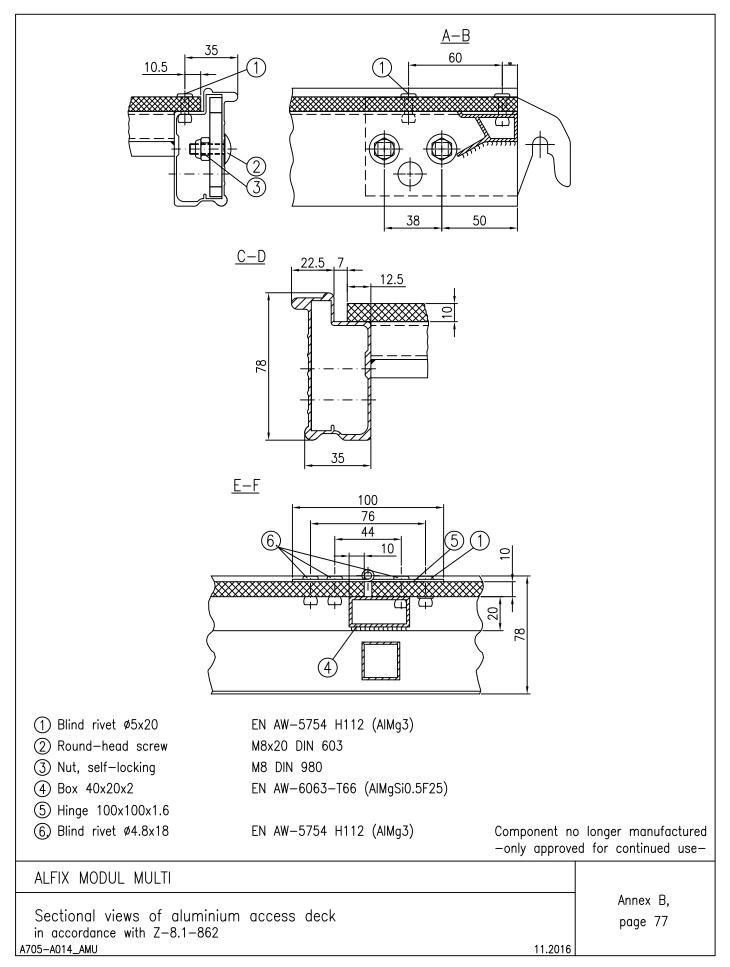


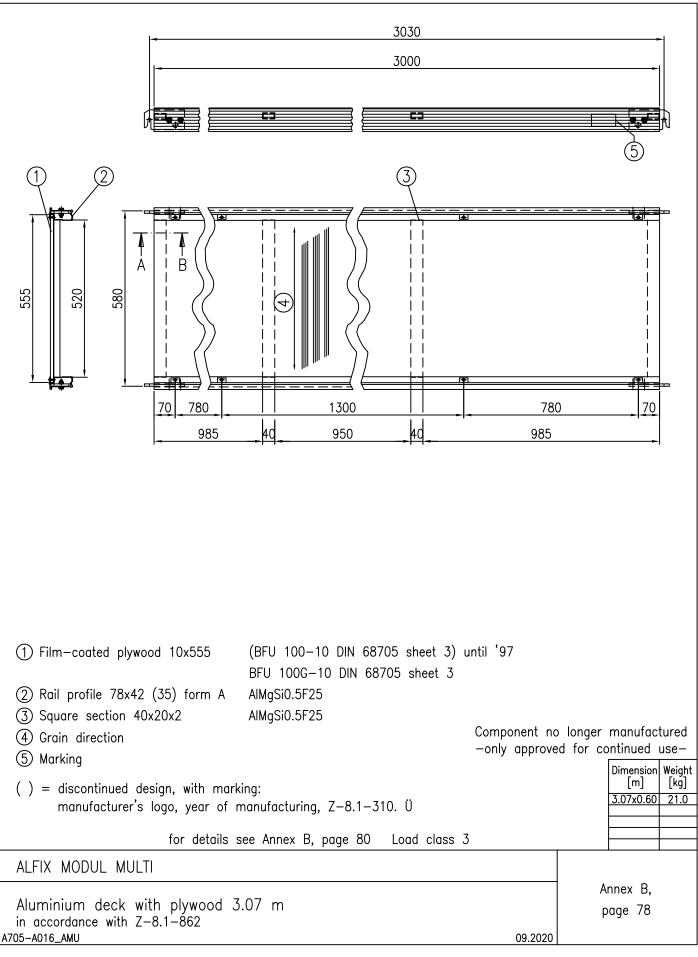


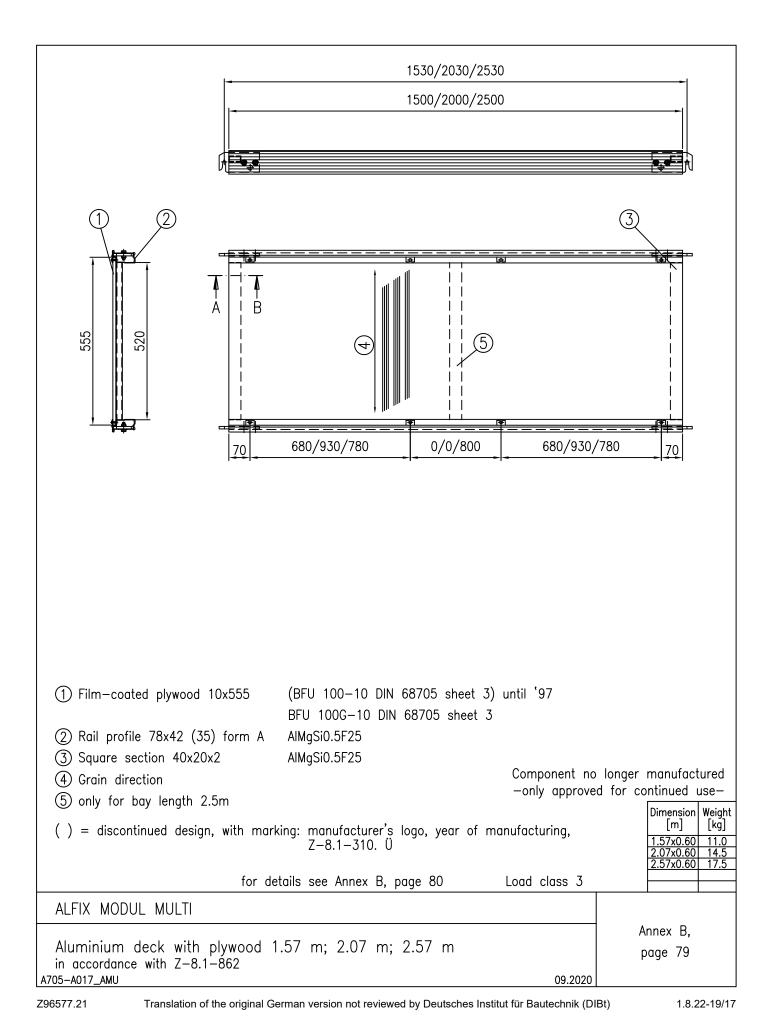


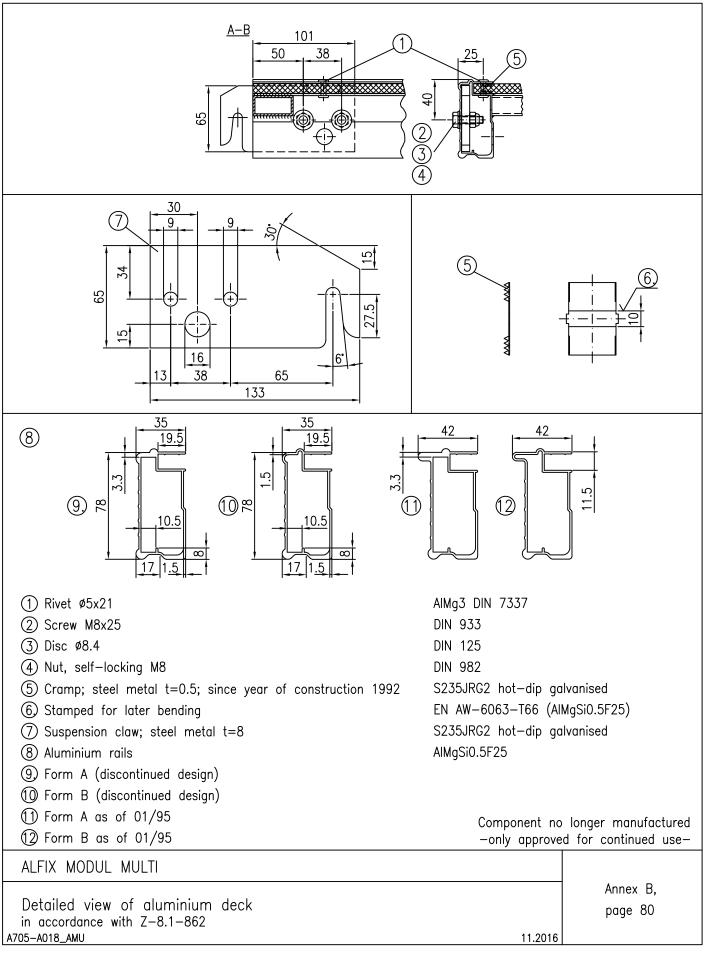


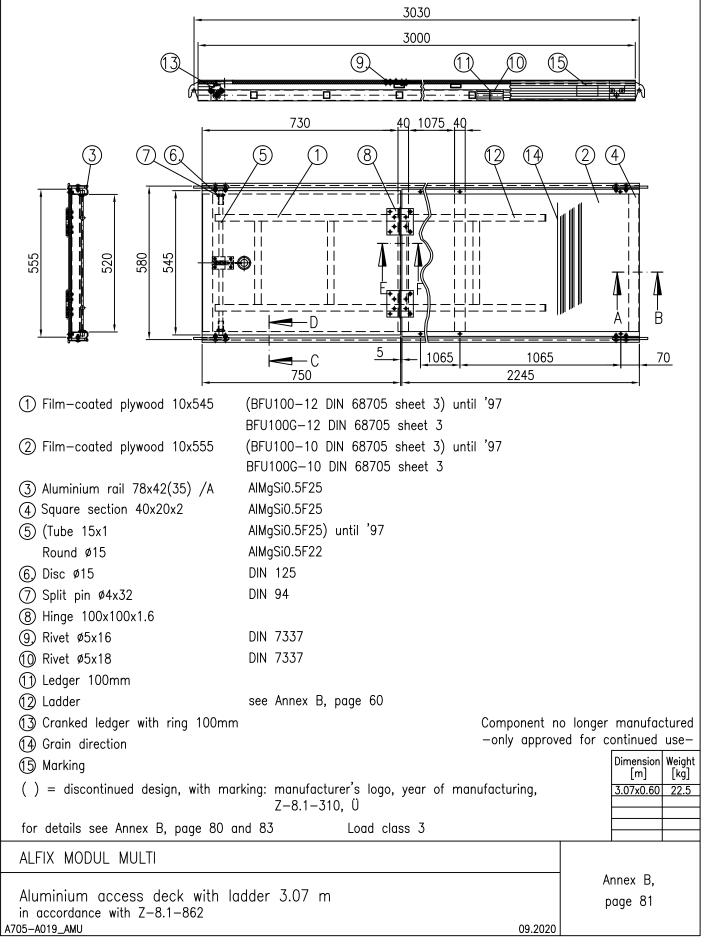


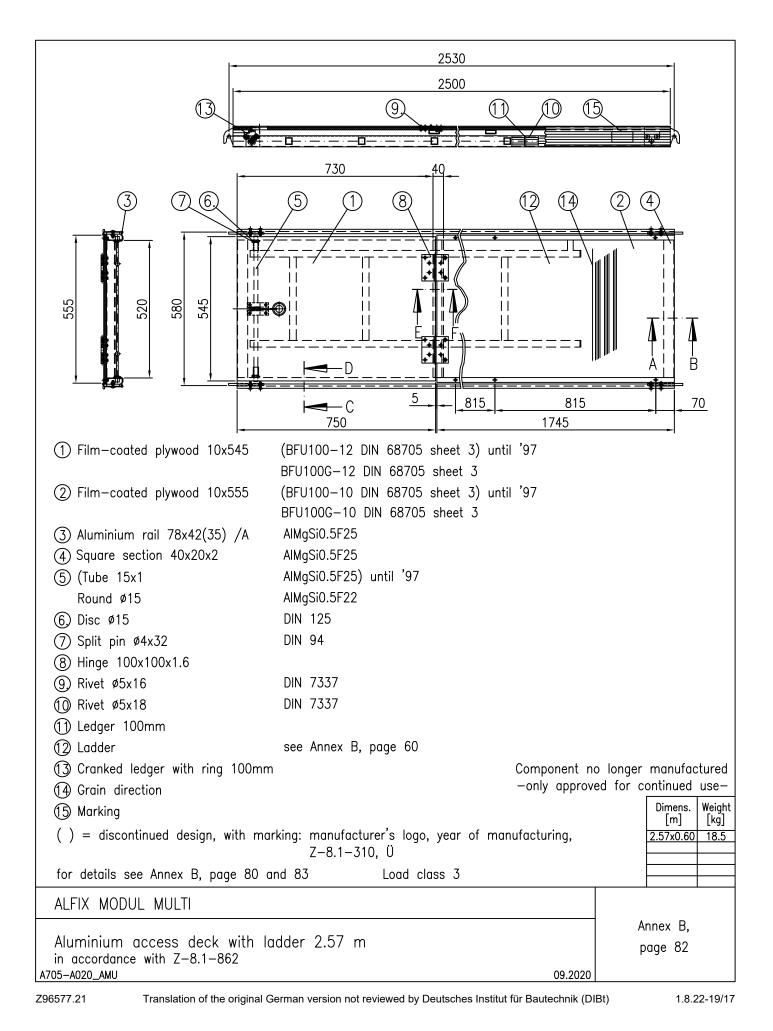


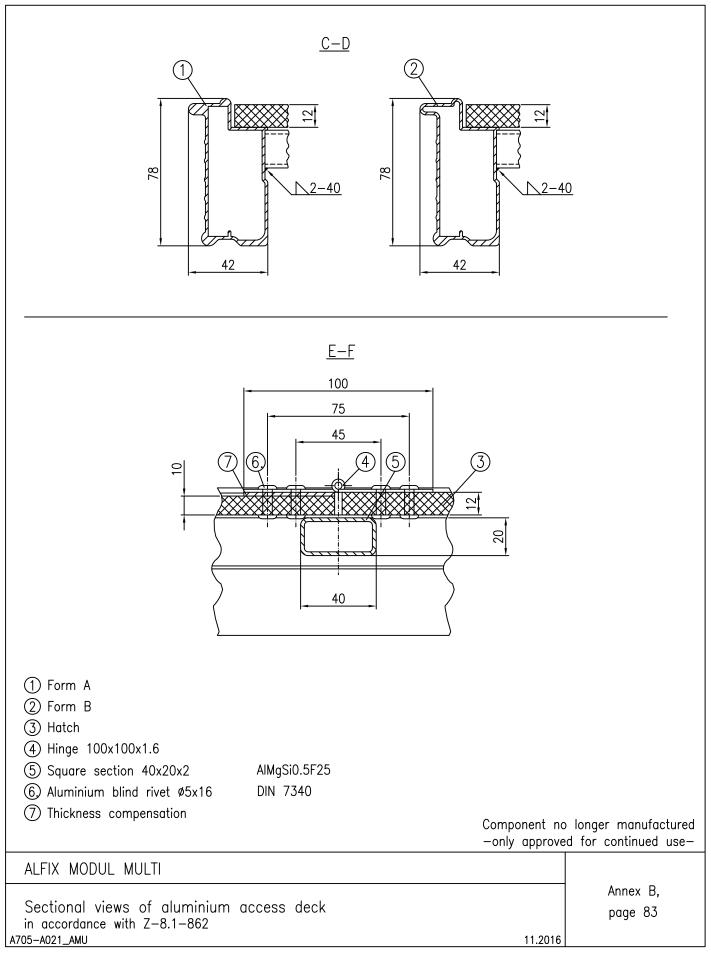


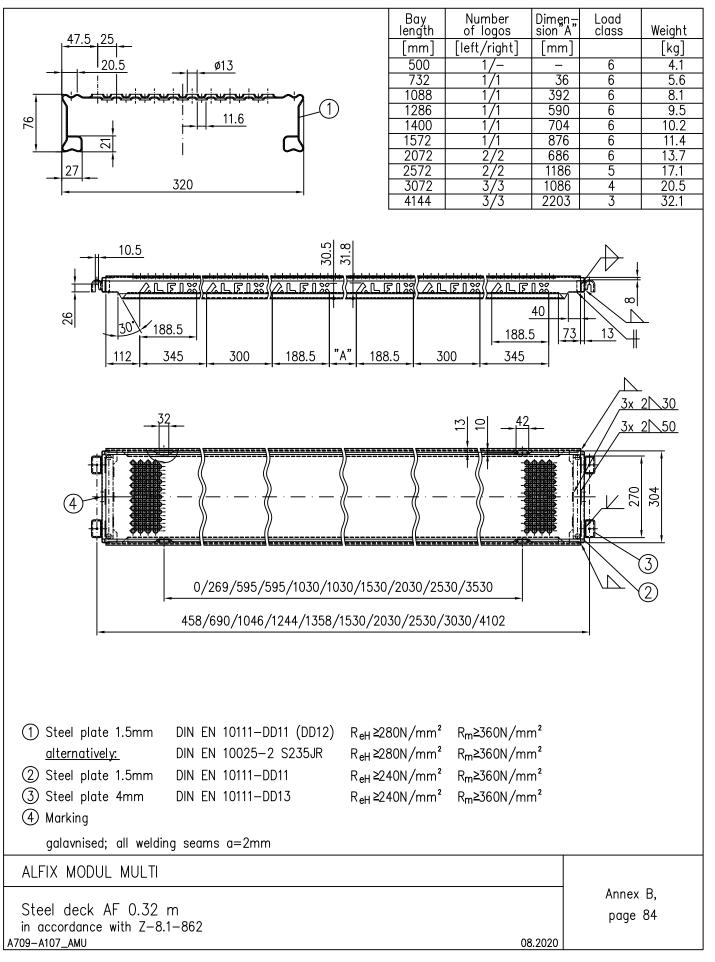




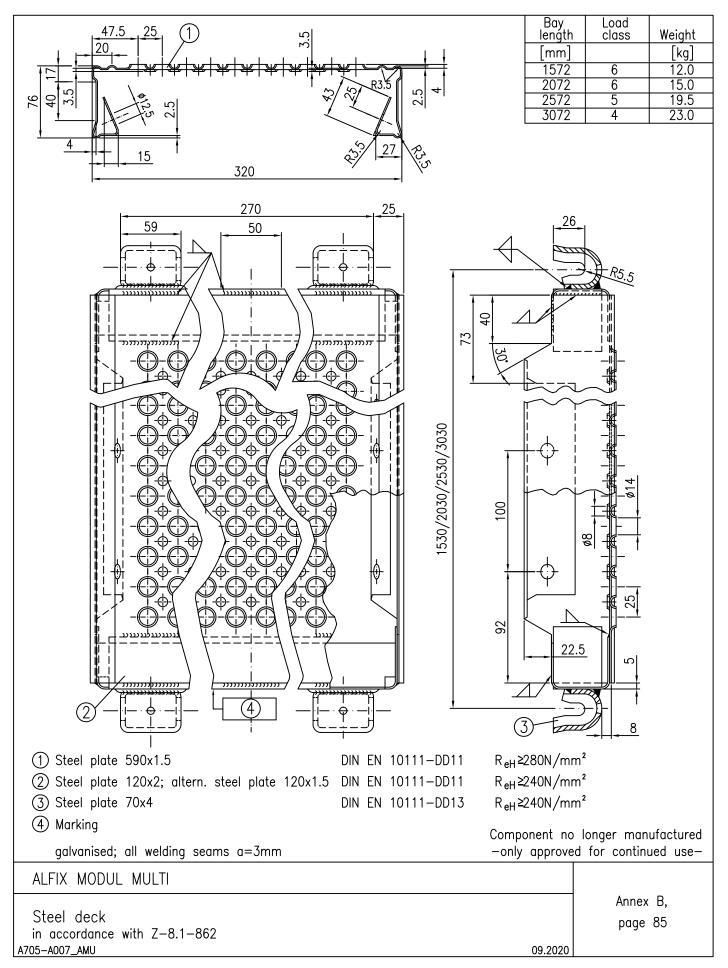


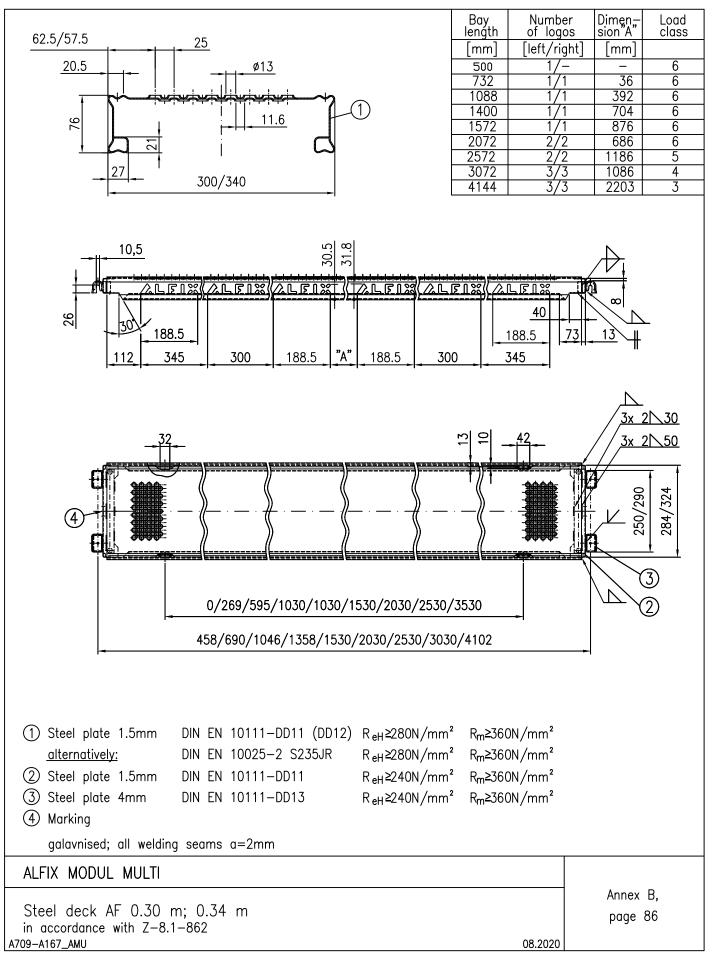


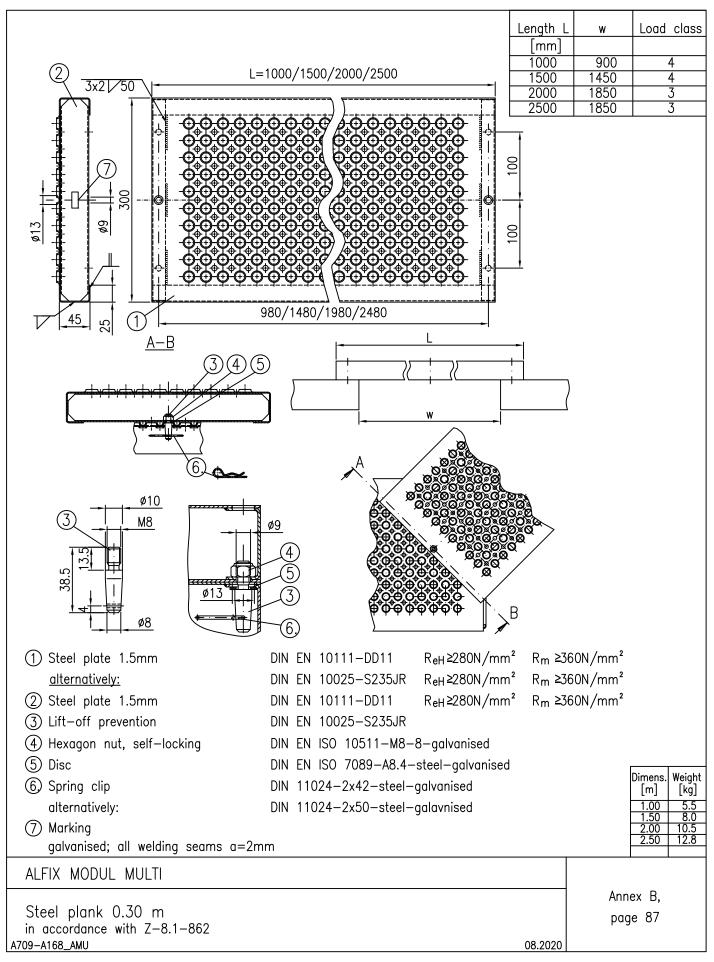


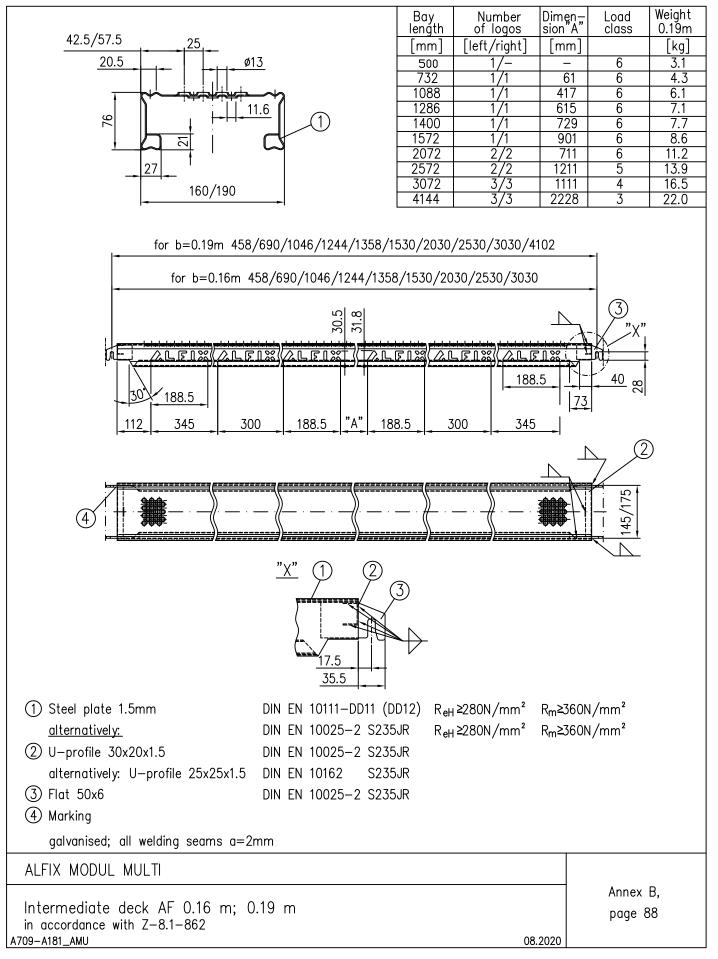


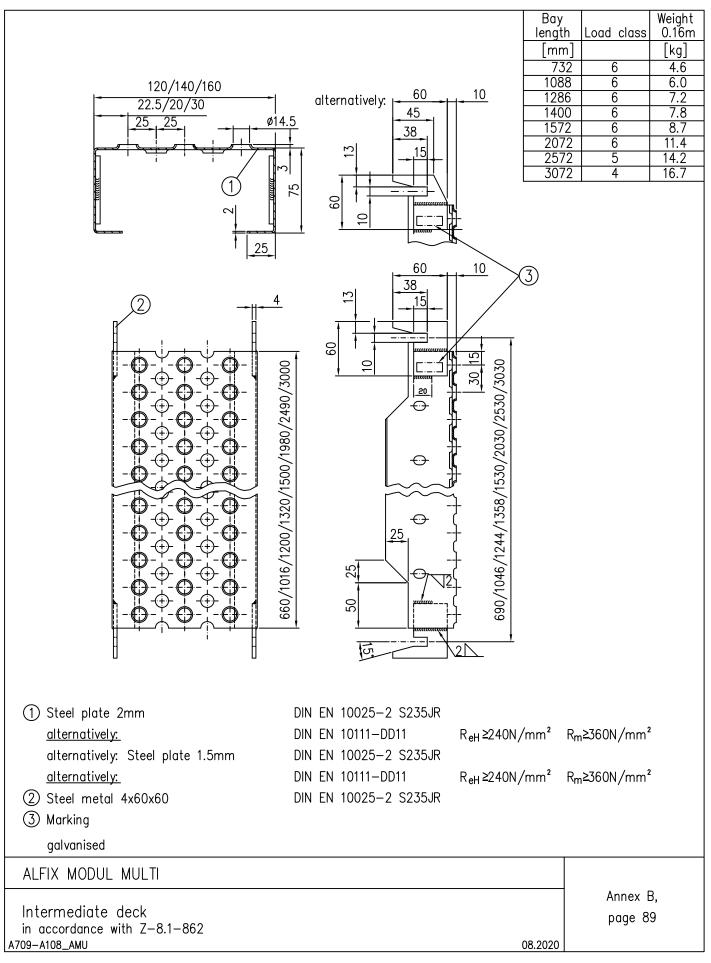
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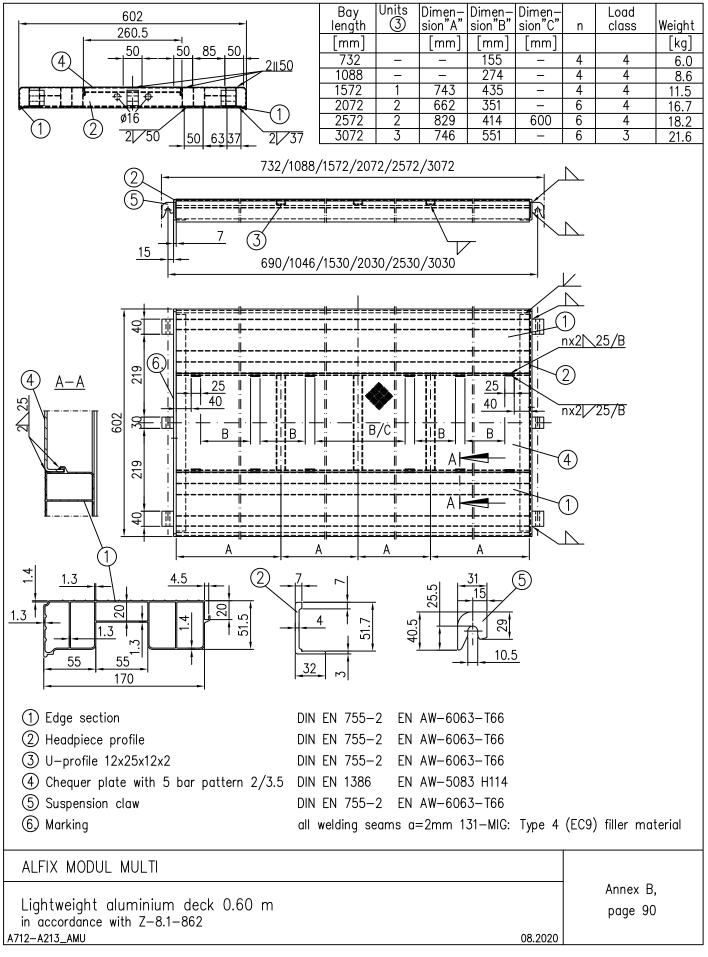


