



NATIONAL TECHNICAL APPROVAL

ALFIX 70

[Seal: Deutsches Institut für Bautechnik]

Notification

of amendents and supplement and the extension of the period of validity of the national technical approval / general construction technique permit of 16 December 2020 An institution under public law jointly funded by the German Federation and the federal states (Länder)

Approval Body for Construction Products & Techniques

 Date:
 Reference number:

 03 January 2022
 | 37.1-1.8.1-71/21

Approval number: **Z-8.1-862**

Period of validity: from: **5 January 2022** to: **5 January 2027**

Applicant: Alfix GmbH Langhennersdorfer Straße 15 09603 Großschirma (Germany)

Subject of approval: "Frame Scaffold ALFIX 70" scaffolding system

This notification amends, supplements and extends the period of vailidity of the national technical approval / general construction technique permit no. Z-8.1-862 of 16 December 2020. This national technical approval includes eight pages and 24 annexes. It shall only be valid in connection with above mentioned national technical approval / general construction technique permit and shall only be used in conjuction with it.

[Seal: Deutsches Institut für Bautechnik]

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I GENERAL PROVISIONS

The general provisions of the national technical approval and the general construction technique permit Z-8.1-862 are replaced by the following version:

- 1 This decision confirms the usability and / or applicability of the subject matter of the permit as defined by the Building Codes of the federal states.
- 2 This decision does not replace statutory approvals, authorisations and certifications specified for carrying out construction works.
- 3 This decision is granted without prejudice to the rights of third parties, especially private property rights.
- 4 The user of the subject matter of the decision must, without prejudice to further regulations laid out under "Special Provisions", make copies of this decision available to the persons using or applying the subject matter of the decision. The user of the subject matter shall also be informed that the decision must be present at the place of use. Upon request, copies must be provided to the relevant authorities.
- 5 This decision may only be reproduced in its entirety. Publication of the approval in excerpts requires the prior consent of the Deutsches Institut für Bautechnik (DIBt). Text and drawings of promotional material must be consistent with this decision. Translations must include the following note: "Translation of the original German version not reviewed by Deutsches Institut für Bautechnik".
- 6 This decision is issued as a revocable decision. The provisions of this decision may be amended or modified at a later time, particularly if new technical knowledge requires this.
- 7 This decision relates to the information on the subject matter of the decision made available by the applicant during the approval process and the documents submitted. Any change made to these decision bases is not covered by this decision and must be disclosed to the Deutsches Institut für Bautechnik without delay.

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II SPECIAL PROVISIONS

The special provisions of the national technical approval are changed and supplemented as follows:

a) Table 1 is changed as follows:

Table 1: Scaffold components of the ALFIX 70 frame scaffold

Designation	Annex A, page	Detailed view / components in accordance with annex A, page
Vertical frame 18/70 1.5m and 2.0m, steel	1a	3a
Protective net post AF 2.00 x 0.36 / 0.50 / 0.73 / 1.09m	128a	3a
Vertical frame AF 1.50m and 2.00m, steel	129a	3a
Aluminium frame platform AB with plywood 0.50m – 3.07m	146a	147

b) The following components are no longer listed in Table 1:

Table 1: Scaffold components of the ALFIX 70 frame scaffold

Designation	Annex A, page	Detailed view / components in accordance with annex A, page
Aluminium trapdoor deck with aluminium chequer plate 2.57m	117	18, 119
Aluminium trapdoor deck with aluminium chequer plate 3.07m	118	18, 119
Aluminium trapdoor deck with aluminium chequer plate- 1.57m; 2.07m without ladder	120	119

c) Table 1 is supplemented as follows:

Table 1: Scaffold components of the ALFIX 70 frame scaffold

Designation	Annex A, page	Detailed view / components in accordance with annex A, page
Aluminium frame platform AB with hatch 2.57m; 3.07m	148	18, 147, 149
Aluminium frame platform AB with hatch 1.57m – 3.07m without ladder	150	147, 149
Aluminium frame platform AB with hatch, aluminium chequer plate 2.57m; 3.07m	151	18, 152
Aluminium frame platform AB with hatch, aluminium chequer plate 1.57m – 3.07m without ladder	153	152

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d) Section 2.2.2 is replaced by the following version:

2.2.2 Marking

The delivery notes for scaffolding components according to table 1 shall be marked in accordance with the regulations for the mark of conformity of the federal states (Länder).

In addition, scaffolding components shall be permanently and easily recognisably marked with

- the uppercase letter "Ü",
- at least the abbreviated approval number "862",
- the identifying mark (logo) of the manufacturer, and
- the last two digits of the year of manufacture.

For more detailed information on marking, please refer to annex A, page 154. These identifying marks may only be applied if the requirements under Section 2.3 are fulfilled.

e) Table 3 is changed as follows:

Table 3: Further scaffolding components for use in ALFIX 70 frame scaffold

Designation	Annex A, page	Detailed view / components in accordance with annex A, page	Regulations for manufacturing, marking and certificate of conformity
Aluminium frame platform with plywood 0.50m – 2.07m	110a	112a	
Aluminium frame platform with plywood 2.57m; 3.07m	111a	112a	according to Z-8.22-906
Aluminium frame platform with internal hatch 2.57m; 3.07m	113a	18, 112a, 115a	2-0.22-900
Aluminium frame platform with internal hatch 1.09m – 3.07m without ladder	114a	112a, 115a	
Aluminium trapdoor deck with aluminium chequer plate 2.57m	117a	18, 119a	according to
Aluminium trapdoor deck with aluminium chequer plate 3.07m	118a	18, 119a	Z-8.1-862 (No longer
Aluminium trapdoor deck with aluminium chequer plate 1.57m; 2.07m without ladder	120a	119a	manufactured.)

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f) The page numbers in table 12 are changed and supplemented as follows:

Table 12: Assignment of decking to scaffold load classes (service classes)

Designation	Annex A, page	Bay length ∦ [m]	Use in load class (service class)
Aluminium frame platform with plywood	110a, 111a, 146a	≤ 3.07	≤ 3
Aluminium frame platform with internal hatch	113a, 114a	≤ 3.07	≤ 3
Aluminium trapdoor deck with aluminium chequer plate	117a, 118a, 120a,	≤ 3.07	≤ 3
Aluminium frame platform AB with hatch	148, 150, 151, 153	≤ 3.07	≤ 3

g) Tables 13.1, 13.2 and 13.3 are replaced by table 13:

<u>Table 13:</u>	Design values of the horizontal travel springs

			per			ess <i>c⊥,d</i> l/cm]		۲
Deck	Annex A, page	Bay length { [m]	Number of decks period scaffold bay	Clearance $f_{oar{L}}$ [cm]	$0 < N_\perp \leq N_{1,2}$	$N_{1,2} < N_{\perp} \leq N_{\perp Rd}$	<i>N_{1,2}</i> [kN]	Spring force <i>N</i> _{4,Rd} [kN]
Steel deck AF	7	3.07	2	4.7	0.62	0.20		
Steel deels	0	3.07	2	4.7	0.62	0.20		2.73
Steel deck	8	≤ 2.57	2	3.8	0.69	0.27	1 00	
Aluminium deck with	12, 13,	3.07	- 1	2.0	0.38	0.26	1.82	1.86
plywood	19, 20	≤ 2.57		2.2	0.65	0.34		1.00
Wooden deck	28	≤ 2.57	2	3.3	0.51	0.31		2.35
Solid wood deck 48	26	≤ 3.07	2	3.9	0.41	0.22	2.00	2.35
Solid wood deck 45	27	≤ 2.57	2	5.9	0.41	0.22	2.00	2.55
Aluminium frame platform	110a,	3.07	1	2.0	0.38	0.26	1.82	1.86
with plywood	111a	≤ 2.57		2.2	0.65	0.34	1.02	1.00
Aluminium deck 0.60m, lightweight	116	≤ 3.07	1	4.7	0.69	0.20	1.50	2.08
Aluminium frame platform AB with plywood	146a	≤ 3.07	1	3.0	0.26			3.00
Access decks	all	≤ 3.07	1	4.4	0.26			1.89

Table 14:

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					Stiffnes	S <i>C ,d</i> [«N/cm]			z
Deck	Annex A, page	Bay length { [m]	Number of decks per scaffold bay	Clearance <i>f_{oll}</i> [cm]	$0 < F_{ } \leq N_{1,2}$	$N_{1,2} < F_{ } \leq N_{2,3}$	$N_{2,3} < F_{ } \leq F_{ l,Rd}$	N1,2 [kN]	<i>N</i> 2,3 [KN]	Spring force $F_{ll,Rd}$ [kN]
Steel deck AF	7	≤ 3.07	2	1.0	2.22	2.37	1.25			4.55
Steel deck	8	≤ 3.07	2	1.0	2.22	2.37	1.25			4.55
Aluminium deck with plywood	12, 13, 19, 20	≤ 3.07	1	0.3	2.20	2.22	0.94	1.14	2.27	3.94
Wooden deck	28	3.07*)	2	1.0	1.99	1.95	1.22]		4.55
	20	≤ 2.57	2	1.0	1.67	1.63	1.02			3.83
Solid wood deck 48	26	≤ 3.07	2	0.0	2.31	1.38		2.0		4.58
Solid wood deck 45	27	≤ 2.57		0.9	1.93	1.16		3.0		3.83
Aluminium frame platform with plywood	110a, 111a	≤ 3.07	1	0.3	2.20	2.22	0.94	1.14	2.27	3.94
Aluminium deck 0.60m, lightweight	116	≤ 3.07	1	0.4	3.41					3.82
Aluminium frame platform AB with plywood	146a	≤ 3.07	1	0.1	1.27					5.75
Access decks	all	≤ 3.07	1	0.2	1.50					4.08
*) Only in case of anche	orage patt	ern ≤ 4 m	(Figure '	I, type b	in DIN EN	V 12810-	1:2004-03	3)		

Design values of the horizontal coupler springs per scaffold bay

h) Tables 14.1, 14.2 and 14.3 are replaced by table 14:

i) Table 15.1 is replaced by the following version:

Table 15.1: Design values of the horizontal travel springs

Deck	Bay length	Clearance	Stiffne	ss [kN/cm]	<i>N</i> ⊥1,2	$N_{\perp,Rd}$
Deck	ℓ [m]	<i>f₀⊥</i> [cm]	C ⊥1,d	C ⊥2,d	[kN]	[kN]
all decks <u>without</u> hatch	≤ 3.07	4.7	0.62	0.26	0.75	1.86
all decks <u>including</u> hatch	≤ 3.07	4.7	0.27			1.86

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j) Table 15.2 is replaced by the following version:

Table 15.2:Design values of the horizontal coupler springs per scaffold bay

Deck	Bay length	Clearance	Stif	fness [k	N/cm]	N _{111,2}	N _{112,3}	N _{II,Rd}
Deck	ℓ [m]	f _{oll} [cm]	CII1,d	C 112,d	С II3,d	[kN]	[kN]	[kN]
all decks including hatch	≤ 3.07	1.0	1.67	1.63	1.00	1.14	2.27	3.83

REGARDING ANNEX A:

- k) In annex A, pages 1, 3, 109 to 115, 117 to 120, 128, 129 and 146 are replaced by pages 1a, 3a, 109a to 115a, 117a to 120a, 128a, 129a and 146a.
- I) In annex A, pages 147 to 154 are supplemented.

REGARDING ANNEX B:

m) Table B.2 is changed and supplemented as follows:

<u> Table B.2:</u>	Components of the standard system configuration
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Designation	Annex A, page
Vertical frame 18/70; 1.5m and 2.0m, steel	1a
Aluminium frame platform with plywood 1.57m; 2.07m	110a
Aluminium frame platform with plywood 2.57m; 3.07m	111a
Aluminium frame platform with internal hatch 2.57m; 3.07m	113a
Aluminium frame platform with internal hatch 1.57m; 2.07m	114a
Aluminium trapdoor deck with aluminium chequer plate 2.57m	117a
Aluminium trapdoor deck with aluminium chequer plate 3.07m	118a
Aluminium trapdoor deck with aluminium chequer plate 1.57m; 2.07m without ladder	120a
Protective net post AF	128a
Vertical frame AF 1.50m and 2.00m, steel	129a
Aluminium frame platform AB with plywood 0.50m – 3.07m	146a
Aluminium frame platform AB with hatch 2.57m; 3.07m	148
Aluminium frame platform AB with hatch 1.57m – 3.07m without ladder	150
Aluminium frame platform AB with hatch, aluminium chequer plate 2.57m; 3.07m	151
Aluminium frame platform AB with hatch, aluminium chequer plate 1.57m – 3.07m without ladder	153

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n) Table B.3 is changed and supplemented as follows:

Table B.3: Scaffold decks in the main bay

Scaffold deck	Deck width [m]	Number per bay	in accordance with annex A, page
Aluminium frame platform with plywood	61	1	110a, 111a
Aluminium frame platform AB with plywood 0.50m – 3.07m	61	1	146a

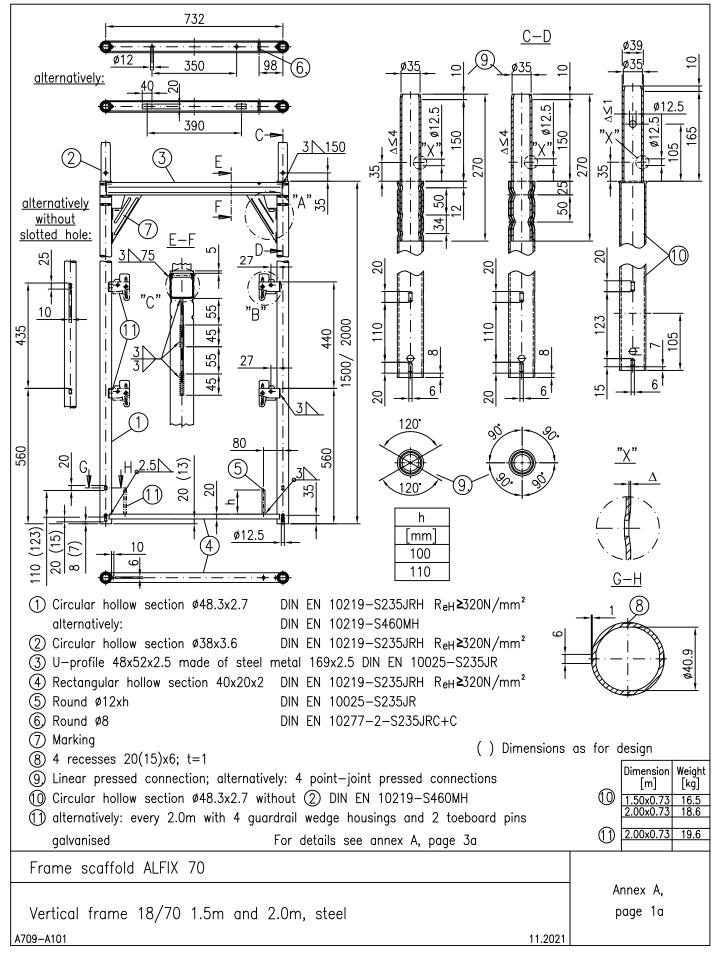
o) Table B.6 is changed and supplemented as follows:

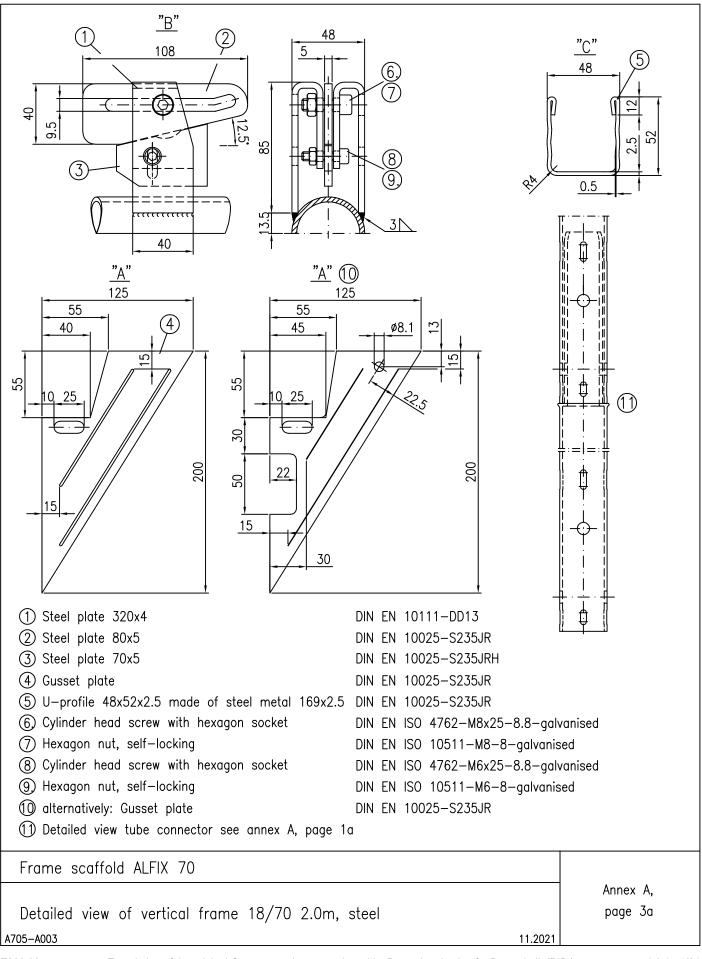
Table B.6: Trapdoor decks

Trapdoor decks	Deck width [m]	Number per bay	in accordance with annex A, page
Aluminium frame platform with internal hatch	61	1	113a, 114a
Aluminium trapdoor deck with aluminium chequer plate	61	1	117a, 118a, 120a
Aluminium frame platform AB with hatch	61	1	148, 150, 151, 153

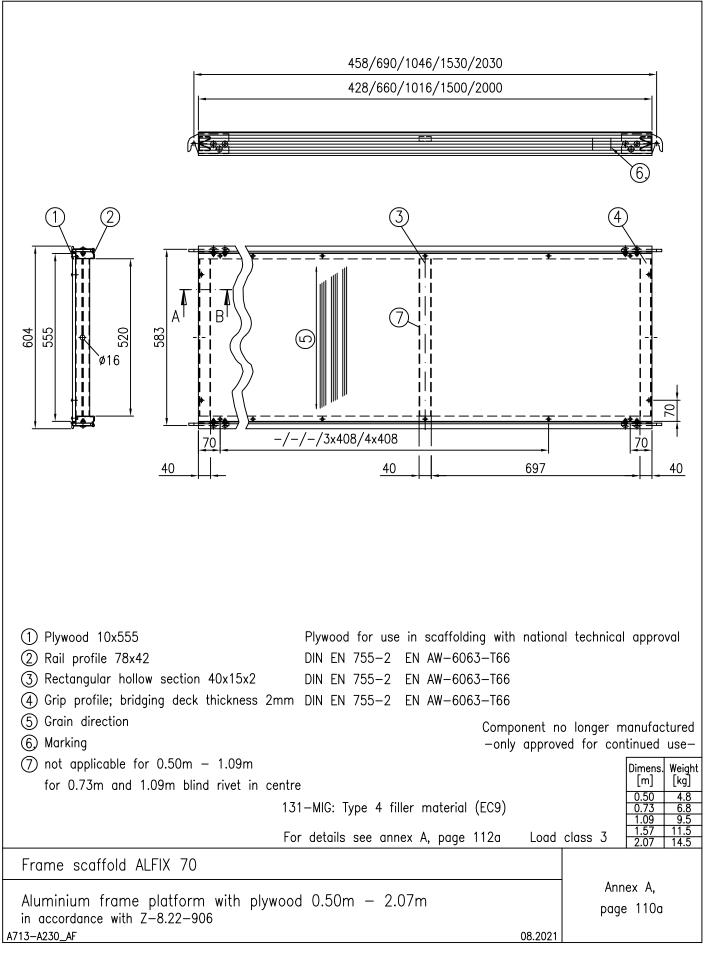
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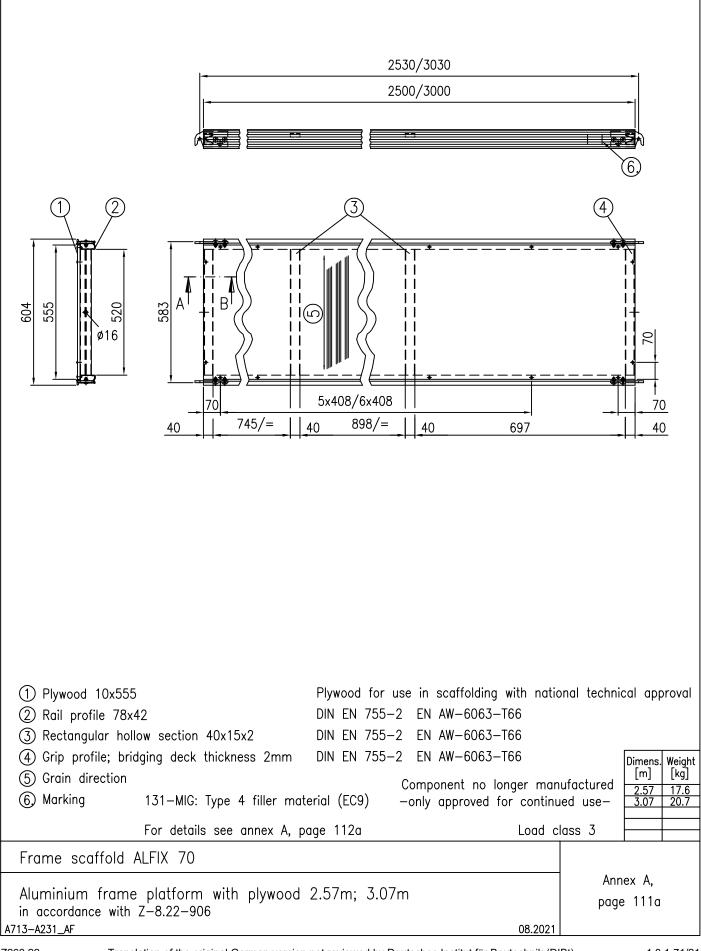
Certified

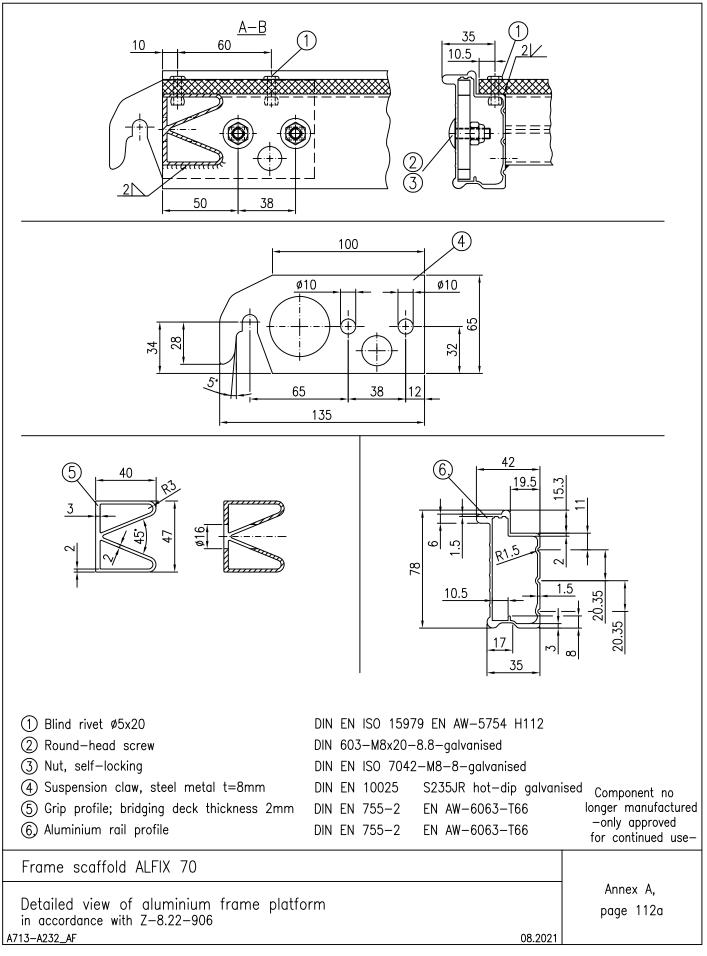


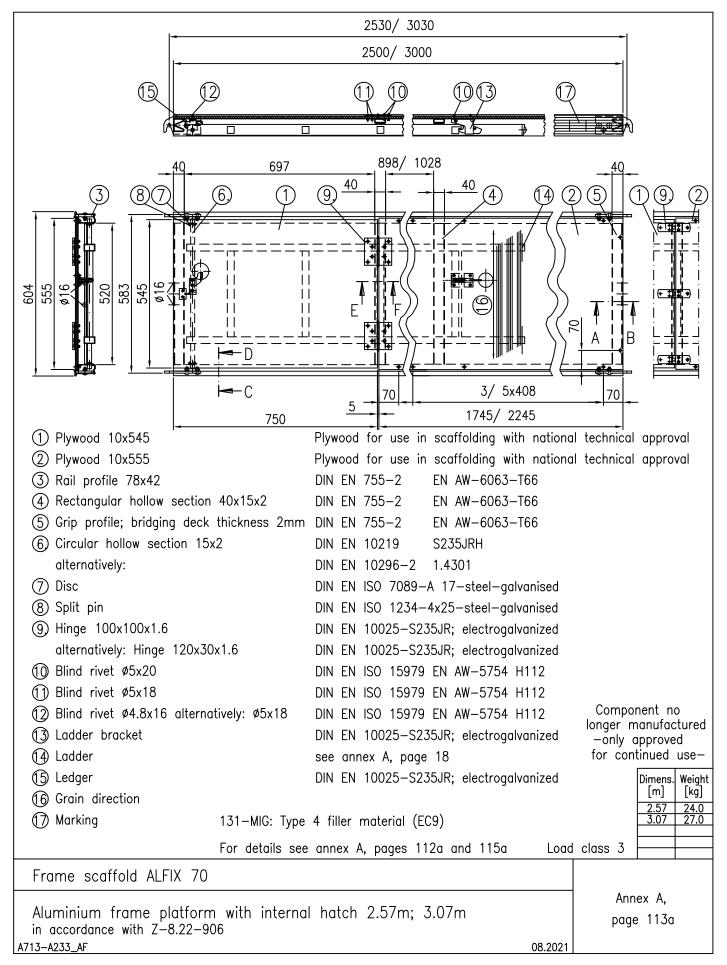


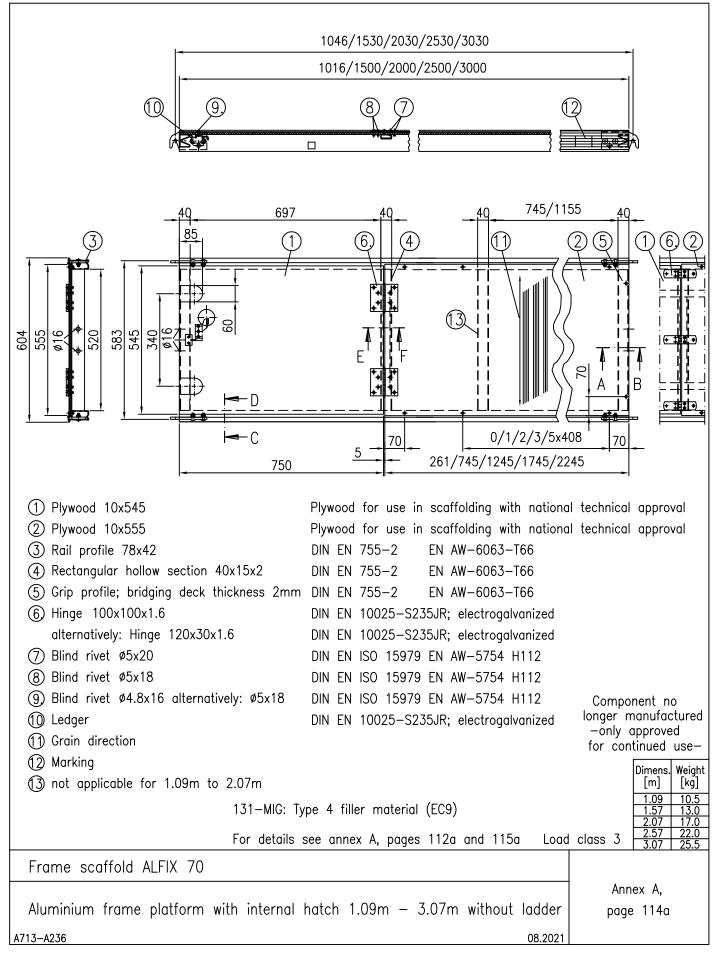
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Frame scaffold ALFIX 70	Annex A,
Blank page	page 109a

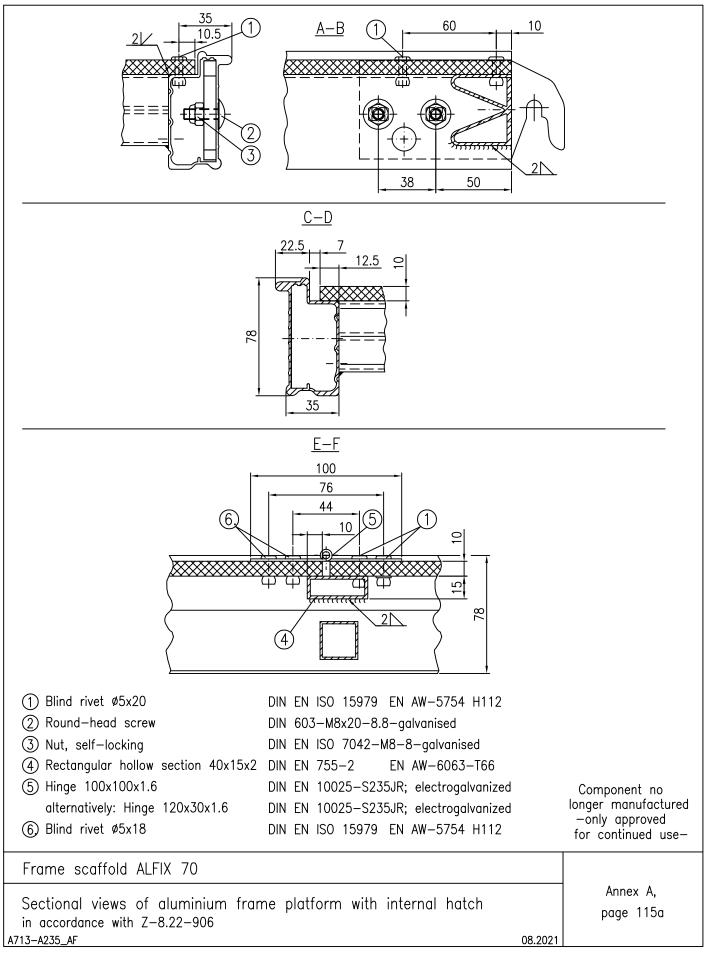


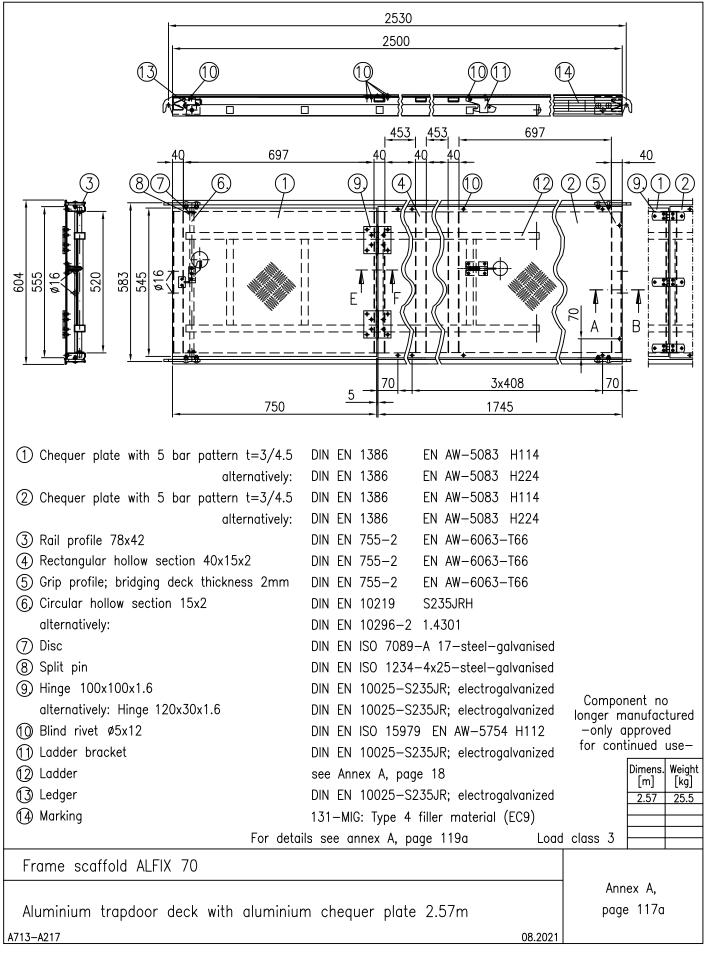


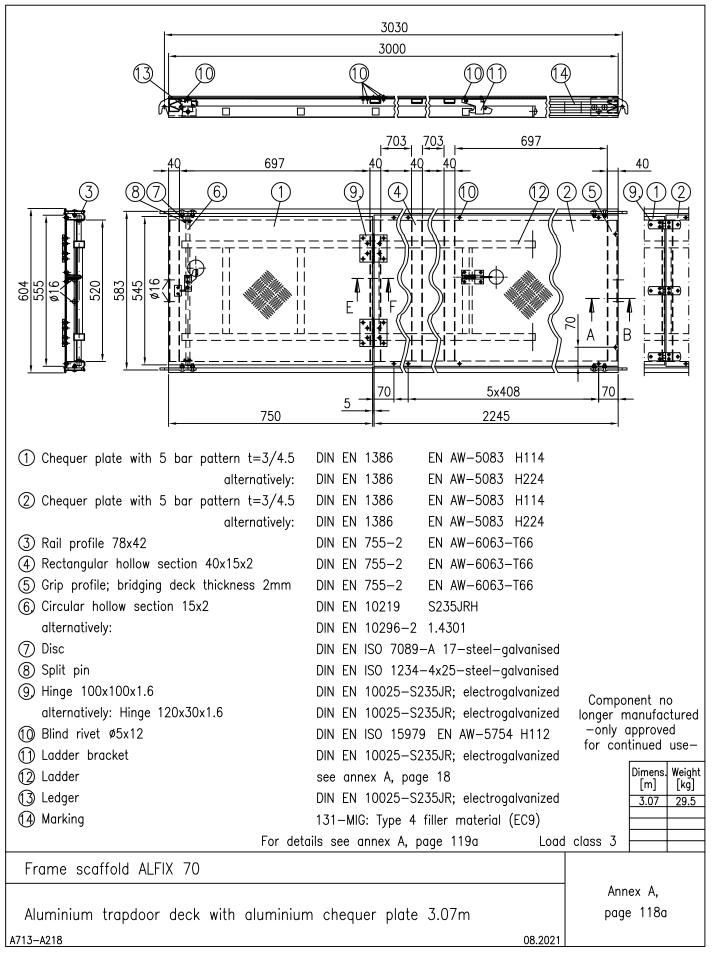




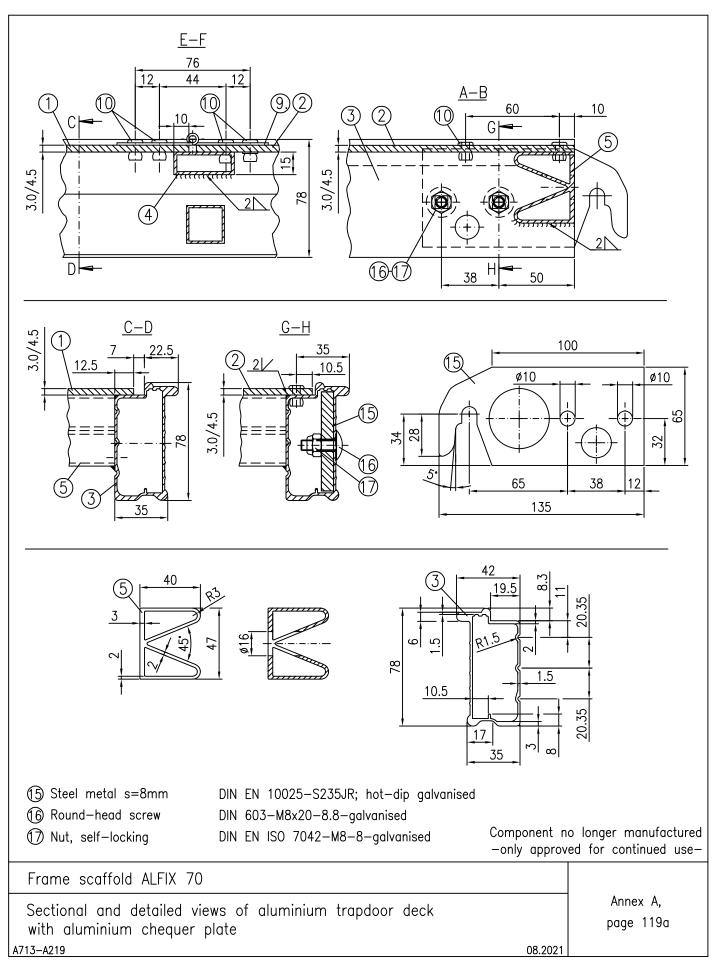


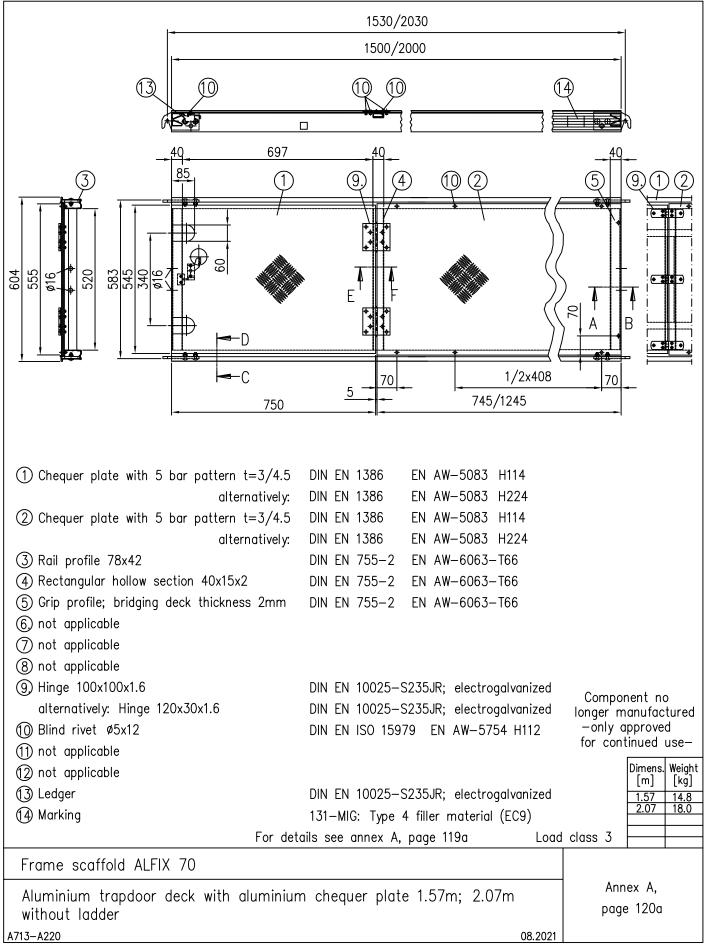


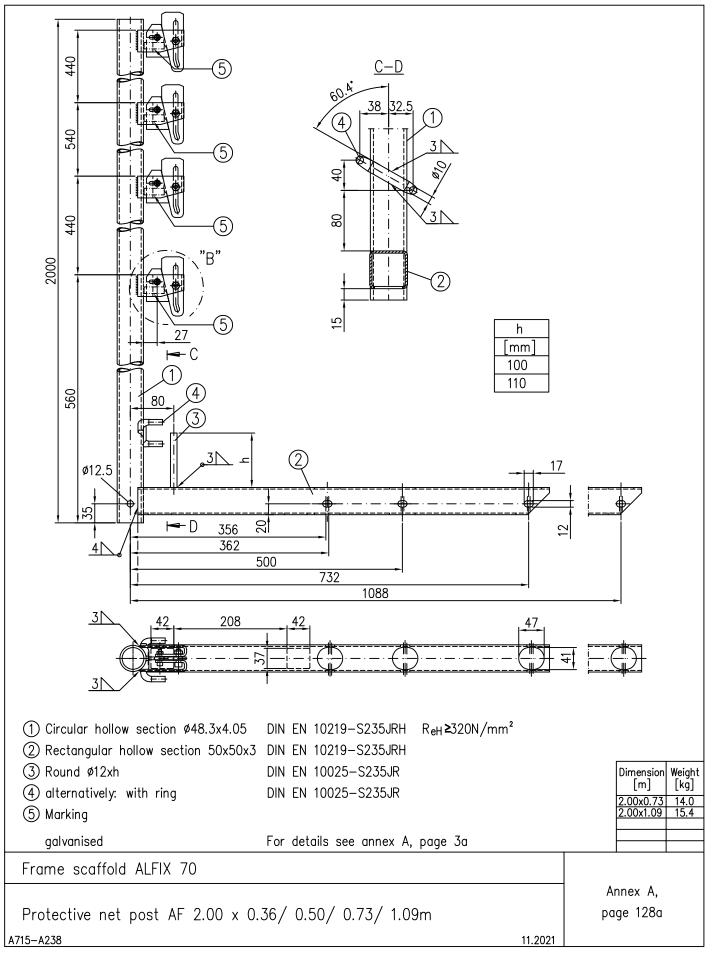


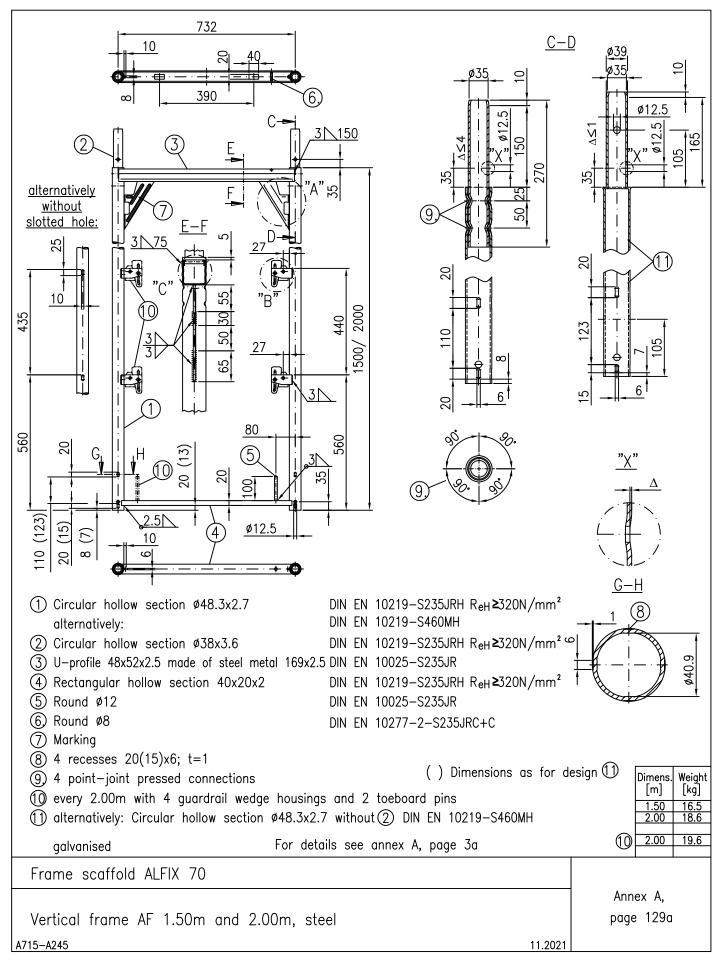


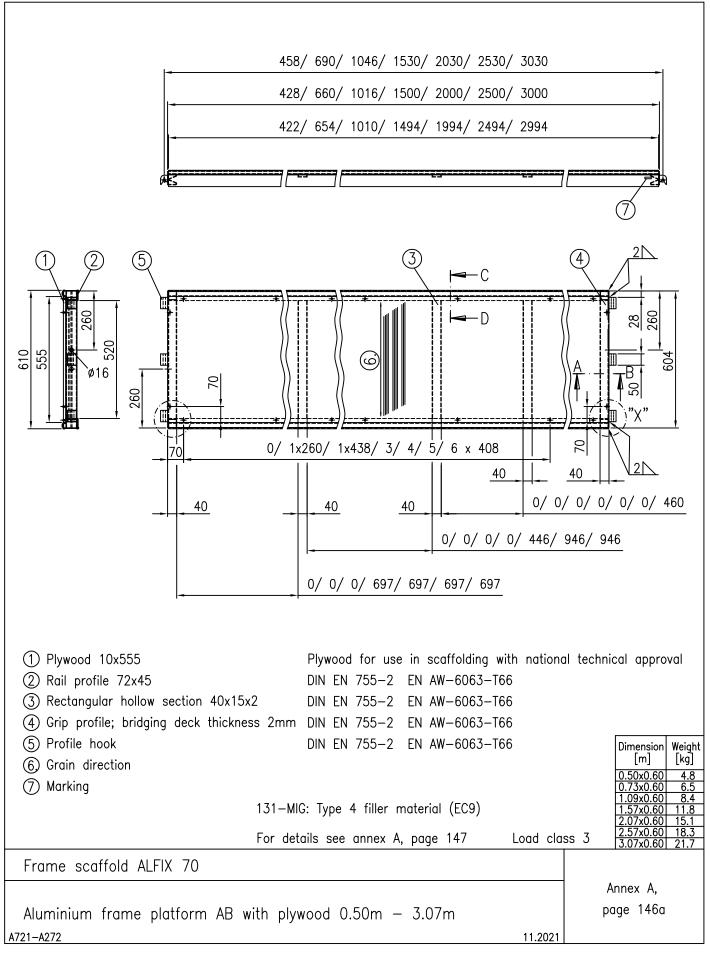
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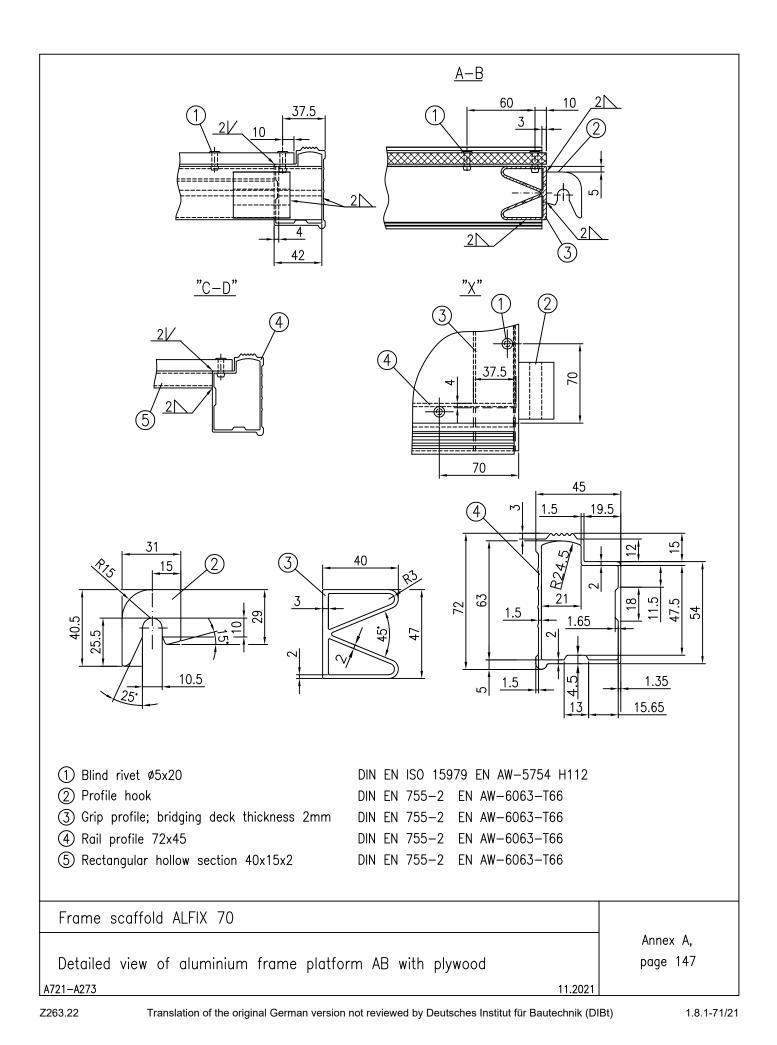