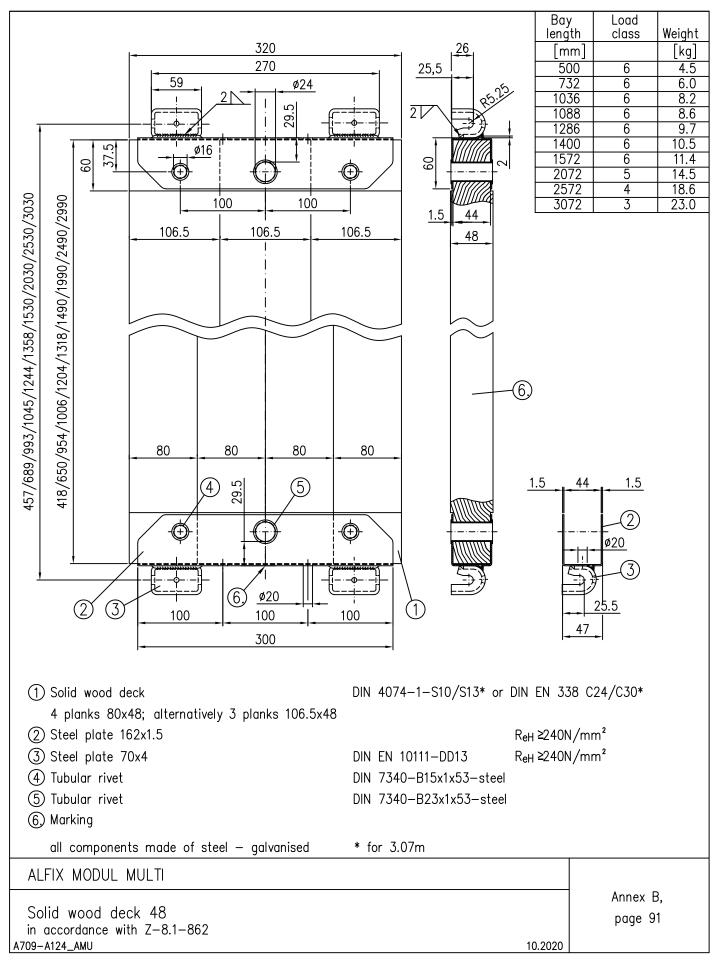


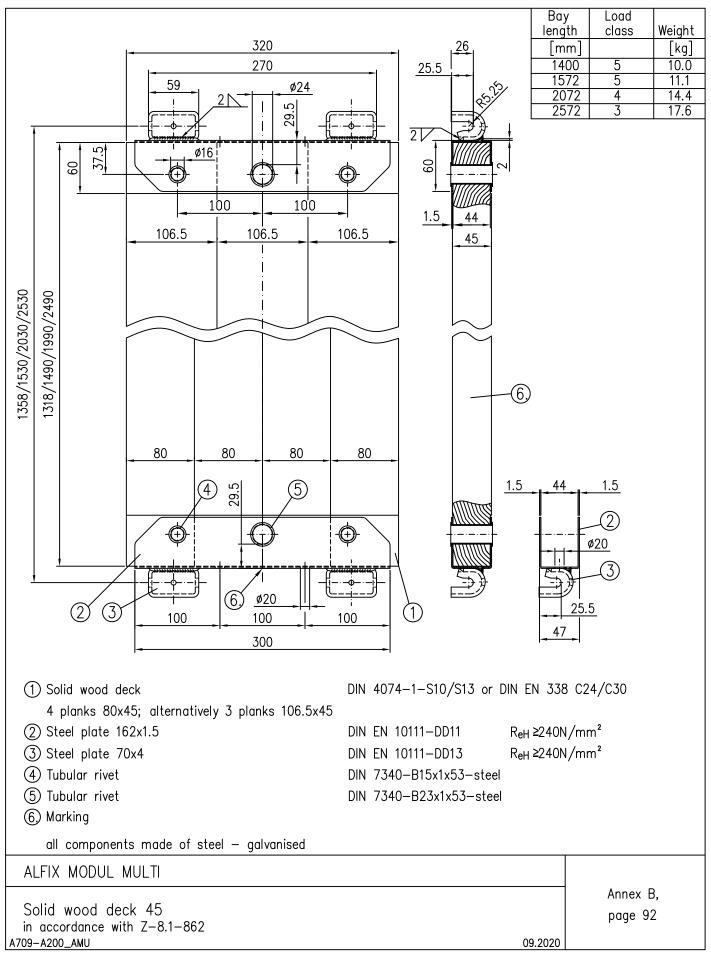


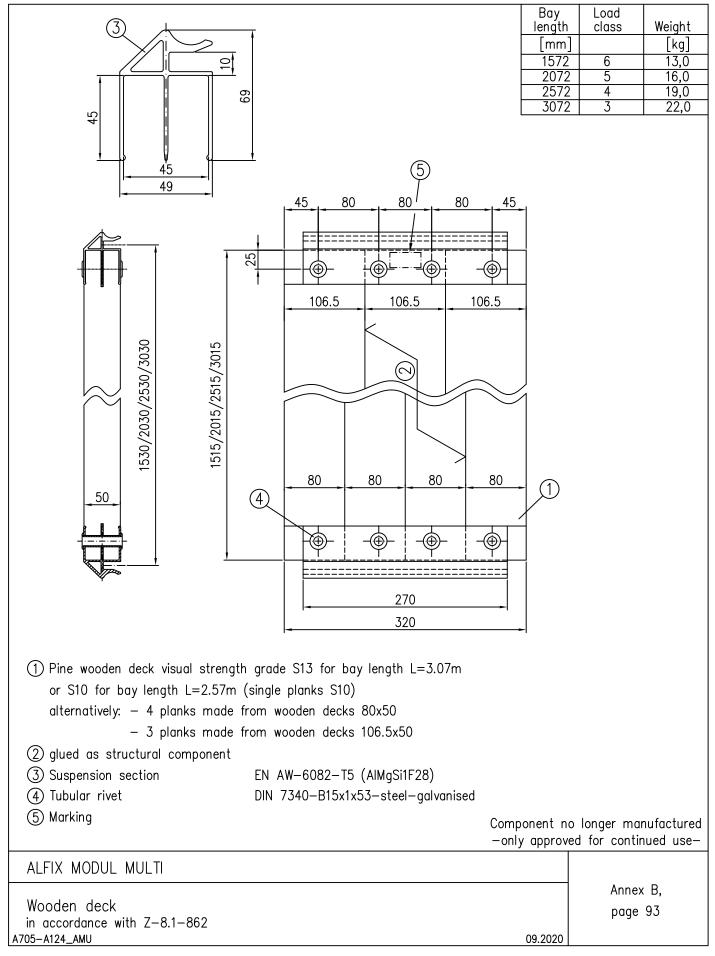






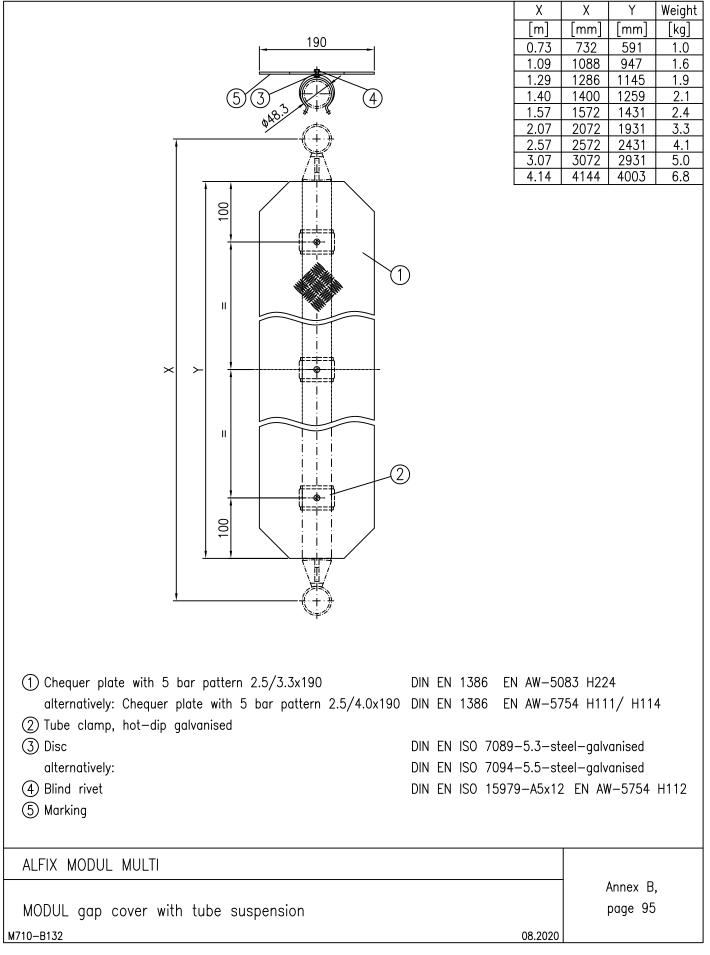


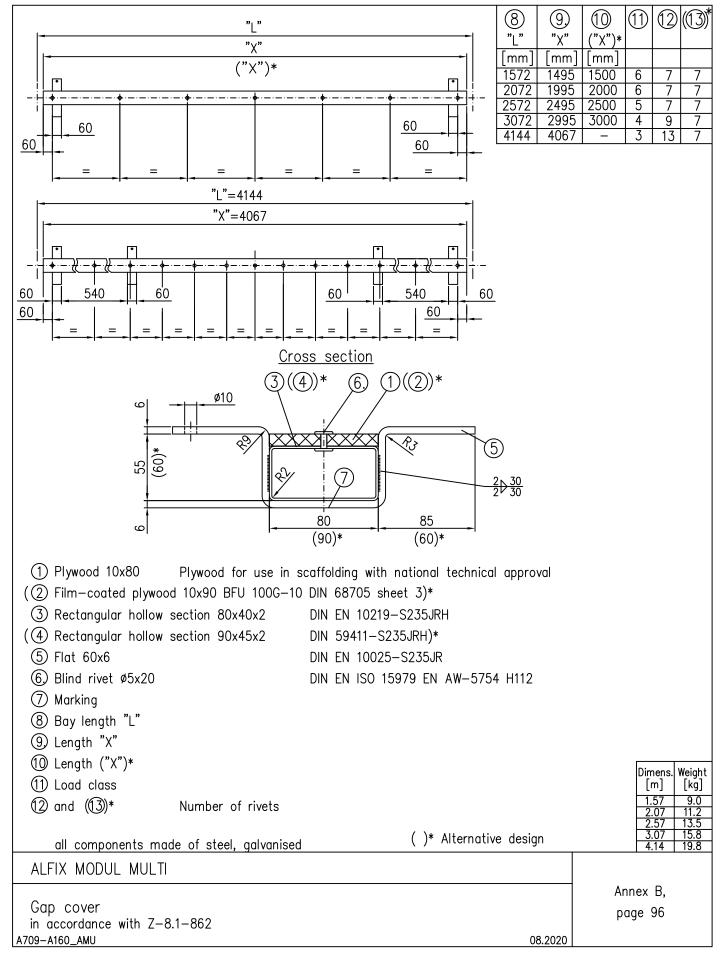


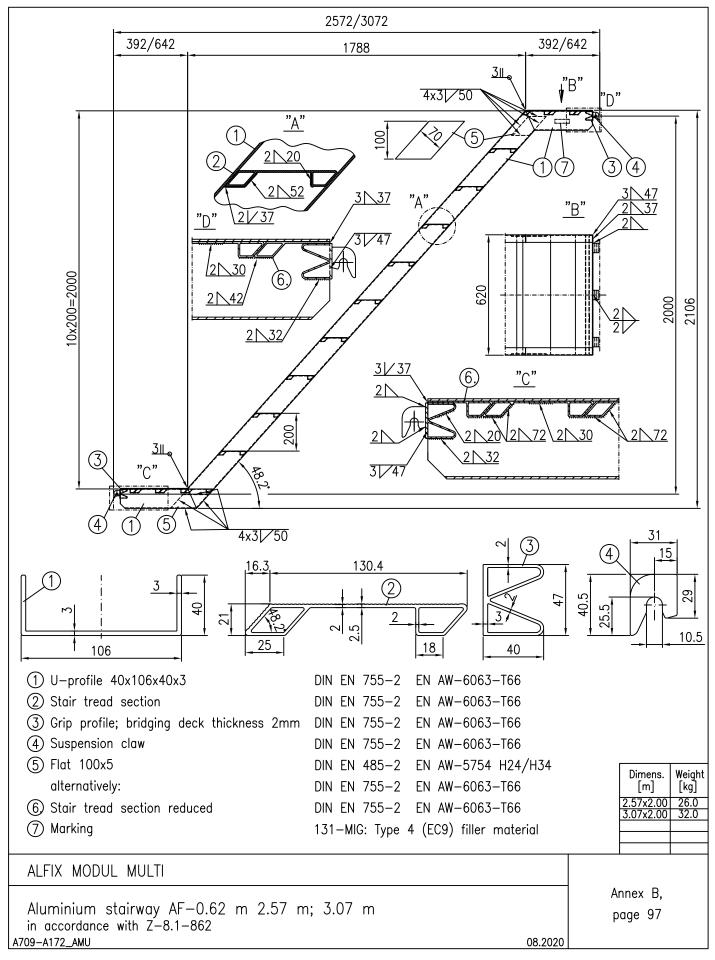


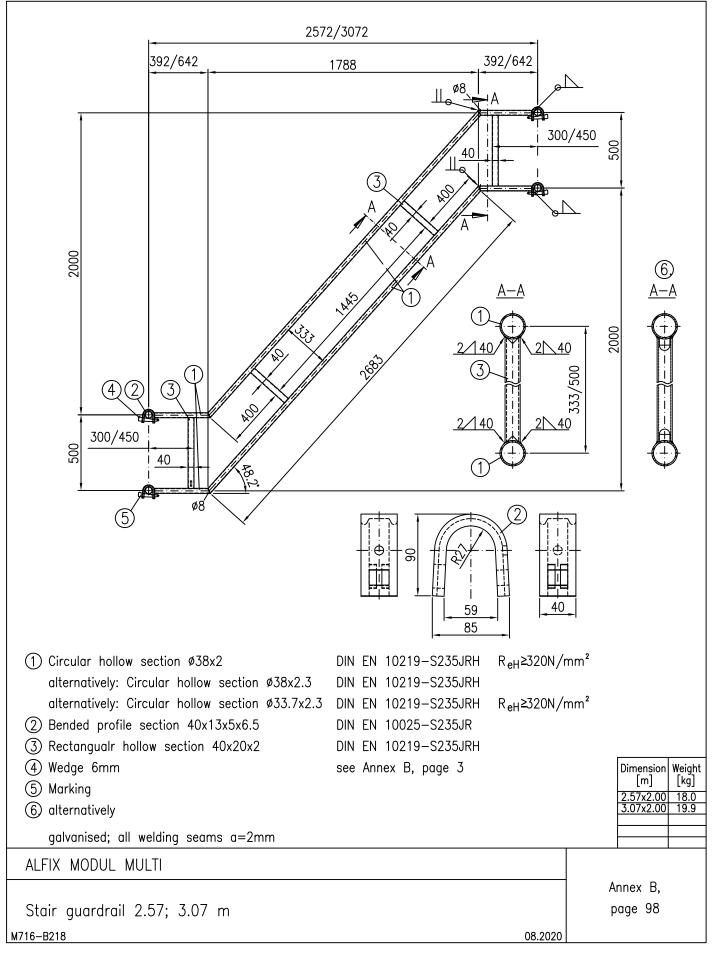
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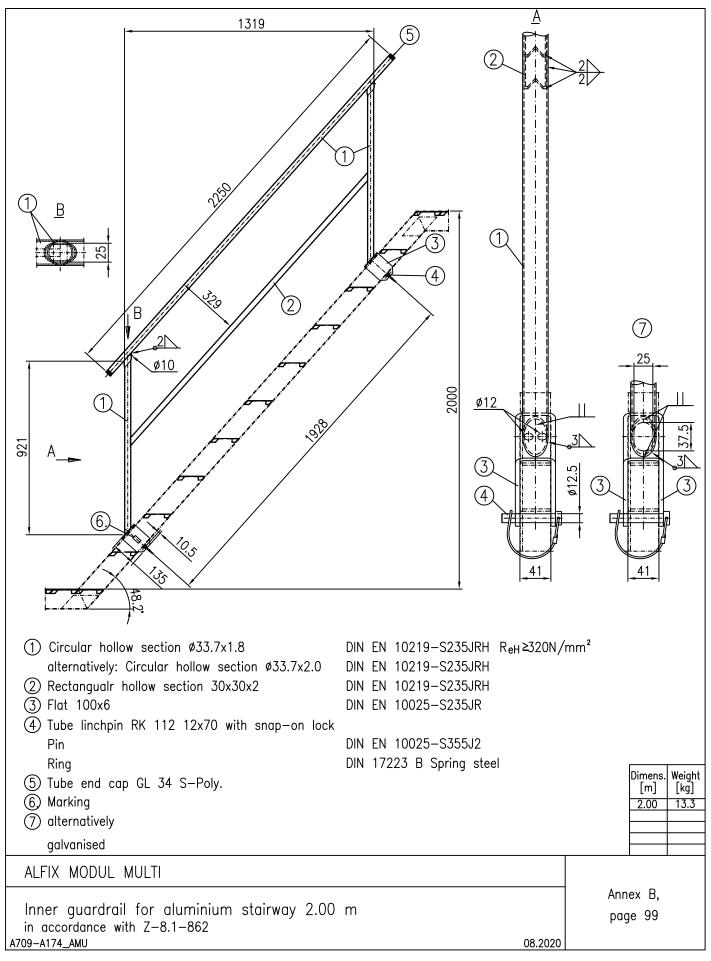
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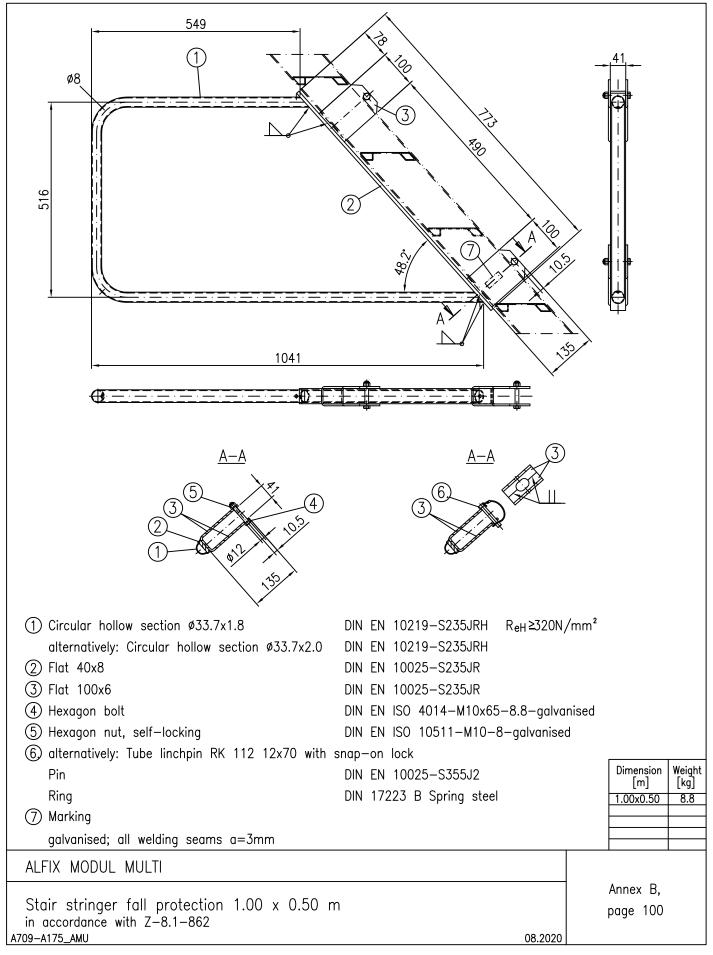