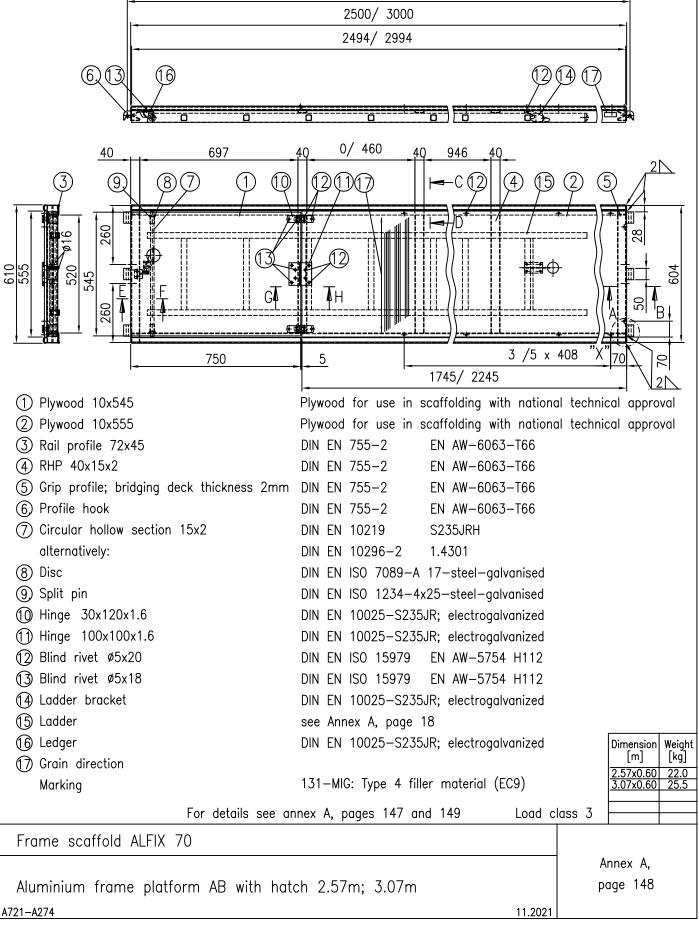






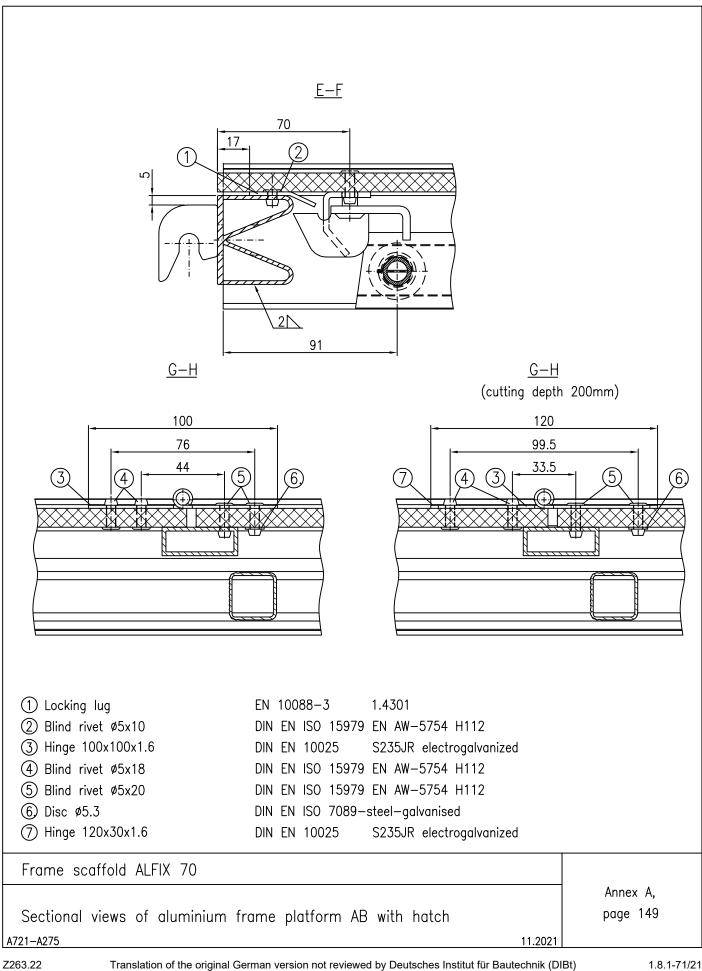


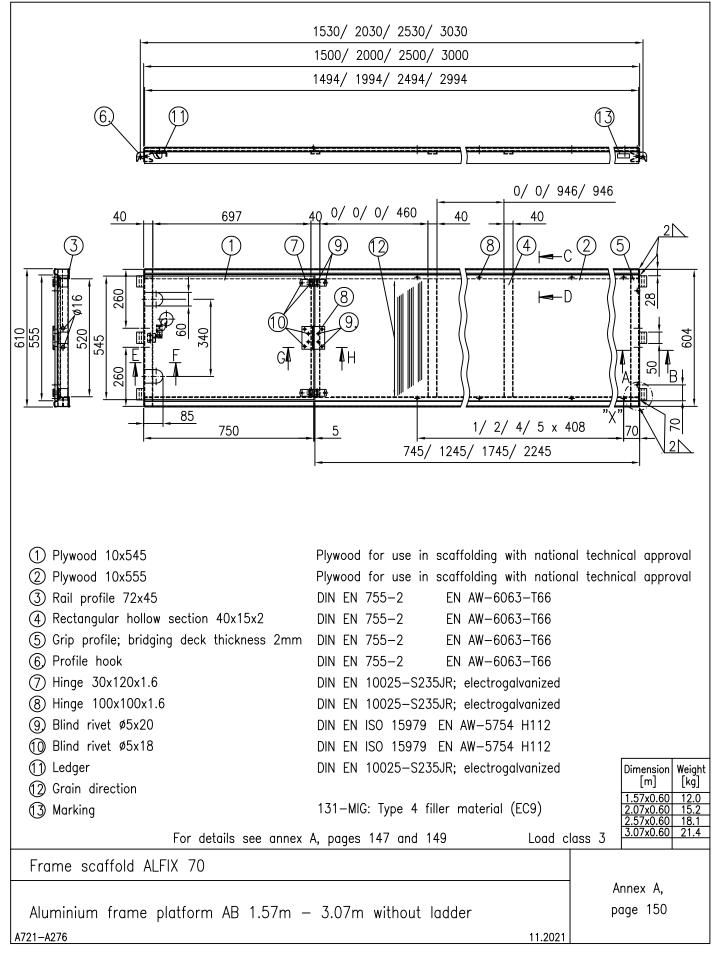


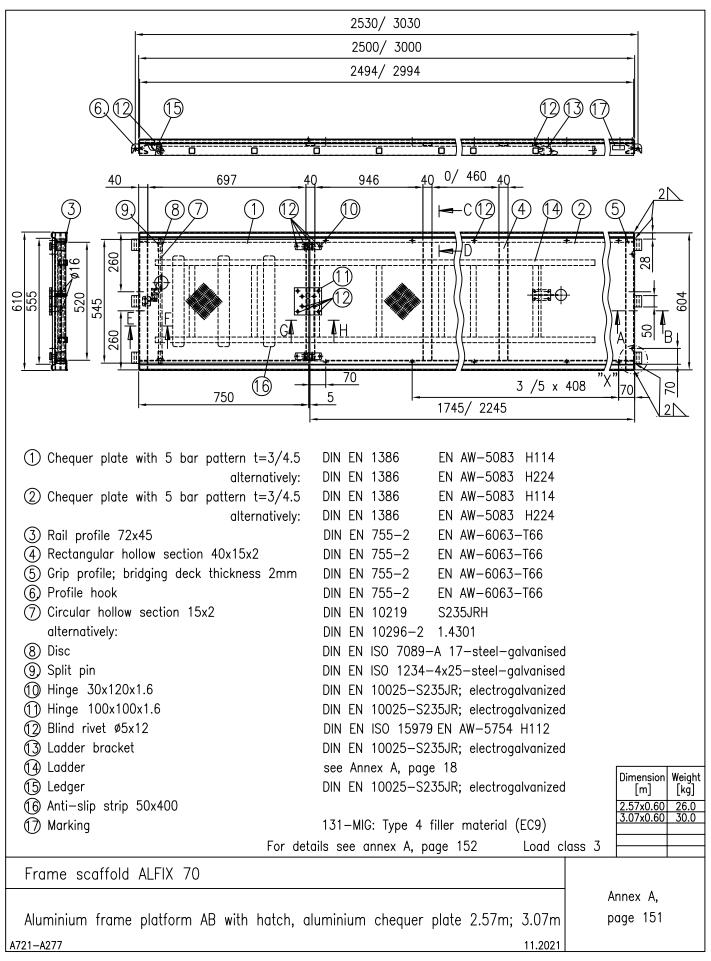


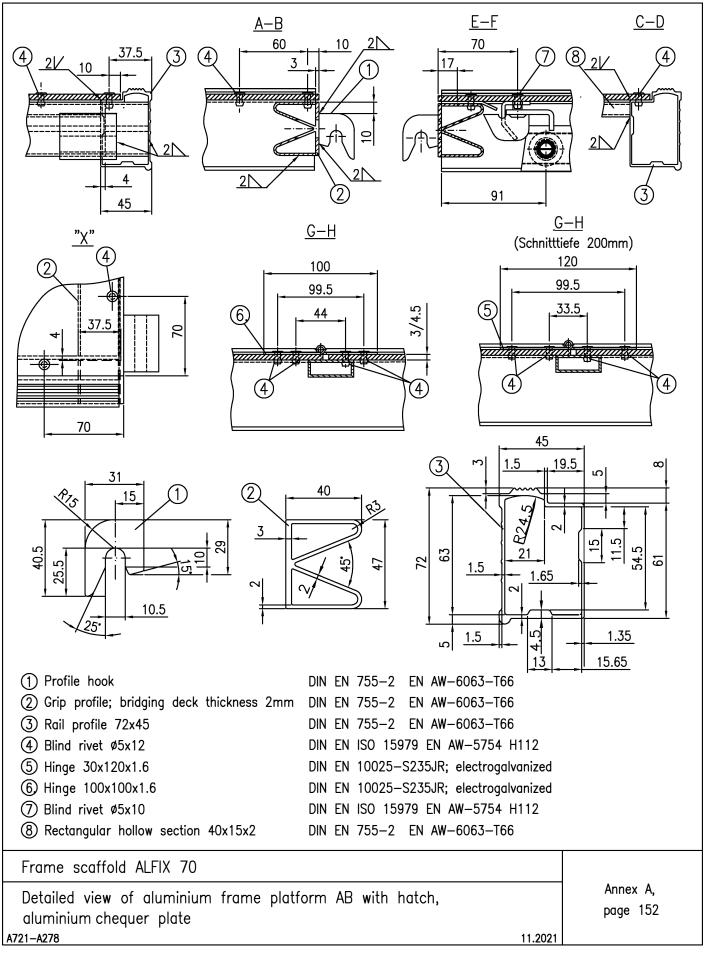
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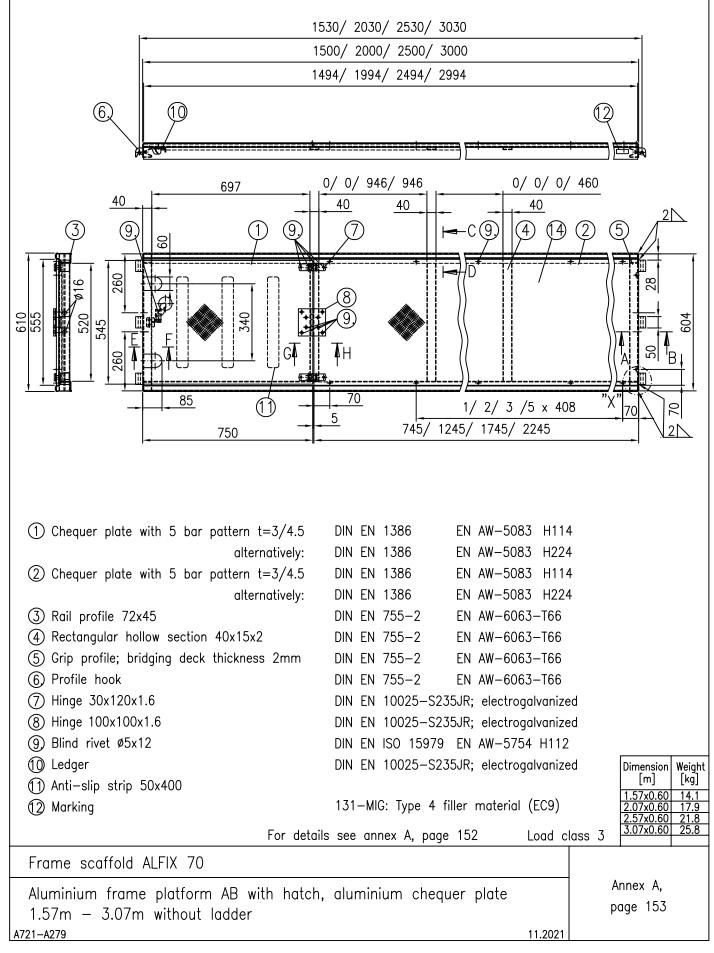
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Product marking code key

AF XX Ü 862 XX

AF = ALFIX manufacturer's logo

XX = Year of manufacture

 $\ddot{U} = Mark$ of conformity

- 862 = Abbreviated approval number
- XX = Supplier number or supplier's company logo in case of third-party manufacturing

Year	XX		
2015	15		
2016	16		
2017	17		
2018	18		
2019	19		
2020	20		
2021	21		
2022	22		
etc.	etc.		

Frame scaffold ALFIX 70	
Product marking code key AF	Annex A, page 154
A717–A257	08.2021

[Seal: Deutsches Institut für Bautechnik]

Approval body for construction products and construction techniques

Structural safety control authority

An institution under public law jointly funded by the German Federation and the federal states (Länder) Member of EOTA, UEAtc and WFTAO

 Date:
 Reference number:

 16 December 2020
 | 37.1-1.8.1-33/20

National technical approval / general construction technique permit

Approval number: **Z-8.1-862**

Period of validity: from: **16 December 2020** to: **4 January 2022**

Applicant:

Alfix GmbH Langhennersdorfer Straße 15 09603 Großschirma (Germany)

Subject of approval: "Frame Scaffold ALFIX 70" scaffolding system

The above-mentioned subject is hereby granted general construction technique permit.

This decision comprises 26 pages as well as annex A (pages 1 to 146), annex B (pages 1 to 11), and annex C (pages 1 to 27).

This national technical approval / general construction technique permit replaces national technical approval Z-8.1-862 of 9 December 2016. The subject was first granted general construction technique permit on 27 May 1999.

[Seal: Deutsches Institut für Bautechnik]

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I GENERAL PROVISIONS

- 1 This decision confirms the usability and / or applicability of the subject matter of the permit as defined by the Building Codes of the federal states.
- 2 This decision does not replace statutory approvals, authorisations and certifications specified for carrying out construction works.
- 3 This decision is granted without prejudice to the rights of third parties, especially private property rights.
- 4 The user of the subject matter of the decision must, without prejudice to further regulations laid out under "Special Provisions", make copies of this decision available to the persons using or applying the subject matter of the decision. The user of the subject matter shall also be informed that the decision must be present at the place of use. Upon request, copies must be provided to the relevant authorities.
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- 7 This decision relates to the information on the subject matter of the decision made available by the applicant during the approval process and the documents submitted. Any change made to these decision bases is not covered by this decision and must be disclosed to the Deutsches Institut für Bautechnik without delay.
- 8 The general construction technique permit covered by this decision also serves as national technical approval for the construction technique.

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II SPECIAL PROVISIONS

1 Subject matter of the decision / approval and scope of use and application

Subject matter of the approval are prefabricated scaffolding components in accordance with Table 1 for use in the "ALFIX 70 frame scaffold".

Subject matter of the decision is the planning, design, and execution of the "ALFIX 70 frame scaffold", consisting of scaffolding components

- according to table 1,
- according to table 3 and
- MVV TB (Model Administrative Provisions Technical Building Rules), section C 2.16 according to the respective scope of application.

The main load-bearing structure consists of steel vertical frames b = 0.732 m, decks $\ell \le 3.07$ m and diagonal braces (vertical diagonal braces) in the outer vertical plane.

The scaffold system has been verified for use as a working and service scaffold according to the DIN EN 12811-1:2004-03 in connection with the "Application Guideline for working scaffolds in accordance with DIN EN 12811-1¹ and DIN 4420-1:2004-03.

2 Provisions for the scaffolding components

2.1 Properties

2.1.1 General provisions

The scaffolding components according to table 1 must comply with the provisions of annex A the provisions in the documents filed at the Deutsches Institut für Bautechnik (DIBt) as well as the regulations of the sections below.

Designation	Annex A, page	Detailed view / components in accordance with annex A, page
Vertical frame 18/70; 1.5m and 2.0m, steel	1	3
Vertical frame 18/70; 1.0m and 0.67m, steel	2	1, 3
Steel deck AF 0.32m	7	
Steel deck AF 0.30m; 0.34m	9	
Intermediate deck AF 0.16m; 0.19m	10	
Intermediate deck	11	
Aluminium corner deck with toeboard, rigid	25	
Solid wood deck 45	26	
Solid wood deck 48	27	
Diagonal brace 3.07m	29	95
Diagonal brace 2.57m	30	95
Diagonal brace 2.07m	31	95
Horizontal strut	32	95
Scaffold tie	33	
Quick-release scaffold tie	34	

Table 1: Scaffolding components of the "ALFIX 70 frame scaffold"

see DIBt-Mitteilungen (notifications of the DIBt), issue 2/2006, p. 61 et seq.

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<u> Table 1:</u>	(continued)
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Designation	Annex A, page	Detailed view / components in accordance with annex A, page
Guardrail AF	36	
Double guardrail AF	38	
Double guardrail AF, aluminium	40	
Advanced guardrail post	42	
Telescopic guardrail, aluminium	44	
Double end guardrail AF, Single end guardrail	47	95
Guardrail post AF, single	49	1, 3
Guardrail post AF	51	1, 3
End guardrail frame	53	1, 3
Protective wall post AF	56	3
Bracket AF 0.36m	58	3
Bracket AF 0.73m	60	1, 3
Protective roof extension	62	1, 3
Lift-off preventer	64	
Transom 0.73m; 1.09m	65	3
Side-protection meshguard	66	
Lattice girder, steel	68	
Passage frame, in sections, 1.57m, upper part	71	3
Passage frame, in sections, tubular post, 1.90m	72	3
Gap cover	73	
Double guardrail AF 4.14m	74	
Steel plank 0.30m	75	
Aluminium stairway AF -0.62m 2.57m; 3.07m	78	
Stair guardrail AF 2.57m; 3.07m	79	
Inner guardrail for aluminium stairway 2.00m	80	
Stair stringer fall protection 1.00 x 0.50m	81	
Cantilever frame 2.00 x 0.37m	82	1, 3
Cantilever frame 2.00 x 0.53m	83	1, 3
Roof guard extension frame 2.00 x 0.73m to 1.09m	84	1,3
Bracket 0.36m, special design	85	
Assembly frame 0.37m; 0.67 – 2.00m	86	3
Starter transom 0.73m; 1.09m	87	3, 65
DS bracket frame 0.99 x 0.73m	88	1, 3
Aluminium stairway AF-0.62m 1.09m; 1.40m	89	
Spacer tube	90	
Bracket AF 0.50m	91	1, 3
Lattice girder suspension	92	
Lattice girder cross brace 0.73m; 1.09m	93	3, 87

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Table 1: (continued)

Designation	Annex A, page	Detailed view / components in accordance with annex A, page
Tube connector for lattice girder	94	
Guardrail coupler AF	95	
Toeboard coupler; Halfcoupler with hook	96	
Squared timber coupler	97	
Lattice girder, aluminium	98	
Toeboard support	99	
Locking pin	100	
Putlog coupler	101	
Diagonal cross brace	103	
Scaffold tie EIFS	105	
EIFS deck AF 190; Tube linchpin	106	
EIFS anchor 300/350/475	107	
Aluminium deck 0.60m, lightweight	116	
Aluminium trapdoor deck with aluminium chequer plate 2.57m	117	18, 119
Aluminium trapdoor deck with aluminium chequer plate 3.07m	118	18, 119
Aluminium trapdoor deck with aluminium chequer plate 1.09m – 2.07m without ladder	120	119
AB Base jack	121	
Base jack, with swivel base	122	
Anchor coupler	123	
Toeboard, End toeboard AF	124	
Toeboard 4.14m AF	125	
Aluminium toeboard; Aluminium end toeboard AF	126	
Steel toeboard; Steel end toeboard AF	127	
Protective net post AF 2.00 x 0.36 / 0.50 / 0.73m	128	3
Vertical frame AF 1.50m and 2.00m, steel	129	3
Vertical frame AF 1.0m and 0.67m, steel	130	3, 129
Aluminium double guardrail AF 1.57m; 2.07m; 2.57m; 3.07m	131	
Guard net system	132	36
Gusset coupler	133	
ALBLITZ stair guardrail post 1.10m	134	3
Inner guardrail post 1.00m	135	3
Guardrail holder for internal corner	136	3
Bracket AF 0.36m, lift-off preventer for inner face	137	3
Weather protection add-on unit 2.00 x 0.73m	138	3
Weather protection sleeve 2.00m	139	3
Corner guardrail wedge housing	140	

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Table 1:	(continued)
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Designation	Annex A, page	Detailed view / components in accordance with annex A, page
DS bracket frame 0.99 x 1.09m	141	3
Aluminium stairway AF-0.62m 2.07m	142	
TRBS guardrail (compliant with technical regulations for occupational safety) 2.07m; 2.57m; 3.07m, folding	143	
TRBS guardrail (compliant with technical regulations for occupational safety) 0.73m; 1.09m; 1.57m, rigid	144	
TRBS end guardrail (compliant with technical regulations for occupational safety) 0.73m; 1.09m	145	

2.1.2 Materials

2.1.2.1 Metals

Metal materials must comply with the technical rules according to table 2. Their properties must be confirmed by means of a material test certificate in accordance with table 2.

Material test certificates for aluminium alloys must contain at least information on the chemical composition, tensile strength R_m , yield point $R_{p0,2}$ and strain A or A_{50mm} .

Components for which the material specifications are filed at the Deutsches Institut für Bautechnik, the properties shall be confirmed by means of the following material test certificates:

- For structural steel without an increased nominal yield strength and with a defined minimum nominal yield strength of ≤ 275 N/mm² a test report 2.2 issued by the factory is sufficient.
- For all other metal materials, an inspection report 3.1 is mandatory.

The wedges used in connection with some of the components must be manufactured, inspected and marked in accordance with the rules laid out in Z-8.22-906.

Table 2:	Technical provisions and material test certificates for the metal materials of the
	scaffolding components

Material	Material number	Designation	Technical regulation	Material test certificate according to DIN EN 10204: 2005-01
	1.0039	S235JRH *)		2.2 ^{*)}
	1.0576	S355J2H	DIN EN 10219-1: 2006-07	2.4
	1.8849	S460MH	2000-07	3.1
Structural steel	1.0038	S235JR *)	DIN EN 10025-2:	2.2 ^{*)}
	1.0577	S355J2	2019-10	3.1
	1.0122	S235JRC+C	DIN EN 10277: 2018-09	2.2
Steel plate and	1.0242	S250GD+Z275**)	DIN EN 10346:	3.1
steel metal	1.0918	DX52D+Z275**)	2015-10	3.1
	1.0332	DD11**)	DIN EN 10111:	
	1.0335	DD13**)	2008-06	
Rolled flats	1.0330	DC01	DIN EN 10130:	
	1.0330	0001	2007-02	
	1.0982	S460MC	DIN EN 10149-1:	
	1.0002	0-100mo	2013-12	

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Table 2: (con	tinued)			
Material	Material number	Designation	Technical regulation	Material test certificate according to DIN EN 10204: 2005-01
Cold extruded	1.0214	C10C	DIN EN 10263-2: 2018-02	
steel	1.1122	C10E2C	DIN EN 10263-3: 2018-02	
Cast steel	1.0446	GE240+N	DIN EN 10293: 2015-04	
	5.4201 (EN-JM1020)	EN-GJMW-360-12		
Mellachia agot iran	5.4202 (EN-JM1030)	EN-GJMW-400-5	DIN EN 1562: 2019-06	
Malleable cast iron	5.4205 (EN-JM1140)	EN-GJMB-450-6		
Ductile iron (nodular cast iron)	5.3106 (EN-JS1030)	EN-GJS-400-15	DIN EN 1563: 2019-04	
	EN AW-5083 H114 EN AW-5083 H224	EN AW- Al Mg4,5Mn0,7	DIN EN 1386: 2008-05	
	EN AW-5754 H114	EN AW-Al Mg3		
Aluminium alloy	EN AW-5754 H24 / H34	EN AW-Al Mg3	DIN EN 485-2: 2018-12	3.1
	EN AW-6060 T66	EN AW-Al MgSi		
	EN AW-6063 T66 EN AW-6082	EN AW-Al Mg0,7Si EN AW-	DIN EN 755-2: 2016-10	
	T5	Al Si1MgMn		
*) For some scaffolding	components, a high	ner nominal yield strength	$R_{eH} \geq 280 \mbox{ N/mm}^2 \mbox{ or } R_e$	$_{\rm H} \ge 320 \ \rm N/mm^2 \ has$
been determined. T	hese components l	have been marked acco	rdingly in the drawings	in annex A. The
		be lower than 15 %. For a		
strain at fracture of A _{80mm} shall be determined. The conversion of A _{80mm} to A shall be done in accordance with				
DIN EN ISO 2566-1.				
The values of the nominal yield strength, the strain at fracture and the tensile strength shall be confirmed by means of an inspection certificate 3.1 in accordance with DIN EN 10204:2005-01. The purchase requisition				
5 S	eu nominai yield sti	rength shall be indicated i	in the inspection certifica	ale J. I as a desifed
value.				

 $R_{\mbox{\scriptsize eH}}$ in accordance with the figures in the annexes

*)

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2.1.2.2 Extruded section profiles

The extruded section profiles must meet the requirements of the DIN EN 755 standards.

2.1.2.3 Solid wood

Solid wood must meet at least the requirements of visual strength grade S10 or S13 in accordance with DIN 4074-1:2012-06 or the minimum strength of strength class C 24 in accordance with DIN EN 338:2010-02.

2.1.2.4 Constructional veneer plywood

Constructional veneer plywood shall meet at least the requirements of the "Approval Principles for the use of constructional veneer plywood in scaffolding"² as well as the specifications in the drawings of annex A.

2.1.3 Couplers

Class B halfcouplers in accordance with DIN EN 74-2:2009-01 must be used as couplers that are fitted to various components. In deviation from DIN EN 74-2:2009-01, proof must be provided for the halfcouplers of the components according to Table 1 that they have a breaking force of $F_{f,c}$ = 30 kN.

2.1.4 Corrosion protection

The technical building regulations shall apply.

2.2 Manufacturing and marking

2.2.1 Manufacturing

Companies that manufacture welded scaffolding components in accordance with the present decision shall demonstrate that they are qualified for this task.

For steel components, this proof shall be considered to be furnished, if welding procedures and welding personnel are qualified in accordance with DIN EN 1090-2:2018-09 and the company holds a welding certificate of at least execution class 2 (EXC 2) in accordance with DIN EN 1090-1:2012-02.

For aluminium components, this proof shall be considered to be furnished, if welding procedures and welding personnel are qualified in accordance with DIN EN 1090-2:2019-07 and the company holds a welding certificate of at least execution class 2 (EXC 2) in accordance with DIN EN 1090-1:2012-02.

Companies that manufacture glued (bonded) scaffolding components in accordance with the present National Technical Approval shall demonstrate that they are qualified for this task. Proof of suitability for the production of glued (bonded) components shall be considered as provided if the company holds at least a C1 certificate in accordance with DIN 1052-10:2012-05.

2.2.2 Marking

The delivery notes for scaffolding components according to table 1 shall be marked in accordance with the regulations for the mark of conformity of the federal states (Länder).

In addition, scaffolding components shall be permanently and easily recognisably marked with:

- the uppercase letter "Ü",
- minat least the abbreviated approval number "862",
- the identifying mark (logo) of the manufacturer, and
- the last two digits of the year of manufacture.

Alternatively, a coded identifying mark in accordance with annex A, page 146, may be used. These identifying marks may only be applied if the requirements under Section 2.3 are fulfilled.

2

see also DIBt-Mitteilungen (notifications of the DIBt) issue 3/1999, p. 122 et seq.

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2.3 Attestation of conformity

2.3.1 General provisions

Attestation of conformity of the scaffolding components according to section 2.1 with the provisions of the National Technical Approval covered by this decision must be provided for each manufacturer's work by a declaration of conformity on the basis of factory production controls and a certificate of conformity issued by a recognised certification body as well as regular external supervision, including a product test of scaffolding components and their components in accordance with the provisions below by a recognised inspection body.

The manufacturer of the scaffolding components must involve a recognised certification body as well as a recognised inspection body to obtain a certificate of conformity and to carry out the external supervision, including the product tests.

The declaration that a certificate of conformity has been issued must be indicated by the manufacturer by marking the scaffolding components with the mark of conformity (Ü mark) with reference to the intended use.

The certifying body shall provide Deutsches Institut für Bautechnik (DIBt) with a copy of the certificate of conformity issued by the former, and the supervisory body shall provide it with a copy of the supervision report upon request.

DIBt shall be given a copy of the initial test report upon request of the same.

2.3.2 Factory production control

A factory production control system must be set up and operated at each production site. Factory production control is to be understood as a continuous monitoring of production to be carried out by the manufacturer, by means of which the manufacturer ensures that the scaffolding components manufactured by them are in compliance with the rules of this National Technical Approval.

The factory production control must include at least the following measures:

Scaffolding components in accordance with table 1:

- In the case of template or automatic production of scaffolding components, the respective templates and / or machine settings shall be checked and documented before commissioning.
- Checks and inspections on the starting material:
 - It shall be checked whether inspection certificates as per Section 2.1.2 are available for the materials and that the attested inspection results meet the requirements.
 - At least 1‰ of the components shall be checked for conformity with dimensions and tolerances as specified in the design drawings.
 - At least 0.1‰ of the cold worked U-claws of some of the fittings in accordance with this decision, shall be subjected to a factory production control according to the documents filed with the Deutsches Institut für Bautechnik.
 - A tensile test shall be carried out on at least 0.1‰ of the non-galvanised pressed-in tube connectors. The breaking load value F_{Break} must not be lower than 13.75 kN.
 - Inspections shall be carried out on at least 0.1 ‰ of the integrated tube connectors of the standards according to annex A, pages 1 and 3, however at least once every production week, in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt).

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- The increased breaking load of the class B halfcouplers fitted to some of the components, shall be verified and documented within the course of a factory production control in accordance with Table A.2 of DIN EN 74-2:2009-01, unless this has already been verified for the production of the couplers.
- Checks and inspections on scaffolding components:
 - At least 1‰ of the scaffolding components shall be checked for conformity with dimensions and tolerances and, if necessary, welding seams and corrosion protection, as specified in the design drawings.
 - At least 0.1‰ of the components with riveted halfcouplers shall be subjected to a factory production control according to the documents filed with the Deutsches Institut für Bautechnik.

The results of the factory production control shall be recorded and evaluated. The records must contain at least the following information:

- Designation of the scaffolding components
- Type of inspection
- Date of manufacturing and inspection of the scaffolding components
- Result of the production controls and inspections and comparison with requirements
- Signature of the person responsible for the factory production controls.

The records shall be kept for at least five years and shall be made available to the external supervisory body in charge of the external supervision. Upon request, these records must be presented to the Deutsches Institut für Bautechnik and to the competent superior building inspection authority.

If inspection results are unsatisfactory, the manufacturer must immediately take corrective actions. Scaffolding components or components that do not meet the requirements must be handled in such a way that they cannot become confused with conforming parts. After the corrective actions, the inspection/test concerned must be repeated immediately, provided this is technically possible and necessary to prove that the defect has been rectified.

2.3.3 External supervision

In each manufacturer's work, factory production controls shall be supervised by an external supervision body on a regular basis, at least every 5 years for scaffolding components in accordance with Table 1. In deviation thereof, the integrated tube connector shall be inspected in accordance with annex A, pages 1 and/or 3 at least two times a year by a recognised inspection body.

External supervision includes an inspection of the factory and the factory production control system, including a product inspection. Sampling and inspections/tests shall be the responsibility of the recognised body.

At least the following inspections/tests are to be carried out:

- Inspection of the requirements in terms of personnel and equipment for proper manufacturing of the scaffolding components
- Inspection of the factory production control system
- Checks on random samples for conformity of scaffolding components with the provisions of the approval in terms of:
 - Construction type, form and dimensions
 - Corrosion protection
 - Marking
- Inspection of the required welding and bonding certificate

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- At least 5 each of the following components / details manufactured in accordance with this decision shall be inspected by the external supervisory body in accordance with the documents filed with the Deutsches Institut für Bautechnik (DIBt):
 - cold worked U-claws of several decks
 - integrated and pressed-in tube connectors
 - components with rivited halfcouplers
- The increased breaking load of the class B halfcouplers fitted to some of the components shall be verified in accordance with level M according to Table A.2 of DIN EN 74-2:2009-01, unless this has already been verified within the course of the production of the couplers.

The scaffolding components shall be taken from current production.

The results of the certification and external supervision shall be kept for at least five years. Upon request, they must be presented to Deutsches Institut für Bautechnik and to the competent superior building inspection authority by the certification body and/or supervisory body.

3 **Provisions for planning, dimensioning and execution**

3.1 Planning

3.1.1 General provisions

The "ALFIX 70 frame scaffold" consists of scaffolding components in accordance with section 1. Scaffolding components in accordance with table 3, which refer to provisions for manufacturing, marking and mark of conformity set out in this decision, are no longer manufactured and are, therefore, only approved for continued use.

Table 3:	Further scaffolding components for use in the "ALFIX 70 frame scaffold"

Designation	Annex A, page	Detailed view / components in accordance with annex A, page	Regulations for manufacturing, marking and certificate of conformity	
Vertical frame 70; 2.0m, steel	4	6		
Vertical frame 70; 1.0 and 0.66m, steel	5	4, 6		
Steel deck	8			
Aluminium deck with plywood 2.57m; 3.07m	12	14		
Aluminium deck with plywood 1.57m; 2.07m	13	14		
Aluminium access deck 3.07m with ladder	15	14, 17, 18	according to Z-8.1-862 (No longer manufactured.)	
Aluminium access deck 2.57m with ladder	16	14, 17, 18		
Aluminium deck with plywood 3.07m	19	21	manalaotaroa.j	
Aluminium deck with plywood 1.57m; 2.07m; 2.57m	20	21		
Aluminium access deck with ladder 3.07m	22	18, 21		
Aluminium access deck with ladder 2.57m	23	18, 21		

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Table 3: (continued)			
Designation	Annex A, page	Detailed view / components in accordance with annex A, page	Regulations for manufacturing, marking and certificate of conformity
Wooden deck	28		
Base jack	35		
Handrail	37		
Double guardrail	39		
Double guardrail, aluminium	41		
Advanced guardrail post	43		
Advanced end guardrail	44		
Telescopic guardrail 2.00 – 3.07m	45		
Toeboard; End toeboard	46		
Double end guardrail	48		
Guardrail post, single	50	6	e e e e e e e e e e e e
Guardrail post	52	6	according to Z-8.1-862
End guardrail post AF	54	3	(No longer
End guardrail post	55	6	manufactured.)
Protective wall post	57	6	
Bracket 0.36m	59		
Bracket 0.73m	61	4,6	
Protective roof extension	63		
Protection net	67		
Passage frame AF	69	6	
Passage frame	70	6	
Toeboard 4.14m	76		
Aluminium toeboard; Aluminium end toeboard	77		
Protective wall post, telescopic 0.36m – 1.73m	102	3	
Protective wall post, telescopic 0.73m – 1.09m	104	3	
Base jack with swivel base	108		
Aluminium frame platform with plywood 0.50m – 2.07m	110	112	
Aluminium frame platform with plywood 2.57m; 3.07m	111	112	according to
Aluminium frame platform with internal hatch 2.57m; 3.07m	113	18, 112, 115	Z-8.22-906
Aluminium frame platform with internal hatch 1.09m – 3.07m without ladder	114	112, 115	

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3.1.2 Standard system configuration

The use of the scaffolding components in facade scaffolding is described in the provisions of a standard system configuration. Proof of structural stability for said fully erected scaffolds in their standard system configuration has been furnished. Assembly configurations of facade scaffolds are considered to be standard system configuration when they comply with the provisions of annex B and C. Any assembly configurations that deviate from the standard system configuration shall be assessable and verified in each individual case.

The standard system configuration applies to facade scaffolds with a structural height that does not exceed 24 m, not including the spindle extension length above the ground. In its standard system configuration, the scaffold system may be used with system widths b = 0.732 m and bay length ≤ 3.07 m for working scaffolds of load classes ≤ 3 in accordance with DIN EN 12811-1:2004-03, and as a protection scaffold and roof edge protection scaffold with a maximum falling height of class 1 (FL1) and as roof edge protection scaffold with protective walls of class SWD 1 in accordance with DIN 4420-1:2004-03.

3.1.3 Deviations from the standard system configurations

If assembly configurations deviate from the standard system configurations in accordance with annex B or annex C, proof of structural stability of the scaffolds shall be provided for each individual configuration or by means of a structural design calculation in accordance with the Technical Building Rules [Technische Baubestimmungen] and the provisions of this decision. The characteristic values to be used for the proof of structural stability are specified in this decision.

Other anchorage patterns are possible and other nettings may be used as scaffold cladding. In a scaffold, any increased stresses / loads (e.g. from higher dead weights and wind loads or from increased live loads) must be verified up to the anchors and the supporting surface (ground). The impact of building hoists or other lifting equipment must also be taken into account if they are not operated independently of the scaffold.

3.2 Dimensioning

3.2.1 General provisions

Unless otherwise specified in this decision, particular attention in relation to the design and calculation of scaffolds erected using the scaffold system shall be paid to the Technical Building Regulations, in particular for working and service scaffolds of DIN EN 12811-1:2004-03 in conjunction with the "Application guideline for working scaffolds according to DIN EN 12811-1" ¹, DIN 4420-1:2004-03, and the "Approval principles for working and service scaffolds, requirements, calculation, tests and proof of conformity". ^{3 4}

Only the transmission of normal forces at the connections of the diagonals is allowed.

In the event that other configurations are used and it is not clear which component shall be used, any proof of structural stability shall assume the least favourable variant.

Please also take into consideration the advisory results of the "Scaffold Expert Committee", the so-called "SVA-Gerüste", available on the DIBt homepage.

³ To be obtained from the Deutsches Institut für Bautechnik.

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3.2.2 Vertical frame

3.2.2.1 Corner plate in vertical frame

Proof of structural stability of the scaffold system may be furnished assuming the values of Figure 1 for the corner plate according to annex A, pages 3 and 6 in the vertical frame.

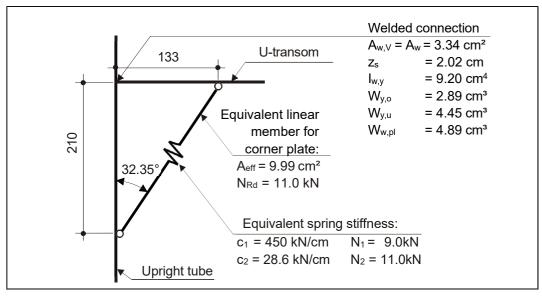


Figure 1: Characteristic values of the corner plate

3.2.2.2 Connection lower transom – upright tube

Proof of structural stability of the scaffold system may be furnished taking into account the connection of the lower transom to the upright tube of the vertical frames with a rotational restraint and a load-bearing capacity according to Table 4. Please note that the connection is related to the outer side of the upright tubes.

<u> Table 4:</u>	Characteristic	values of the	connection low	/er transom -	upright tube
------------------	----------------	---------------	----------------	---------------	--------------

Component	Resistance capacity M _{y,Rd} [kNcm]	Rotation ϕ_d [rad]
Vertical frame	±33.5	$\varphi_d = \frac{M_y}{4\ 520\ -\ 24,7\cdot\ M_y} \text{with } M_y \ in \ [kNcm]$

- 3.2.2.3 Configuration and proof of structural stability of the standard joints
- 3.2.2.3.1 General provisions

Unless otherwise specified below, the joints of the standards in the ALFIX 70 frame scaffold are to be constructed and proof of structural stability is to be provided in accordance with the current Technical Building Rules. Please also refer to the "Calculation of standard joints with one-sided, centrally fixed joint pins for working and service scaffolds, and for shoring scaffolds made of steel"⁵.

The decision comprises four configurations of this detail. Table 5 summarizes these configurations and lists the most important characteristic values. If it is not possible to ensure which standard joint configuration shall be used, any proof of structural stability shall assume the least favourable variant.

see DIBt Newsletter 4/2017

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Standard and tube connector configurations

Turno	Standa	rd	with tube	Material	
Туре	Annex A, page	Tube	Provisions for execution	Tube	$f_{y,k}(R_{eH})$
1	1, 2, 129, 130		integrated	Ø 39.0x3.3	460 N/mm ²
2	1, 2, 82, 83, 84, 88, 90, 129, 130	Ø 48.3x2.7	plugged in,	Ø 38.0x3.6	
3	69, 71, 72, 88	Ø 48.3x3.2	pressed		320 N/mm²
4	4, 5, 70, 141	Ø 46.3X3.2		Ø 38.0x4.0	

3.2.2.3.2 Load-bearing model "lap joint"

In accordance with the "Calculation notes for standard joints with one-sided centrally fixed joint pins for working and service scaffolds, and for shoring scaffolds made of steel" ⁵ ", the standard joint parameters as per Table 6 shall be taken into account for the structural analysis of type 1 standard joints with standard tubes Ø 48.3 x 2.7 mm made of steel grade S460MH with integrated tube connectors in the load-bearing model "lap joint".

<u>Table 6:</u> Load-bearing capacities and load-deformation behaviour fo
--

Forces and moments	moments connector Resistance capacity Load-deformation behaviour							
Bending momentType 1 $M_{\rm Rd} = 111 \ kNcm^{*}$ Stiffness behaviour: $\varphi_d = \frac{M}{13000 - 41 \cdot M }$ with M [kNcm]								
*) Separate proof of stability of the net section at the tube connector is not mandatory.								

3.2.2.3.3 Structural behaviour under tensile stress

If tensile forces must be transmitted via a standard joint, the tubes shall be connected by means of bolt connections whereby the looseness at the connecting members are to be taken into consideration. Connecting members are to be passed through the openings in the joint area intended for this use and secured against unintended lift-off (e.g. by means of a screw joint tightened by hand). Depending on the connecting member used and the standard joint variant, the loads in accordance with Table 7 can be transmitted.

Table 7: Tension stress resistance of the standard joints

Tensile stress resistance $N_{z,\text{Rd}}$ [kN] when using screws							
					Tut	pe II	
Tube II				Type 1	Type 2	Type 3	Type 4
		Type 1	M10-8.8	30.1	23.6	27	' .9
		Type 1 -	M12-8.8	40.6	28.3	33	8.5
	Tube I	Туре 2					
Tube I		Туре 3		10.0			
		Ту	pe 4				

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For standard joints with standard tubes \emptyset 48.3 x 2.7 mm made of steel grade S460MH with integrated tube connector \emptyset 39 x 3.3 mm according to annex A, pages 1 and 129, it shall additionally be shown that proof of structural stability (eq. 1) has been provided.

$$\frac{M_{Ed}}{M_{Rd} \cdot \cos\left(\frac{\pi}{2} \cdot \frac{N_{Z,Ed}}{50,7 \ kN}\right)} \le 1$$
(Eq. 1)

Where:

$M_{ m Ed}$	Bending stress
$M_{ m Rd}$	Bending capacity according to Table 6
NZ,Ed	Tensile force load

3.2.2.3.4 Structural behaviour under compressive stress

Please refer to table 8 for the compressive stress resistance of the standard joints in accordance with the respective assembly variant.

Proof of interaction for the lap joint supporting structure model is not mandatory in case standard joints are simultaneously subject to compressive and bending stress.

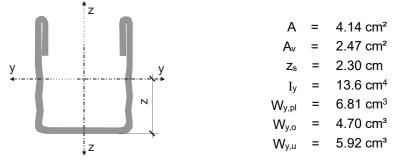
e standards
¢

Stand	ard joint	Compressive stress resistance N _{D,Rd} [kN]			Tuk	oe II	
	Tube II			Type 1	Type 2	Type 3	Type 4
			Type 1	162	113	12	29
		Tubal	Type 2	74.3		70.1	
		Tube I	Туре 3	00			
	Tube I		Type 4	82.9		80.9	

3.2.3 Cross-section properties of the U-profiles

3.2.3.1 U-profile 53 without holes

U-profile 53 without holes according to annex A, pages 3 and 6, e.g. the upper transom U48 x 52 x 2.5 of the vertical frame according to annex A, pages 1, 2, 4, 5, 129 and 130 shall be verified assuming the characteristic values in accordance with figure 2.





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3.2.3.2 U-profile 53 with holes

U-profile 53 with holes Ø 12 mm according to Annex A, pages 3 and 6, e.g. the upper transom U48 x 52 x 2.5 of the vertical frame according to Annex A, pages 1, 2, 4, 5, 129 and 130 shall be verified assuming the characteristic values in accordance with figure 3.

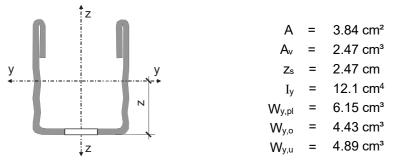


Figure 3: Characteristic values of U-profile with holes Ø 12 mm according to annex A, pages 3 and 6

U-profile 53 with holes Ø 8 mm according to Annex A, pages 3 and 6, e.g. the upper transom U48x52x2.5 of the vertical frame according to Annex A, pages 1, 2, 4, 5, 129 and 130, shall be verified assuming the characteristic values in accordance with figure 4.

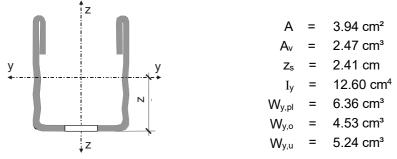


Figure 4: Characteristic values of U-profile 53 with holes Ø 8 mm according to annex A, pages 3 and 6

3.2.3.3 U-profile 60 without holes

U-profile 60 without holes according to annex A, pages 6, 71 and 141, e.g. the upper transom U49 x 60 x 3 of the different frames according to annex A, pages 69, 70, 71 and 141 shall be verified assuming the characteristic values in accordance with figure 5.

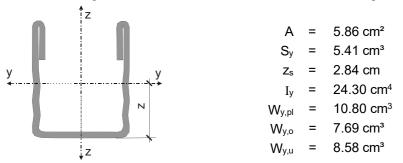
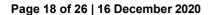


Figure 5: Characteristic values of U-profile 60 without holes according to annex A, pages 6, 71 and 141

3.2.3.4 U-profile 60 with holes

U-profile 53 with holes Ø 12 mm according to Annex A, pages 6, 71 and 141, e.g. the upper transom U49x60x3 of the vertical frame according to Annex A, pages 69, 70, 71 and 141, shall be verified assuming the characteristic values in accordance with figure 6.



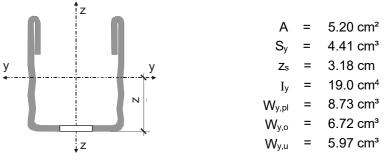


Figure 6: Characteristic values of U-profile 60 with holes \square 20 x 40 mm according to annex A, pages 6, 71 and 141

3.2.4 Vertical diagonal braces

For the vertical diagonal braces according to Annex A, pages 29 to 31, proof shall be provided that the loads shall not exceed the load-bearing capacities given in table 9.

In the overall system, the vertical diagonal braces may be taken into account as an equivalent strut which is connected in an articulated manner between the node points formed by upright tubes and decks, with an effective equivalent cross-sectional area A_{eff} in accordance with table 9 and a corresponding equivalent stiffness E • A_{eff} , as well as the connection eccentricities in accordance with figure 7.

	Table 9:	Characteristic values of the vertical of	diagonal braces
--	----------	--	-----------------

Component	Annex A, page	Bay length ℓ [m]	Stiffness E _d • A _{eff} [kN]	Resistance capacity N _{Rd} [kN]		
Diagonal brace 2.07m	29	2.07	1102	7.65		
Diagonal brace 2.57m	30	2.57	1154	6.51		
Diagonal brace 3.07m	31	3.07	1212	5.37		
when $E_{1} = (21,000,11,1) \text{ kN}/\text{cm}^{2}$						

when $E_d = (21.000 / 1.1) \text{ kN/cm}^2$

with A_D Cross-sectional area of the diagonal tube

Effective equivalent cross-sectional area

A_{eff} N_{Rd}

Load-bearing capacity of the diagonal braces

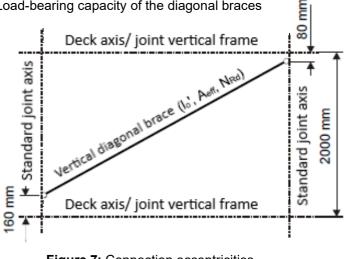


Figure 7: Connection eccentricities

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3.2.5 Longitudinal ledger

For the proof of structural stability of the overall system, the longitudinal ledgers (horizontal struts) according to Annex A, page 32 in connection with the equivalent stiffness values and load-bearing capacities given in table 10 shall be taken into consideration.

Table 10: Characteristic values of longitudinal ledgers

Scaffold bay length [m]	Load	Stiffness Ed • A _{eff} [kN]	Resistance capacity N _{Rd} [kN]			
1 - 2 07	Compression	2620	6.94			
<i>ℓ</i> = 3.07	Tension	5950	18.2			
<i>l</i> = 2.57	Compression	3180	9.54			
1 - 2.57	Tension	5090	18.2			
	Compression	3360	13.3			
<i>ℓ</i> = 2.07	Tension	4190	18.2			
R = 4 EZ	Compression	3010	18.2			
<i>ℓ</i> = 1.57	Tension	3260	18.2			
when $E_d = (21.000 / 1.1) \text{ kN/cm}^2$						

3.2.6 Diagonal cross brace

For the proof of structural stability of the overall system, the diagonal cross braces according to Annex A, page 103 in connection with the equivalent stiffness values and load-bearing capacities given in table 11 shall be taken into consideration.

Scaffold width	System length	Load	Stiffness	Resistance capacity	
[m]	[m]		Ed • Aeff [kN]	N _{Rd} [kN]	
0.732	1.05	Compression	2730	10.2	
1.088	1.95	Tension	2890	10.2	
0.732	1 77	Compression	2570	10.2	
1.088	1.77	Tension	2670	10.2	
when E _d = (21.000 / 1.1) kN/cm ²					

Table 11: Characteristic values of the diagonal cross brace

3.2.7 Vertical load-bearing capacity of decks

The decks of the "ALFIX 70 frame scaffold" are verified in accordance with table 12 for live loads of the scaffold load classes / service classes according to DIN EN 12811-1:2004-03, table 3 and for use in protection scaffolds and roof edge protection scaffolds with fall heights of up to 2 m (top fall arresting layer of class FL 1) according to DIN 4420-1:2004-03 (class D according to DIN EN 12810-1:2004-03).

Table 12: Assignment of decks to scaffe	old load classes (service classes)
---	------------------------------------

Designation	Annex A, page	Bay length ℓ [m]	Use in load class (service class)
		≤ 2.07	≤ 6
Steel deck AF	7.0	2.57	≤ 5
Sleer deck AF	7, 9	3.07	≤ 4
		4.14	≤ 3
Steel deck	8	≤ 3.07	≤ 4

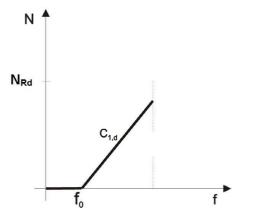
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Designation	Annex A, page	Bay length ℓ [m]	Use in load class (service class)
		≤ 2.07	≤ 6
		2.57	≤ 5
Intermediate deck AF 0.19m		3.07	≤ 4
	10	4.14	≤ 3
	1 [≤ 2.07	≤ 6
Intermediate deck AF 0.16m		2.57	≤ 5
		3.07	≤ 4
		≤ 2.07	≤ 6
Intermediate deck	11	2.57	≤ 5
		3.07	≤ 4
Aluminium deck with plywood	12, 13, 19, 20	≤ 3.07	≤ 3
Aluminium access deck with ladder	15, 16, 22, 23	≤ 3.07	≤ 3
Aluminium corner deck with toeboard, rigid	25		≤ 3
		≤ 1.57	≤ 5
Solid wood deck 45	26	2.07	≤ 4
		2.57	≤ 3
		≤ 1.57	≤ 6
Solid wood deck 48		2.07	≤ 5
	27 -	2.57	≤ 4
		3.07	≤ 3
		1.57	≤ 6
		2.07	≤ 5
Wooden deck	28 -	2.57	≤ 4
		3.07	≤ 3
		≤ 2.07	≤ 6
Concerver	70	2.57	≤ 5
Gap cover	73 -	3.07	≤ 4
	Γ	4.14	≤ 3
		≤ 1.57	≤ 4
Steel plank 0.30m	75	2.07	≤ 3
		2.57	≤ 3
EIFS deck AF 190	106	≤ 4.14	≤ 3
Aluminium frame platform with plywood	110, 111	≤ 3.07	≤ 3
Aluminium frame platform with internal hatch	113, 114	≤ 3.07	≤ 3
Aluminium deck 0.60m lightwoight	116	≤ 2.57	≤ 4
Aluminium deck 0.60m, lightweight		3.07	≤ 3
Aluminium trapdoor deck with aluminium chequer plate	117, 118, 120	≤ 3.07	≤ 3

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3.2.8 Elastic support of the vertical frame sections

Non-anchored nodes of vertical frame sections on the frame level (in case of facade scaffolding perpendicular to the facade) may be assumed to be elastically supported by the horizontal level (decking), provided that the neighbouring horizontal nodes are anchored. This elastic support can be taken into account for scaffolds of load classes / service classes \leq 3 depending on the points of contraflexure by assuming a trilinear travel spring in accordance with figure 9, having the design values given in tables 13.1 to 13.3.



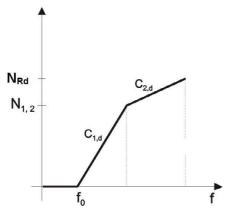


Figure 8: Bilinear spring characteristics

Figure 9: Trilinear spring characteristics

					Stiffne: [kN/c		
Deck	Annex A, page Bay length { [m]		Number of decks per scaffold bay	Clearance $f_{o\perp}$ [cm]	0 < N⊥ ≤ 1.82 kN	$1.82 kN < N_{\perp} \leq N_{\perp,Rd}$	Spring force $N_{\perp,Rd}$ [kN]
Steel deck AF	7	3.07	2	4.7	0.62	0.20	
Steel deck	8	3.07	2	4.7	0.62	0.20	2.73
Sleer deck	0	≤ 2.57	2	3.8	0.69	0.27	
Aluminium deck	12, 13,	3.07	1	2.0	0.38	0.26	1.86
with plywood	19, 20	≤ 2.57	I	2.2	0.65	0.34	1.00
Wooden deck	28	≤ 2.57	2	3.3	0.51	0.31	2.35
Aluminium frame	110, 111	3.07	1	2.0	0.38	0.26	1.86
platform with plywood	110, 111	≤ 2.57		2.2	0.65	0.34	1.00

Table 13.1: Design values for the horizontal travel spring (N_{1,2} = 1.82 kN)

Table 13.2: Design va	Design values of the horizontal travel springs ($N_{1,2}$ = 2.00 kN)						
					Stiffne: [kN/d	,	
Deck	Annex A, page	Bay length { [m]	Number of decks per scaffold bay	Clearance $f_{lpha ar L}$ [cm]	0 < N⊥ ≤ 2.00 kN	$2.0 kN < N_\perp \leq N_{\perp,Rd}$	Spring force $M_{\perp,Rd}$ [kN]
Solid wood deck 48	26	≤ 3.07	0	2.0	0.44	0.00	0.05
Solid wood deck 45	27	≤ 2.57	2	3.9	0.41	0.22	2.35

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Tabelle 13.3: Design values of the horizontal travel springs ($N_{1,2} = 1.50 \text{ kN}$)

					Stiffnes [kN/c	5	
Deck	Annex A, page	Bay length <i>【</i> [m]	Number of decks per scaffold bay	Clearance $f_{o \perp}$ [cm]	$0 < N_\perp \leq 1,50 kN$	$1.50kN < N_\perp \leq N_{\perp,Rd}$	Spring force $N_{\perp,Rd}$ [kN]
Aluminium deck 0.60m, lightweight	116	≤ 3.07	1	4.7	0.69	0.20	2.08

3.2.9 Elastic coupling of the vertical levels

The inner and outer vertical level of a scaffold may be assumed to be elastically coupled to each other by the decking in the direction of these levels (in the case of facade scaffolding parallel to the facade). This elastic coupling can be taken into account for scaffolds of load classes / service classes ≤ 3 depending on the spring characteristics by assuming a bi-, trior multilinear coupling spring in accordance with figure 8 or figure 9, having the design values given in tables 14.1 to 14.3.

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<u> Table 14.1:</u>	Design values of the horizontal coupler springs per scaffold bay
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					Stiffne	ess _{C//,d} [(N/cm]	
Deck	Annex A, page	Bay length $\ell \;\;$ [m]	Number of decks per scaffold bay	Clearance $f_{\it oll}[{\sf cm}]$	$0 < F_{ } \le 1.14 kN$	1.14 < F∥≤2.27 kN	$2.27 < F_{ } \leq F_{ ,kd}$	Spring force $F_{j/Rd}$ [kN]
Steel deck AF	7	≤ 3.07	2	1.0	2.22	2.37	1.25	4.55
Steel deck	8	≤ 3.07	2	1.0	2.22	2.37	1.25	4.55
Aluminium deck with plywood	12, 13, 19, 20	≤ 3.07	1	0.3	2.20	2.22	0.94	3.94
Wooden deck	20	3.07*)	2	1.0	1.99	1.95	1.22	4.55
	28	≤ 2.57		1.0	1.67	1.63	1.02	3.83
Aluminium frame platform with plywood	110, 111	≤ 3.07	1	0.3	2.20	2.22	0.94	3.94
*) Only in case of anchor	age patter	m ≤ 4 m (l	Figure	1, type b	o in DIN E	N 1281)-1:2004-	03)

Table 14.2: Design values of the horizontal coupler springs per scaffold bay

				Ē	Stiffnes [kN/cr	[kN]	
Deck	Annex A, page	Bay length $\ell \;\; [m]$	Number of decks per scaffold bay	Clearance $f_{olt}[{ m cm}]$	$0 < F_{ } \leq 3.0 kN$	$3.0 < N_{ll} \leq F_{l1,Rd}$	Spring force $F_{l/,Rd}$ [kN]
Solid wood deck 48	26	≤ 3.07	2	0.90	2.31	1.38	4.58
Solid wood deck 45	27	≤ 2.57	Z	0.90	1.93	1.16	3.83

Table 14.3:Design values of the horizontal coupler springs per scaffold bay

		Ŀ	ß	[m:	Stiffness <i>c_{//,d}</i> [kN/cm]	$F_{/\!/_{Rd}}$ [kN]
Deck	Annex A, page	Bay length { [m]	Number of decks per scaffold bay	Clearance f_{oll} [cm]	$0 < N_{ll} \leq F_{l1,Rd}$	Spring force $F_{ll,R}$
Aluminium deck 0.60m, lightweight	116	≤ 3.07	1	0.35	3.41	3.82

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3.2.10 Summary of the horizontal travel and coupling springs for "all deck types"

The stiffness relationships given in tables 15.1 and 15.2 for "all deck types" represent the minimum value for all decks listed in tables 13.1 to 14.3. These values provide a secure basis for the structural analysis. The values given in tables 15.1 and 15.2 apply to all load (service) classes \leq 3.