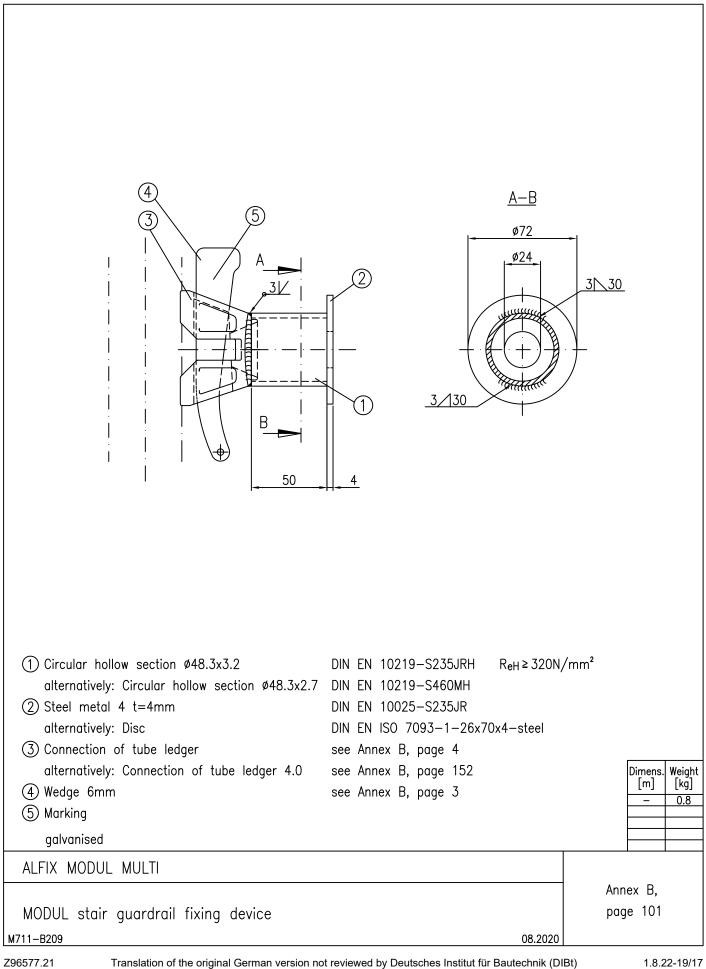




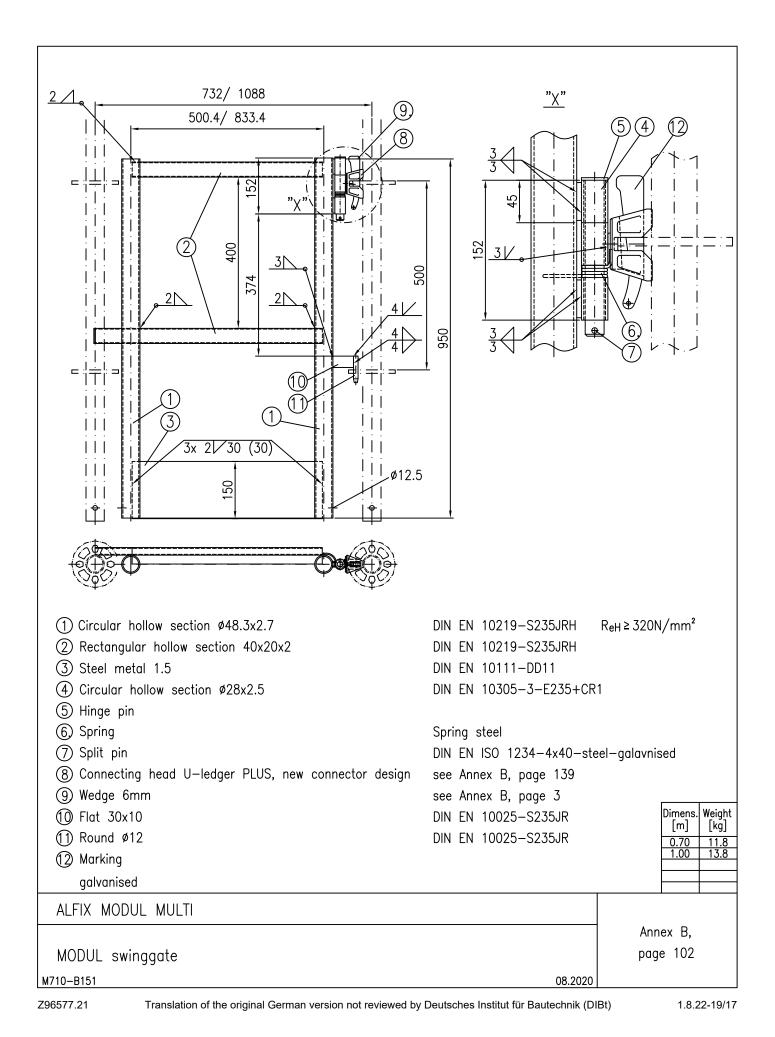


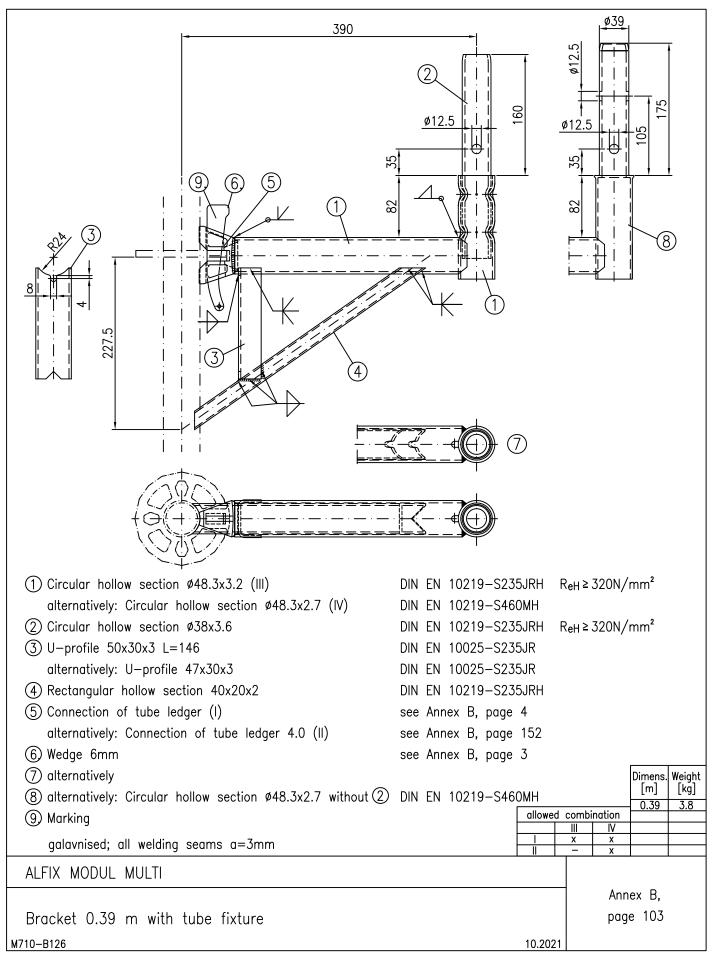


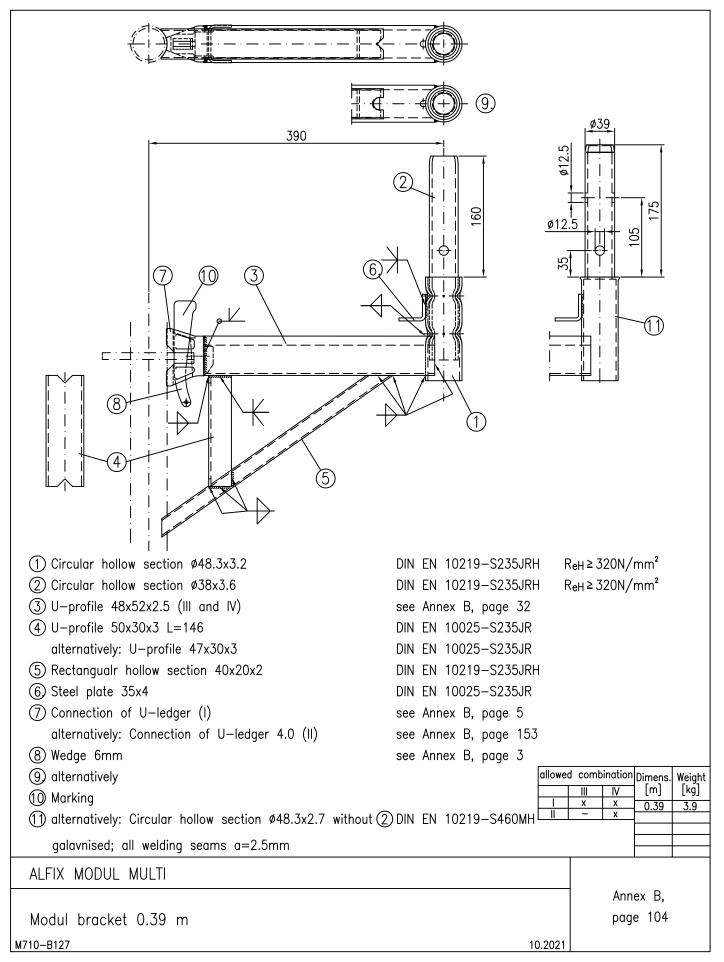


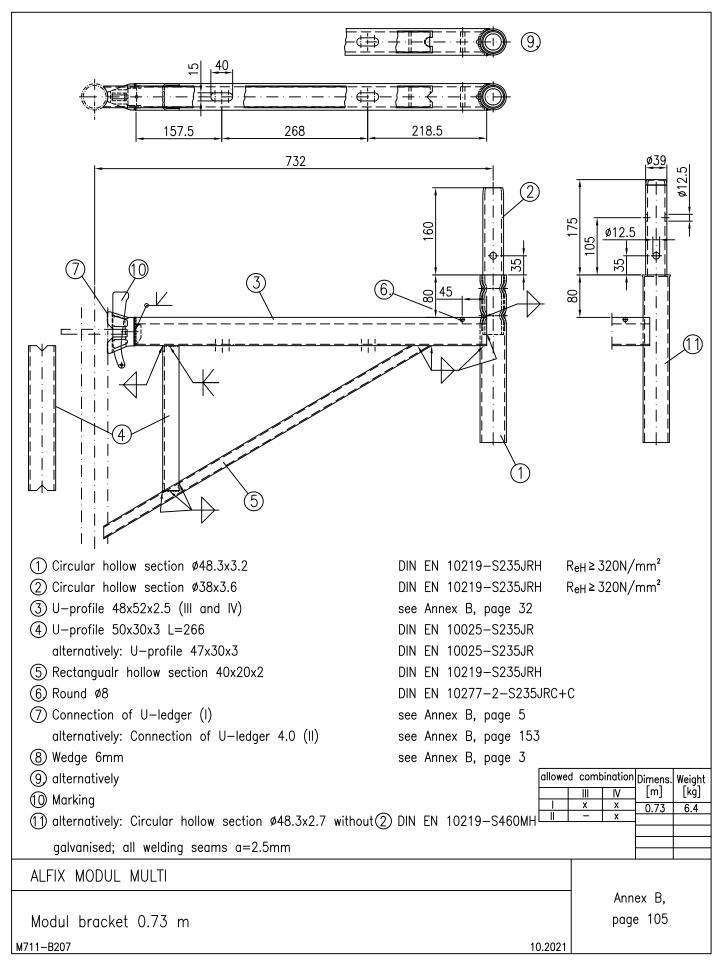


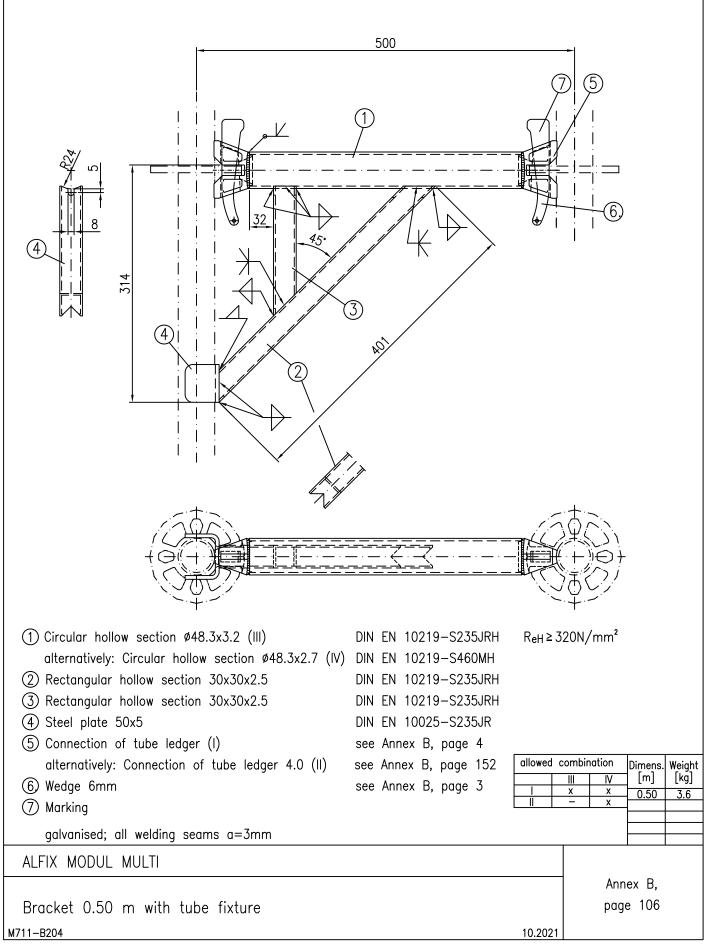
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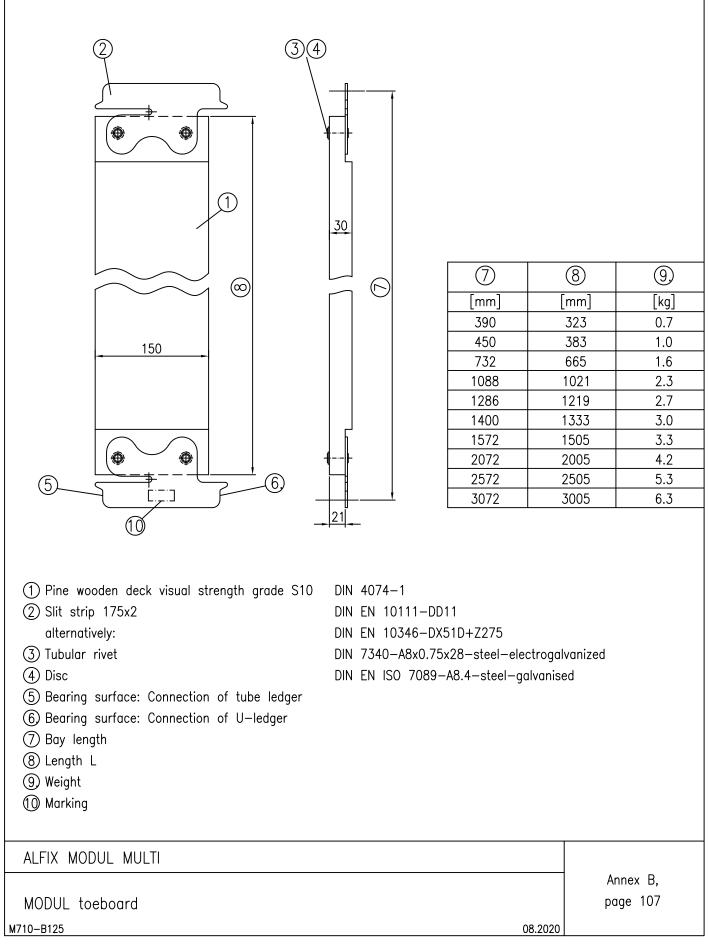