	Table 15.1:	Design values of the horizontal travel springs
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Deck	Bay length ℓ [m]	Clearance $f_{o\perp}$ [cm]	Stiffness	[kN/cm]	<i>N⊥1,2</i> [kN]	<i>N⊥,_{Rd}</i> [kN]
Deck			C ⊥1,d	C ⊥2,d		
all decks	≤ 3.07	4.7	0.62	0.41	1.00	1.86

Table 15.2: Design values of the horizontal coupler springs per scaffold bay

Deck	Bay length	Clearance	Stiffness [kN/cm]			<i>N</i> ⊥1,2	<i>N⊥2,3</i>	<i>N⊥,Rd</i>
	ℓ [m]	<i>f₀⊥</i> [cm]	C 111,d	C 112,d	C 113,d	[kN]	[kN]	[kN]
all decks	≤ 3.07	1.00	1.67	1.63	1.00	1.14	2.27	3.83

3.2.11 Material parameters

For components made of S235JR/S235JRH steel with an increased nominal yield strength ($R_{eH} \ge 280 \text{ N/mm}^2$ or $R_{eH} \ge 320 \text{ N/mm}^2$) - any such components are marked accordingly in the drawings of annex A - the design value of the nominal yield strength $f_{y,d}$ = 254 N/mm² or $f_{y,d}$ = 291 N/mm² may be used for the calculation. All other parameters are to be applied according to the basic material.

3.2.12 Scaffolding spindles / base jacks

The substitute section properties for the base jacks for the stress and / or interaction analyses and calculations of deformation according to DIN 4425:2017-04 (see also annex B of DIN EN 12811-1:2004-03) shall be assumed as follows:

- Scaffolding spindles (base jacks) according to annex A, pages 35 and 108:

$A = A_S$	=	$3.52cm^2$
Ι	=	4.00 cm ⁴
Wel	=	2.68 cm ³
W_{pl}	=	$1.25 \bullet 2.68 = 3.35 \text{ cm}^3$

- Scaffolding spindles (base jacks) according to annex A, pages 121 and 122:

$A = A_S$	=	$3.85 cm^2$
Ι	=	4.27 cm ⁴
Wel	=	2.83 cm ³
W_{pl}	=	$1.25 \bullet 2.83 = 3.54 \text{ cm}^3$

The cosine interaction in accordance with DIN 4420- 1:1990-12, table 7 may be used to provide proof of stability of the load-bearing capacity of the scaffolding spindles / base jacks.

3.2.13 Couplers

For the verification of the halfcouplers attached to the various components, the load-bearing capacities and stiffnesses shall be applied in accordance with annex A and the data provided in DIN EN 74-2:2009-01.

Notwithstanding DIN EN 74-2:2009-01, a load-bearing capacity of the breaking load of $F_{t,Rd} = 27.3 \, kN$ may be assumed for the proof of structural stability.

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3.3 **Provisions for execution**

3.3.1 General provisions

The assembly, alteration and dismantling of the scaffold must be carried out in compliance with the Instructions for Assembly and Use⁶ and is not the subject of this decision.

3.3.2 Condition of components

All components shall be inspected for proper condition prior to assembly. Damaged components may not be used.

3.3.3 Structural design

3.3.3.1 General provisions

Scaffolds in accordance with this decision shall be erected using the components listed in section 1. Only use components that have been marked in accordance with the provisions of this decision.

The wedges of the connector heads and the wedges of the guardrail wedge housings are to be fixed by driving the wedge from top to bottom to the end-stop with a 0.5 kg hammer (or heavier).

3.3.3.2 Base area

The lower vertical frames (scaffold frames) must be placed on scaffolding spindles / base jacks and aligned in such a way that the working areas are horizontal. It must be ensured that the base plates of the base jacks are horizontal and supported over the entire area to absorb and transmit the forces resulting from the scaffolding in the supporting surface.

3.3.3.3 Height equalisation

The vertical frames 1.0 m and 0.666 m according to annex A, pages 2, 5 and 130 may be used as adjustment frames for height equalisation. Do not work on working areas directly below these frames.

3.3.3.4 Scaffolding decks

Scaffolding decks must be secured to prevent them from accidental lift-off.

3.3.3.5 Side protection

The provisions of DIN EN 12811-1:2004-03 apply to the side protection. Primarily use components intended for this use and only exceptionally use components such as steel tubes and couplers according to DIN EN 12811-1:2004-03 as well as scaffold decks and planks according to DIN 4420-1:2004-03. Toeboards are mandatory in add-on (external) stairway accesses.

The TRBS-Guardrails according to annex A, pages 143 to 145 are connected to the standards by driving the wedges to their end-stop with a 0.5 kg hammer (or heavier).

3.3.3.6 Bracing

Scaffolds must be braced.

For facade scaffolds, the outer vertical level is to be braced parallel to the facade by means of vertical diagonal braces according to section 3.2.4 fitted spaced along the scaffolding or one above another. The required number of diagonal braces is determined by means of the structural analysis. However, at least 1 diagonal shall be fitted per 5 scaffolding bays. Longitudinal ledgers according to section 3.2.5 shall be fitted in at least in the bays to which a diagonal brace is fitted at the height of the base jacks.

Decks or horizontal struts are to be fitted continuously on all scaffold (working) levels to provide horizontal bracing. The characteristic values of the deck levels in accordance with sections 3.2.8 to 3.2.10 and of the horizontal struts in accordance with section 3.2.5 may be assumed for the design calculations.

⁶ The Instructions for Assembly and Use must comply with the requirements of the "Application guideline for working scaffolds according to DIN EN 12811-1", see DIBt-Mitteilungen (notifications of the DIBt) issue 2/2006.

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The EIFS deck AF 190 in accordance with Annex A, page 106 must not be used as a bracing component. Only use these decks in connection with inner brackets.

The steel decks AF 0.30 m, 0.34 m according to Annex A, page 9 is a compensation deck (to compensate for different deck widths) and must not be used as a bracing component.

3.3.3.7 Anchoring

Please refer to the structural analysis for anchor forces and the anchorage pattern.

The anchorage of scaffold ties to the facade or to other parts of the building is not covered by this decision. The user must ensure that the respective forces can be securely absorbed and transmitted from scaffold ties. Vertical forces must not be transferred in this process.

3.3.3.8 Couplers

Couplers with screwed connectors must be tightened with a torque of 50 Nm when connecting to the standards; tolerances of ± 10 % are permitted. According to the manufacturer's instructions for use, bolts/screws must be easy to reposition.

When connecting couplers with wedge-lock to the standards, they must be connected by driving the wedge to the end-stop with a 0.5 kg hammer (or heavier).

3.3.3.9 Protective wall post

Protective wall posts in accordance with Annex A, pages 56, 57, 102 and 104 shall be secured by means of a gravity pin in all standards.

3.3.3.10 Lift-off preventer against uplifting forces

To secure against uplifting forces in accordance with the structural analysis, the standard joints shall be installed in accordance with the Instructions for Assembly and Use.

3.3.4 Attestation of conformity

The building contractor shall submit a declaration of conformity in accordance with §§ 16 a (5) in conjunction with Section 21 (2) Model Building Regulation (MBO) in order to confirm the conformity of the erected working and service scaffold with the General Construction Technique Permit (aBG) covered by this decision.

4 Provisions for use, maintenance and inspection

4.1 General provisions

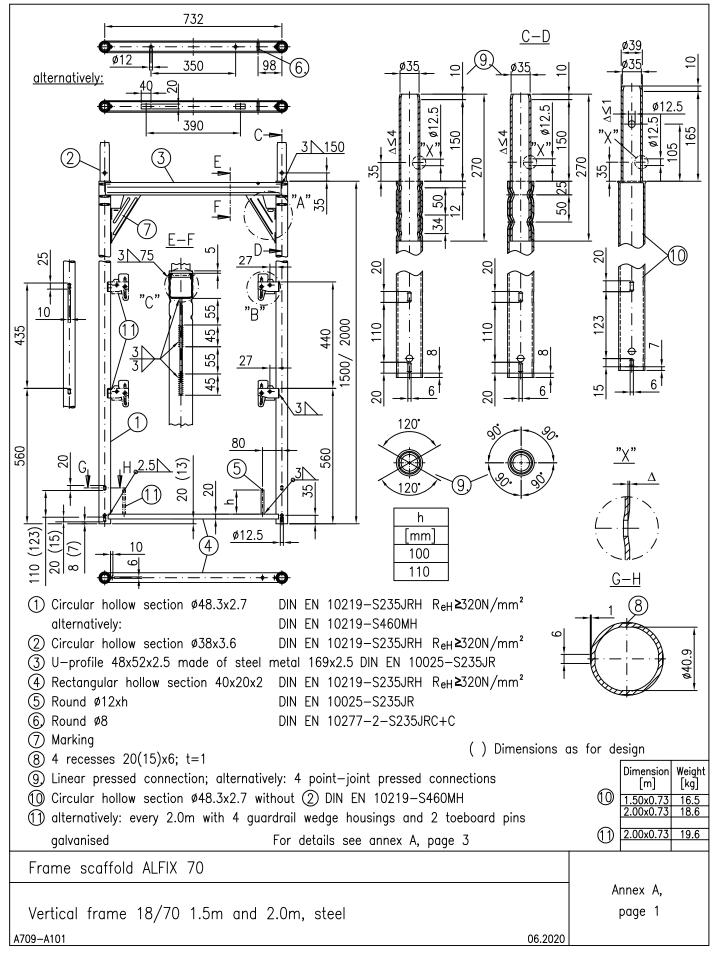
The use of the scaffolding is not covered by this decision...

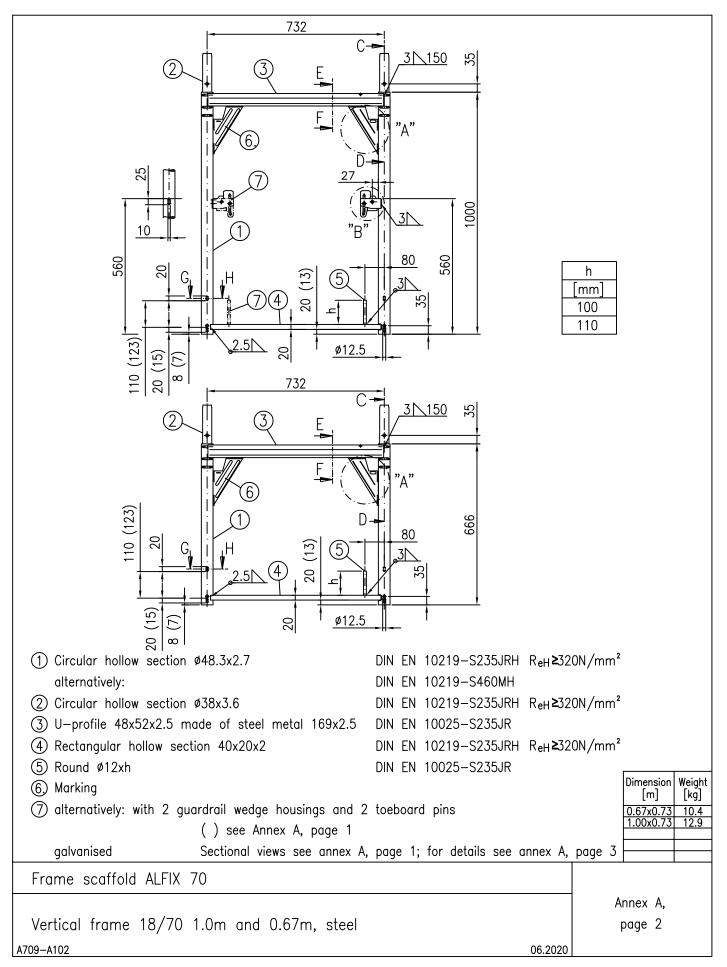
4.2 Wooden scaffolding components

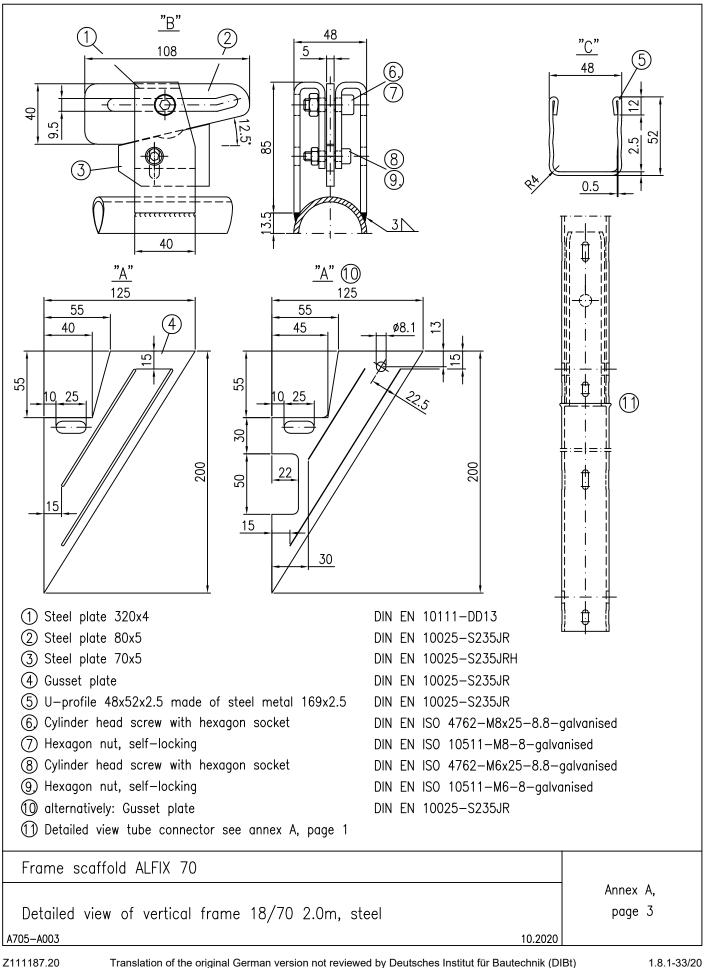
In order to prevent damage caused by moisture to wooden scaffolding components, they must be stored in a dry place, off the ground, and providing adequate ventilation.

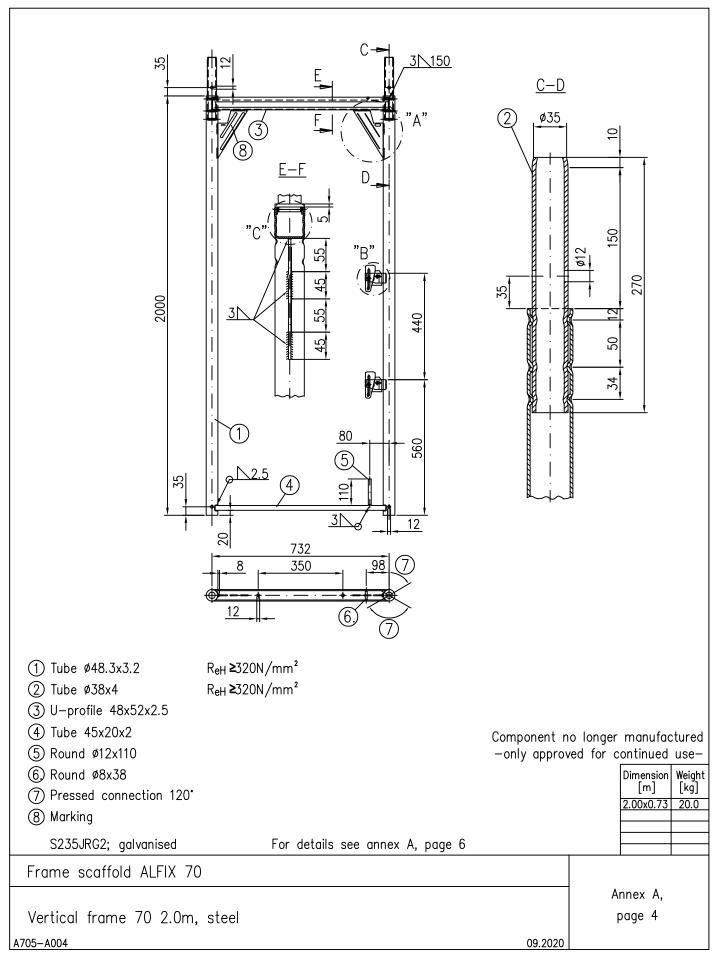
Head of Division

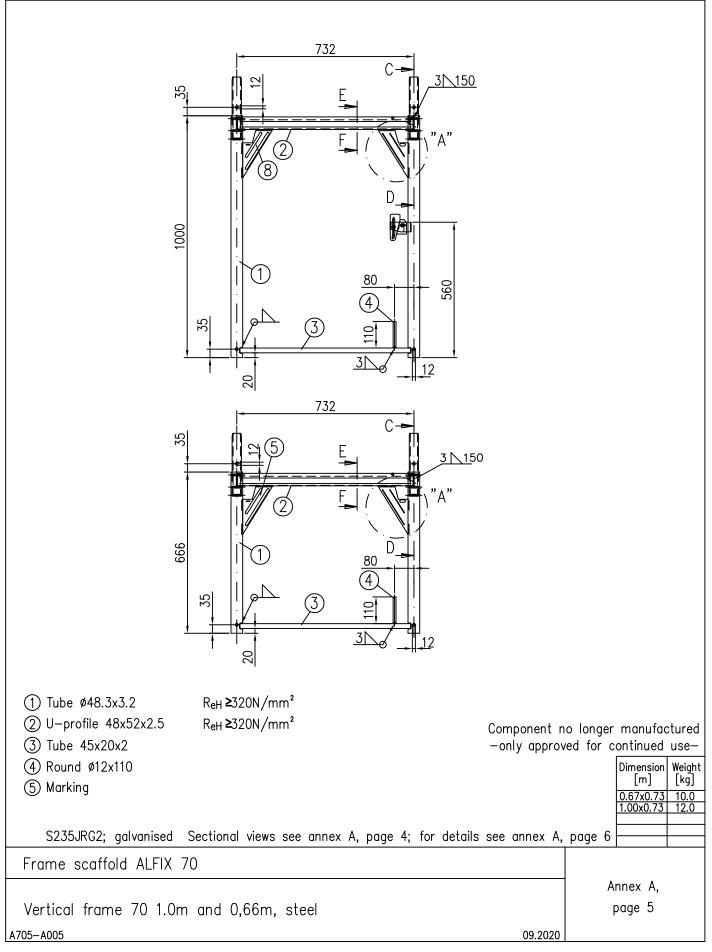
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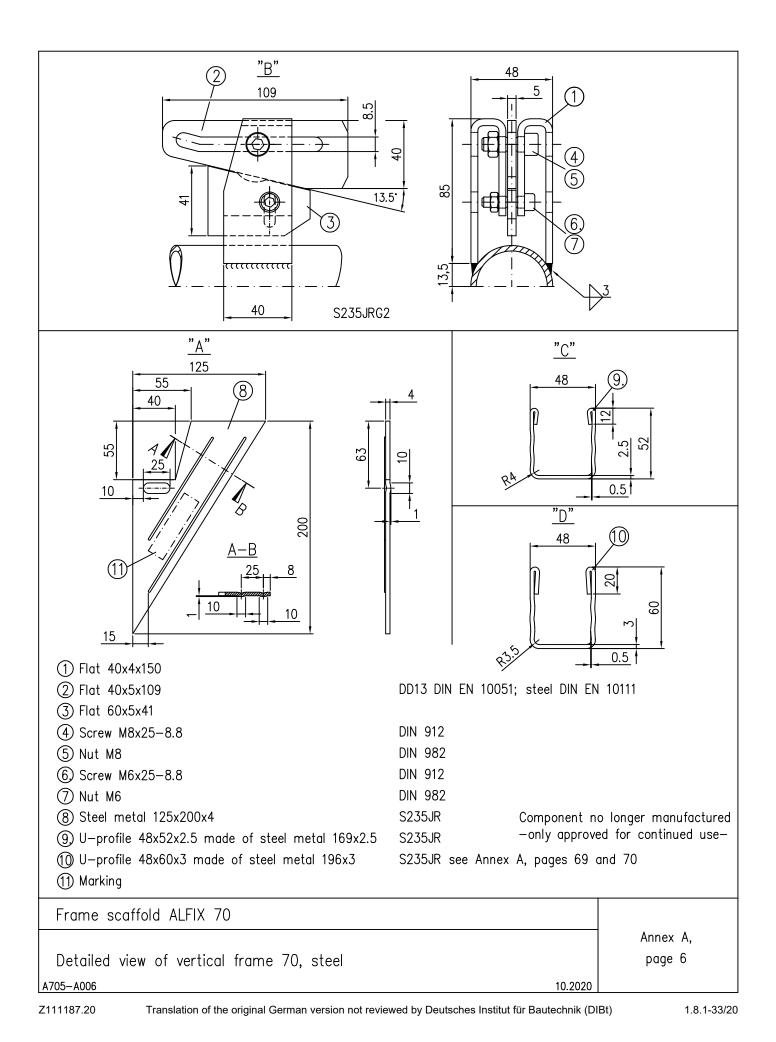