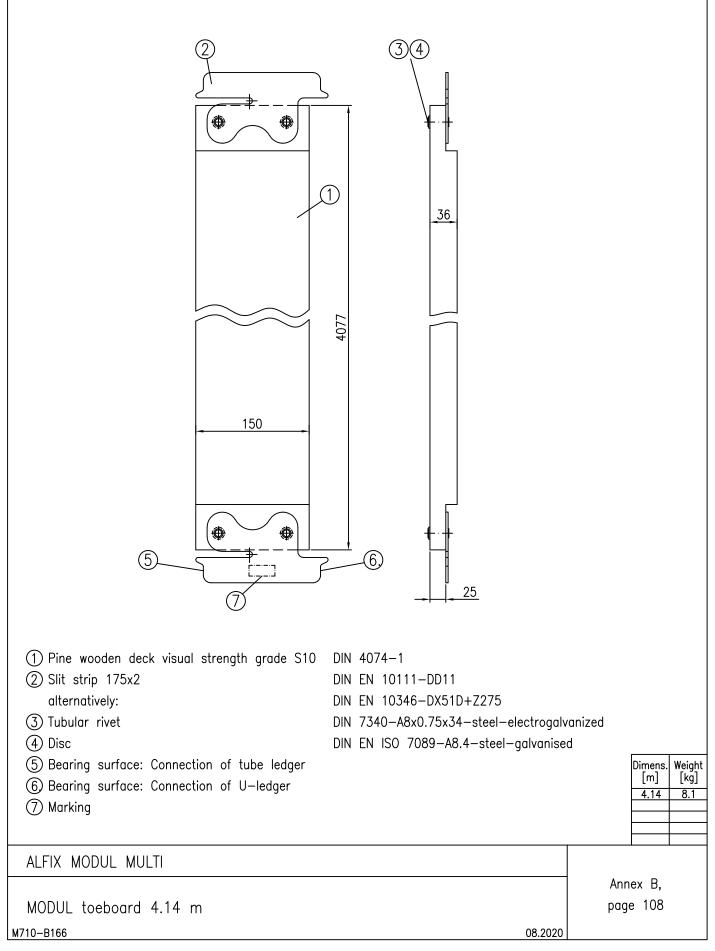


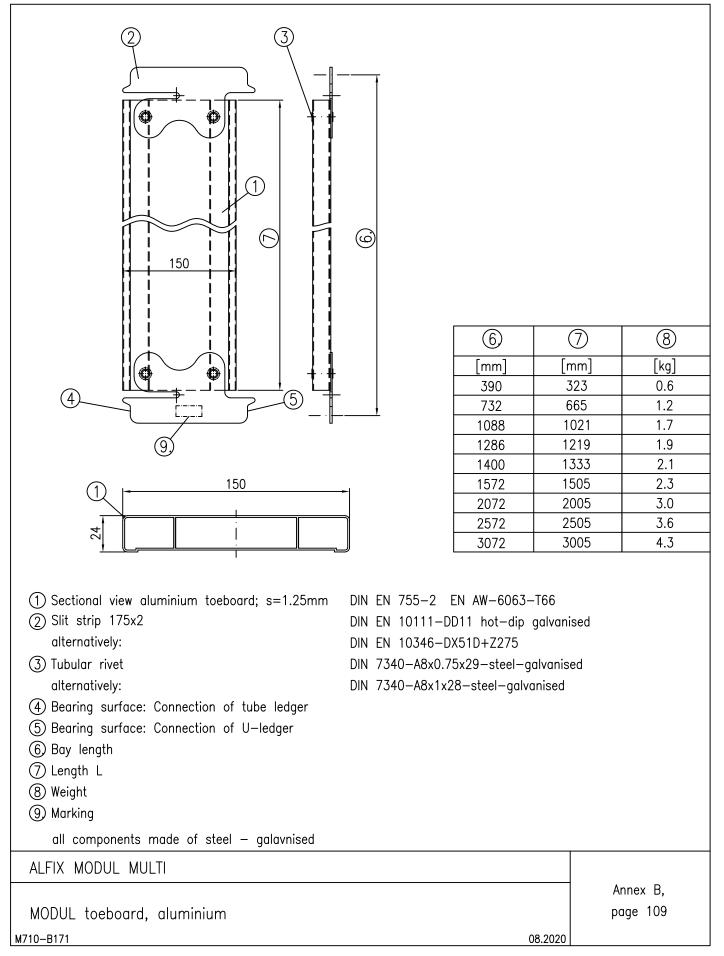


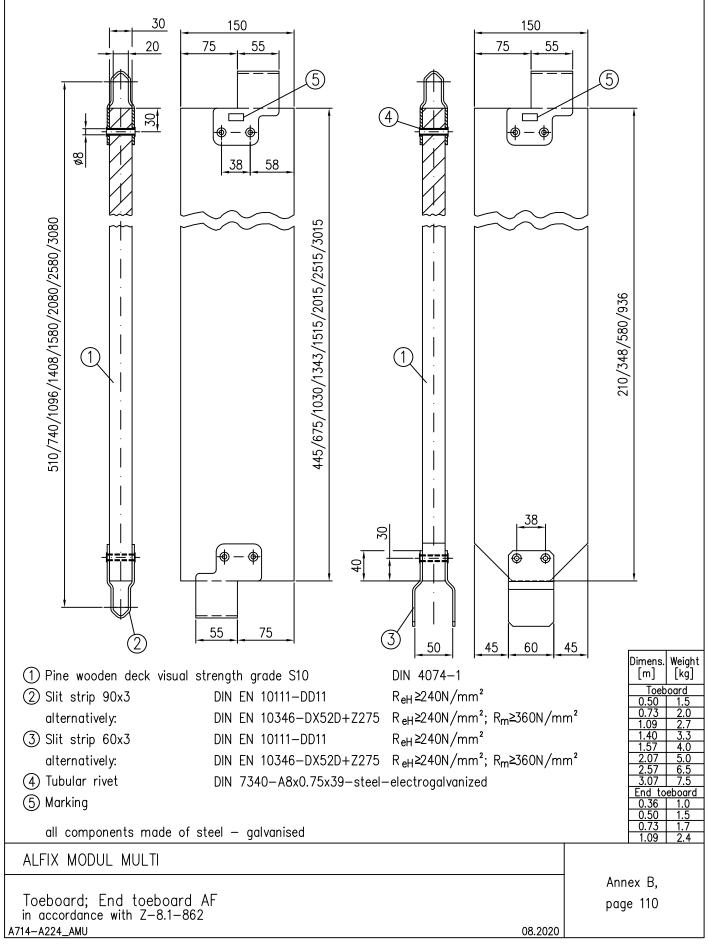


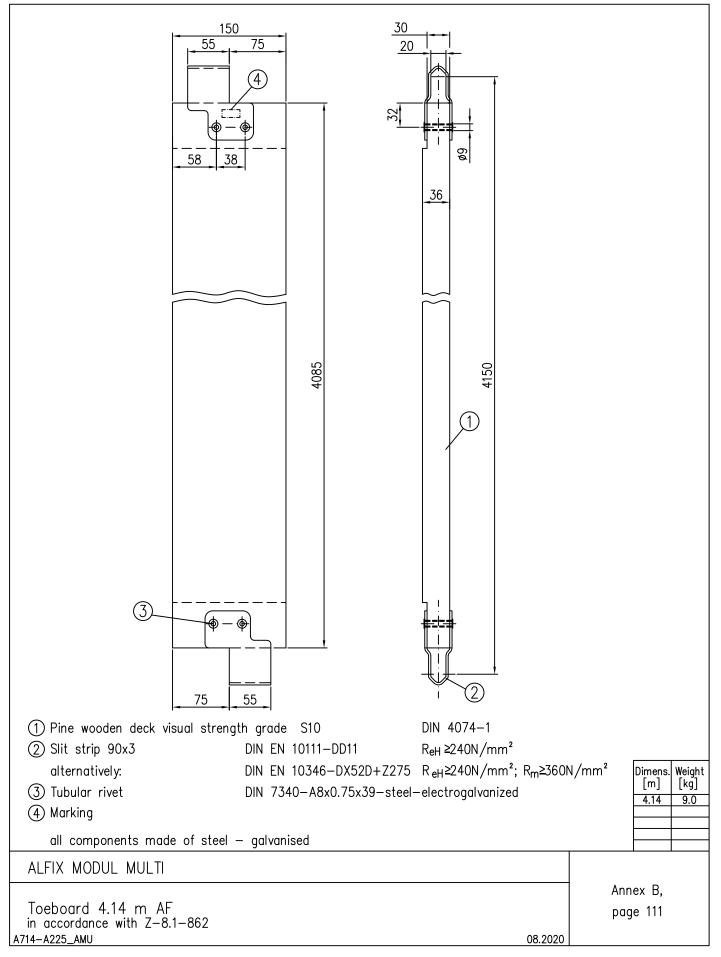


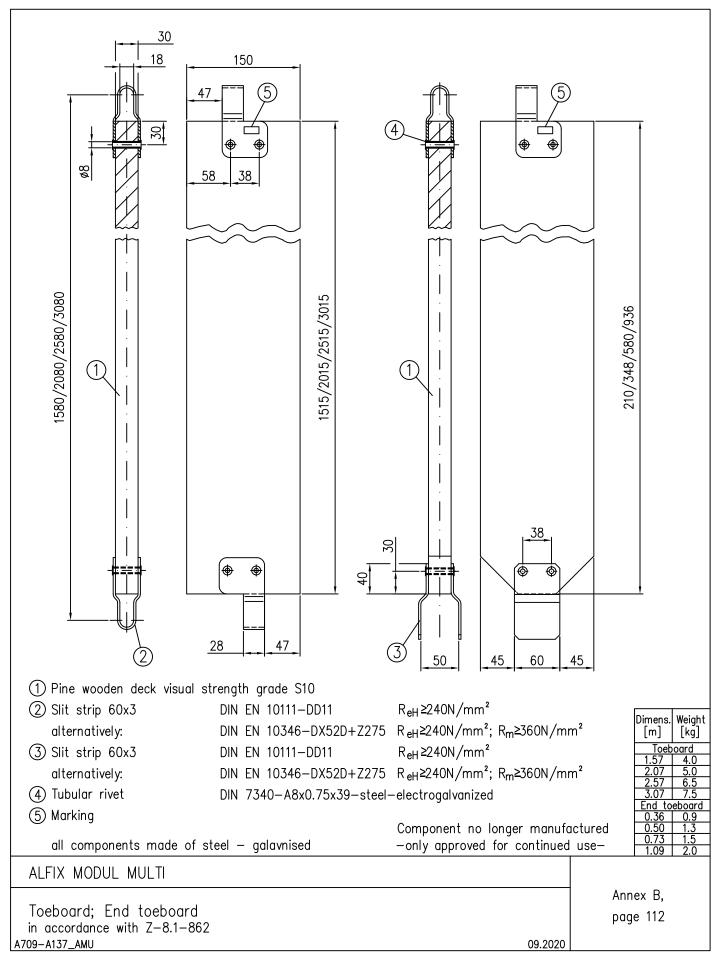


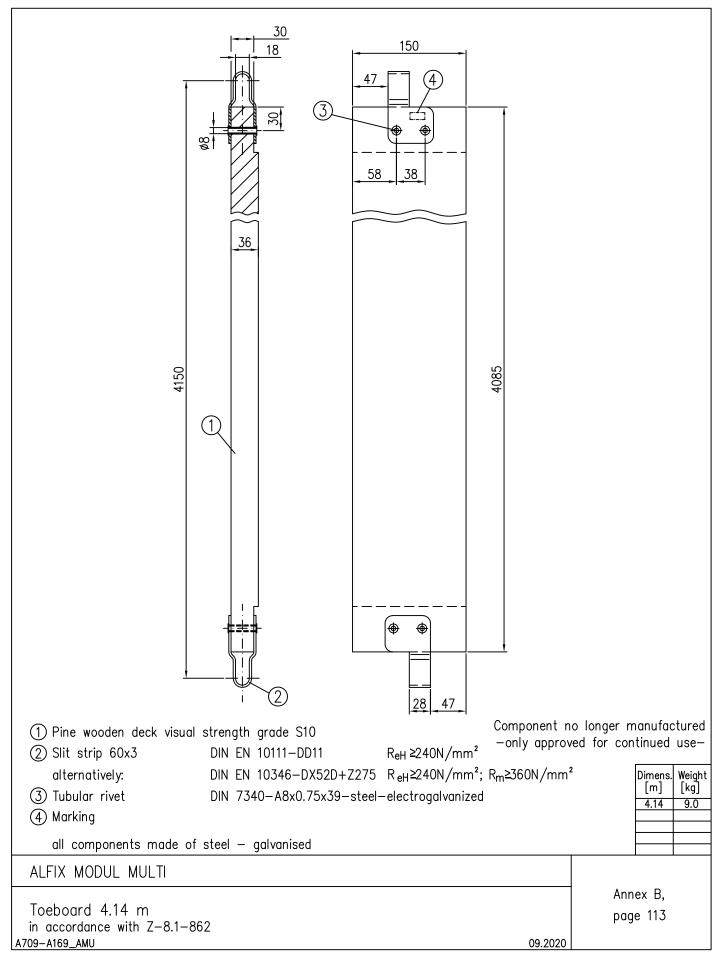


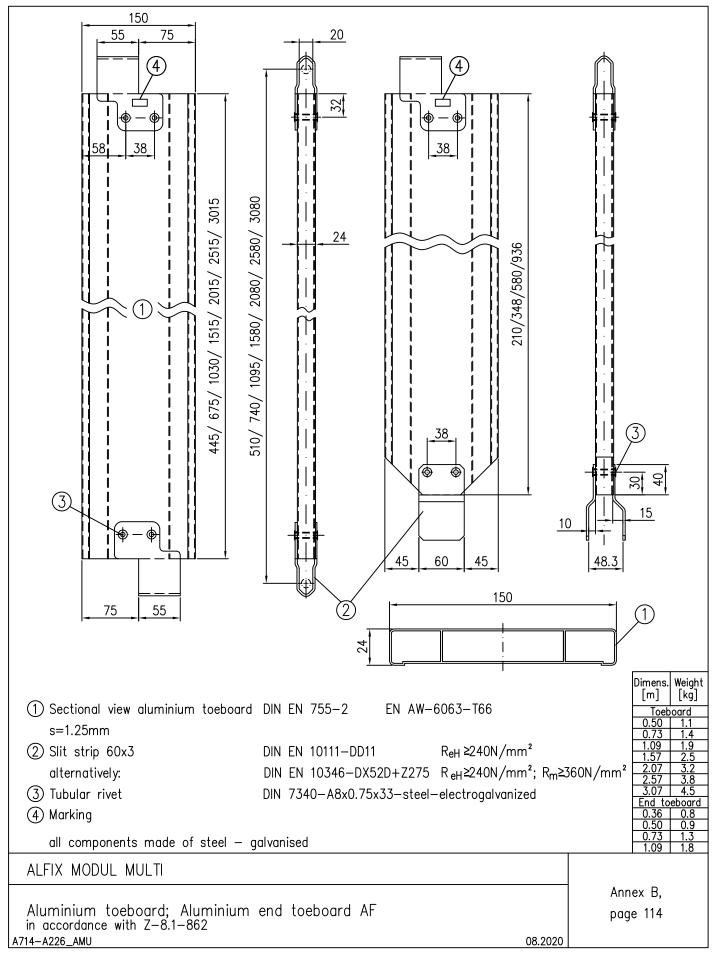


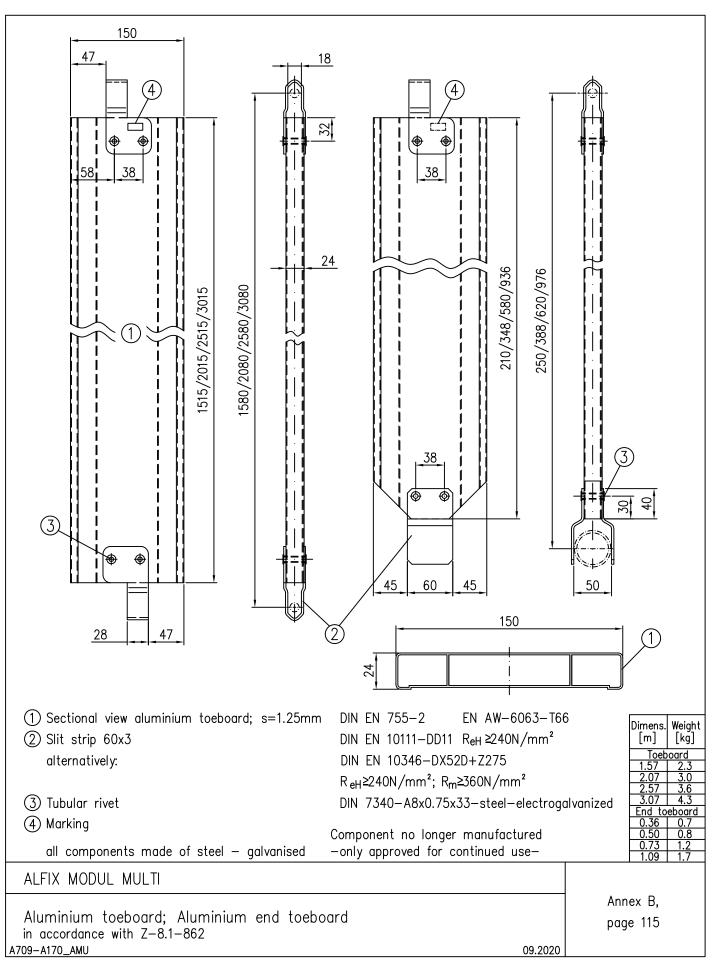


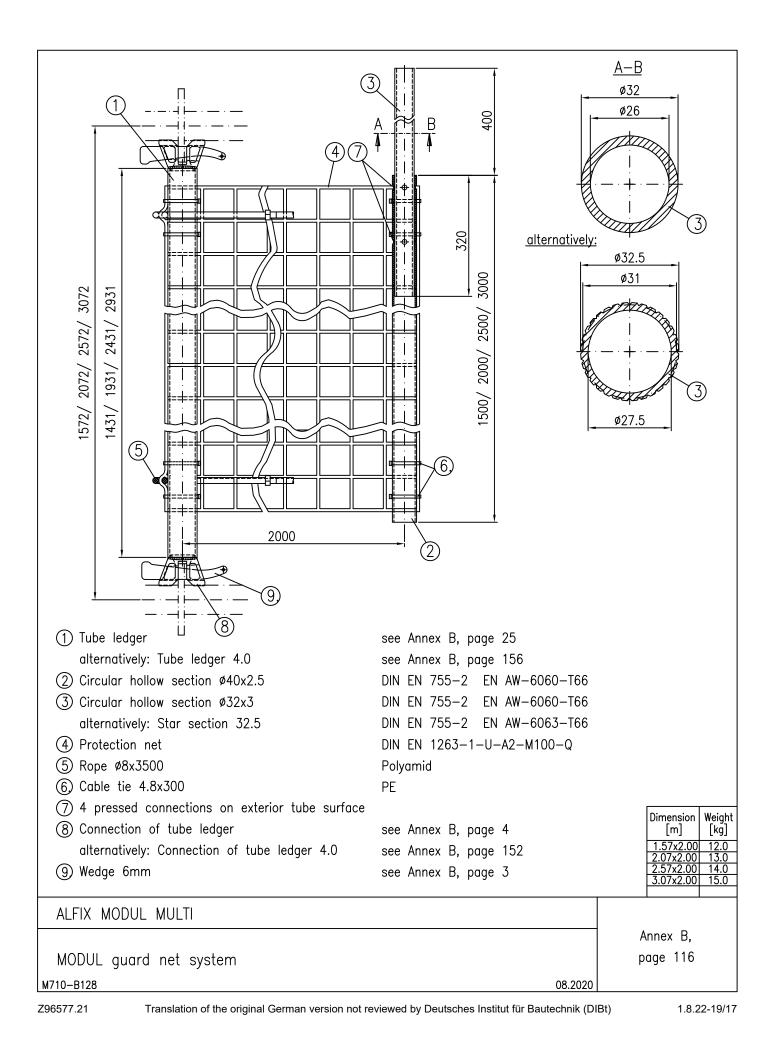


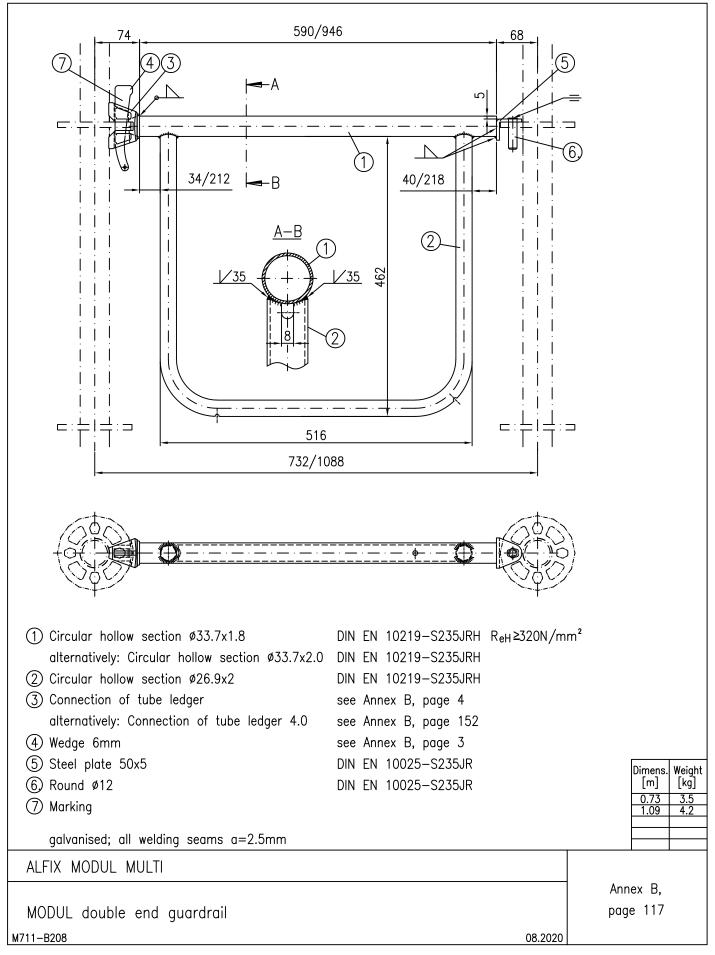


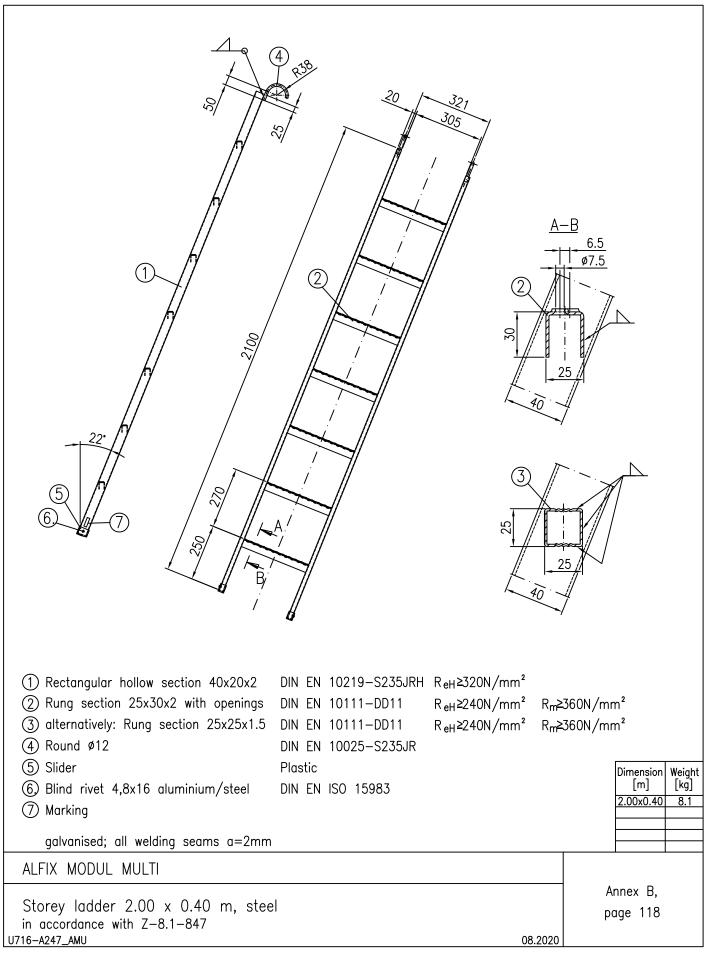


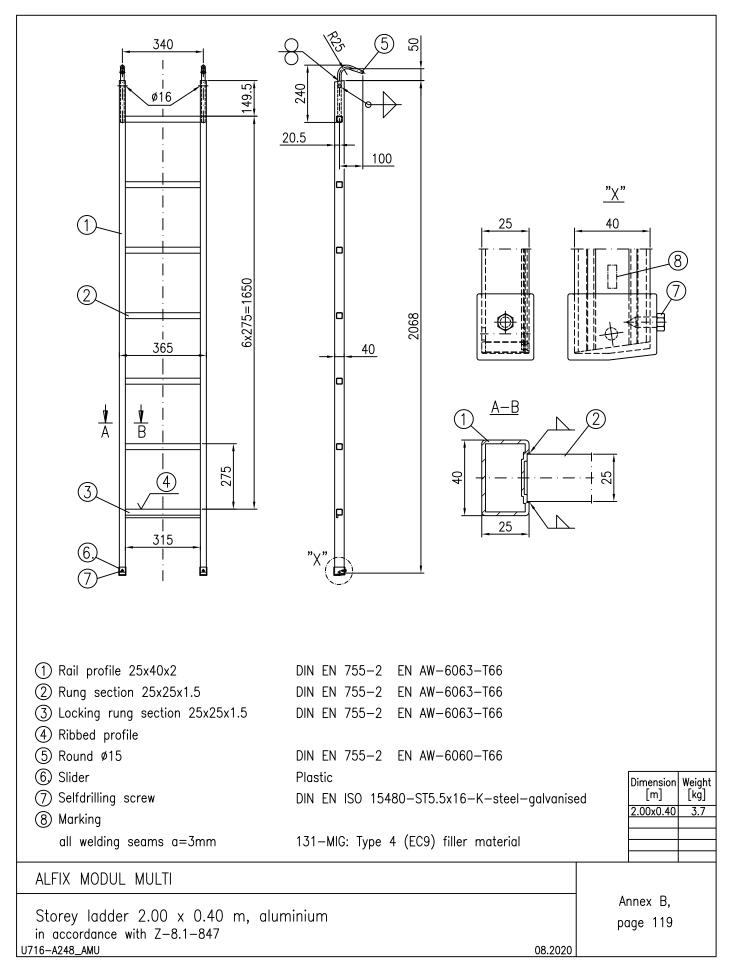


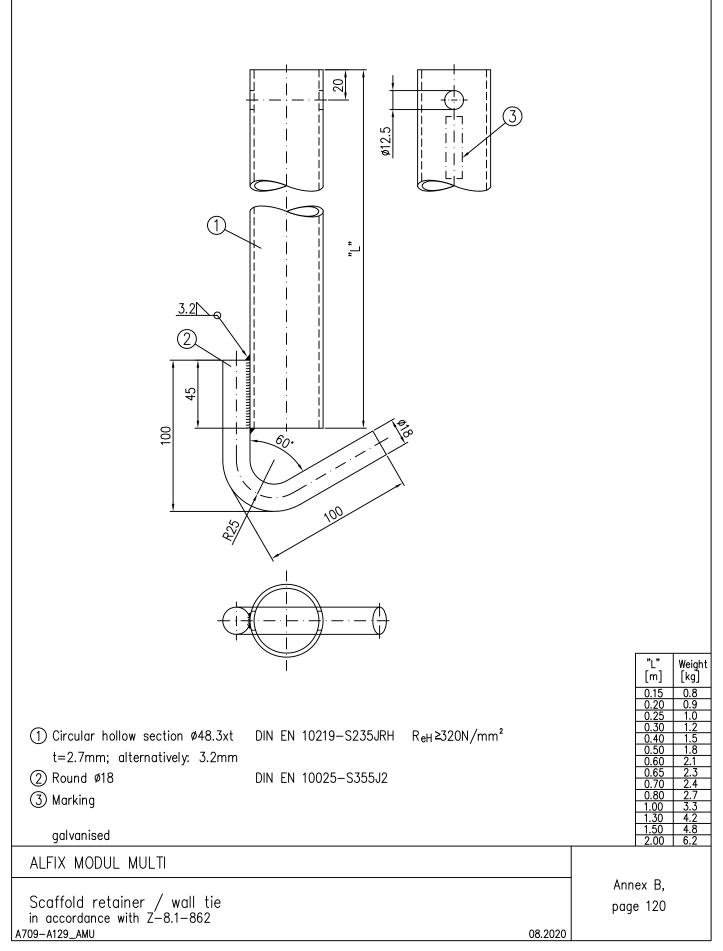


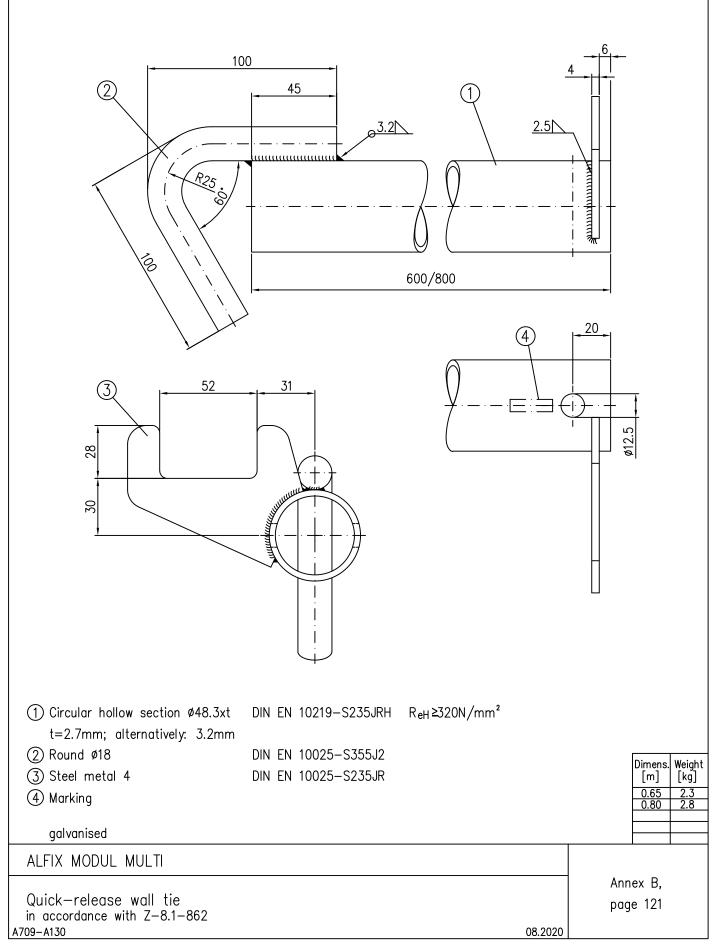


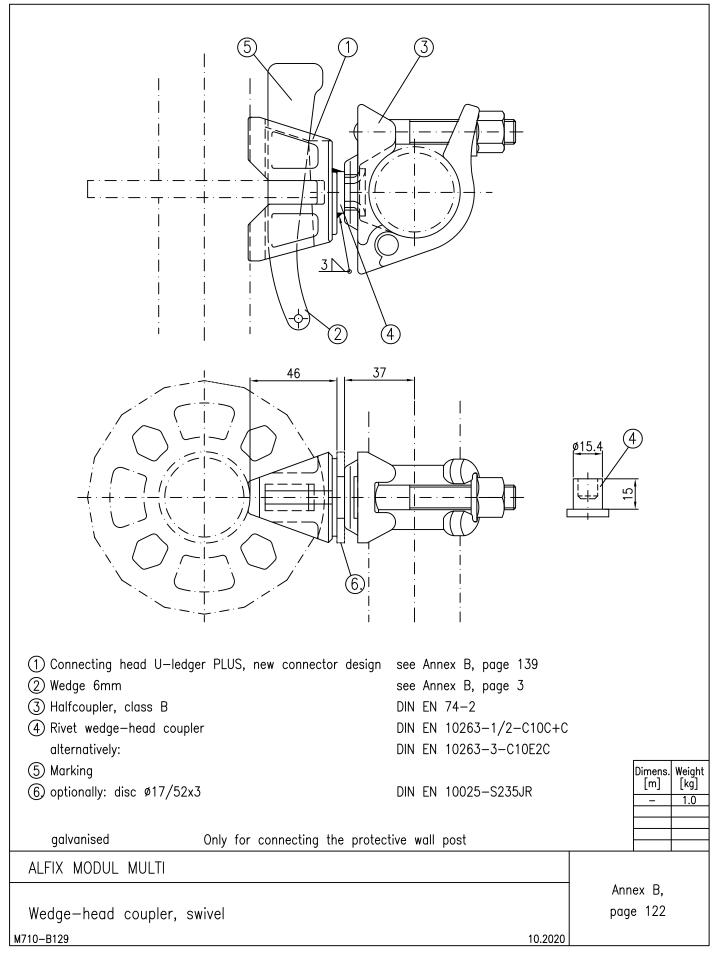


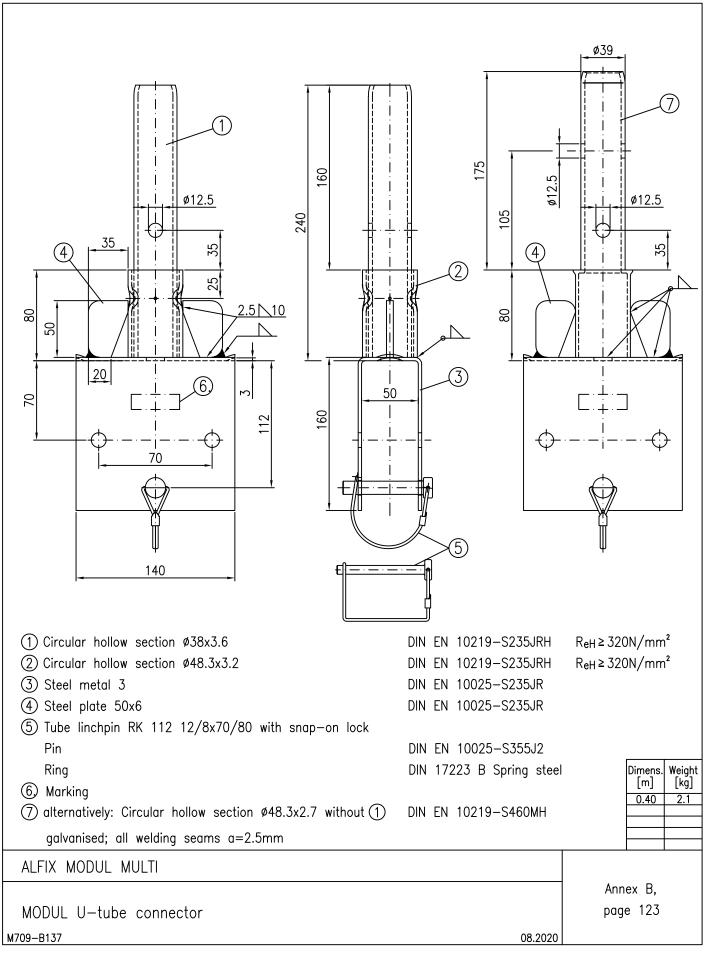


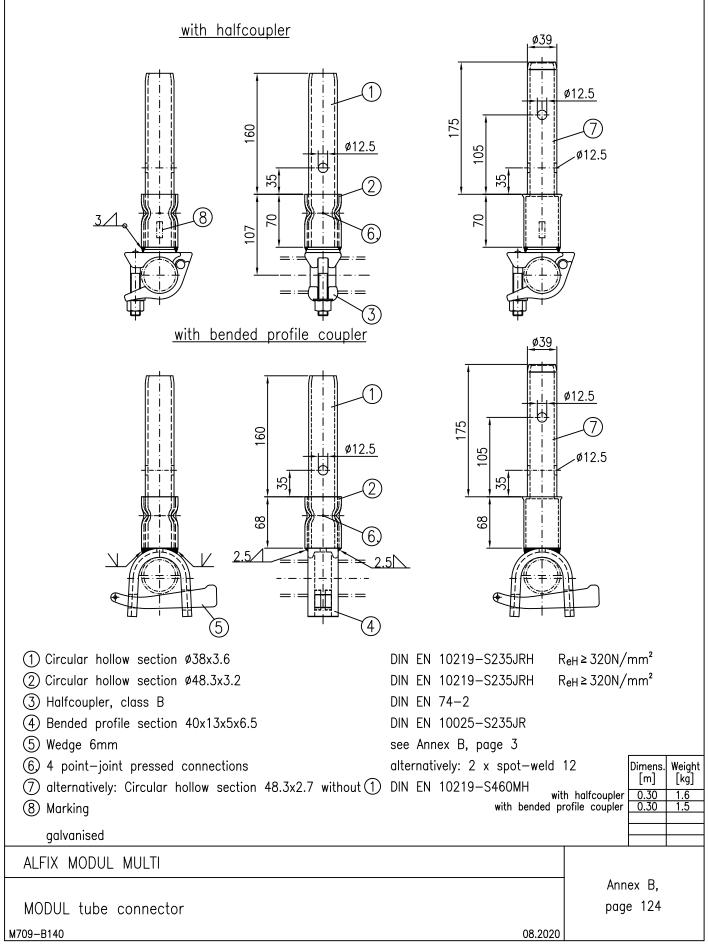


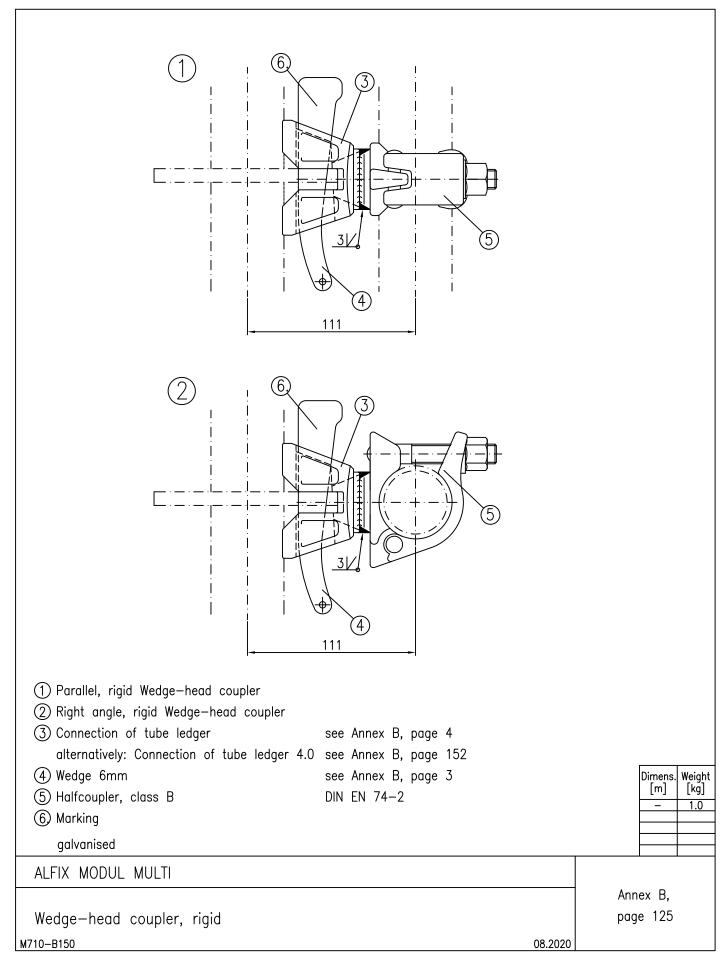


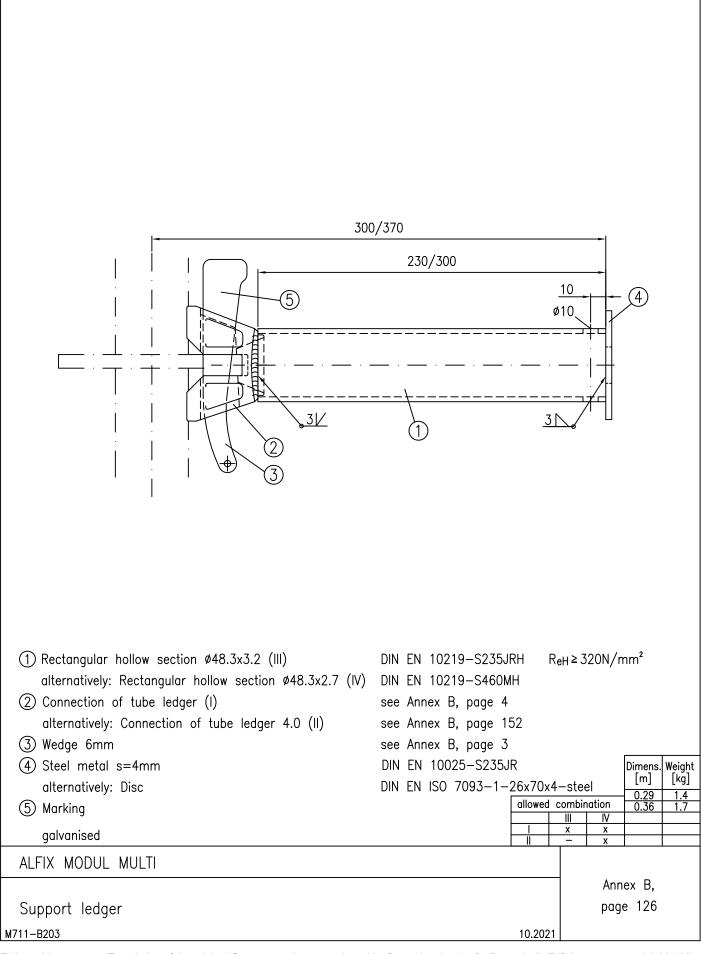


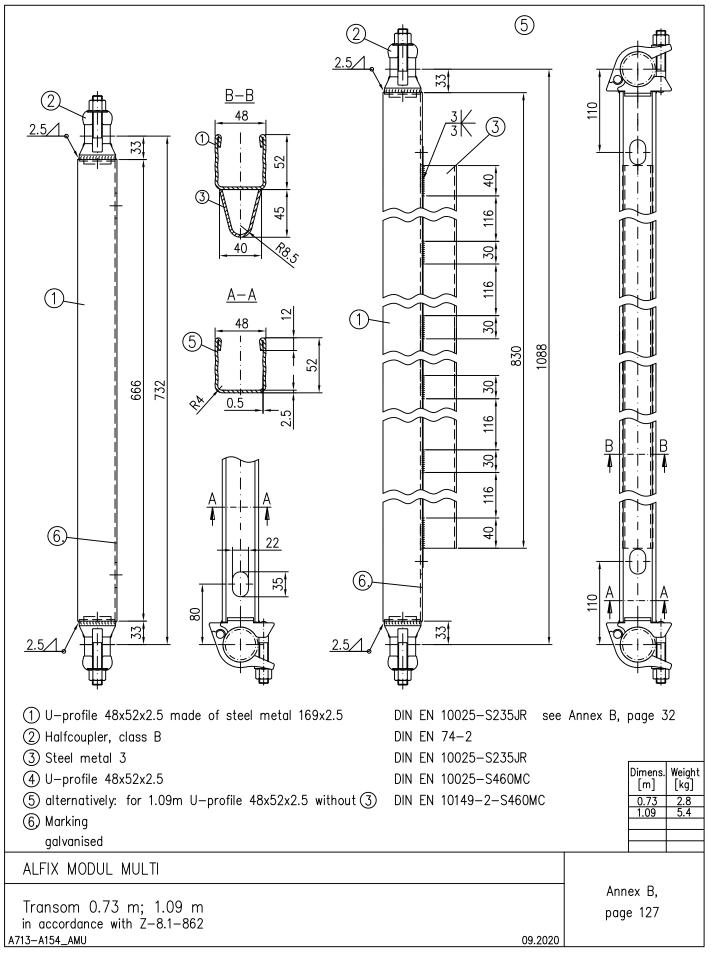


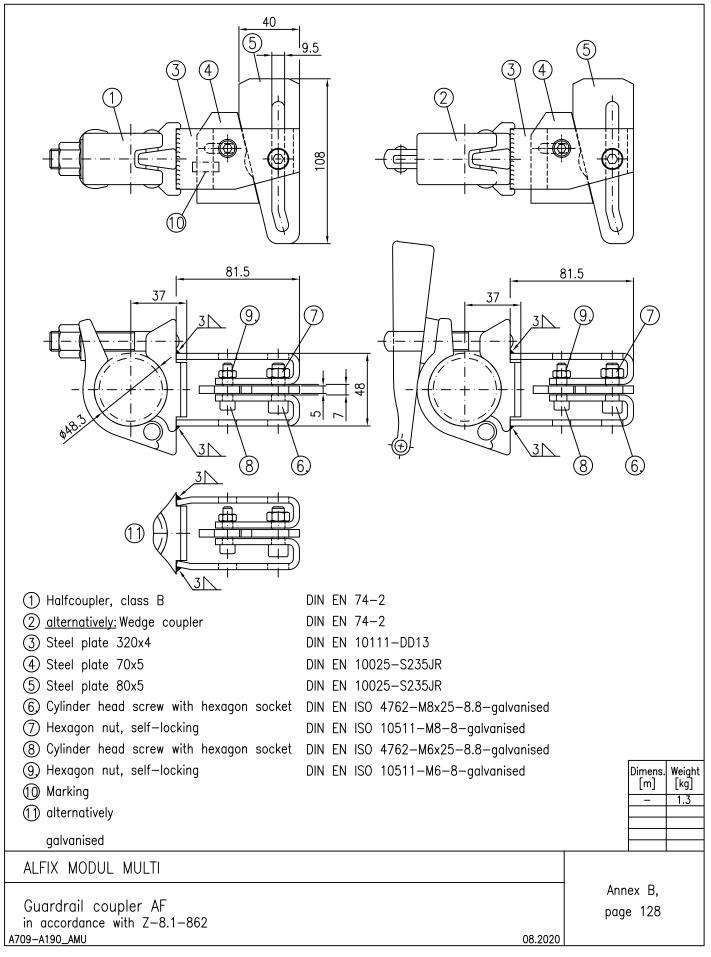


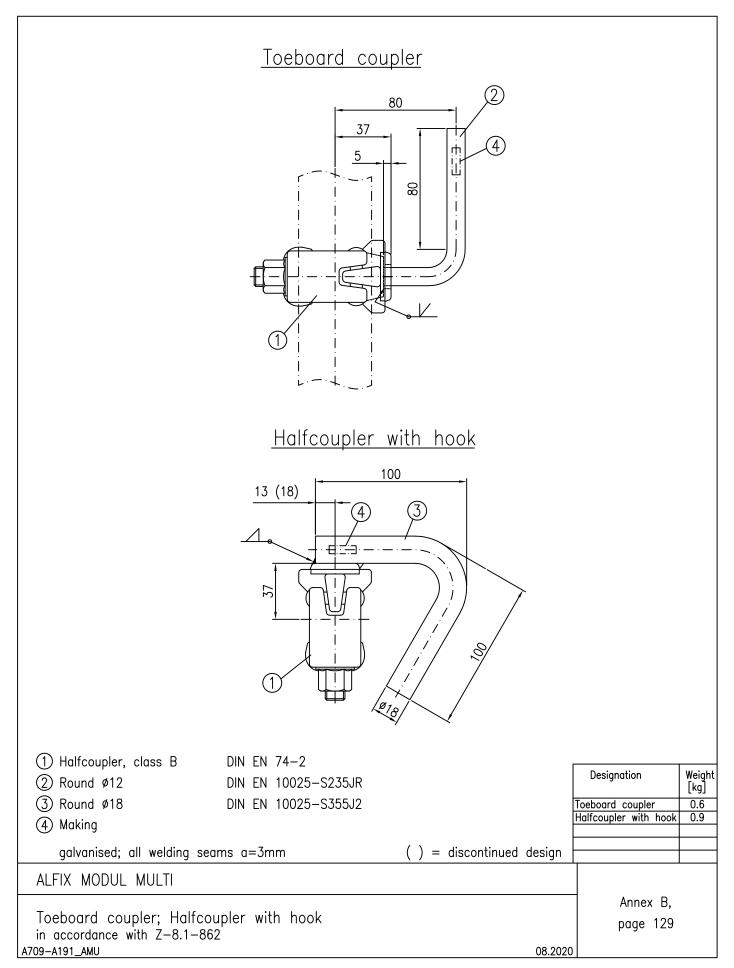


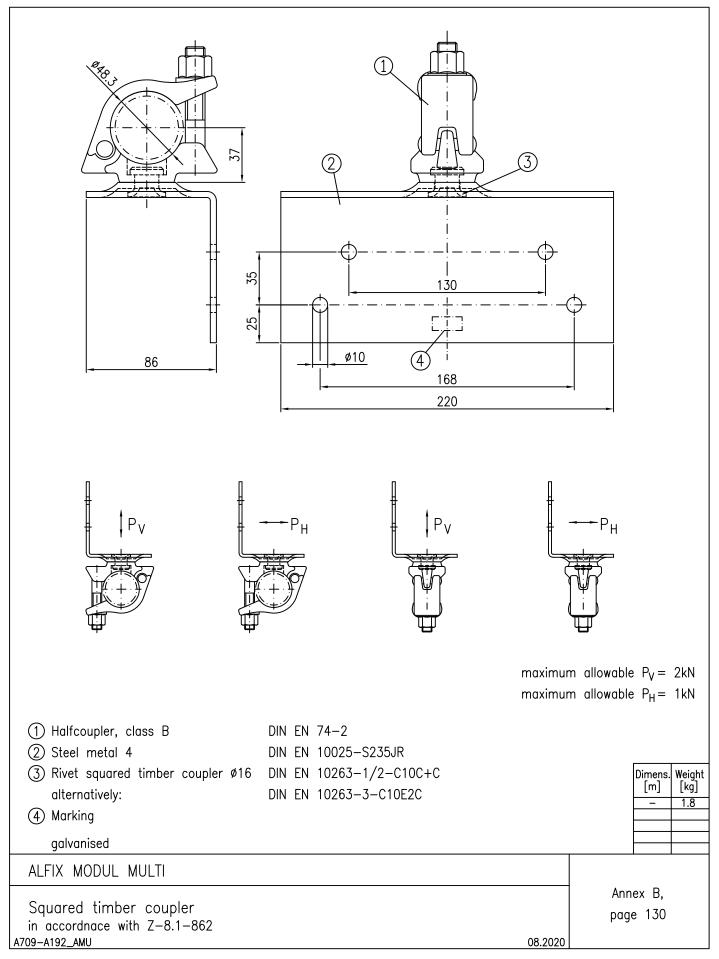


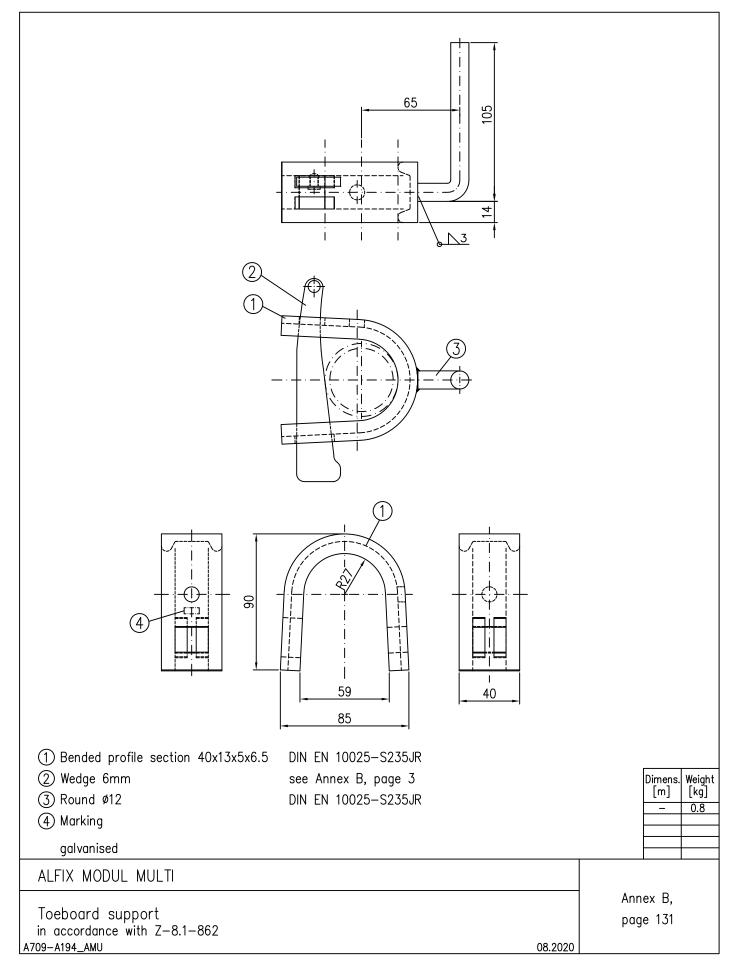


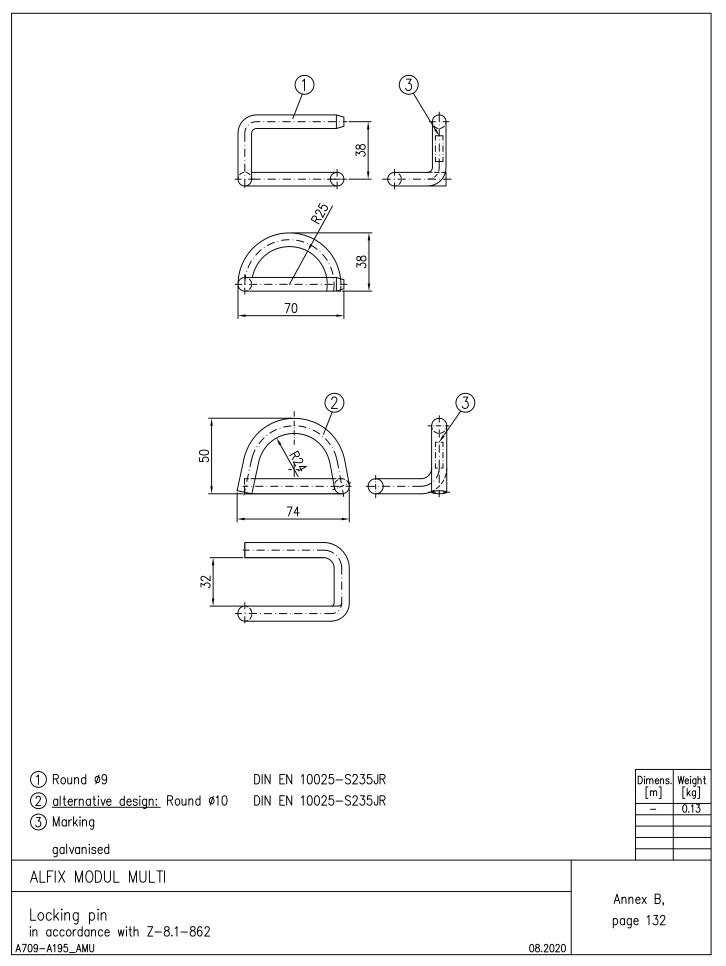


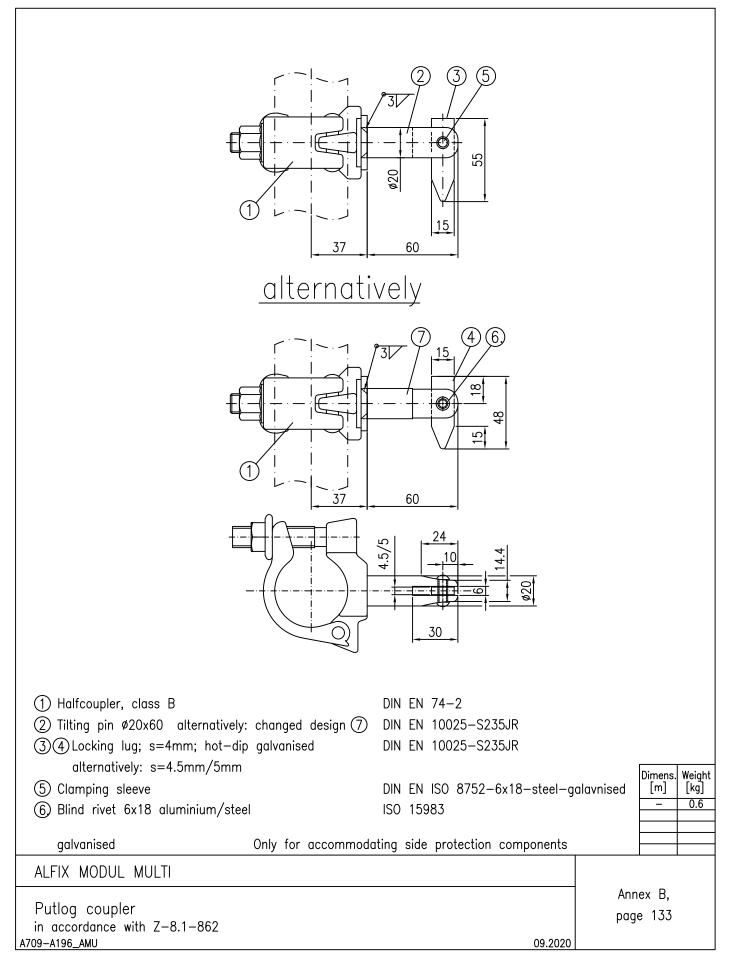


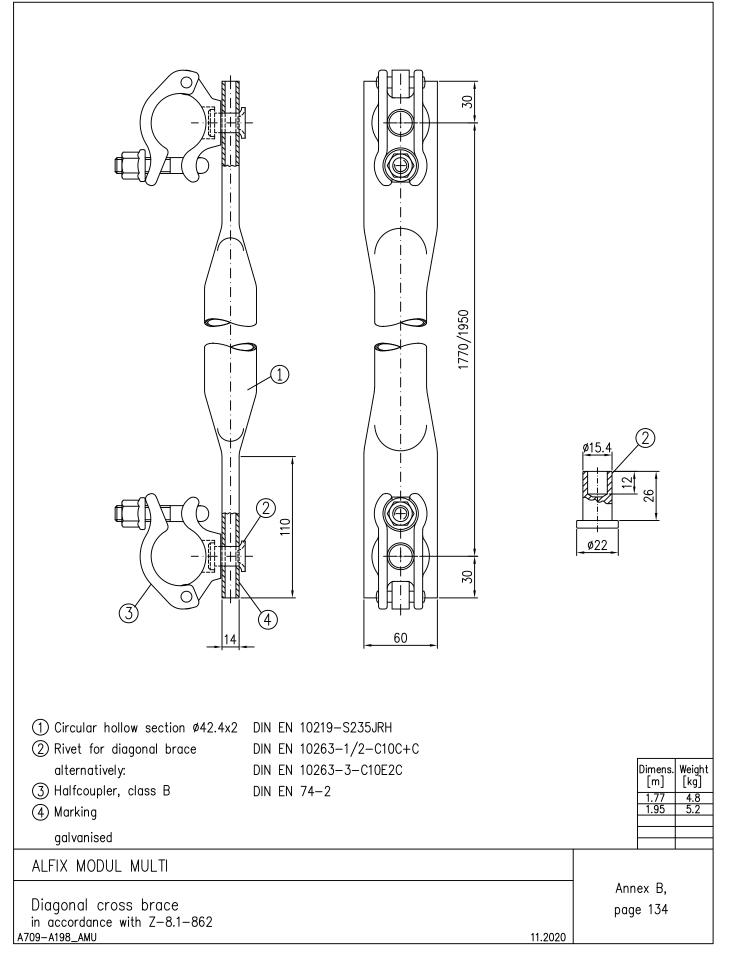


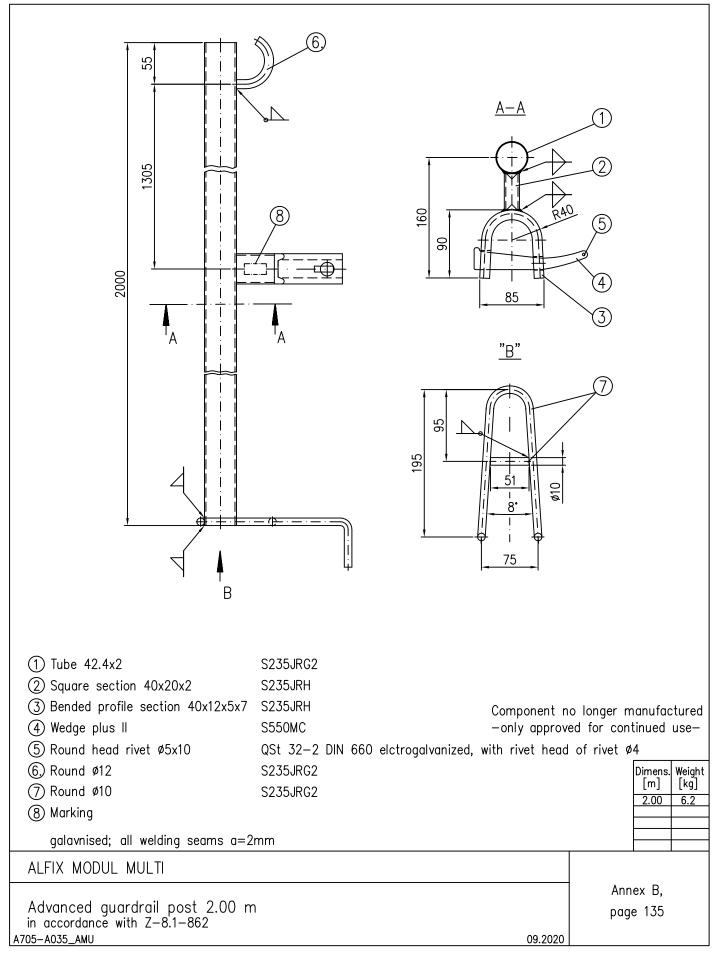


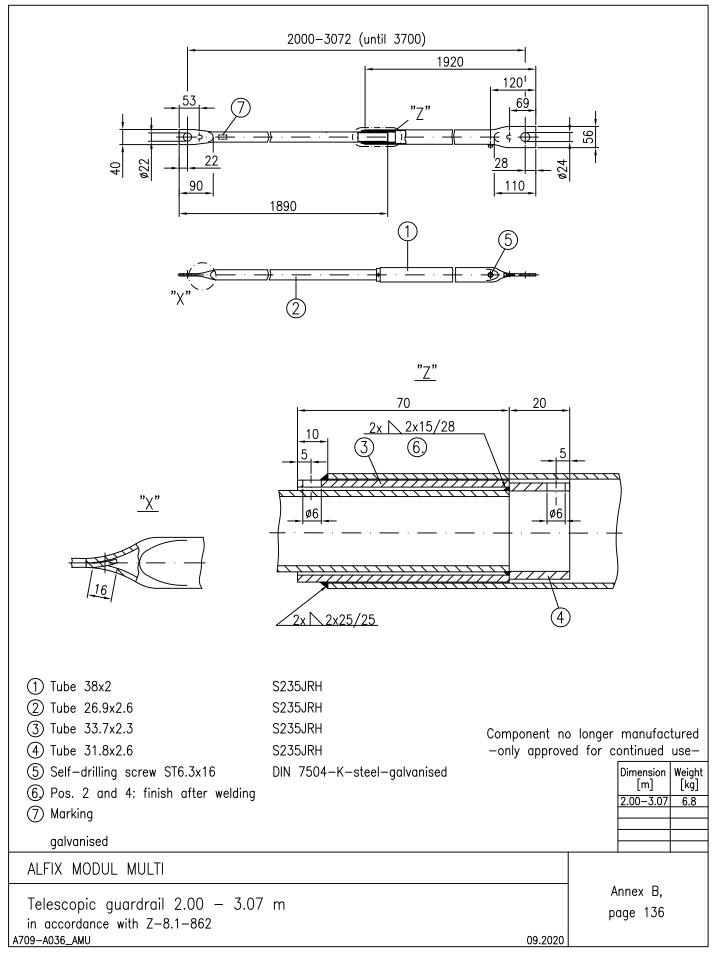


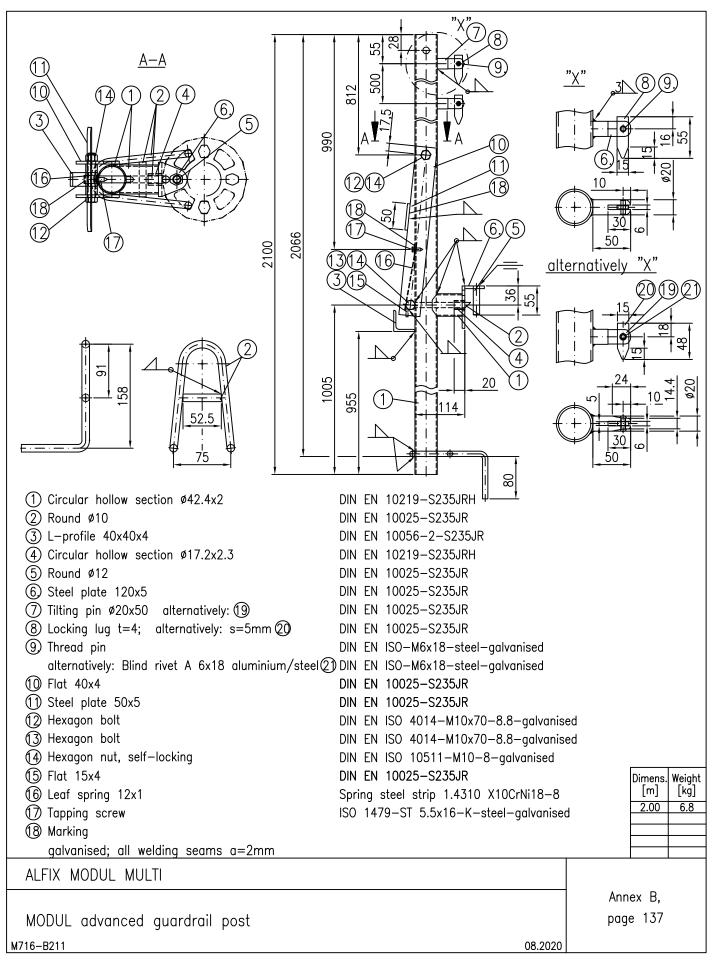


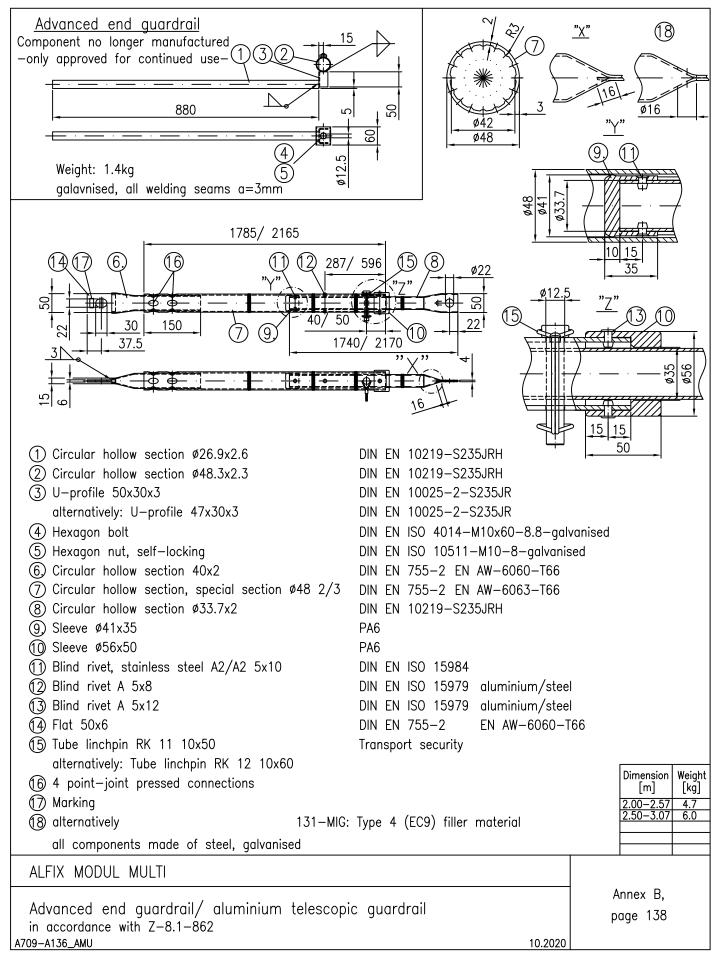


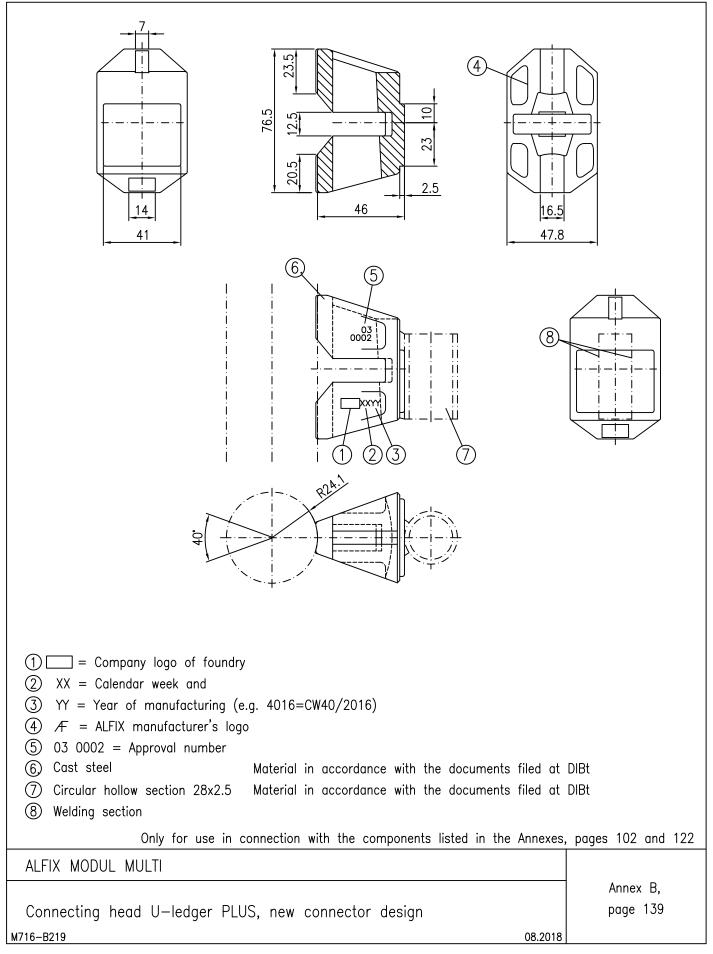


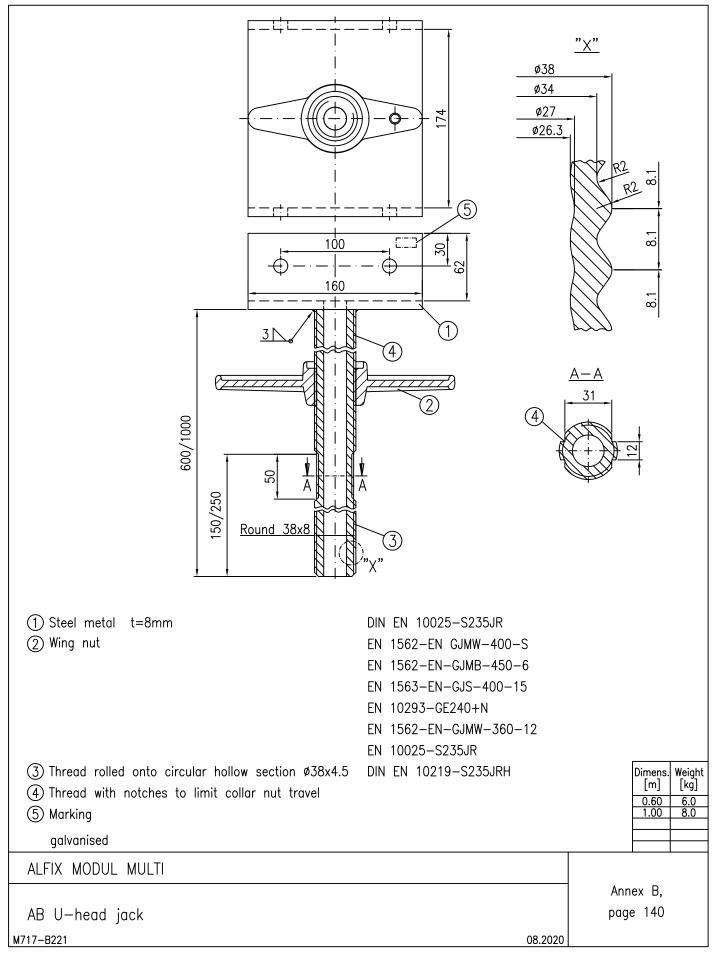


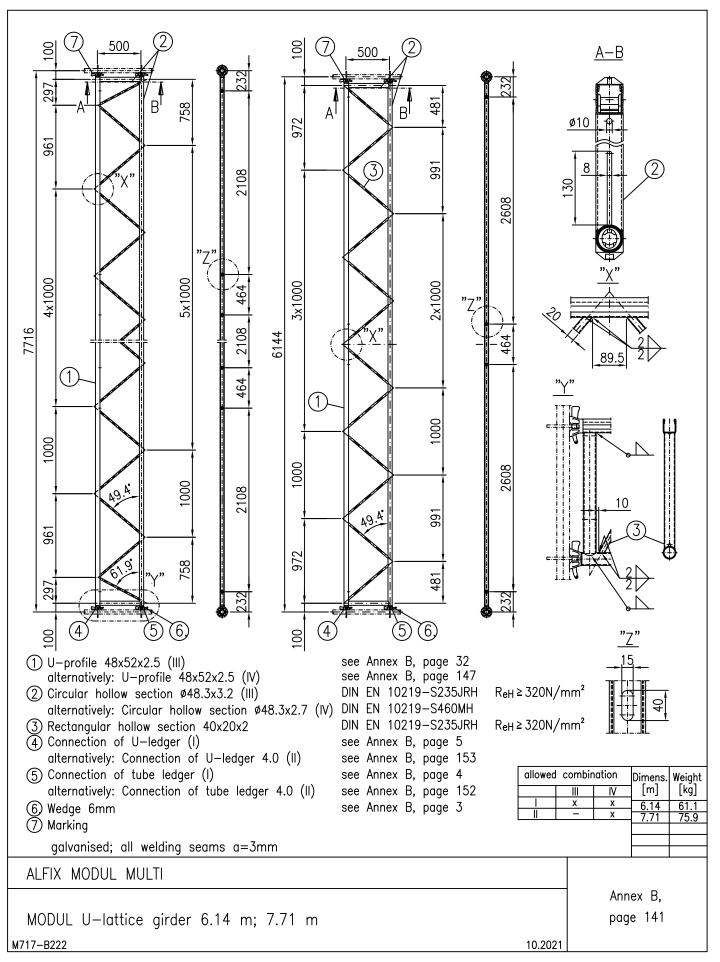


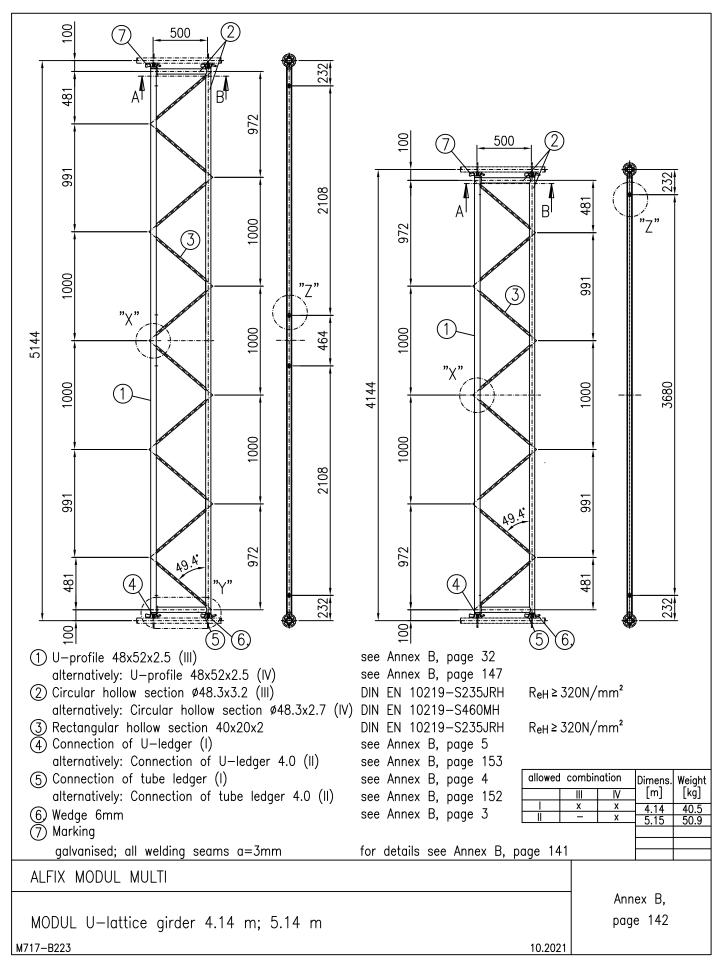


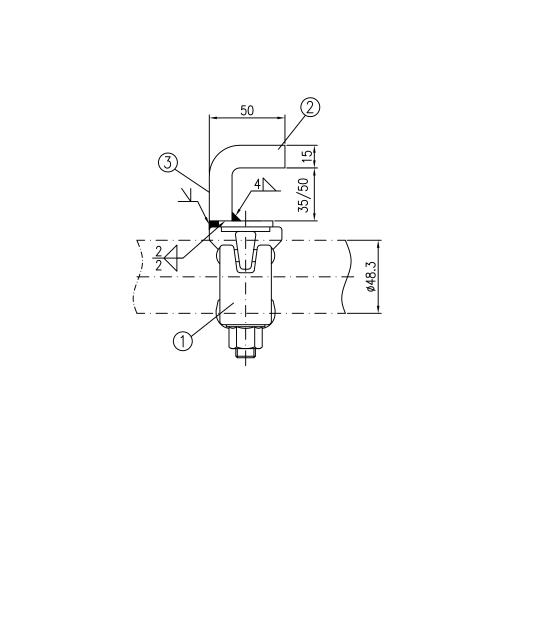












\bigcirc	Halfe	coupler,	class	В
2	Flat	40x15		

3 Marking

galvanised

Claw coupler

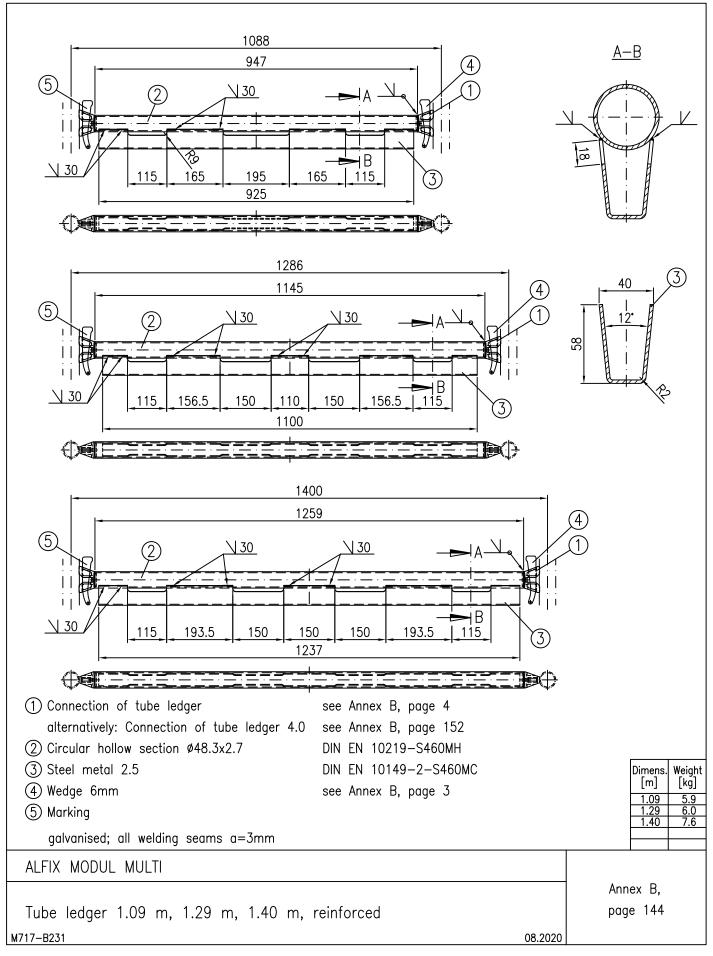
ALFIX MODUL MULTI

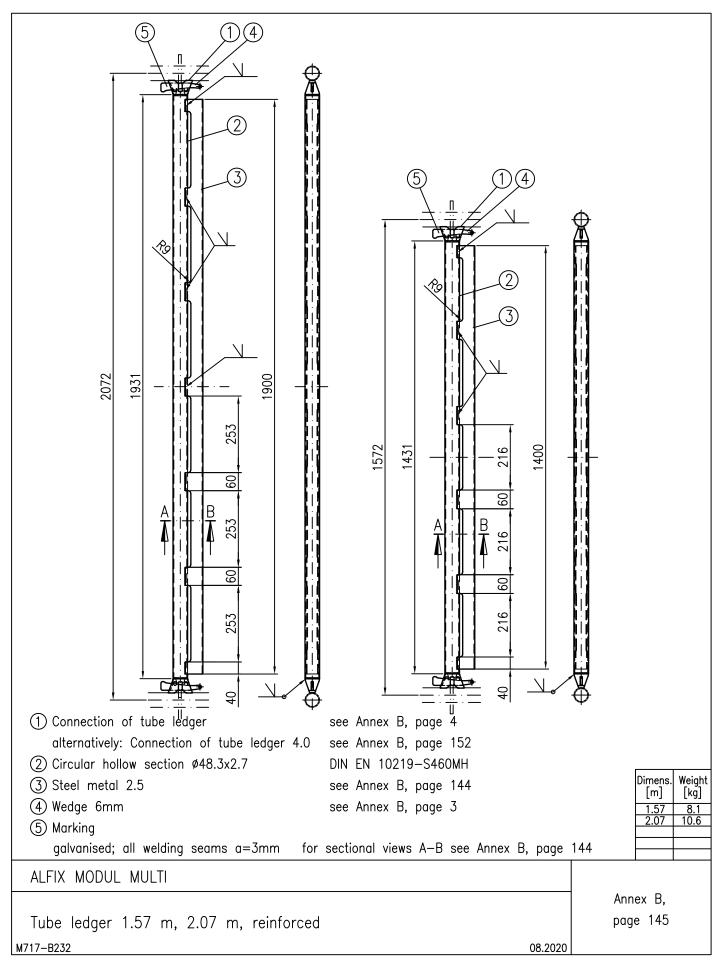
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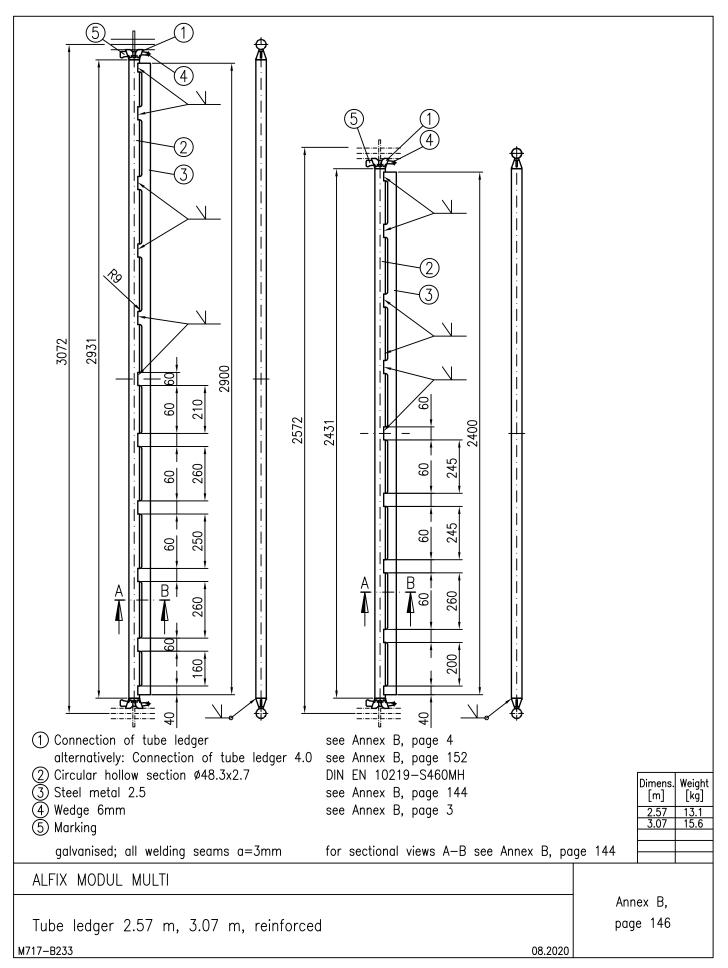
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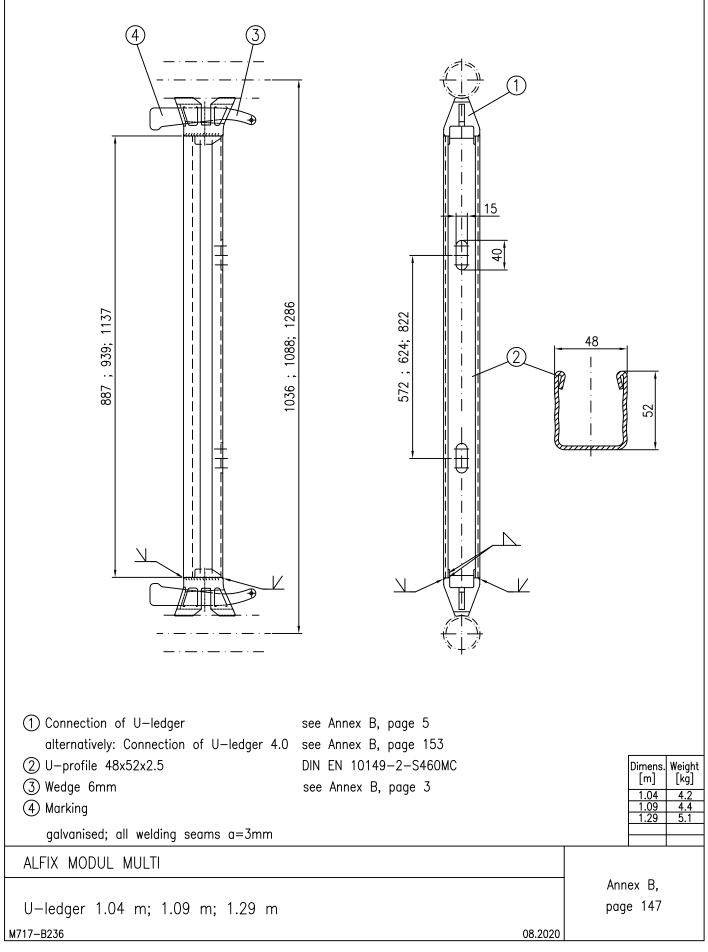
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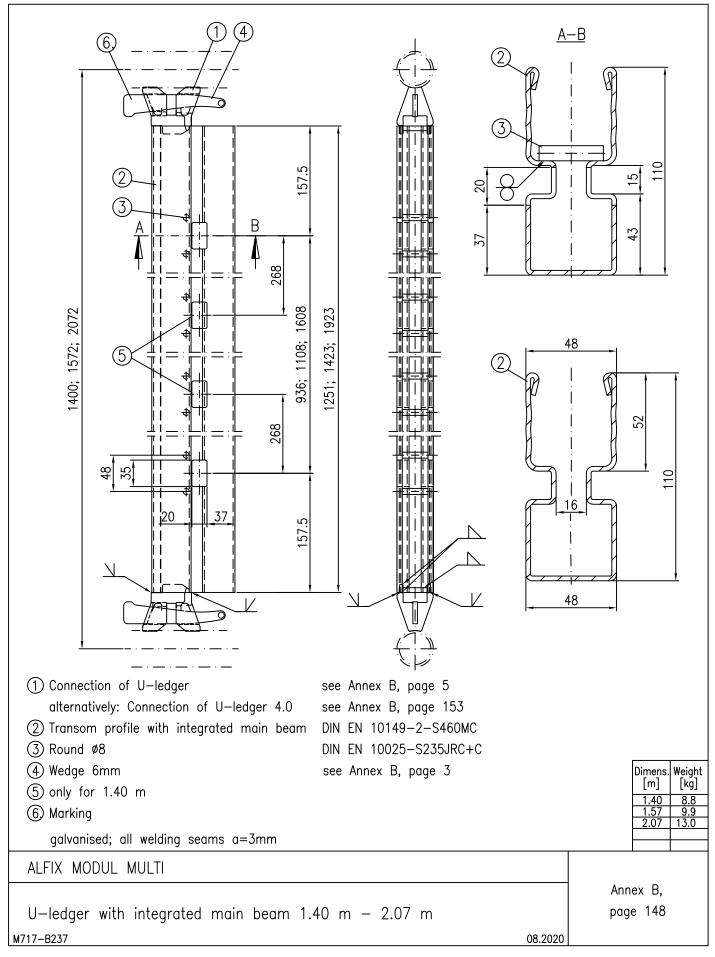


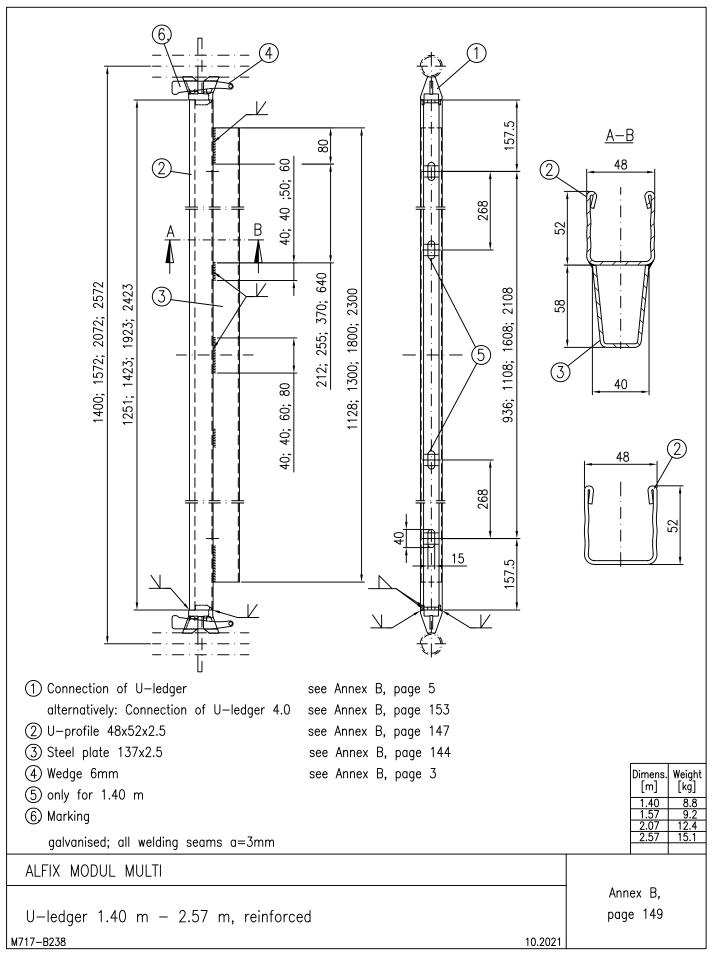


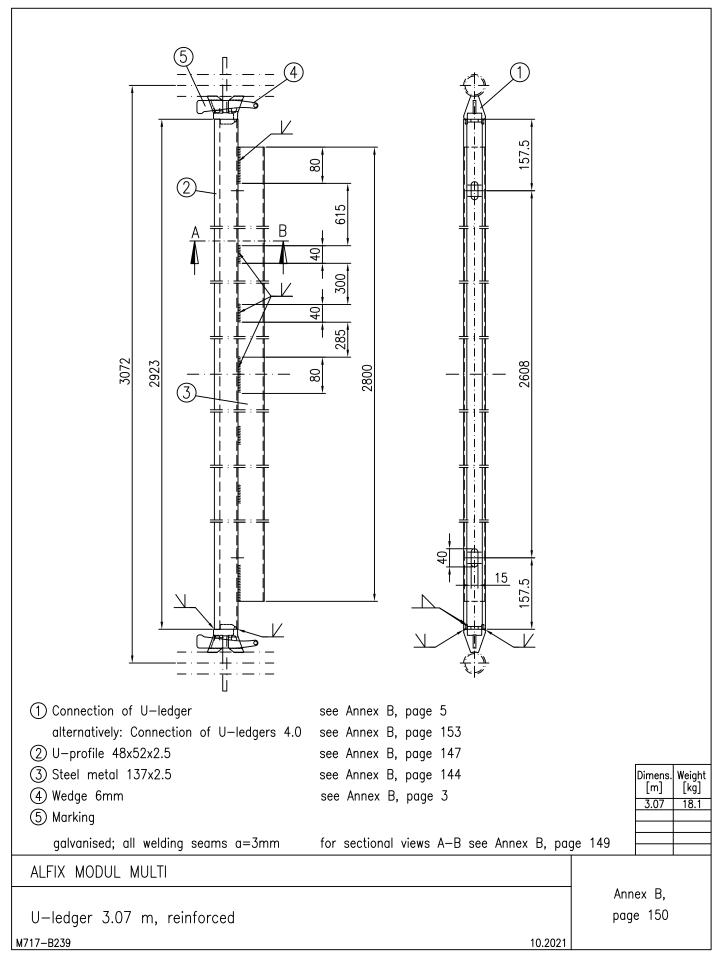




Translation of the original German version not reviewed by Deutsches Institut für Bautechnik (DIBt)







Product marking code key

XX Ü 906/932 AF XX