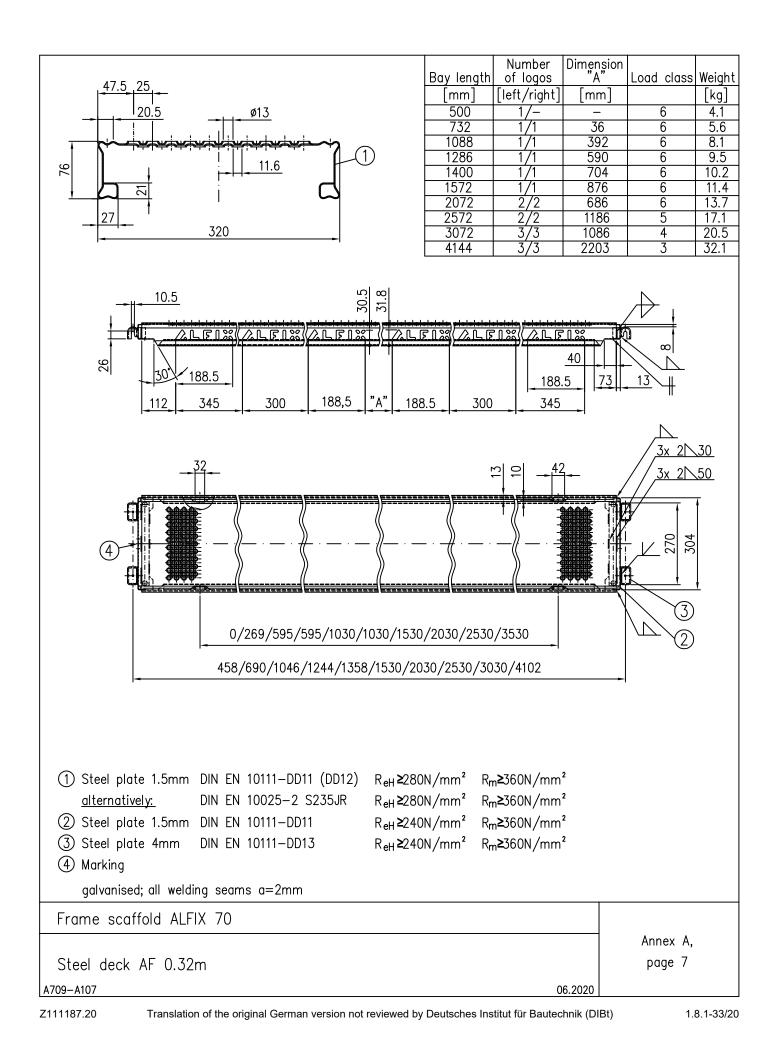


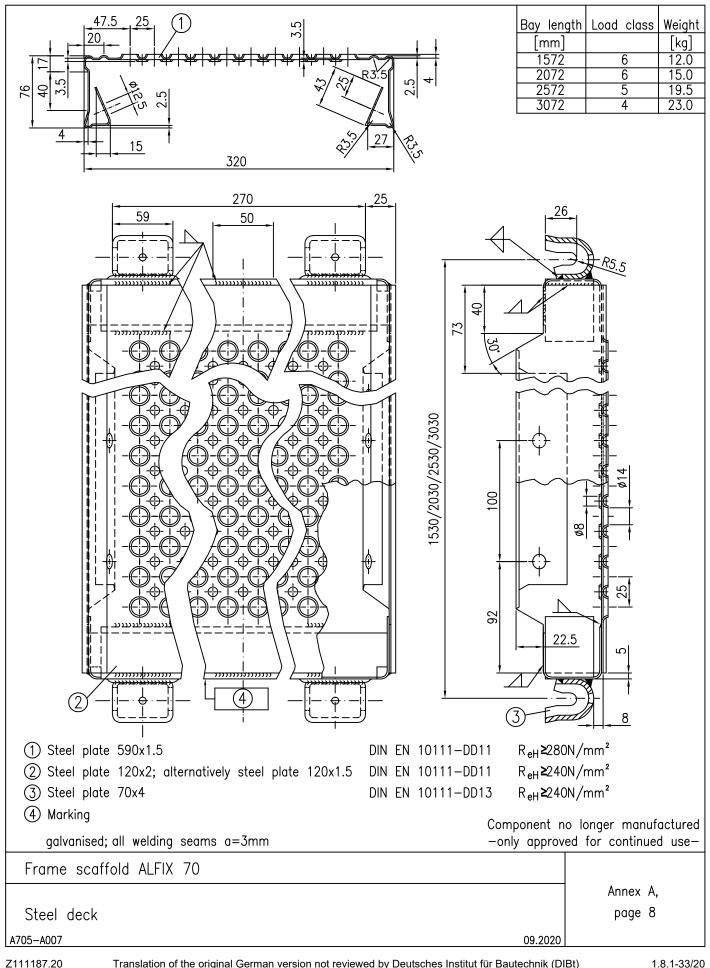


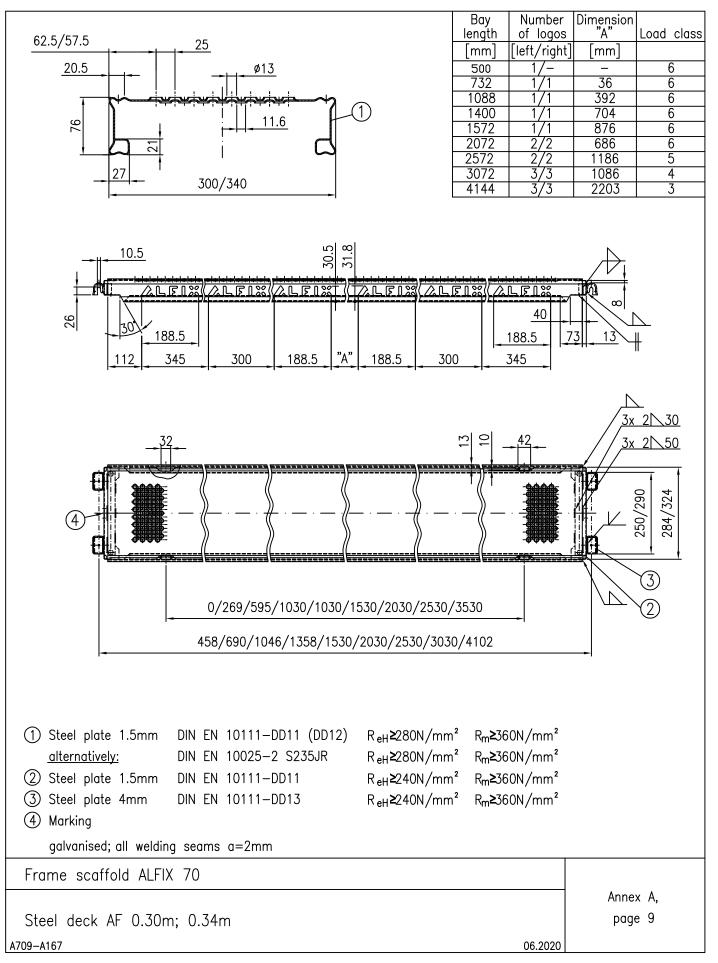




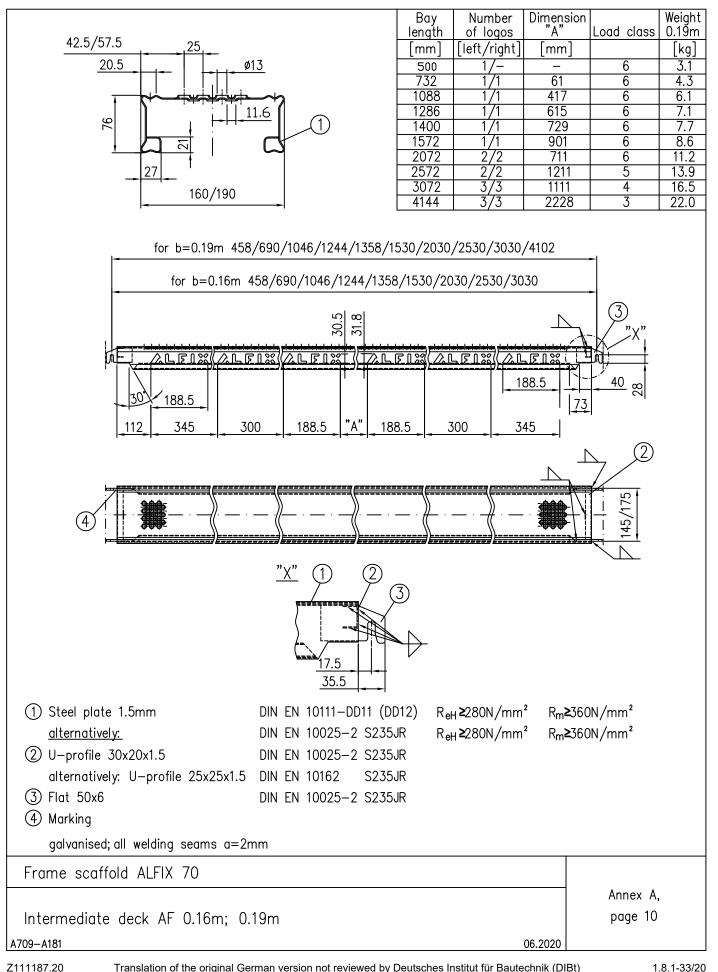


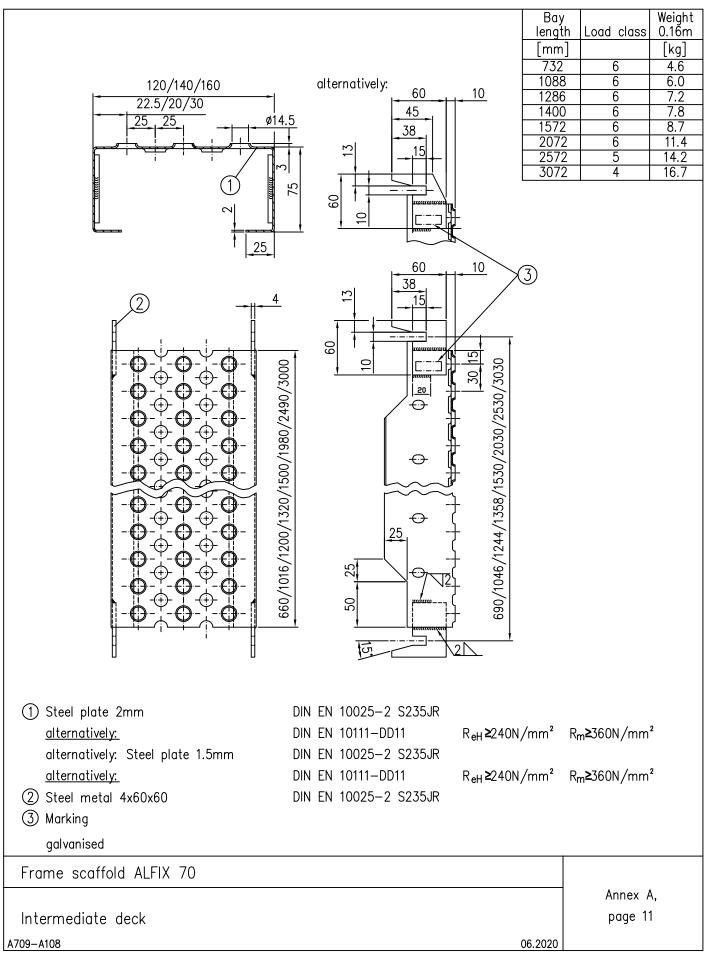


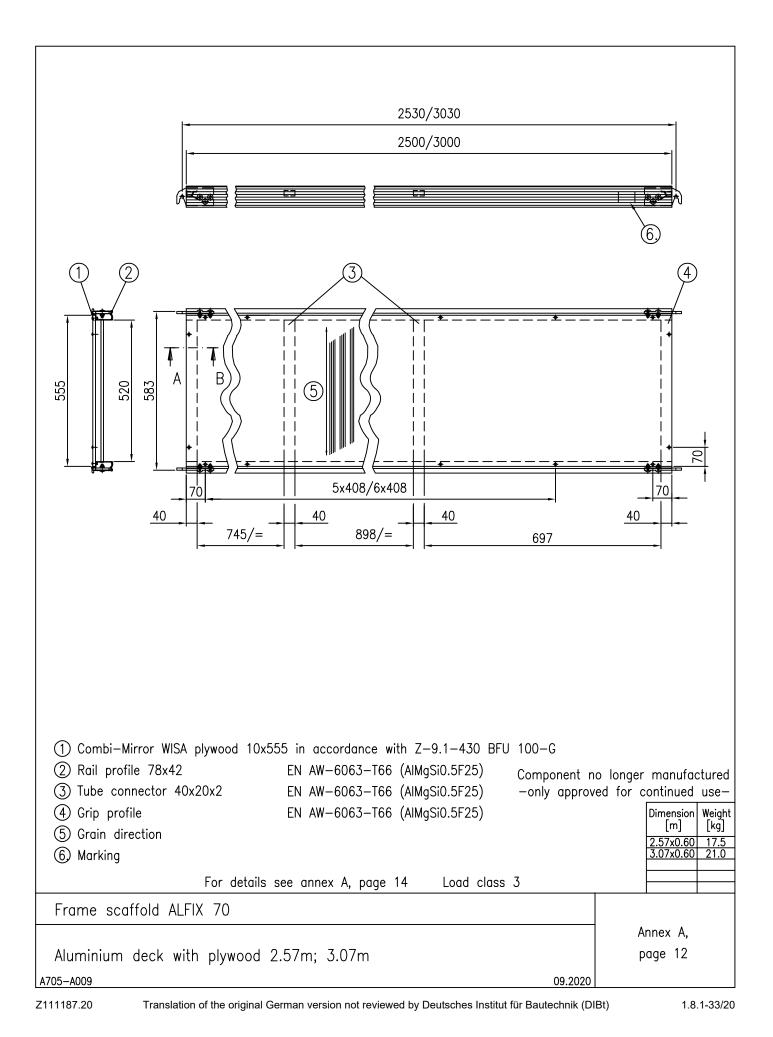


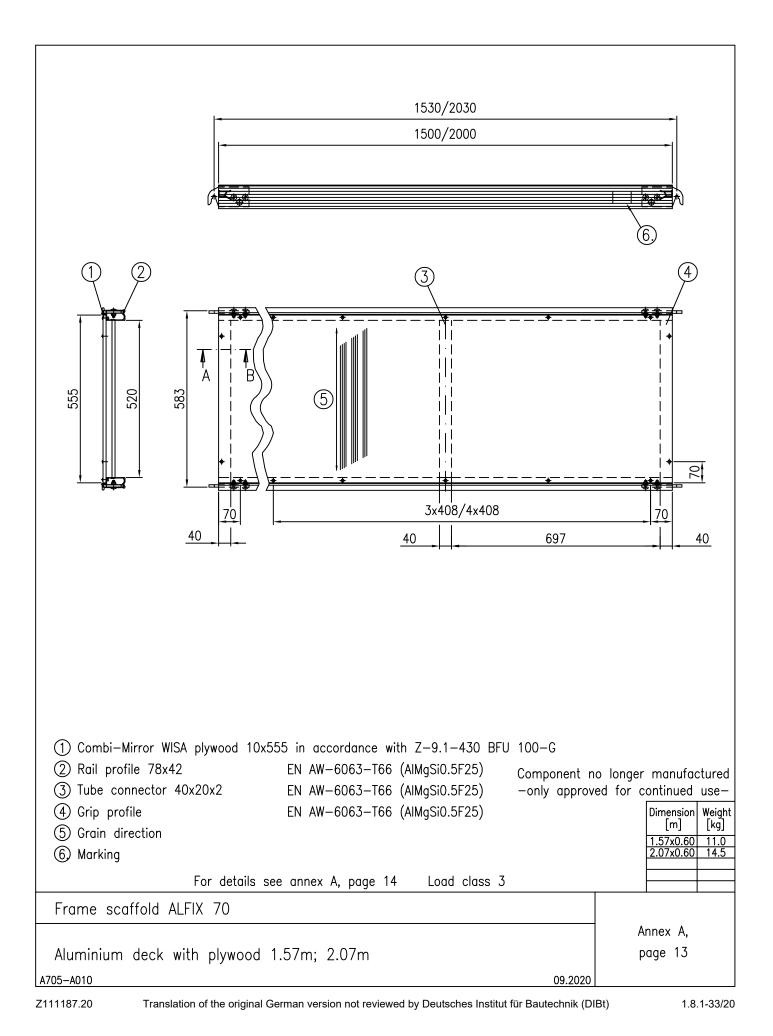


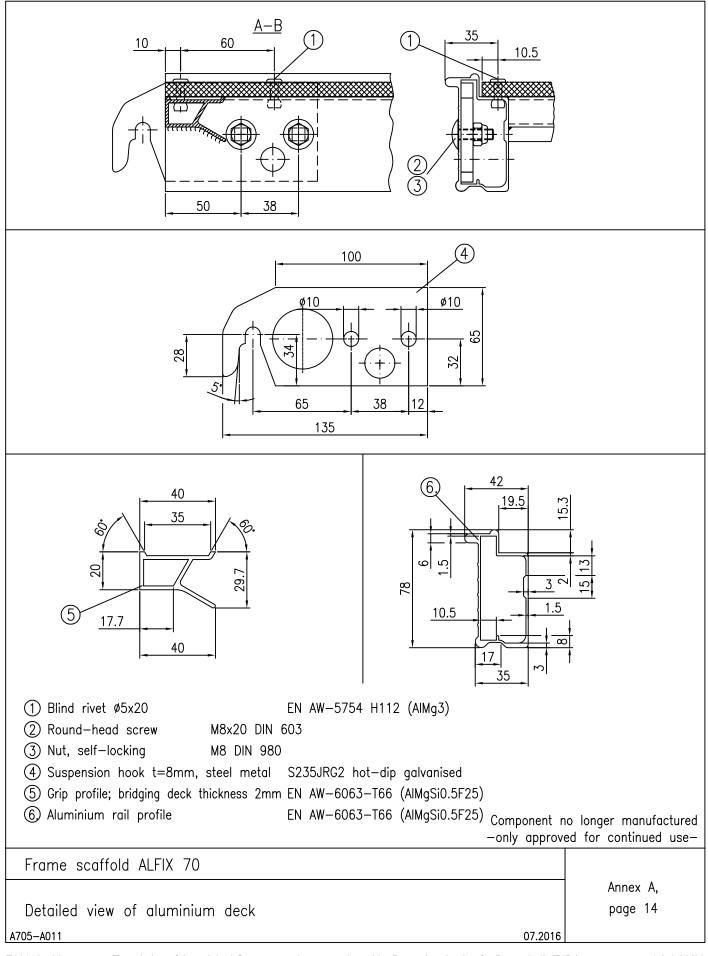
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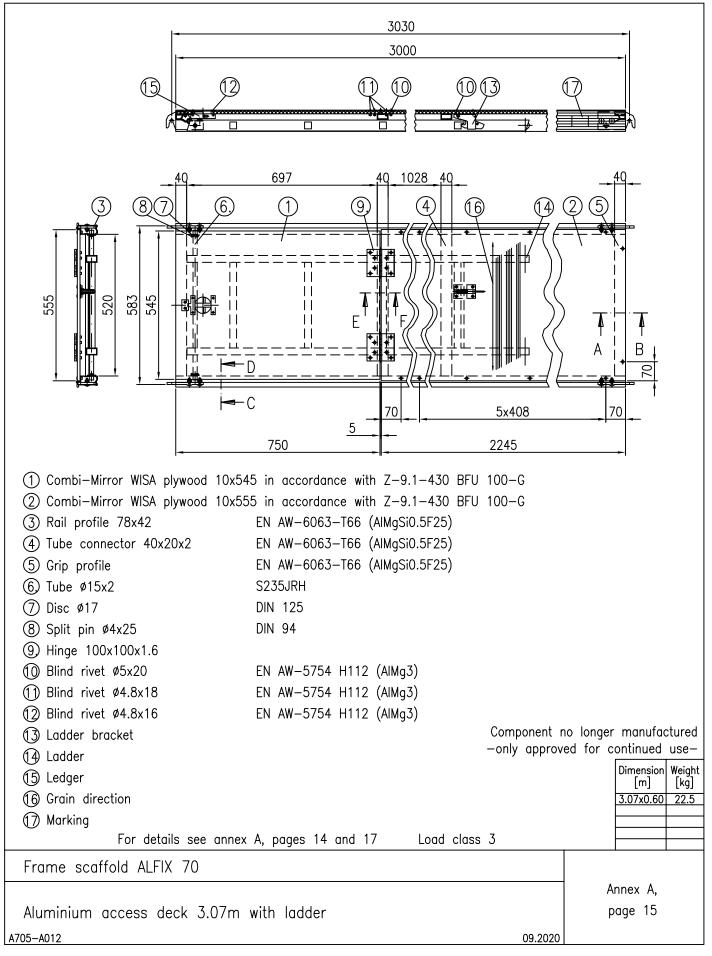