XX = Supplier number

Ü = Mark of conformity Ü

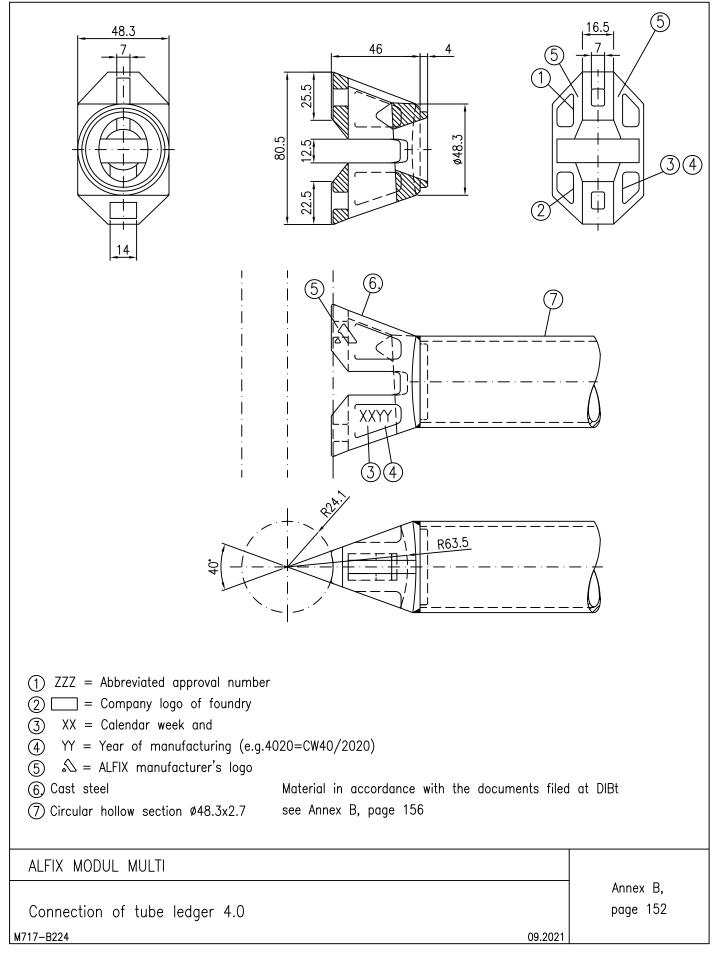
906/932 = Abbreviated approval number

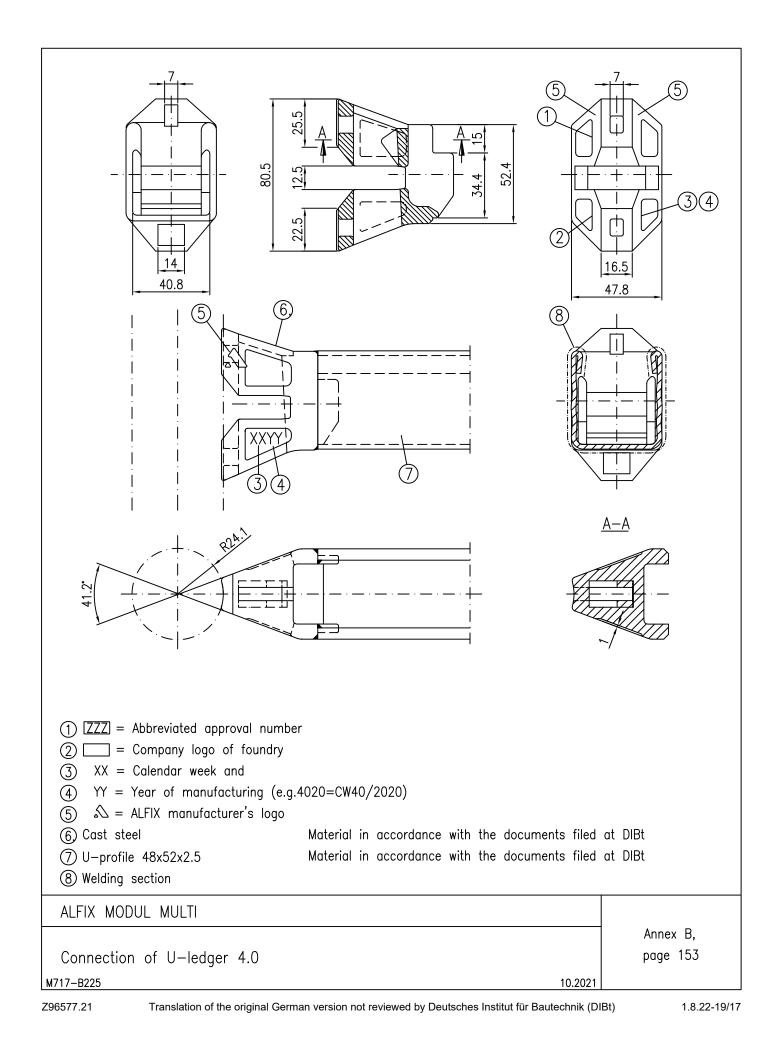
AF = ALFIX manufacturer's logo

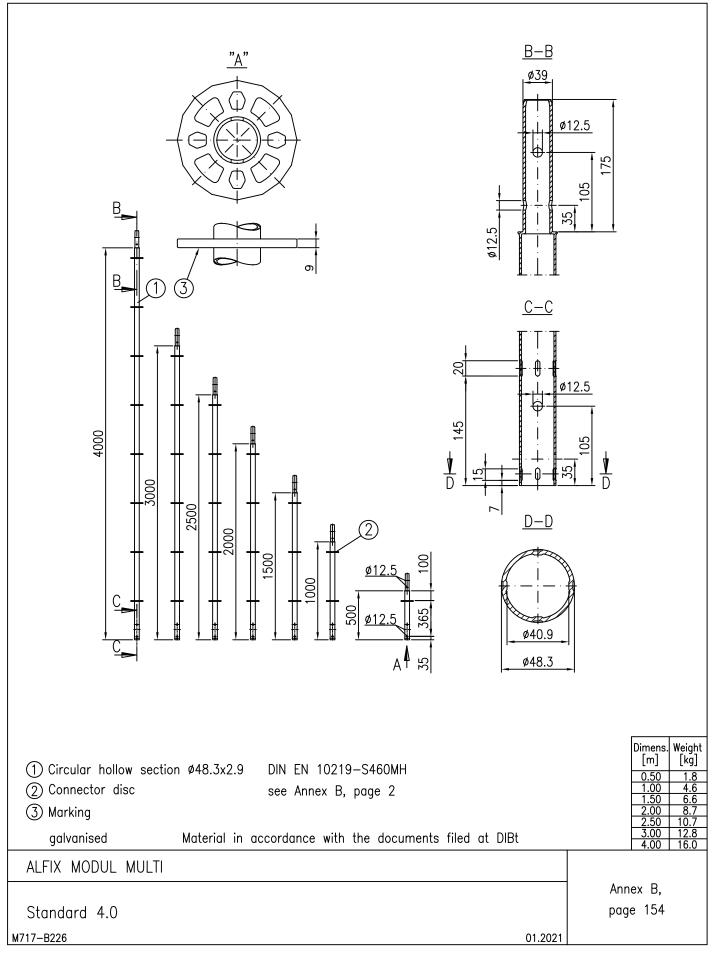
XX = Year of manufacturing

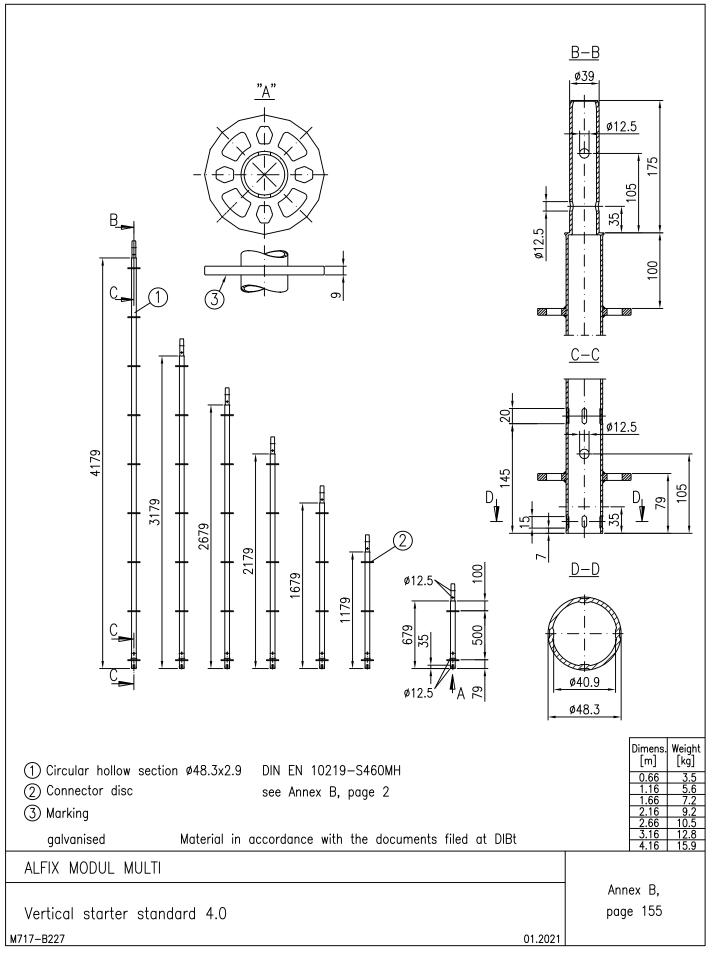
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2015	15
2016	16
2017	17
2018	18
2019	19
2020	20
etc.	etc.

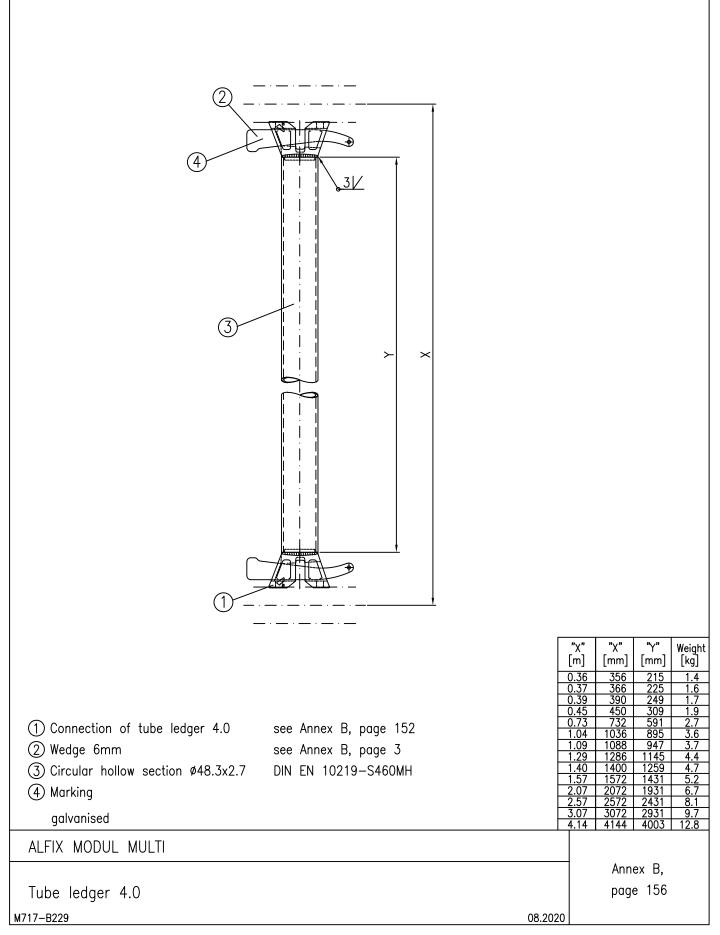
for further marking codes, please refer to Annexes B, page 2, 4-7, 152,	153
ALFIX MODUL MULTI	
Product marking code key	Annex B, page 151
M716-B220 08.202	20

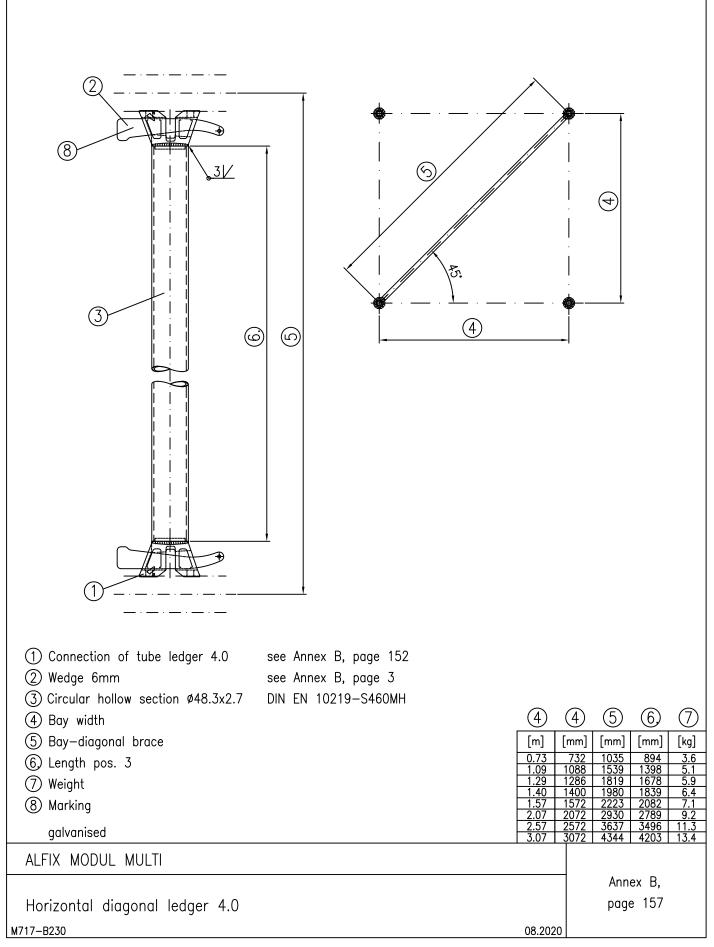


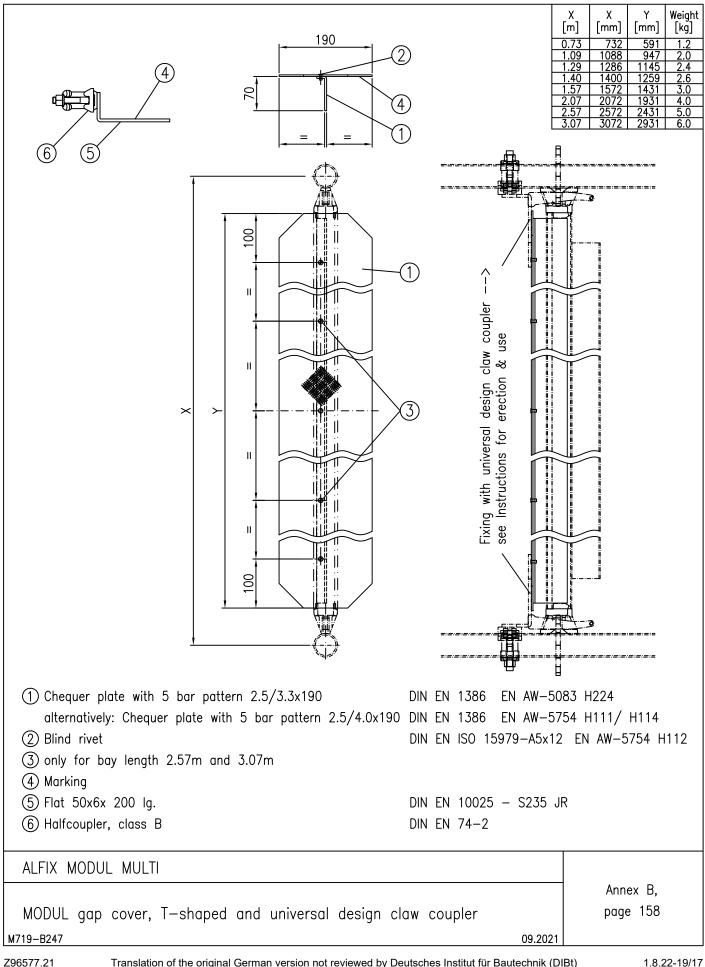


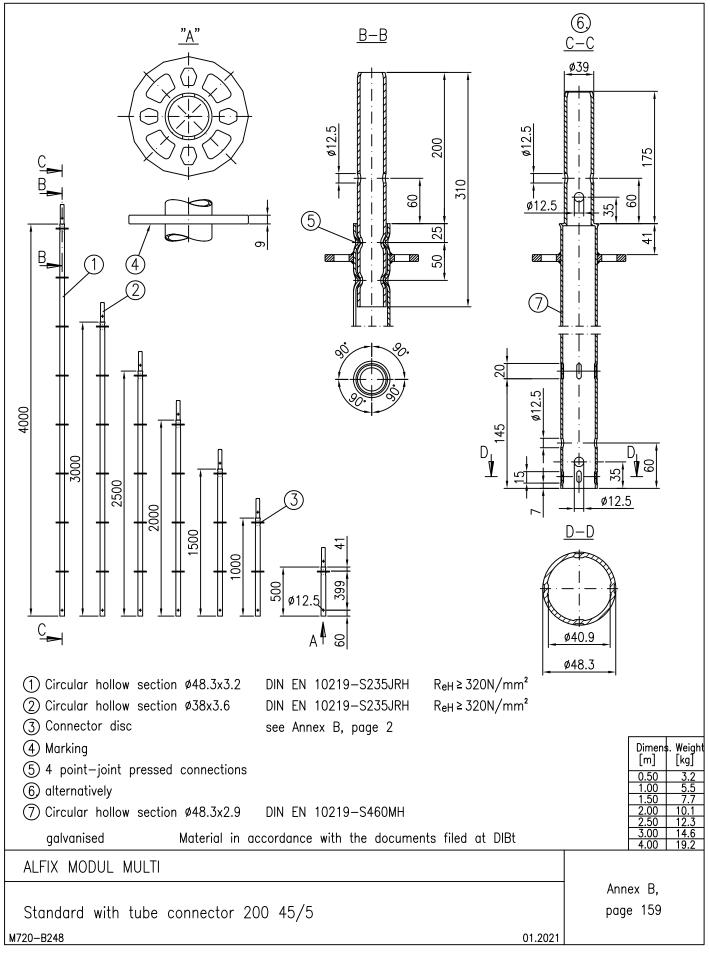












C.1 General provisions

In its standard assembly configuration, the scaffolding system may be used as working scaffold of load classes ≤ 3 with system width b = 0.732 m and bay widths $l \leq 3.07$ m in accordance with DIN EN 12811-1:2004-03, and as protection scaffold and roof edge protection scaffold in accordance with the regulations stipulated in Section C.2.