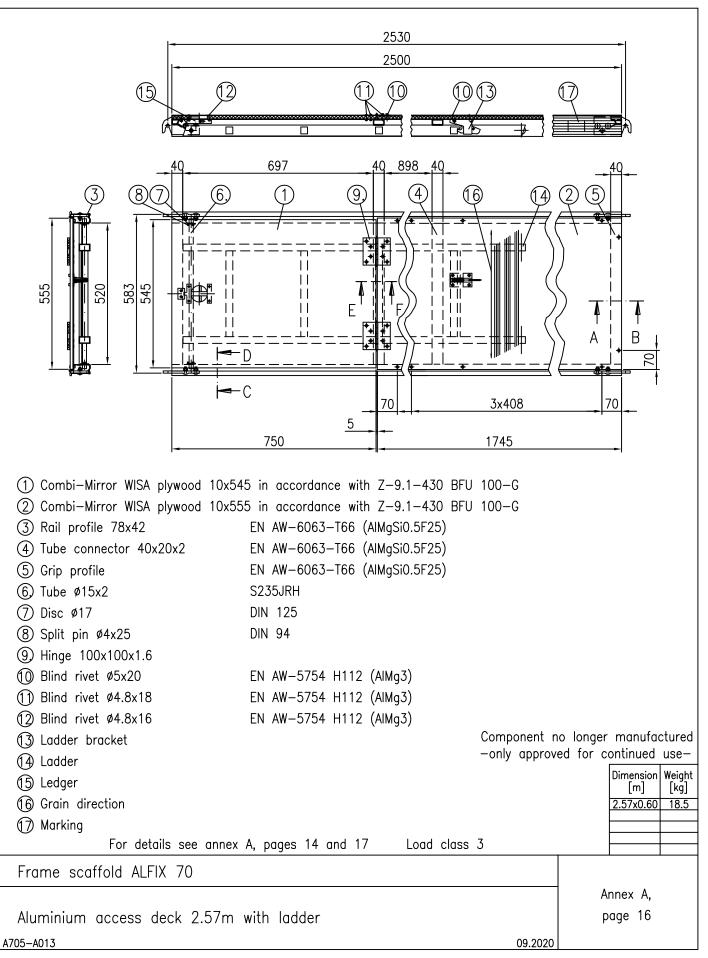


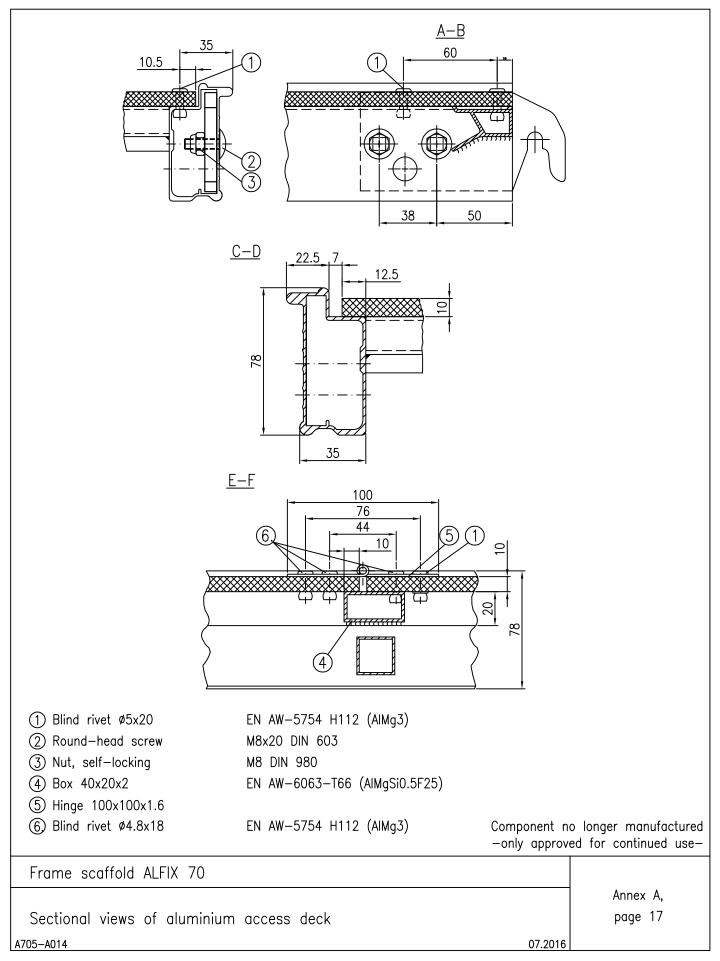


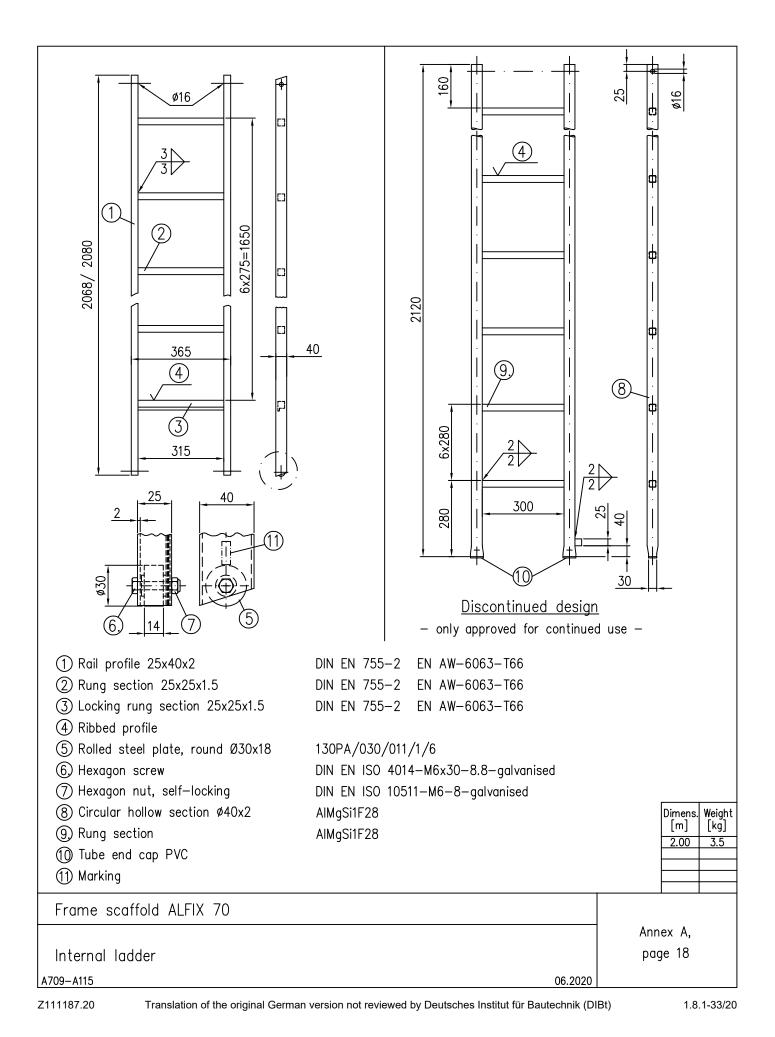


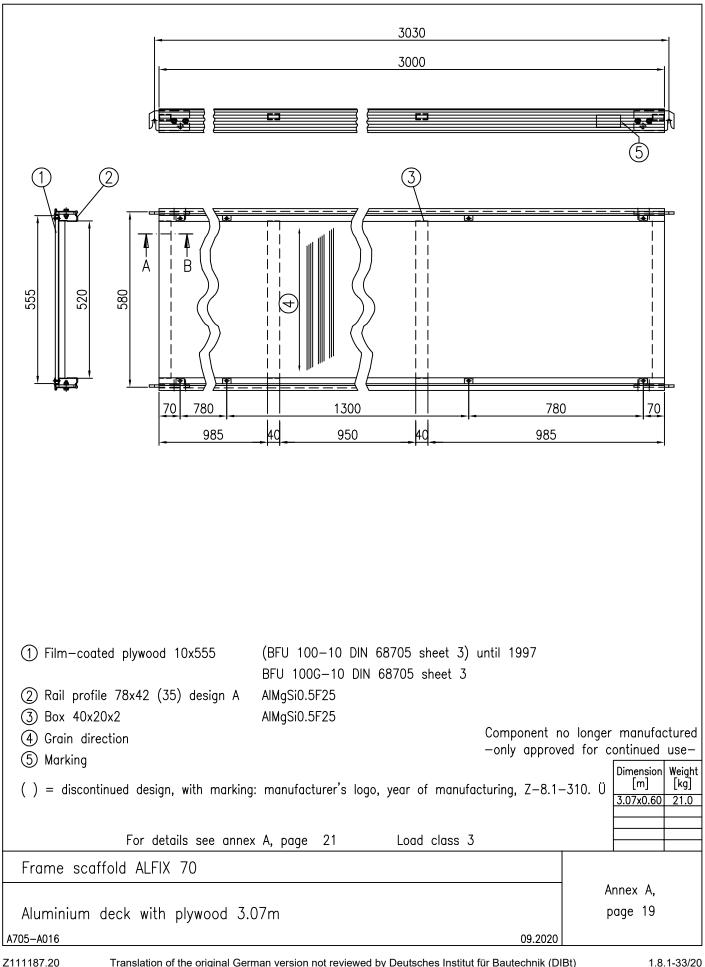


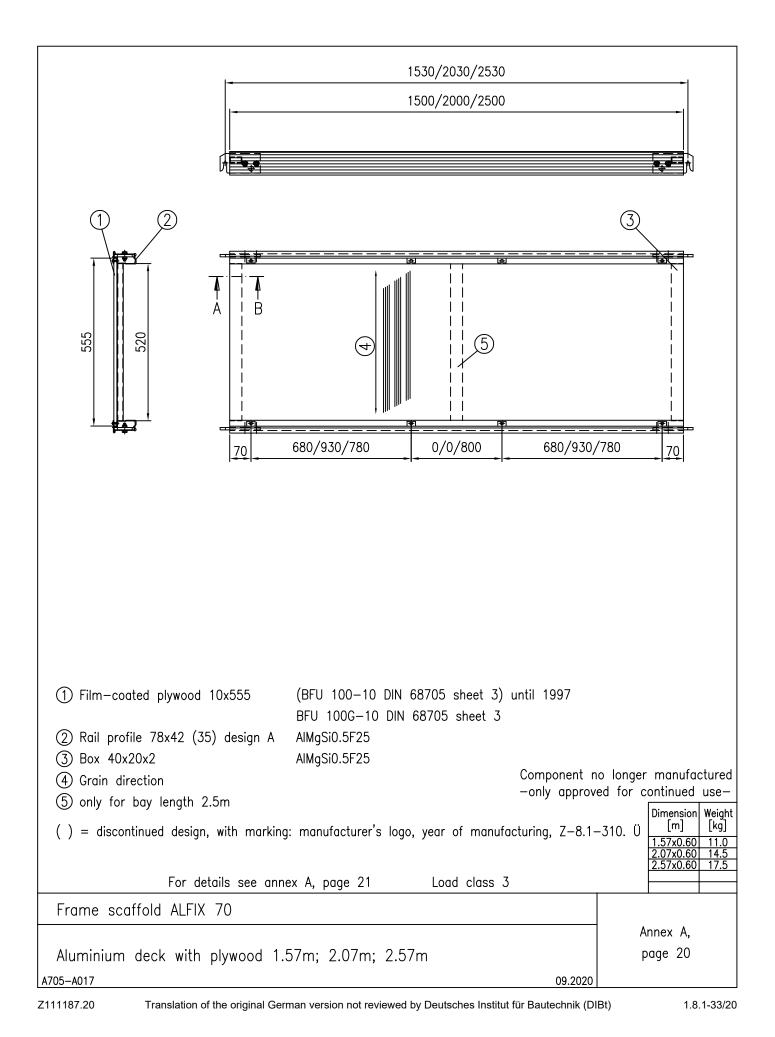


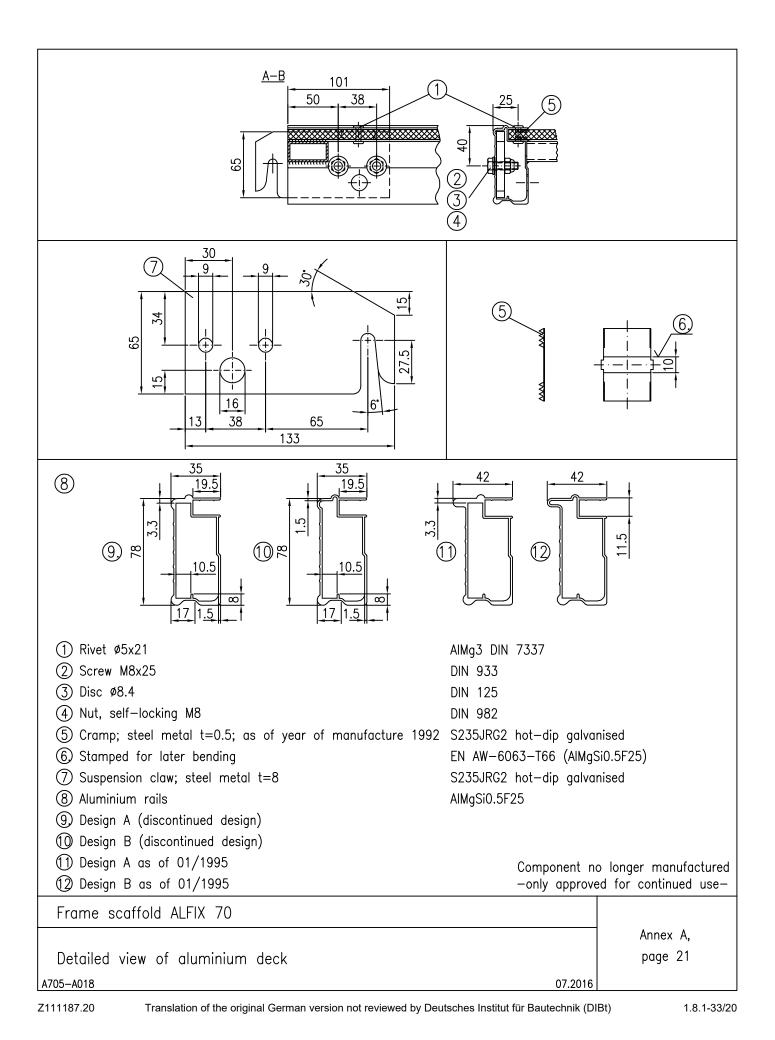


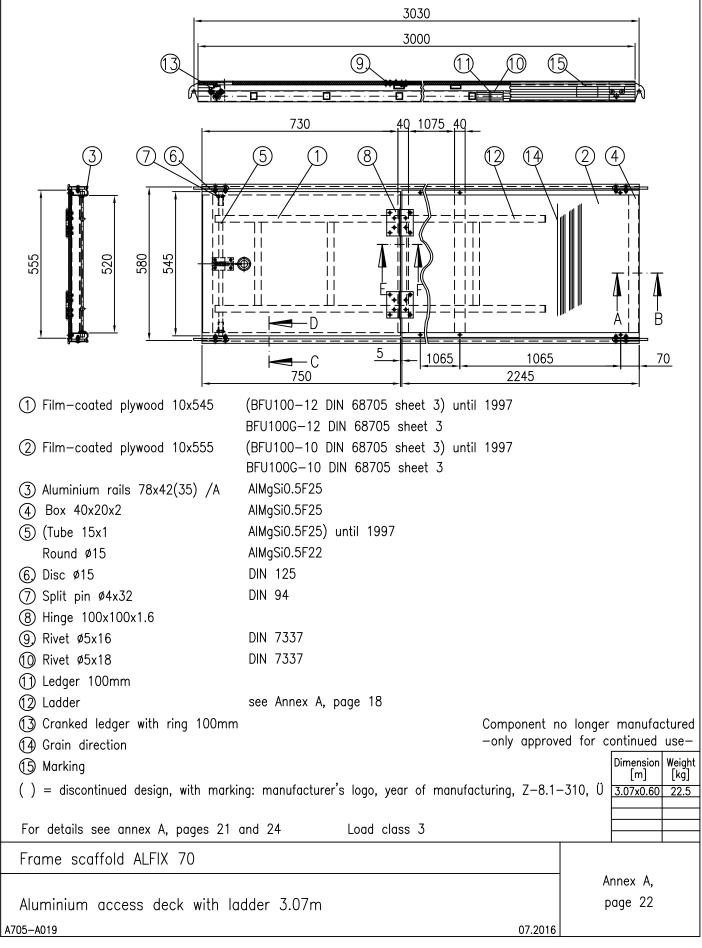


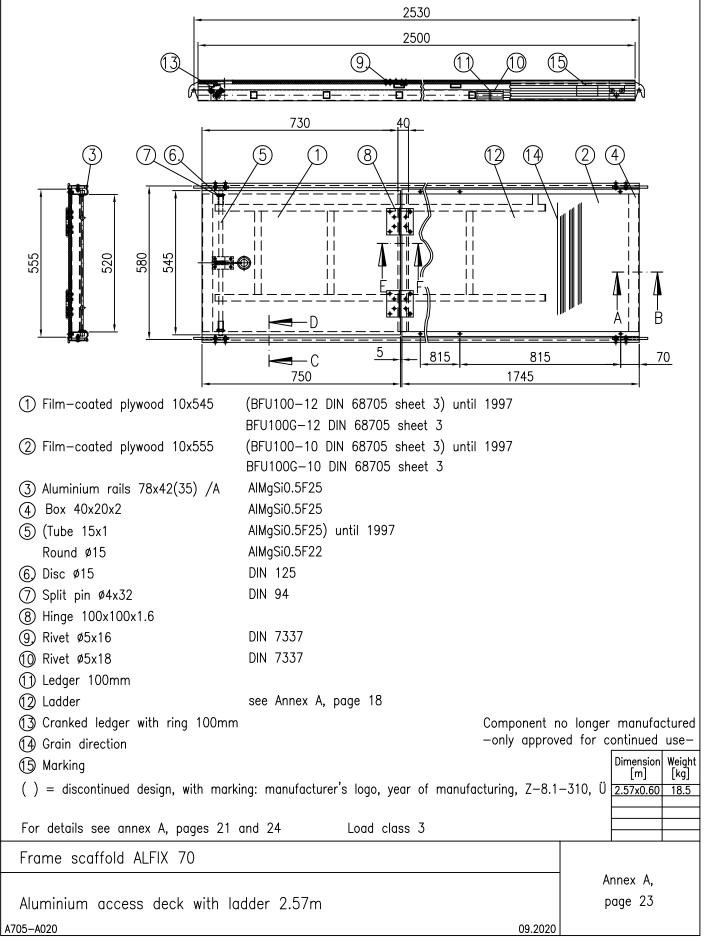


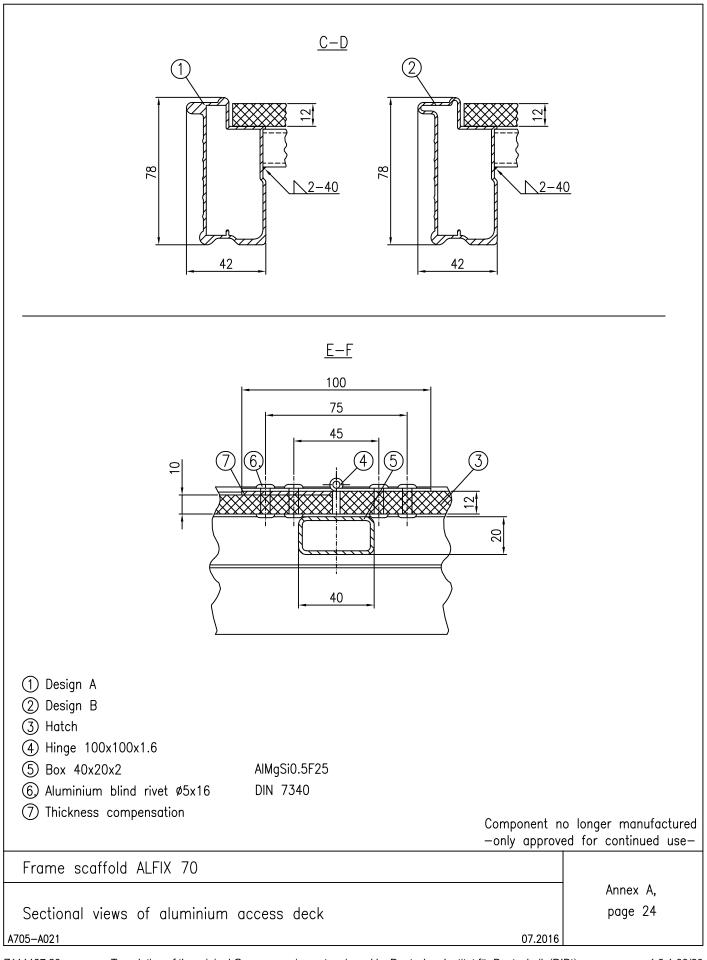


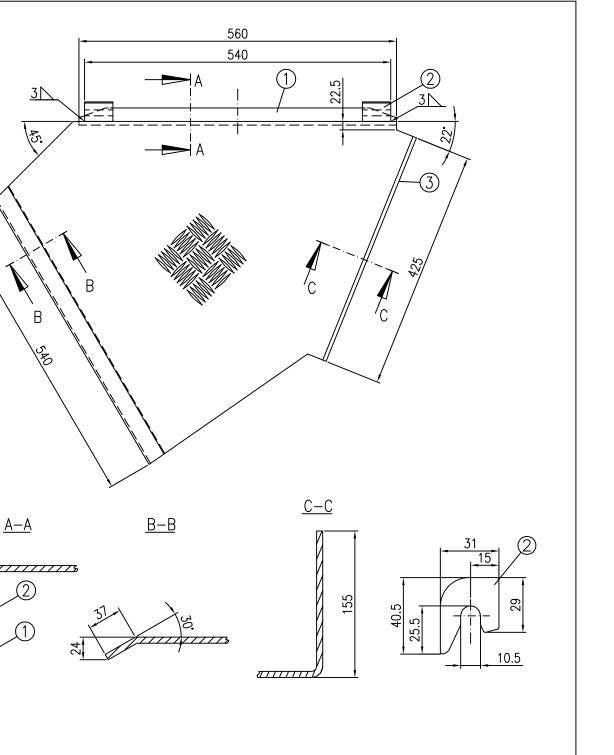












 Chequer plate with 5 bar pattern 5/6.5 Suspension hook Marking 	DIN EN 1386 EN AW-5754-H114 DIN EN 755 EN AW-6063-T66 131-MIG: type 5 filler material (EC9)	Dimens [m] 	. Weight [kg] 10.6
Frame scaffold ALFIX 70			
Aluminium corner deck with toeboard,	rigid	Annex A, page 25	
A709-A122	07.2016	F-9	

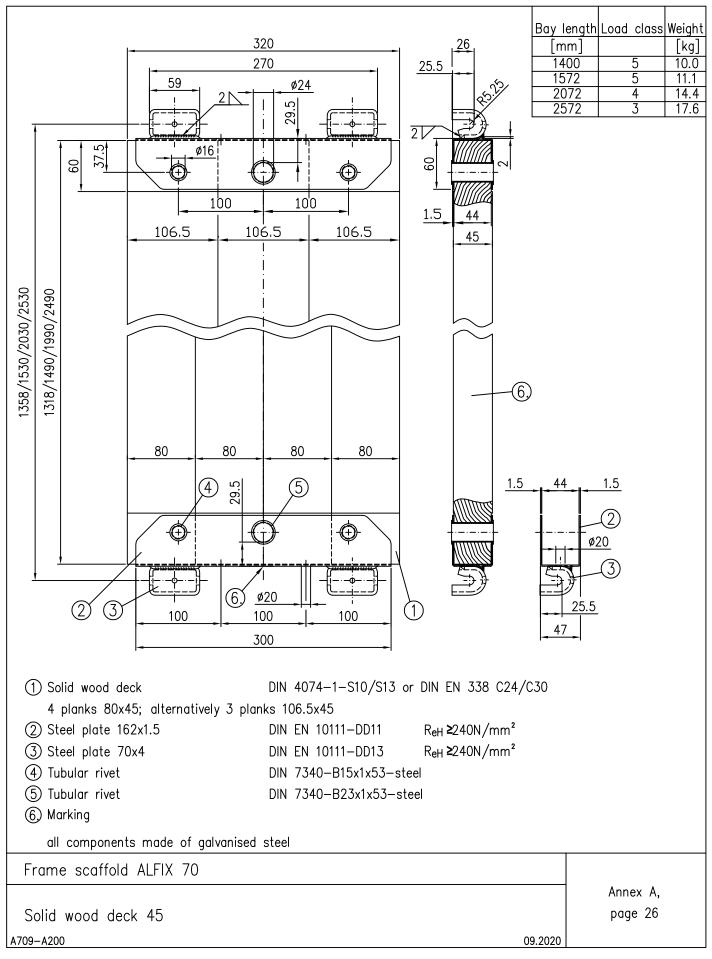
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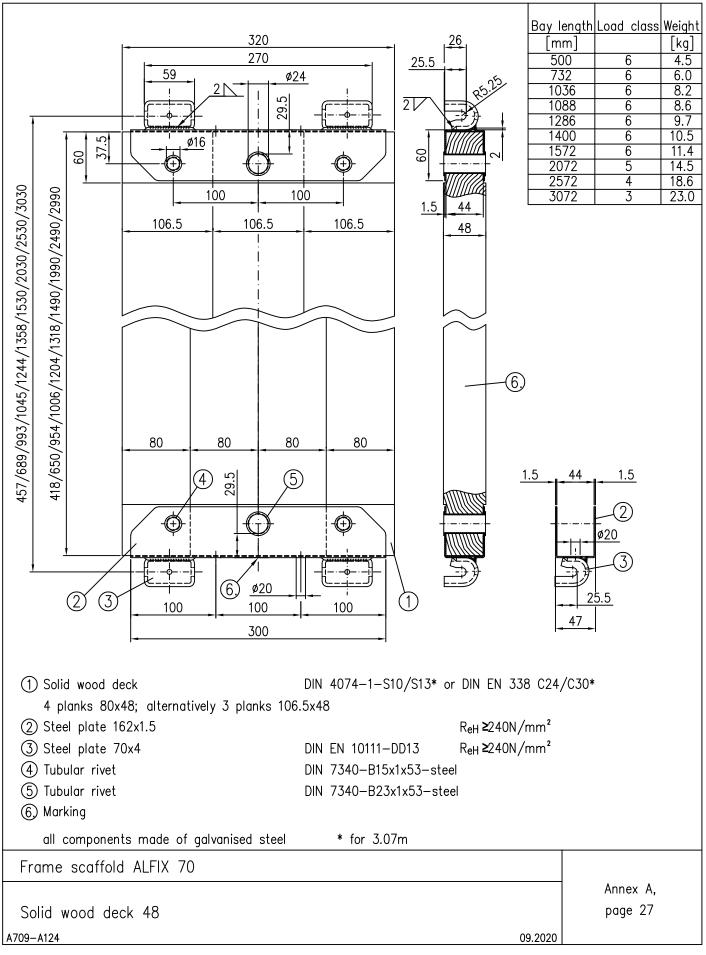
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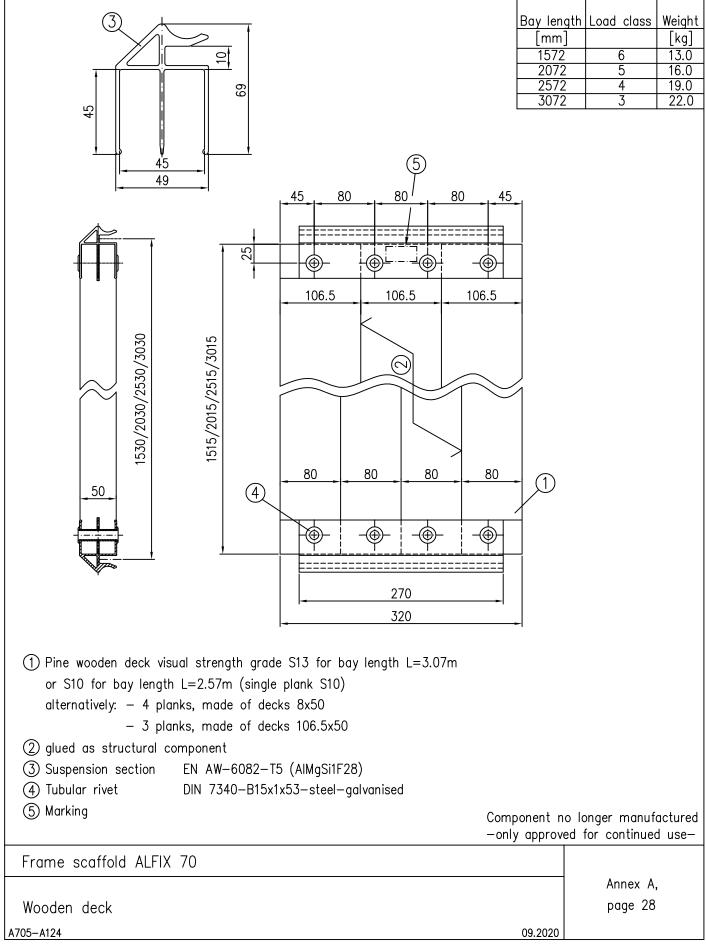
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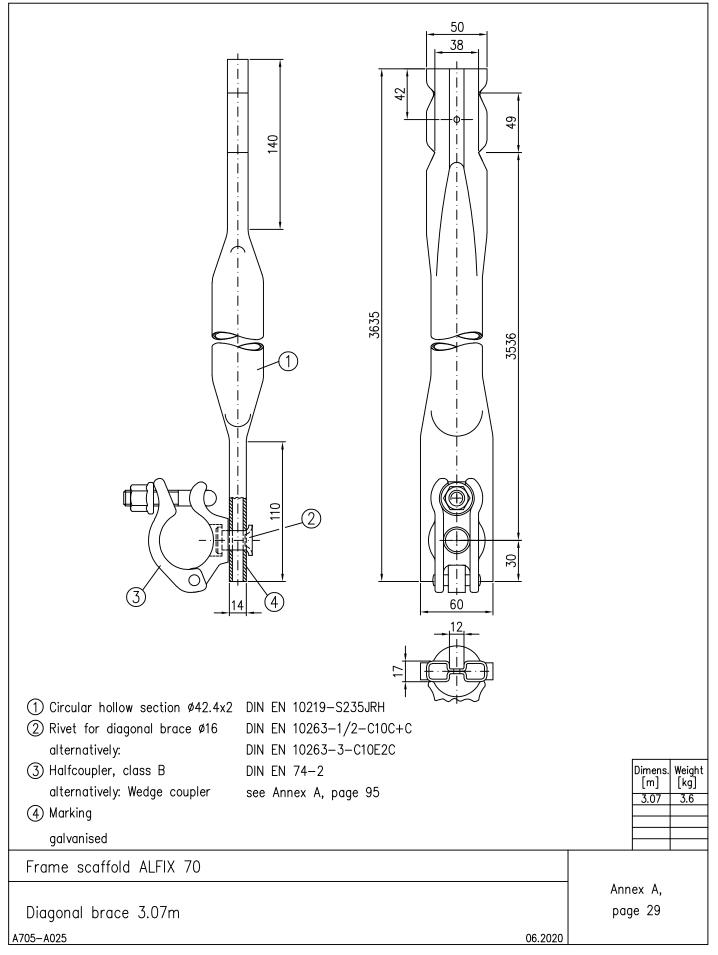
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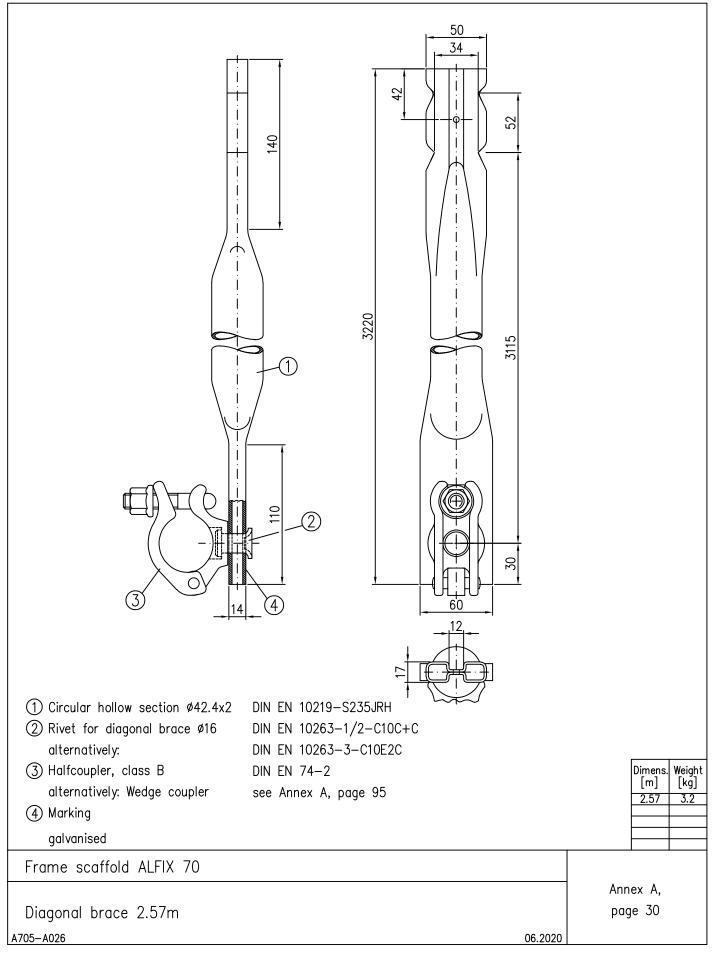
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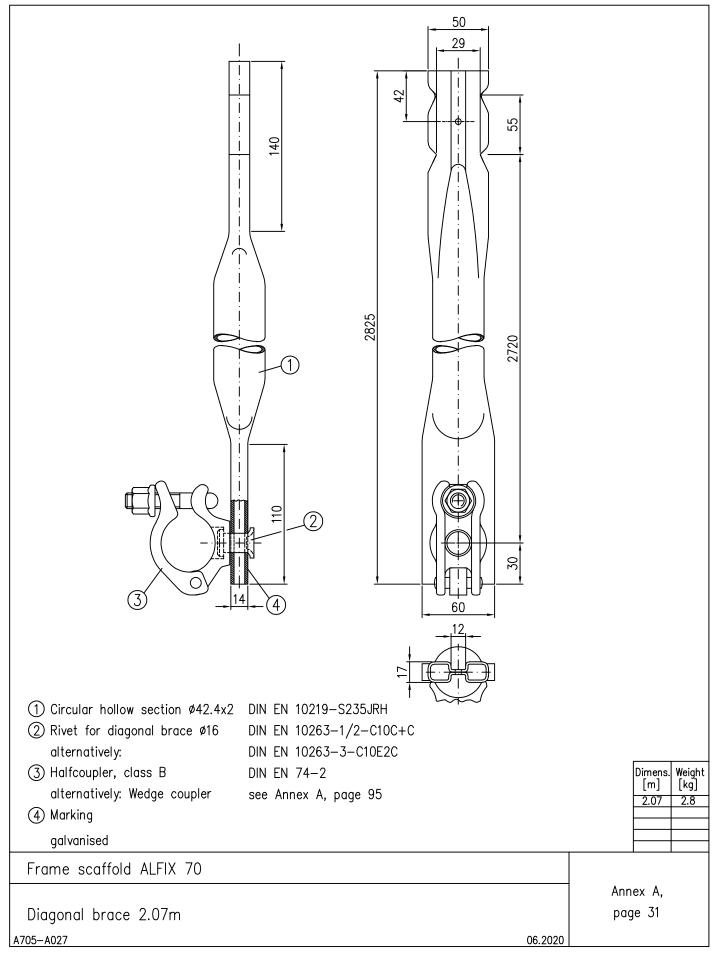