The topmost horizontal level (working level) must not exceed 24 m above ground level, not including the spindle extension length. The standard assembly configuration of the scaffolding system is designed for use on a scaffold level in accordance with the regulations of the DIN EN 12811-1:2004-03 standard, Section 6.2.9.2 in front of a "partially open" facade with an open proportion of no more than 60%, and in front of closed facades. When determining the wind load, a statistical factor of

 χ = 0.7, assuming a maximum service life of 2 years, has been taken into account.

Structural analysis for scaffold cladding with tarpaulins has not been provided in the standard assembly configuration. Structural analysis for scaffold cladding with nets has only been provided in the standard assembly configuration in front of closed facades. Structural analysis for net-covered scaffoldings applies to scaffolds with an aerodynamic force coefficient of the entire structure (net and scaffolding) that is no greater than $c_{f,L,total} = 0.6$

Without any further structural proof, the standard assembly configuration shall only be used if the loads the bays will carry do not exceed the respective live loads in accordance with

DIN EN 12811-1:2004-03, Table 3.

For the standard assembly configuration of the "ALFIX MODUL MULTI" scaffolding system, the following designation in accordance with DIN EN 12810-1:2004-03 shall be used:

Scaffolding EN 12810 - 3D - SW06/307 - H2 - A - LA

The assembly variants of the standard assembly configuration are listed in Table C.4.

C.2 Protection scaffold and roof edge protection scaffold

In its standard assembly configuration, the scaffolding system may be used as a protection scaffold and roof edge protection scaffold with a top fall arresting layer of class FL 1 and as a roof edge protection scaffold with protective walls of class SWD 1 according to DIN 4420:2004-03.

Access decks must not be fitted into brackets.

The protective wall is to be installed in accordance with Annex D, page 7.

Use protective nets in accordance with DIN EN 1263-1:2015-03 with a mesh size of no more than 100 mm.

C.3 Components

In addition to these components, other components may be used: steel tubes \emptyset 48.3 \cdot 3.2 mm and couplers as well as standard couplers for the connection of the wall ties and V-type wall ties to the standards in accordance with DIN EN 12811-1:2004-03.

C.4 Bracing

Horizontal scaffolding levels are to be braced by continuously installing the following members at vertical intervals of 2 m

tube ledgers 0.73 m with one aluminium frame platform with tube fixture two steel decks with tube fixture

two steel decks AF with tube fixture

in accordance with Annex B, page 51 or 52 or

in accordance with Annex B, page 64

in accordance with Annex B, page 61

or

or

Modular scaffolding system "ALFIX MODUL MULTI"

Standard assembly configuration Load Class 3 / SW06 / I ≤ 3.07 m - General provisions

Annex C, page 1

U-ledger 0.73 m and		
one aluminium frame platform with plywoo	od in accordance with Annex B, page 66 or 67	or
one aluminium deck with plywood	in accordance with Annex B, page 72, 73, 78 or	r 79 or
two steel decks	in accordance with Annex B, page 85	or
two steel decks AF	in accordance with Annex B, page 84	or
one aluminium lightweight frame platform	in accordance with Annex B, page 90	
each.		

When installing ladder accesses, the following members must be used instead of platforms and decks: aluminium access frame platforms (with tube fixture) when using tube ledgers or aluminium frame platform with internal hatch with ladder when using U-ledgers.

Secure the platforms, decks and hatches against unintentional lift-off.

The outer vertical planes are to be braced by means of tube ledgers used as guardrails (1 m above deck level) continuously for each scaffolding bay.

Vertical starter pieces are to be installed directly above the base jacks (scaffolding spindles) and connected by means of longitudinal ledgers in the inner and outer plane parallel to the facade and by means of transoms at right angles to the facade.

C.5 Anchoring

Anchoring is to be provided using wall ties in accordance with Annex B, page 120.

Wall ties are to be installed as anchor pairs at an angle of 90° (V-type wall tie) or as "short" wall ties only to the inner vertical frame standard using standard couplers. Depending on the assembly configuration, the node points that are anchored by means of V-type anchors are to be connected on the inner plane parallel to the facade with the adjacent standard section by means of tube ledgers (longitudinal ledgers).

The V-type anchors and wall ties are to be installed in the immediate vicinity of the node points of the standard tubes and transoms.

The anchor forces listed in Table C.2 were determined with the characteristic values of the actions $(\gamma_F = 1.0)$. For the design analysis of the anchorage and the load transfer, the values given must be multiplied by the respective partial safety factor γ_F (generally $\gamma_F = 1, 5$).

Each frame section is anchored at vertical intervals of 8 m; anchoring points of neighbouring vertical frame sections must be arranged with a vertical offset of half the spacing. Frame sections at the edge of a scaffolding must be anchored at a vertical interval of 4 m. On the topmost and second working level, each standard section must be anchored.

C.6 Foundation loads

Depending on the assembly variation, the foundation loads listed in Table C.3 must be absorbed and transferred in the supporting surface.

The foundation loads are given as characteristic values.

For the structural analysis of transfer of loads in the supporting surface, the values given must be multiplied by the partial safety factor γ_F (generally $\gamma_F = 1.5$).

C.7 Bridging construction

The bridging girders may be used at a height of 4m to bridge gate entrances or similar openings when the working levels underneath the bridging part are omitted.

The bridging girders are to be anchored in the supporting and centre section and are to be braced additionally by means of a horizontal bracing unit comprised of tubes and couplers or by means of additional anchorage (see also Annex D, pages 3, 4 and 8).

Modular scaffolding system "ALFIX MODUL MULTI"

Annex C, page 2

Standard assembly configuration Load Class 3 / SW06 / I ≤ 3.07 m - General provisions

C.8 Ladder access

When installing internal ladder accesses, aluminium access frame platforms (with tube fixture) must be installed when using tube ledgers, or aluminium frame platforms with internal hatch or aluminium access frame platforms with ladder must be installed when using U-ledgers.

C.9 Widening bracket

Only use brackets 0.39 m on the inner face of the scaffolding on all working levels.

Table C.1: Components of the standard assembly configuration

Designation	Annex
Vertical starter piece	B, page 10
Standard with tube connector 200	10
	17
Base jack	17
AB Base jack	
Tube ledger I ≤ 3.07 m	25 32
U-ledger 0.73 m U-transom lattice girder 0.73 m V	<u> </u>
Tube-transom lattice girder 0.73 m V	45
MODUL lattice girder 6.14 m	46
MODUL lattice girder 4.14 m / 5.14 m	47
MODUL lattice girder with tube fixture 6.14 m	48
MODUL lattice girder with tube fixture 4.14 m / 5.14 m	49
MODUL lift-off preventer	50
Aluminium frame platform with tube fixture 1.57 m; 2.07 m	51
Aluminium frame platform with tube fixture 2,57 m; 3,07 m	52
Aluminium access frame platform with tube fixture 3.07 m	54
Aluminium access frame platform with tube fixture 2.57 m	55
Aluminium access frame platform with tube fixture 1.57 m – 3.07 m without ladder	57
Aluminium access frame platform with tube fixture 2.57 m – 3.07 m with aluminium chequer plate	58
Steel deck AF with tube fixture 0.32 m	61
Steel deck with tube fixture	64
Aluminium frame platform with plywood 1.57 m; 2.07 m	66
Aluminium frame platform with plywood 2.57 m; 3.07 m	67
Aluminium frame platform with internal hatch 2.57 m; 3.07 m	69
Aluminium deck with plywood 2.57 m; 3.07 m	72
Aluminium deck with plywood 1.57 m; 2.07 m	73
Aluminium access frame platform 3.07 m with ladder	75
Aluminium access frame platform 2.57 m with ladder	76
Aluminium deck with plywood 3.07 m	78
Aluminium deck with plywood 1.57 m, 2.07 m, 2.57 m	79
Aluminium access deck with ladder 3.07 m	81
Aluminium access deck with ladder 2.57 m	82
Steel deck AF 0.32 m	84
Steel deck	85
Lightweight aluminium deck 0.60 m	90

Modular scaffolding system "ALFIX MODUL MULTI"

Standard assembly configuration Load Class 3 / SW06 / I ≤ 3.07 m - General provisions

Annex C, page 3

Designation	Annex B, page
MODUL gap cover $\ell \le 3.07$ m	94
MODUL gap cover with tube fixture $\ell \le 3.07 \text{ m}$	95
Gap cover ℓ ≤ 3.07 m	96
Bracket 0.39 m with tube fixture	103
MODUL bracket 0.39 m	104
MODUL toeboard	107
MODUL toeboard, aluminium	109
Toeboard; end toeboard AF	110
Toeboard; end toeboard	112
Aluminium toeboard; aluminium end toeboard AF	114
Aluminium toeboard; aluminium end toeboard	115
MODUL guard net system	116
MODUL double end guardrail 0.73 m	117
Storey ladder 2.00 x 0.40 m, steel	118
Storey ladder 2.00 x 0.40 m, aluminium	119
Scaffold retainer / wall tie	120
Wedge-head swivel coupler	122
MODUL U-tube connector	123
MODUL tube connector	124
Wedge-head coupler, rigid	125
Standard 4.0	154
Vertical starter piece 4.0	155
Tube ledger 4.0 ℓ ≤ 3.07 m	156
MODUL gap cover, T-shaped and universal design claw coupler	158

Table C.2: Characteristic anchor forces

		Bay	partially open facade		closed facade			
Variant /	Annex D, page	length	GH	DI	RH	GH	DI	RH
configuration		[m]	<i>A⊥</i> [kN]	<i>A_{//}</i> [kN]	<i>A⊥</i> [kN]	<i>A⊥</i> [kN]	<i>A∥</i> [kN]	<i>A⊥</i> [kN]
	ket 1, 3	3.07	3.6	2.4	2.4	1.2	2.4	2.4
without inner bracket		2.57	3.0	2.4	2.4	1.0	2.4	2.4
without inner bracket	2.4	3.07	3.6	3.0	3.0	1.2	3.0	3.0
	2, 4	2.57	3.0	3.0	3.0	1.0	3.0	3.0
(-) Tension								

(-) (+) Compression

GH wall tie (single tube attachment)

DRH V-type wall tie

Modular scaffolding system "ALFIX MODUL MULTI"

Annex C, page 4

Standard assembly configuration Load Class 3 / SW06 / I ≤ 3.07 m - General provisions

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			Structural height				
Standard force for	Fittings	Bay length [m]	24 m [kN]	16 m [kN]	8 m [kN]		
	without inner brackets	3.07	7.9	6.3	4.8		
Inner standard	without inner brackets	2.57	6.6	5.3	4.0		
basic scaffolding F_{IS}	with inner brackets	3.07	17.2	13.9	10.6		
1 15		2.57	14.4	11.6	8.9		
	with / without inner brackets	3.07	11.5	8.7	6.0		
	with / without inner brackets	2.57	9.6	7.3	5.0		
Outer standard	additional loads						
basic scaffolding	aretesti ve vvell	3.07		+ 0.5			
FAS	protective wall	2.57	+ 0.4				
	add-on access bay	3.07	4.2	2.9	1.6		
		2.57	3.5	2.4	1.3		
Outer standard		3.07	10.6	9.3	8.1		
access bay $F_{AS,T}$	without	2.57	8.9	7.8	6.8		
	bridging	. 11	Inner standard: $1.5 \cdot F_{IS}$				
Special configuration	construction F_{U}	all	Outer	standard: 1	$1.5 \cdot F_{AS}$		

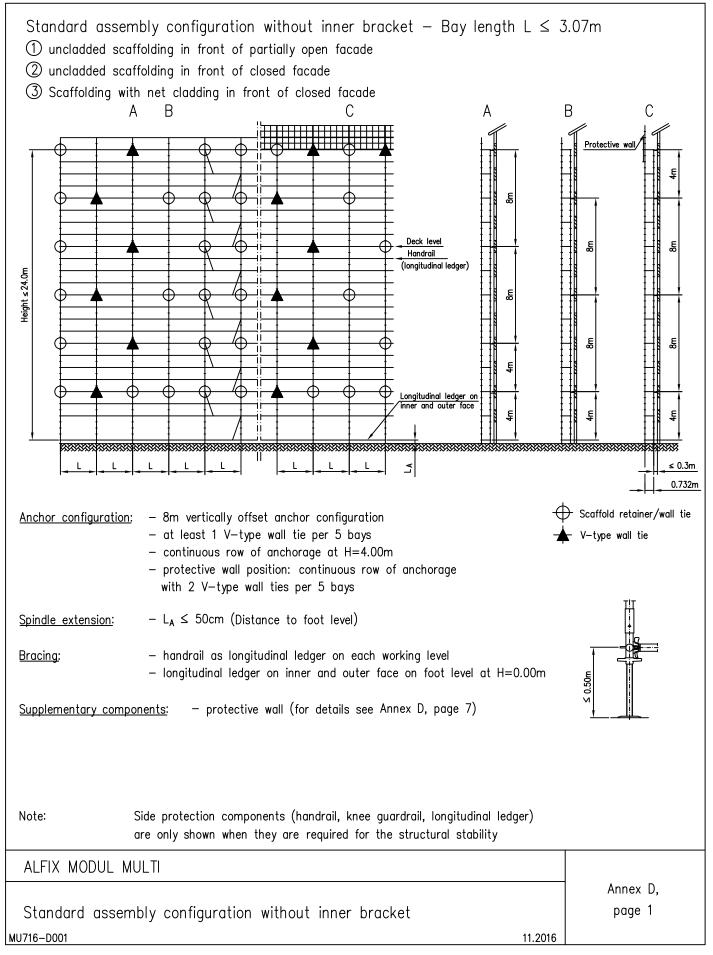
Table C.4: Assembly variants of the standard assembly configuration

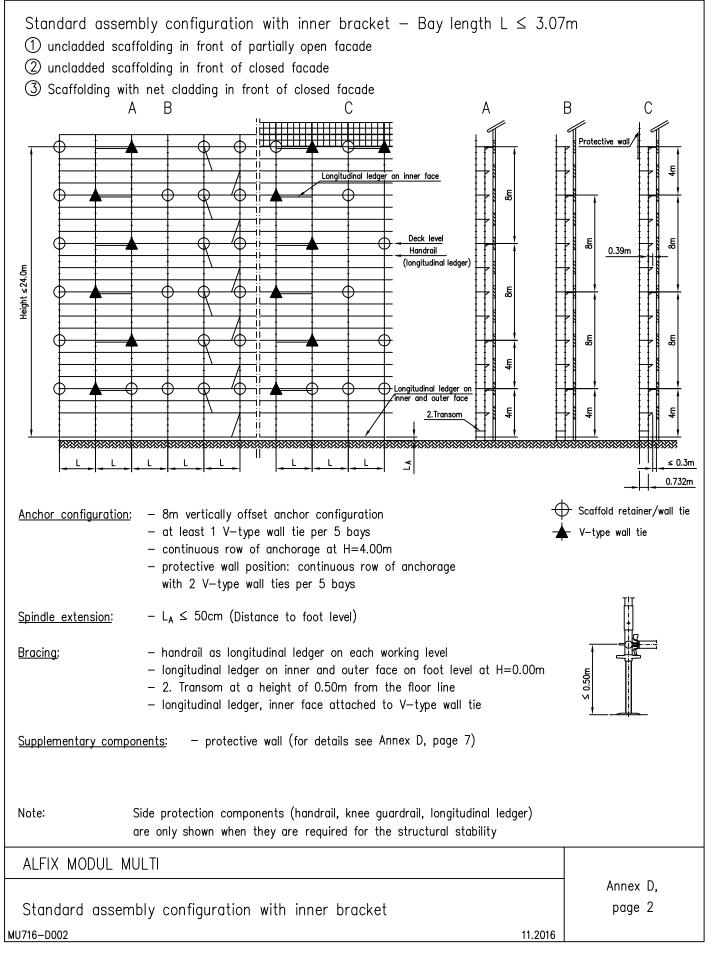
Cladding	Fittings	without inner brackets	with Inner brackets
	no supplementary components	Annex D,	Annex D,
uncladded / partially open facade uncladded / closed facade	protective wall	page 1	page 2
net cladding / closed facade	Bridging girder	Annex D,	Annex D,
		page 3	page 4

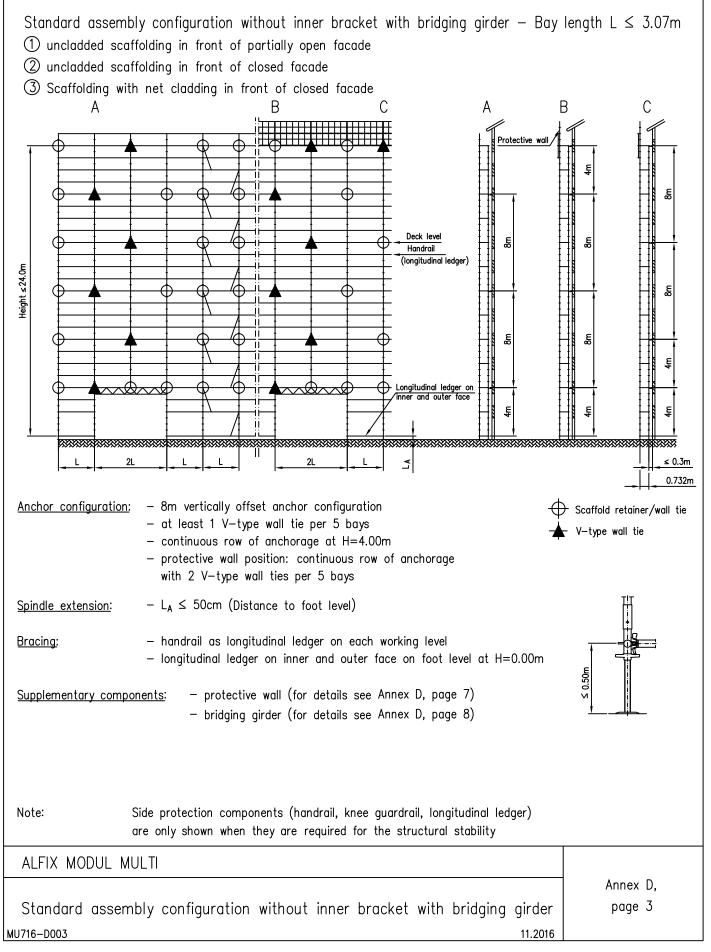
Modular scaffolding system "ALFIX MODUL MULTI"

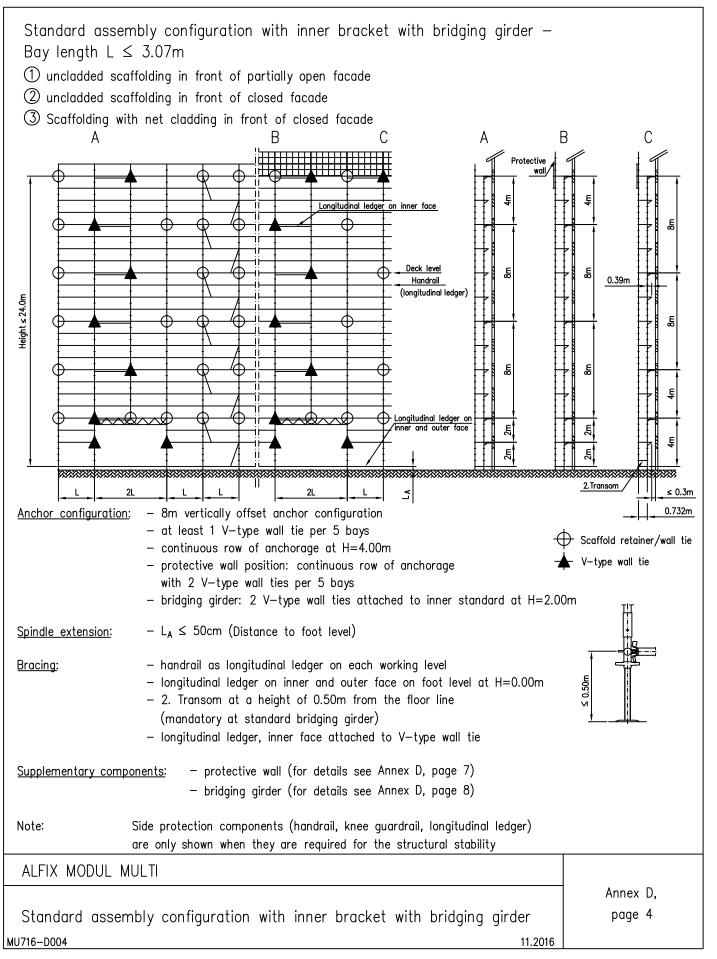
Standard assembly configuration Load Class 3 / SW06 / I ≤ 3.07 m - General provisions

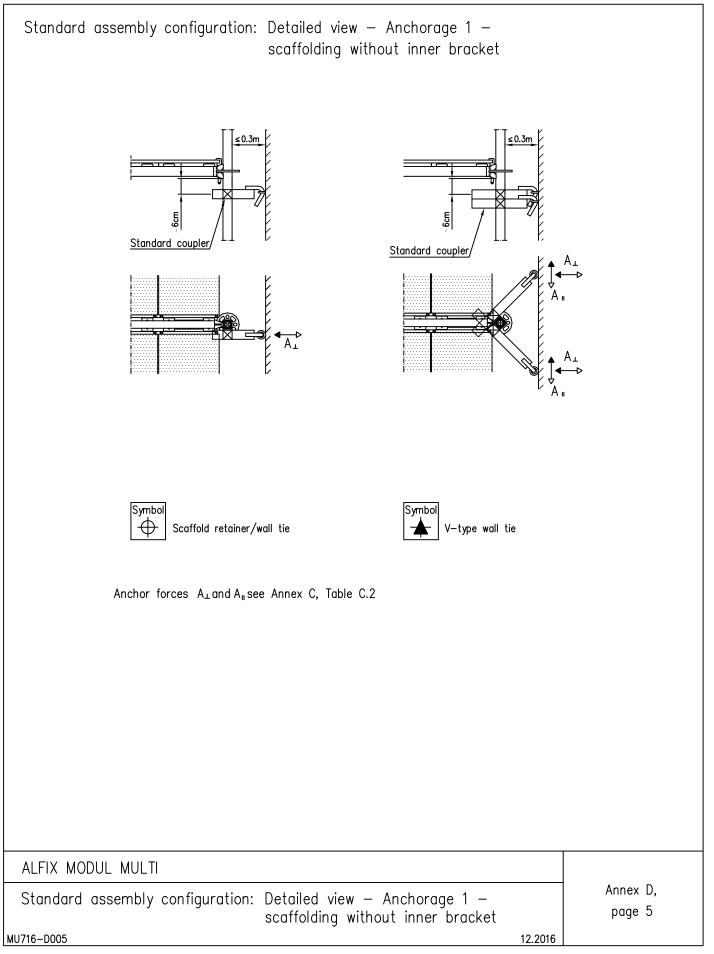
Annex C, page 5

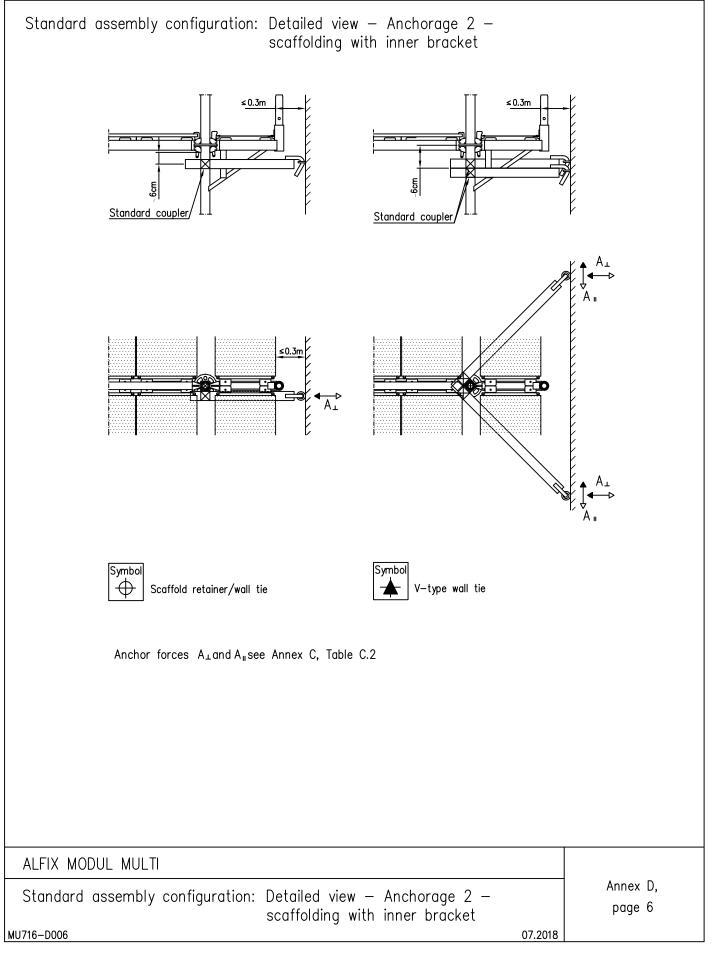


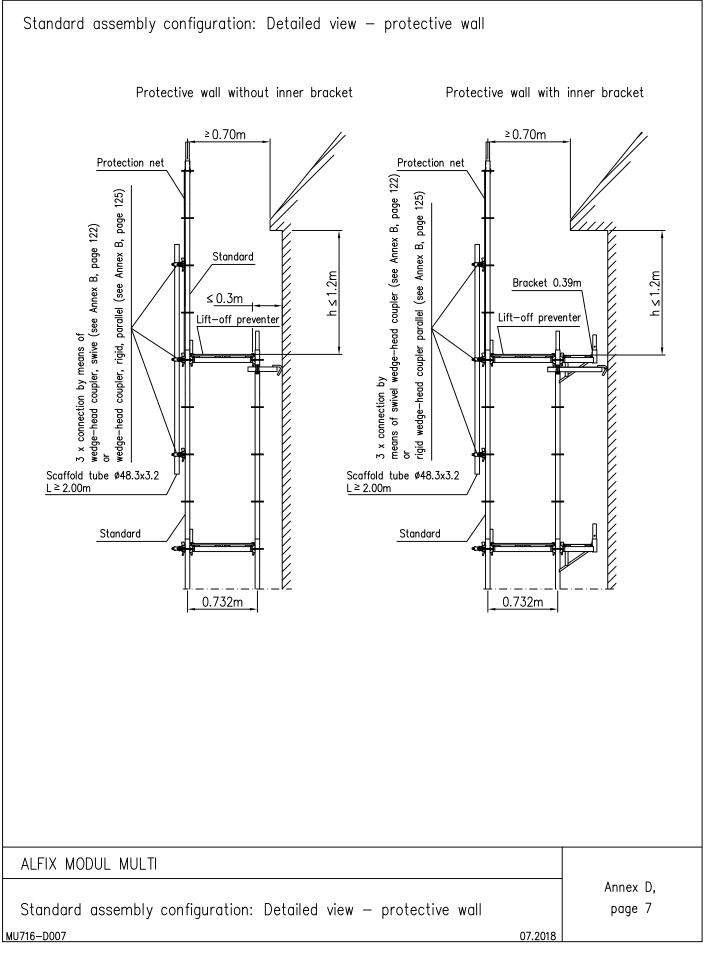


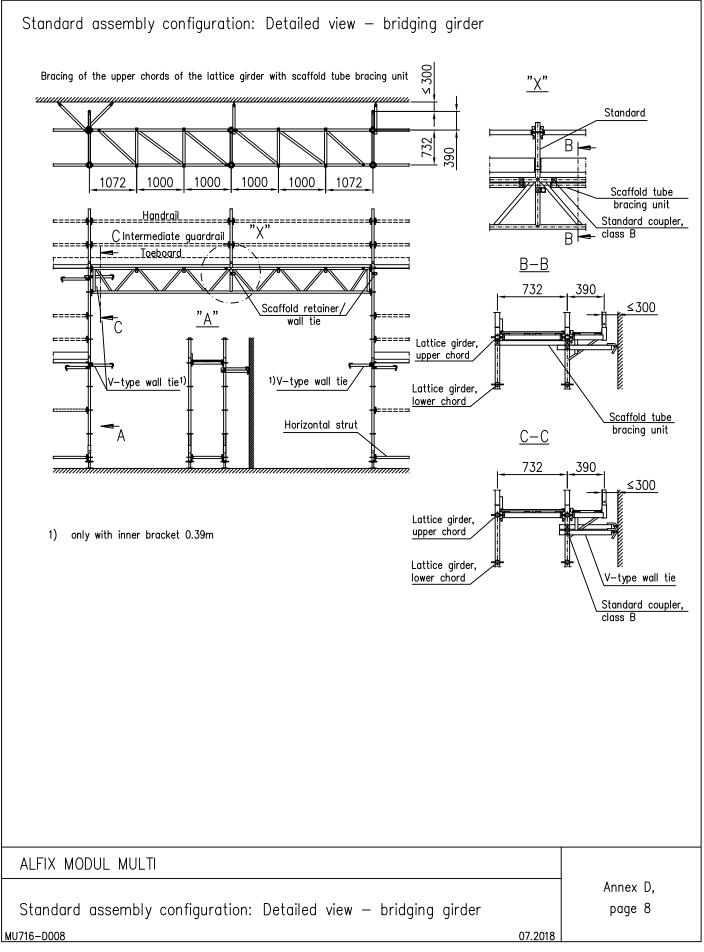












E.1 General Instructions

In its standard assembly configuration, the scaffolding system may be used as working scaffold of load classes ≤ 4 with system width b = 1.09 m and bay widths $\ell \leq 2.57 \text{ m}$ in accordance with DIN EN 12811-1:2004-03, and as protection scaffold and roof edge protection scaffold in accordance with the regulations stipulated in Section E.2.

The topmost horizontal level (working level) must not exceed 24 m above ground level, not including the spindle extension length. The standard assembly configuration of the scaffolding system is designed for use on a scaffold level in accordance with the regulations of the DIN EN 12811-1:2004-03 standard, Section 6.2.9.2 in front of a "partially open" facade with an open proportion of no more than 60%, and in front of closed facades. When determining the wind load, a statistical factor of $\chi = 0, 7$, assuming a maximum service life of 2 years, has been taken into account.

Structural analysis for scaffold cladding with tarpaulins has not been provided in the standard assembly configuration. Structural analysis for scaffold cladding with nets has only been provided in the standard assembly configuration for cladding in front of closed facades. Structural analysis for netcovered scaffoldings applies to scaffolds with an aerodynamic force coefficient of the entire structure (net and scaffolding) that is no greater than $c_{f,L,total} = 0.6$.

Without any further structural proof, the standard assembly configuration shall only be used if the loads of the bays will carry do not exceed the respective live loads in accordance with

DIN EN 12811-1:2004-03, Table 3.

For the standard assembly configuration of the modular scaffolding system" ALFIX MODUL MULTI", the following designation in accordance with DIN EN 12810-1:2004-03 shall be used:

Scaffolding EN 12810 - 4D- SW09/257- H2- A- LA

The assembly variants of the standard assembly configuration are listed in Table E.4.

E.2 Proetction scaffold and roof edge protection scaffold

In its standard assembly configuration the scaffolding system may be used as a protection scaffold and roof edge protection scaffold with a top fall arresting layer of class FL 1 and as a roof edge protection scaffold with protective walls of class SWD 1 according to DIN 4420:2004-03. Access decks must not be fitted into brackets.

The protective wall is to be installed in accordance with Annex F, page 7.

Use protective nets in accordance with DIN EN 1263-1:2015-03 with a mesh size of no more than 100 mm.

E.3 Components

The components intended for use are listed in Table E.1. In addition to these components, other components may also be used: steel tubes \emptyset 48.3 \cdot 3.2 mm and couplers for the protective wall and the horizontal bracing of the bridging girders as well as standard couplers for the connection of the wall ties and V-type wall ties to the standards in accordance with DIN EN 12811-1:2004-03.

E.4 Bracing

Horizontal scaffolding levels are to be braced by continuously installing the following members at vertical intervals of 2 m tube ledger 1.09 m with

vertical intervals of 2 m tube ledger 1.00 m w		
three steel decks with tube fixture	in accordance with Annex B, page 64	or
three steel decks AF with tube fixture	in accordance with Annex B, page 61	or
U-ledger 1.09 m and		
three steel decks	in accordance with Annex B, page 85	or
three steel decks AF each.	in accordance with Annex B, page 84	

Modular scaffolding system "ALFIX MODUL MULTI"

Annex E, page 1

Standard assembly configuration Load Class 4 / SW09 / $I \le 2.57$ m - General provisions

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The steel decks are to be secured against unintentional lift-off using deck lift-off preventers.

The outer vertical planes are to be braced depending on the assembly configuration with tube ledgers used as handrails (1 m above deck surface) and as intermediate guardrail (0.5 m above deck surface) continuously in each scaffold bay as well as vertical diagonal braces.

Vertical starter pieces are to be installed directly above the base jacks (scaffolding spindles) and connected by means of longitudinal ledgers in the inner and outer plane parallel to the facade and by means of transoms at right angles to the facade. Additionally, depending on the assembly configuration diagonal cross braces are to be installed up to the first working level.

E.5 Anchoring

Anchoring is to be provided using wall ties in accordance with Annex B, page 120.

Wall ties are to be installed as anchor pairs at an angle of 90° (V-type wall tie) or as "short" wall ties only to the inner vertical frame standard using standard couplers. Depending on the assembly configuration, the node points that are anchored by means of V-type anchors are to be connected on the inner plane parallel to the facade with the adjacent standard section by means of tube ledgers (longitudinal ledgers).

The V-type anchors and wall ties are to be installed in the immediate vicinity of the node points of the standard tubes and transoms.

The anchor forces listed in Table C.2 were determined with the characteristic values of the actions

($\gamma_F = 1.0$). For the design analysis of the anchorage and the load transfer, the values given must be multiplied by the respective partial safety factor γ_F (generally. $\gamma_F = 1.5$).

Each frame section is anchored at vertical intervals of 4 m. On the topmost working level, each standard section must be anchored.

E.6 Foundation loads

Depending on the assembly variation, the foundation loads listed in Table E.3 must be absorbed and transferred in the supporting surface. The foundation loads are given as characteristic values. For the structural analysis of transfer of loads in the supporting surface, the values given must be multiplied by the partial safety factor γ_F (generally $\gamma_F = 1.5$).

E.7 Bridging construction

The bridging girders may be used at a height of 4 m to bridge gate entrances or similar openings when the working levels underneath the bridging part are omitted.

The bridging girders are to be anchored in the supporting and centre section and are to be braced additionally by means of a horizontal bracing unit comprised of tubes and couplers or by means of additional anchorage (see also Annex F, pages 3 and 8).

E.8 Add-on access bay

When installing the add-on access bay in load class 3, aluminium access frame platforms (with tube fixture) must be installed when using tube ledgers, or aluminium frame platforms with internal hatch or aluminium access frame platforms with ladder must be installed when using U-ledgers. The add-on access bay is to be braced at vertical intervals of 4 m by means of horizontal diagonal braces. The outer vertical planes are to be braced with tube ledgers used as handrails (1 m above deck surface) and as intermediate guardrail (0.5 m above deck surface) continuously in each scaffold bay. Additionally, a longitudinal ledger is to be installed at the outer face of the access bay directly above the scaffolding spindle. (see Annex F, page 4).

E.9 Widening bracket

Only use brackets 0.39 m on the inner face of the scaffolding on all working levels.

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Designation	Annex page
Vertical diagonals braces 0.73 m – 2.57 m x 2.00 m	8
Horizontal diagonal braces 0.73 m – 2.57 m x 1.09 m	9
Vertical starter piece	10
Standard with tube connector 200	11
Base jack	17
AB Base jack	18
Tube ledger $\ell \le 2.57$ m	25
Tube ledger reinforced 1.09 m	27
U-ledger 0.73 m *)	32
U-transom reinforced 1.09 m	33
U-transom lattice girder 1.09 m V	44
Tube-transom lattice girder 1.09 m V	45
MODUL lattice girder 4.14 m / 5.14 m	47
MODUL lattice girder with tube connector 4.14 m / 5.14 m	49
MODUL lift-off preventer 0.73 m, 1.09 m	50
Aluminium access frame platform with tube fixture 2.57 m *)	54
Aluminium access frame platform with tube fixture 1.57 m – 2.57 m without ladder $^{*)}$	57
Aluminium access frame platform with tube fixture 2.57 m $-$ 2.57 m with aluminium chequer plate *)	58
Steel deck AF with tube fixture 0.32 m $\ell \le 2.57$ m	61
Steel deck with tube fixture $\ell \le 2.57$ m	64
Aluminium frame platform with internal hatch 2.57 m *)	69
Aluminium access deck with ladder 2.57 m *)	76
Aluminium access deck with ladder 2.57 m *)	82
Steel deck AF 0.32 m ℓ ≤ 2.57 m	84
Steel deck ℓ ≤ 2.57 m	85
MODUL gap cover 0.73 m, 1.09 m	94
MODUL gap cover with tube fixture	95
Gap cover ℓ ≤ 2.57 m	96
MODUL swing gate	102
Bracket 0.39 m with tube fixture	103
MODUL bracket 0.39 m	104
MODUL toeboard $\ell \le 2.57$ m	107
MODUL aluminium toeboard $\ell \le 2.57$ m	109
Toeboard $\ell \leq 2.57$ m, end toeboard AF	110
Toeboard $\ell \leq 2.57$ m, end toeboard	112
Aluminium toeboard $\ell \le 2.57$ m; Aluminium end toeboard AF	114
Aluminium toeboard $\ell \le 2.57$ m; Aluminium end toeboard	115
MODUL guard net system ℓ ≤ 2.57 m	116
MODUL double end guardrail	117
Storey ladder 2.00 x 0.40 m, steel	118

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Designation	Annex B, page	
Storey ladder 2.00 x 0.40 m, aluminium	119	
Scaffold retainer / wall tie	120	
Wedge head swivel coupler	122	
MODUL U-tube connector	123	
MODUL tube connector	124	
Wedge-head coupler, rigid	125	
Diagonal cross brace	134	
Tube ledger reinforced 1.09 m	144	
U-ledger 1.09 m	147	
Standard 4.0	154	
Vertical starter piece 4.0	155	
Tube ledger 4.0 $\ell \le 2.57$ m	156	
MODUL gap cover, T-shaped and universal claw coupler $\ell \le 2.57$ m	158	

Table E.2:characteristic anchor forces

Variant /		Bay length	partially open facade			closed facade		
			GH	DI	RH	GH	D	RH
configuration		[m]	A_{\perp}	A _{//}	A_{\perp}	A_{\perp}	A _{//}	A_{\perp}
			[kN]	[kN]	[kN]	[kN]	[kN]	[kN]
without inner bracket	1	2.57	1.6	2.9	2.9	0.5	2.9	2.9
		2.07	1.3	2.9	2.9	0.4	2.9	2.9
with inner bracket	2, 3	2.57	1.6	3.5	3.5	0.5	3.5	3.5
		2.07	1.3	3.5	3.5	0.4	3.5	3.5

(-) Tension

(+) Compression

GH wall tie (single tube attachment)

DRH V-type wall tie

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			Structural height				
Standard force	Fittings	Bay length [m]	24 m [kN]	16 m [kN]	8 m [kN]		
Inner standard basic scaffolding F_{LS}	without inner brackets	2.57	11.4	9.5	7.6		
		2.07	9.2	7.7	6.1		
	with inner brackets	2.57	21.3	17.9	14.5		
		2.07	17.2	14.4	11.7		
Outer standard basic scaffolding F_{AS}	with / without	2.57	14.8	11.9	9.0		
	inner brackets	2.07	11.9	9.6	7.2		
	additional loads						
	protective well	2.57	+ 0.5				
	protective wall	2.07	+ 0.4				
	add-on access bay	2.57	3.5	2.4	1.3		
Outer standard access bay <i>F</i> _{AS,T}	without	2.57	8.9	7.8	6.8		
On a sigl as a firm mation	Bridging	- 11	Inner standard: $1.5 \cdot F_{IS}$				
Special configuration	construction Fü	all	Outer standard: $1.5 \cdot F_{AS}$				

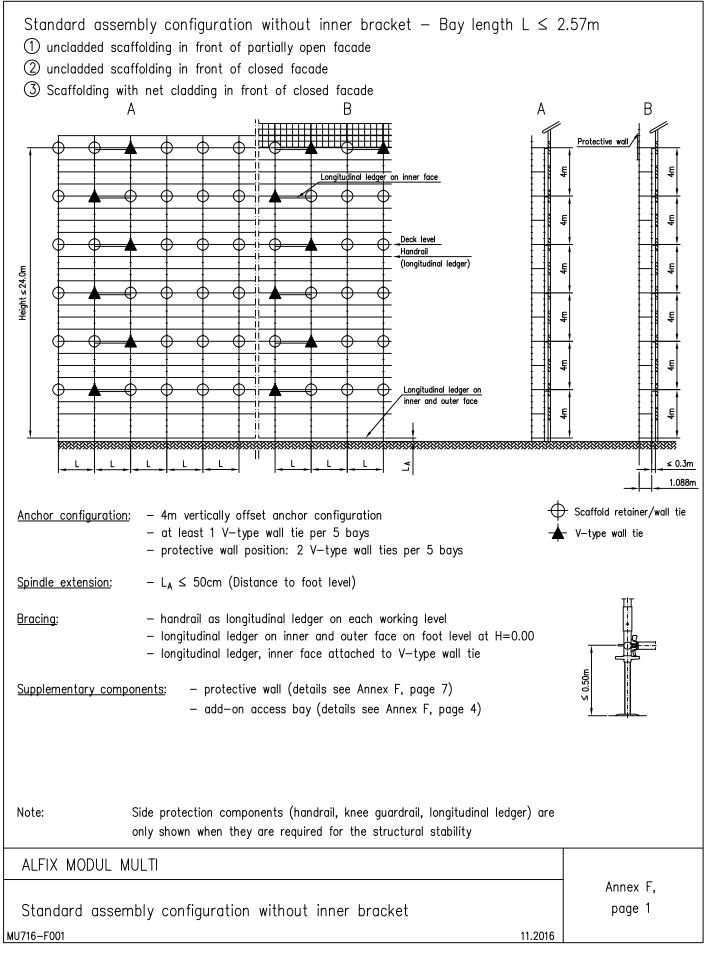
Table E.4:Assembly variants of the standard assembly variants

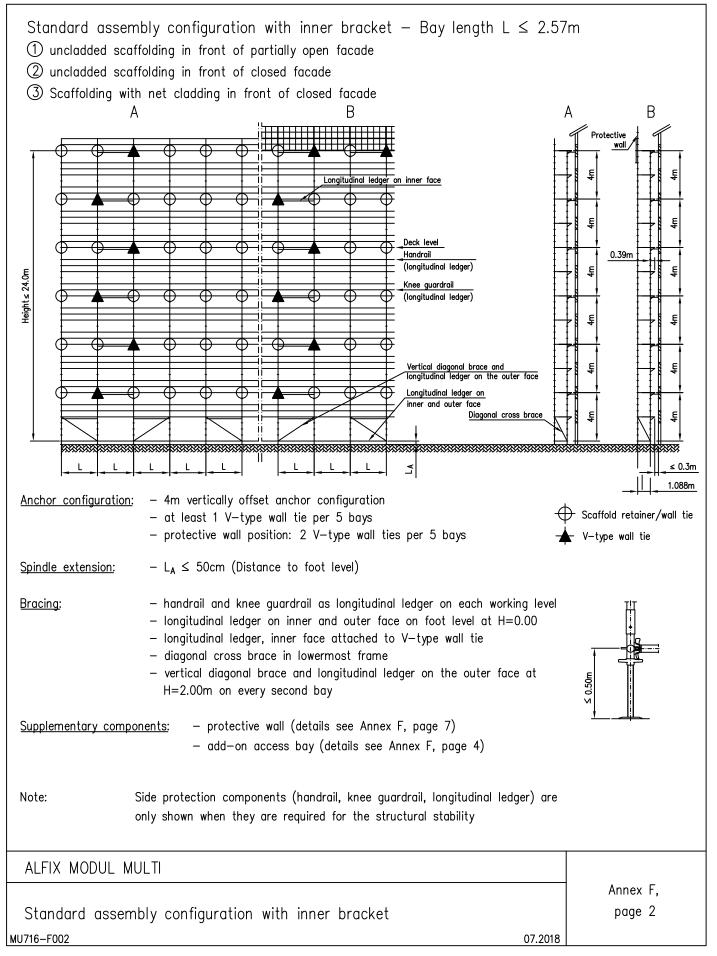
Cladding	Fittings	without inner brackets	with inner brackets
uncladded / partially open facade uncladded / closed facade net cladding / closed facade	no supplementary components protective wall	Annex F, page 1	Annex F, page 2
	Bridging girder		Annex F, page 3
	Add-on access bay	Annex F	, page 4

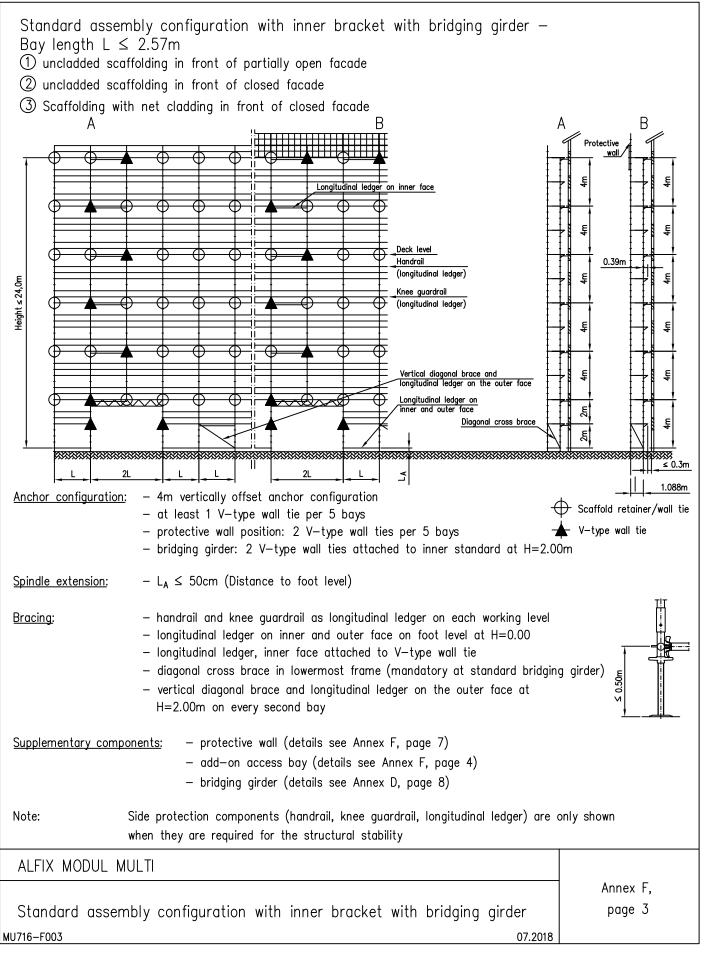
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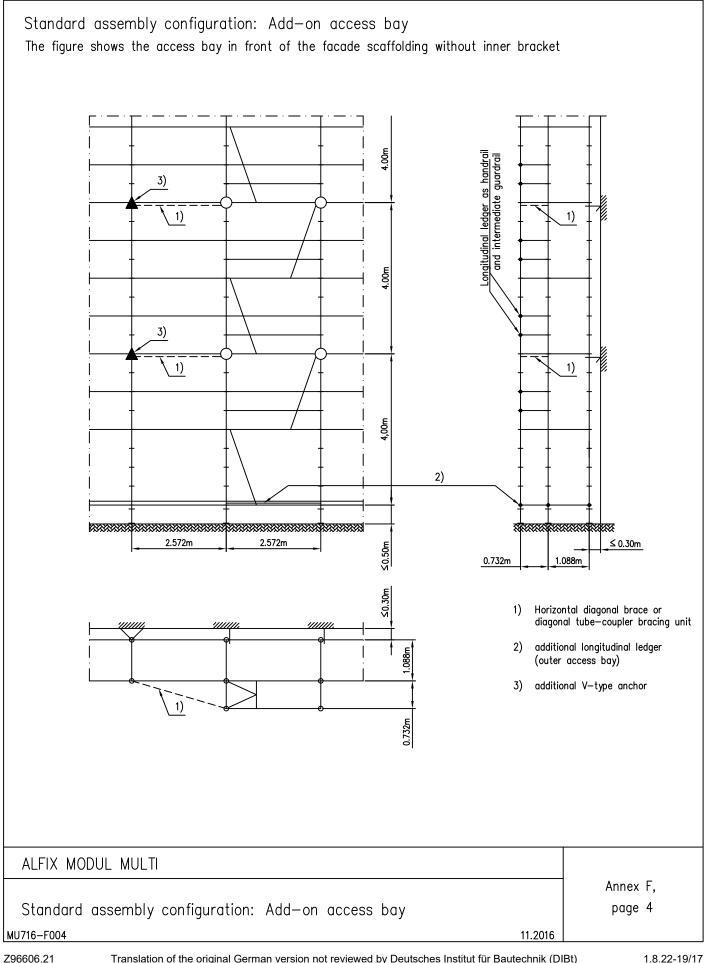
Standard assembly configuration Load Class 4 / SW09 / I ≤ 2.57 m - General provisions

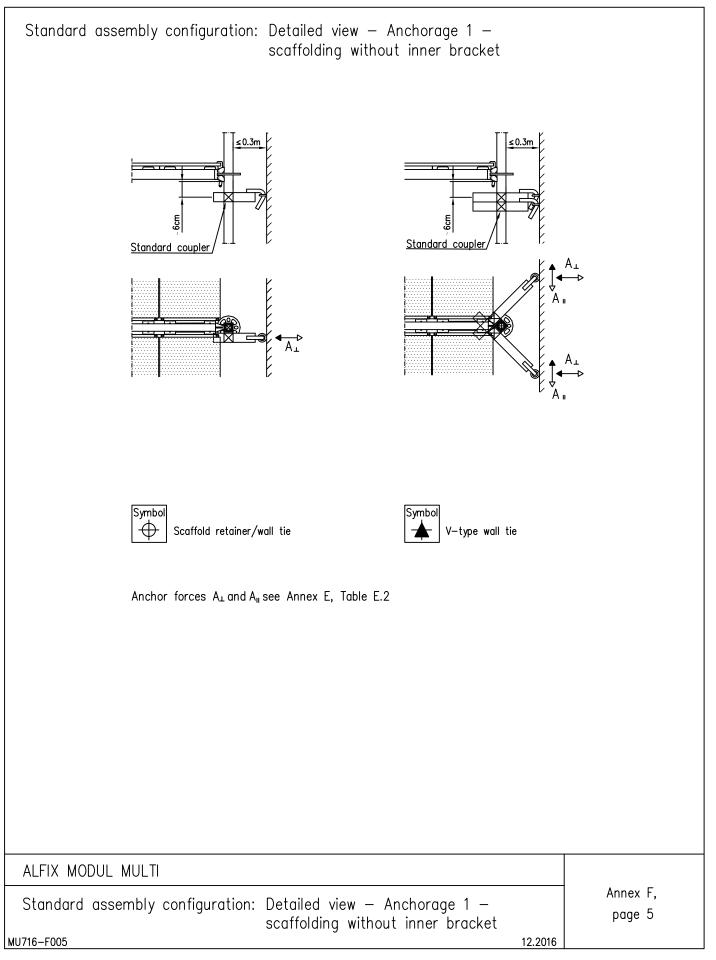
Annex E, page 5

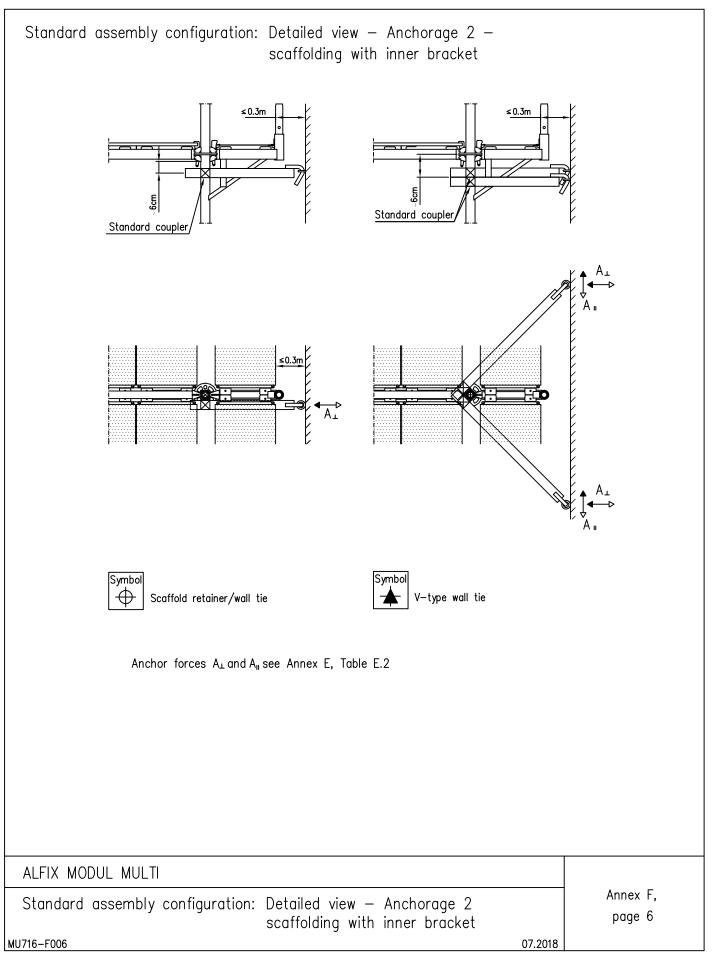


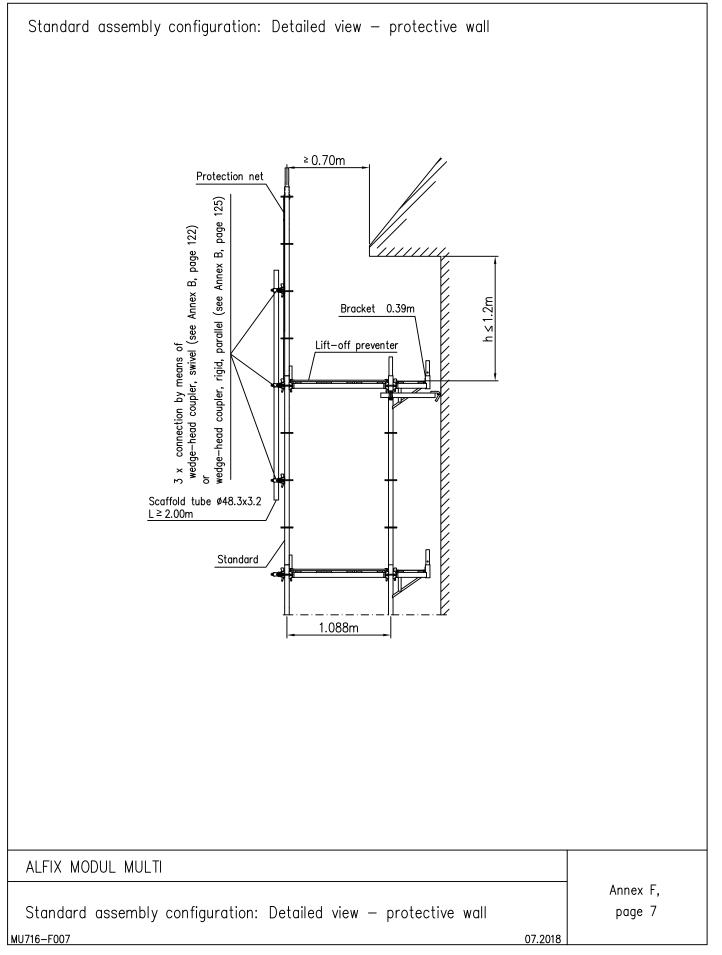


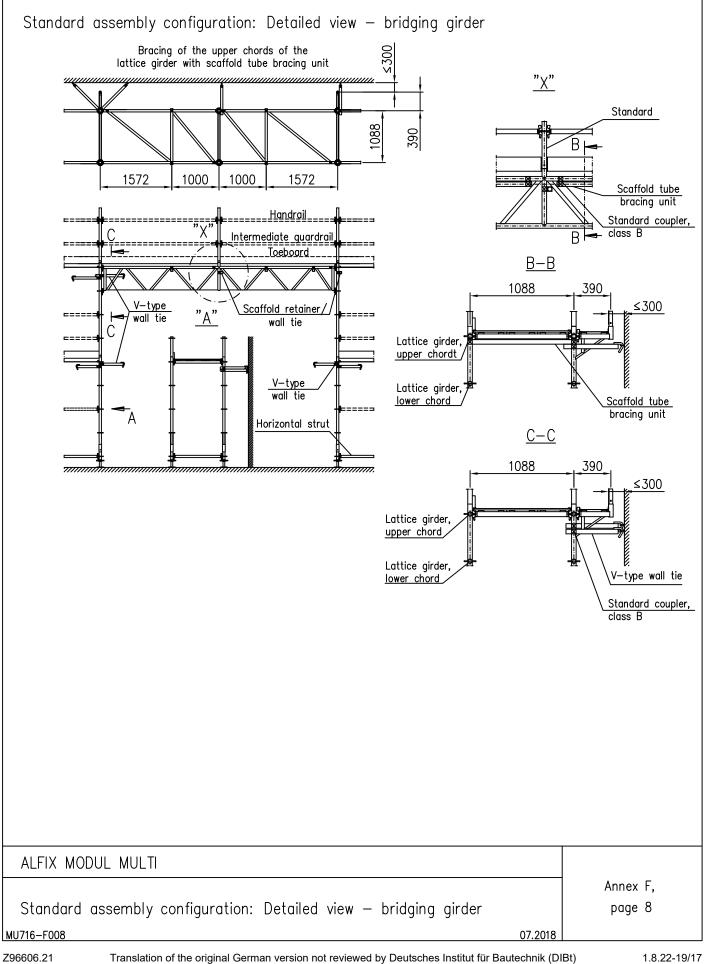












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