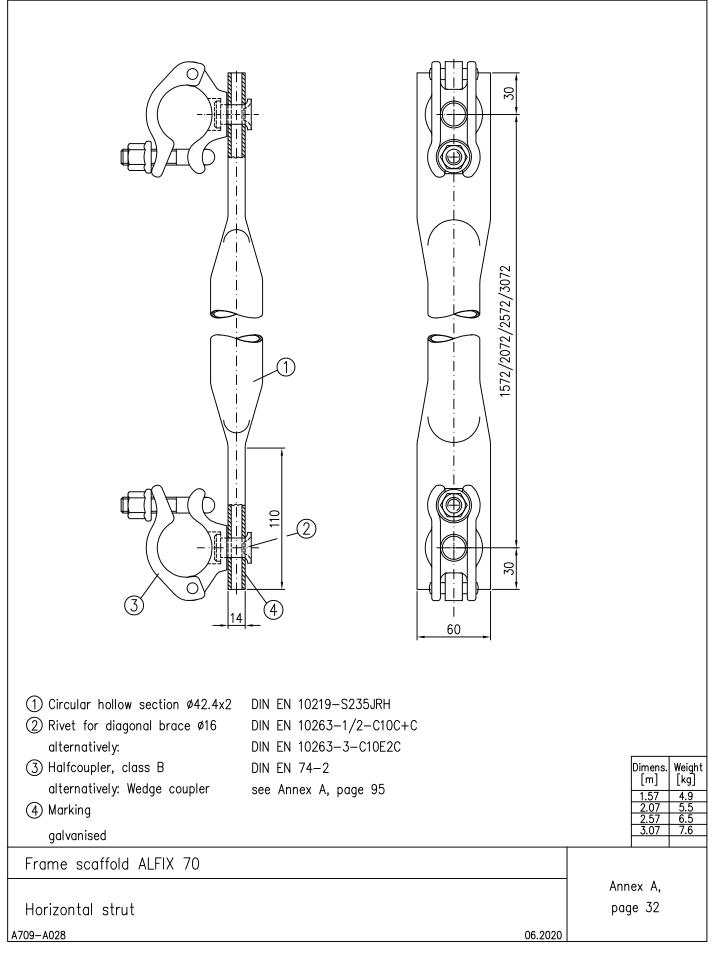


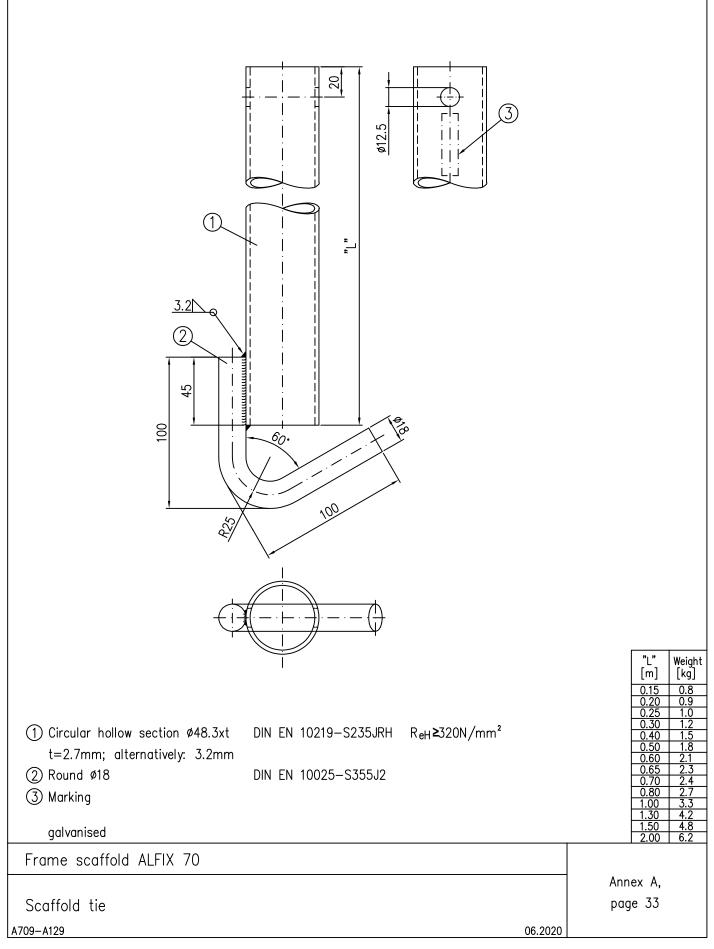


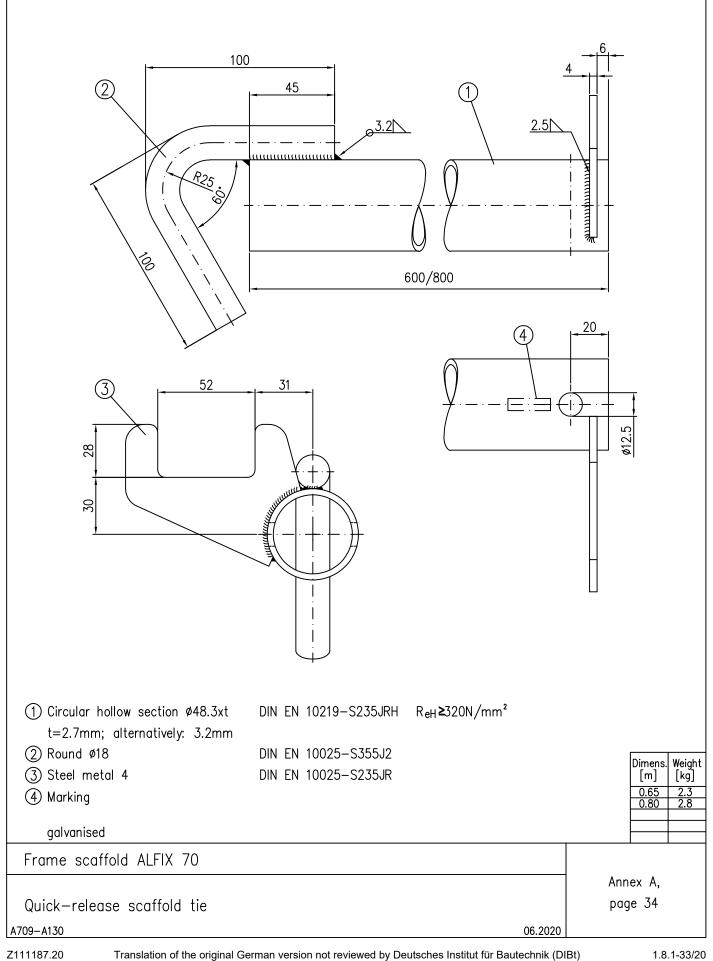


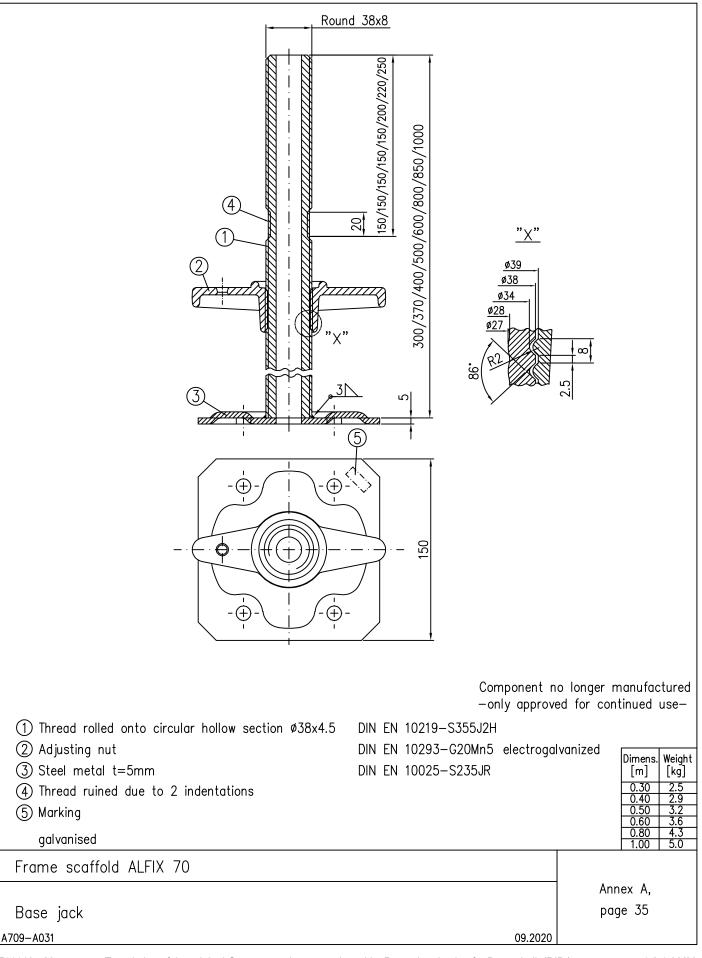
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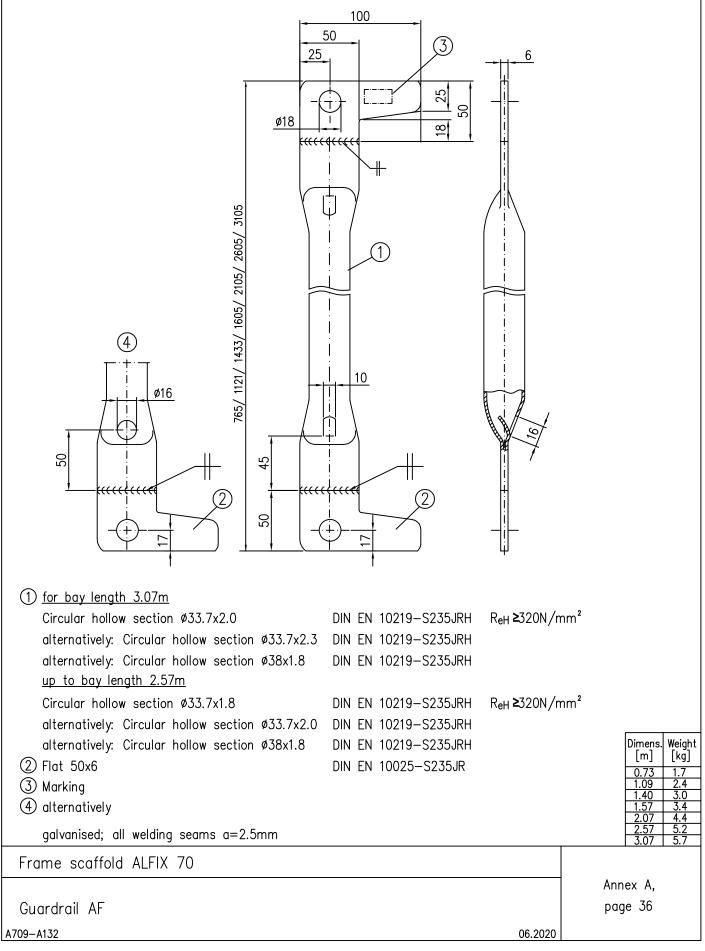


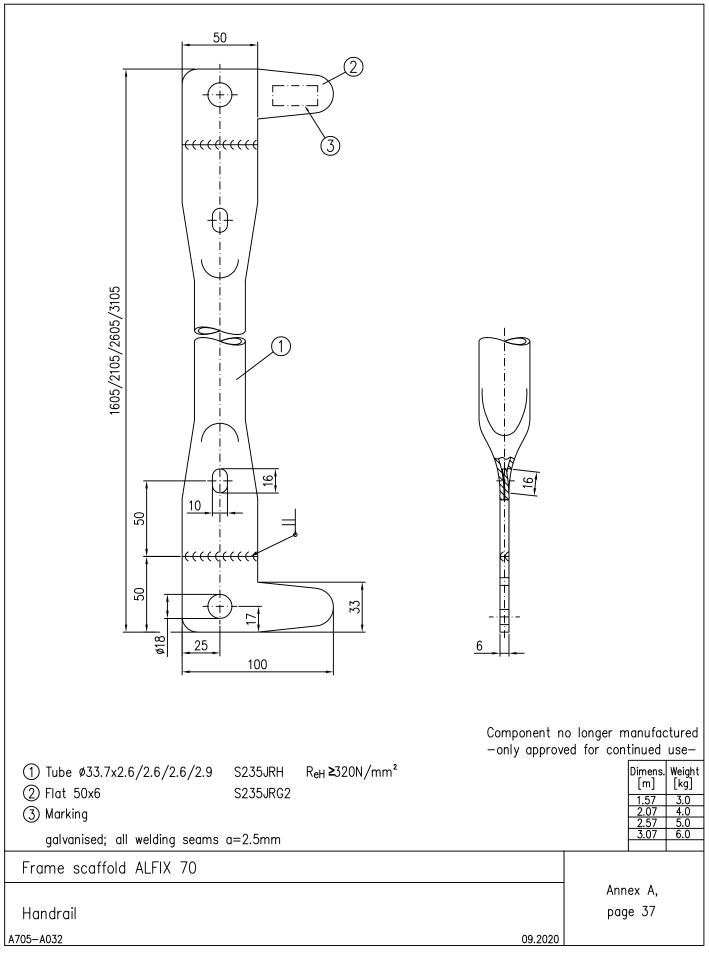




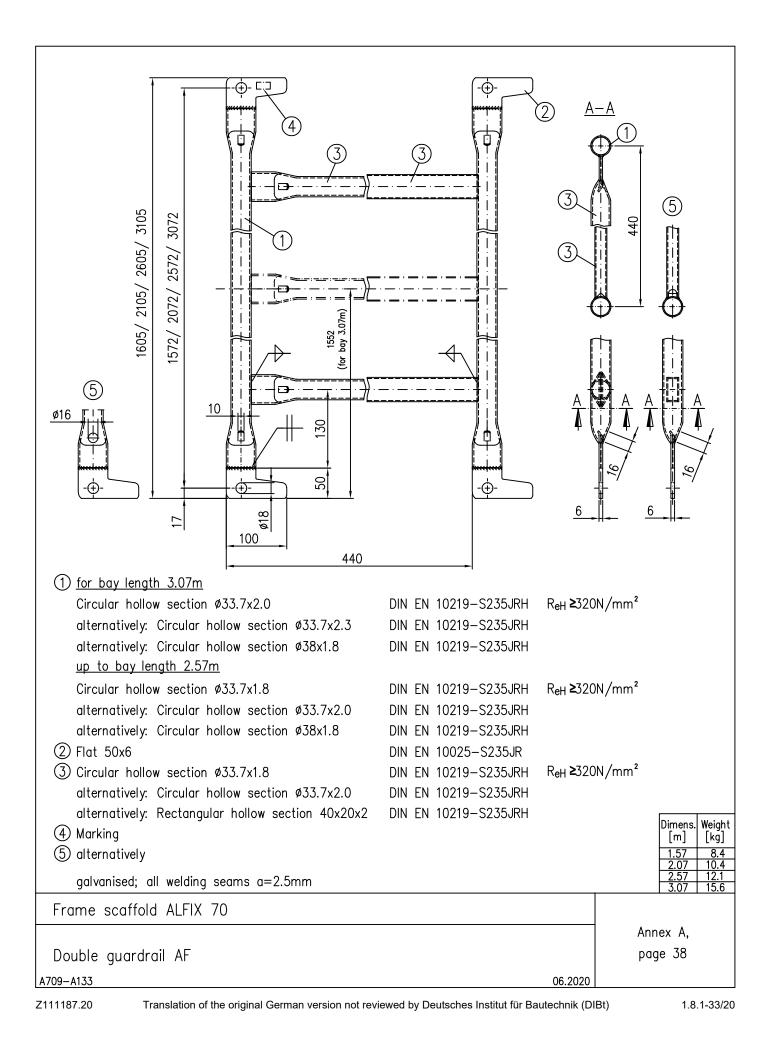


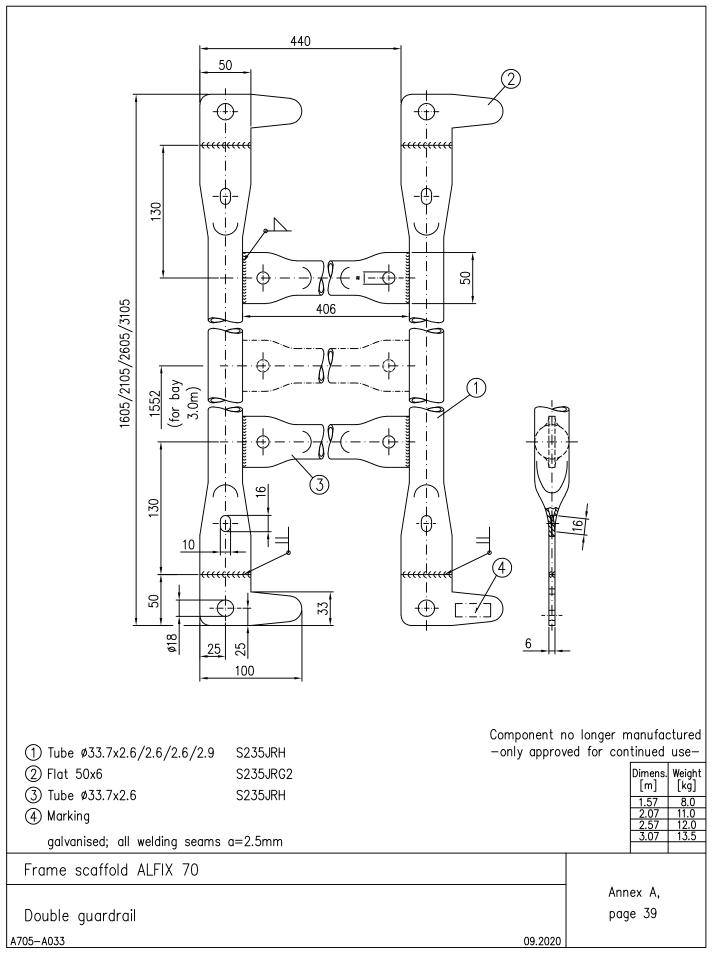


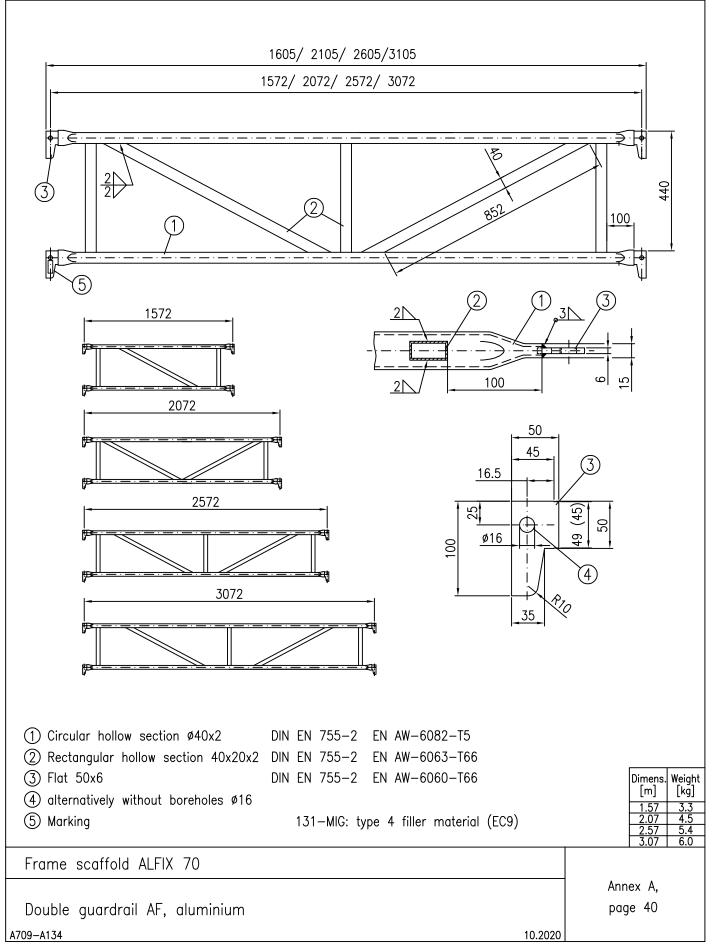


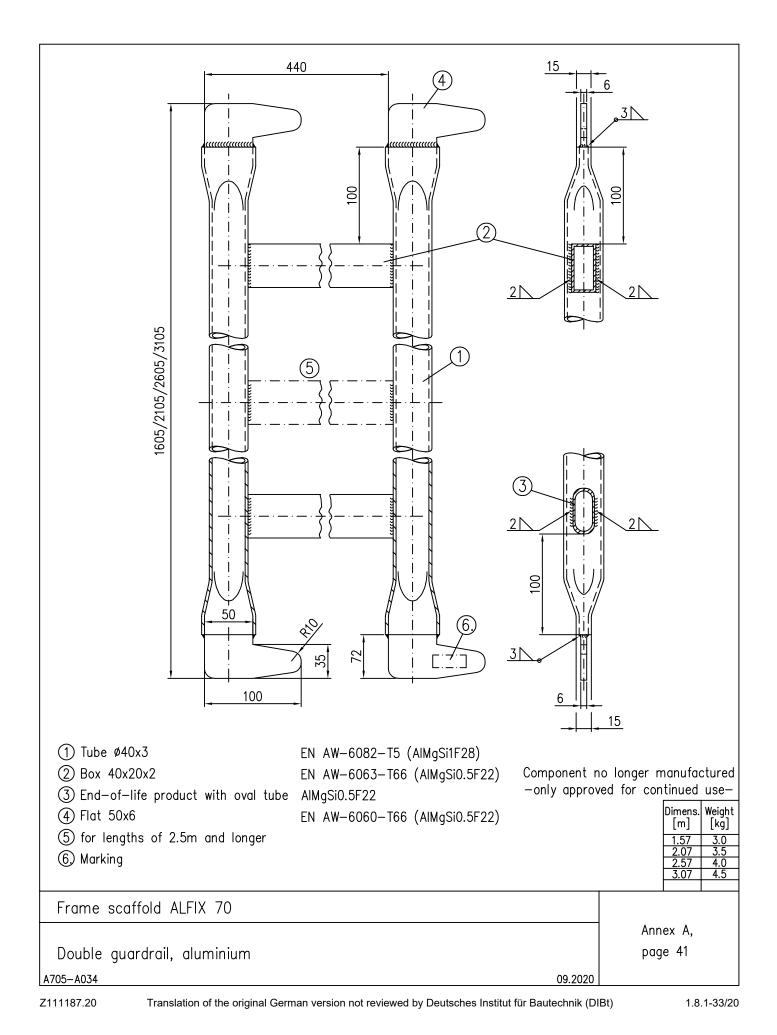


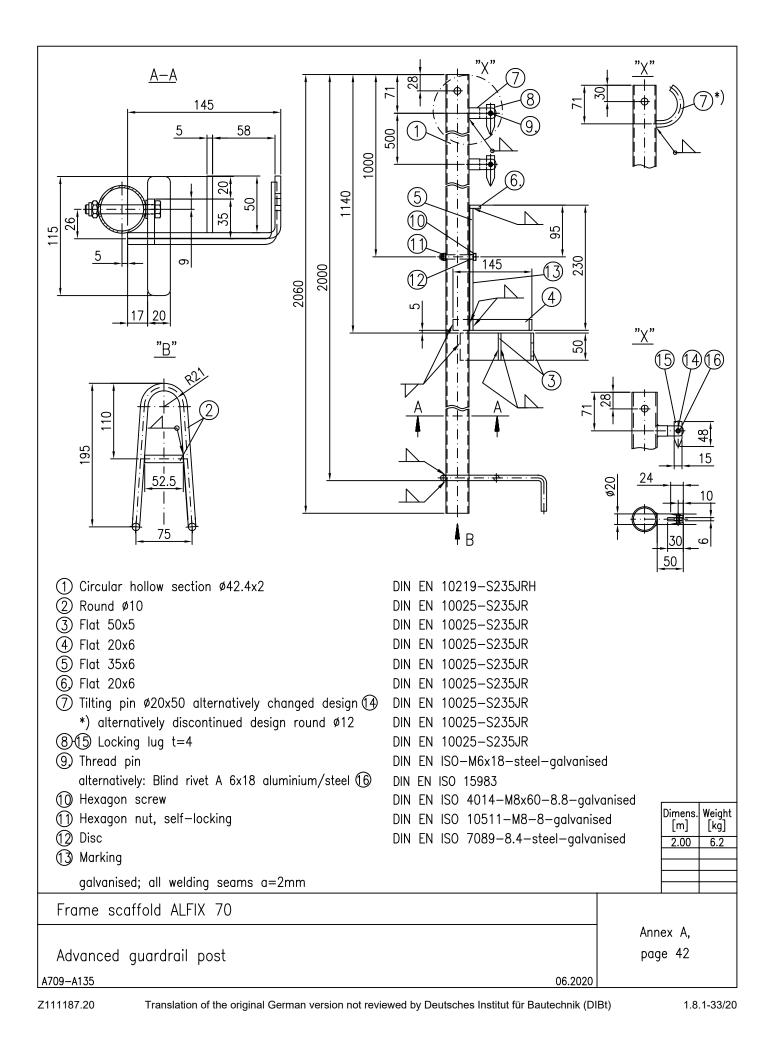
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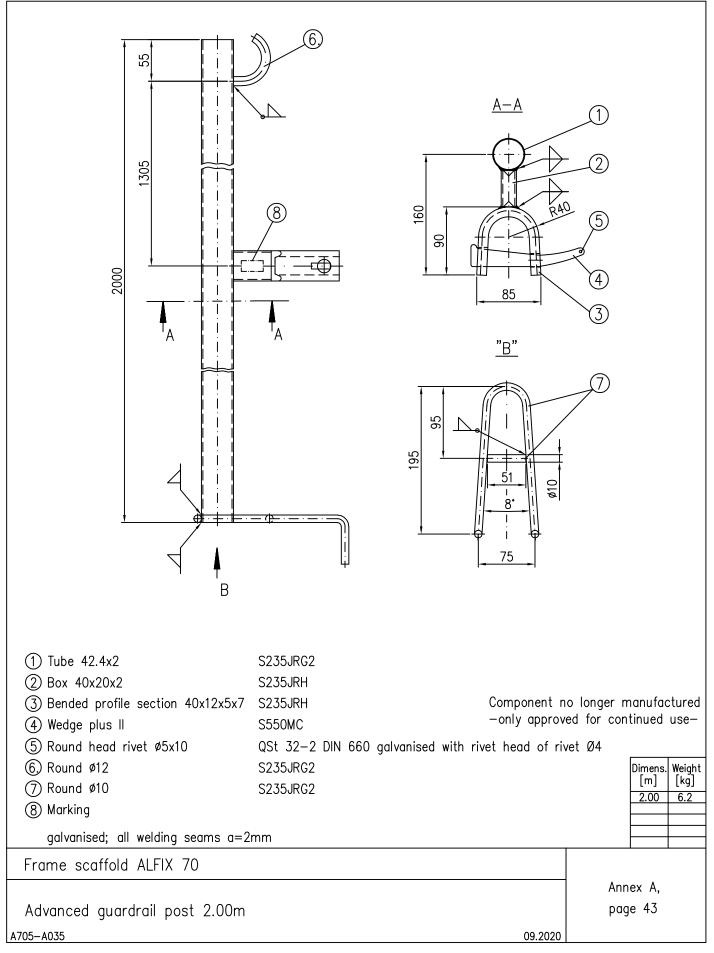


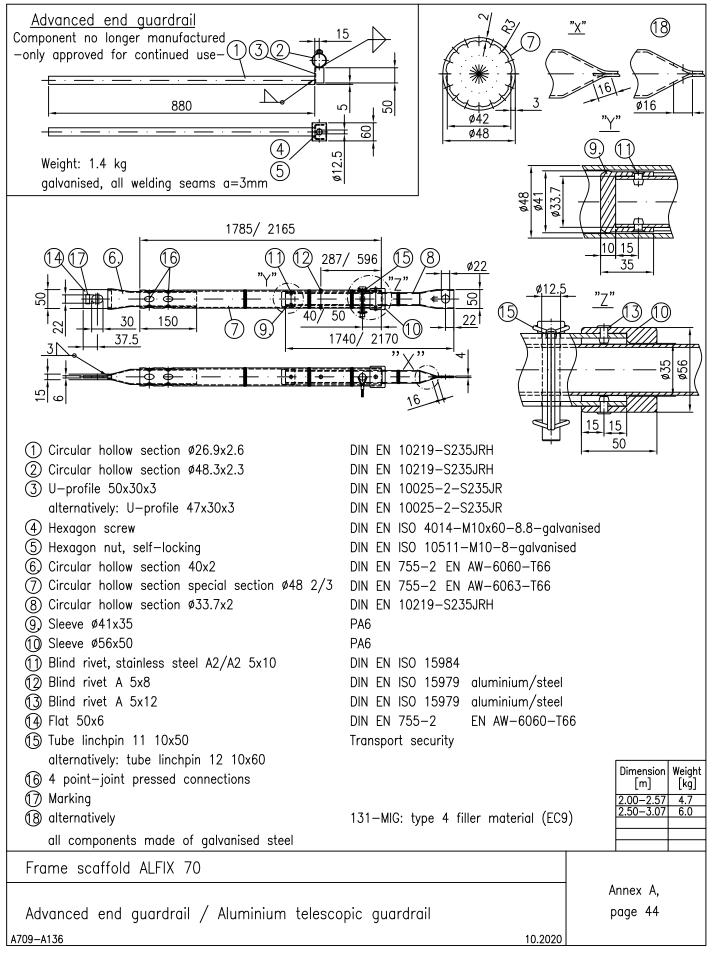


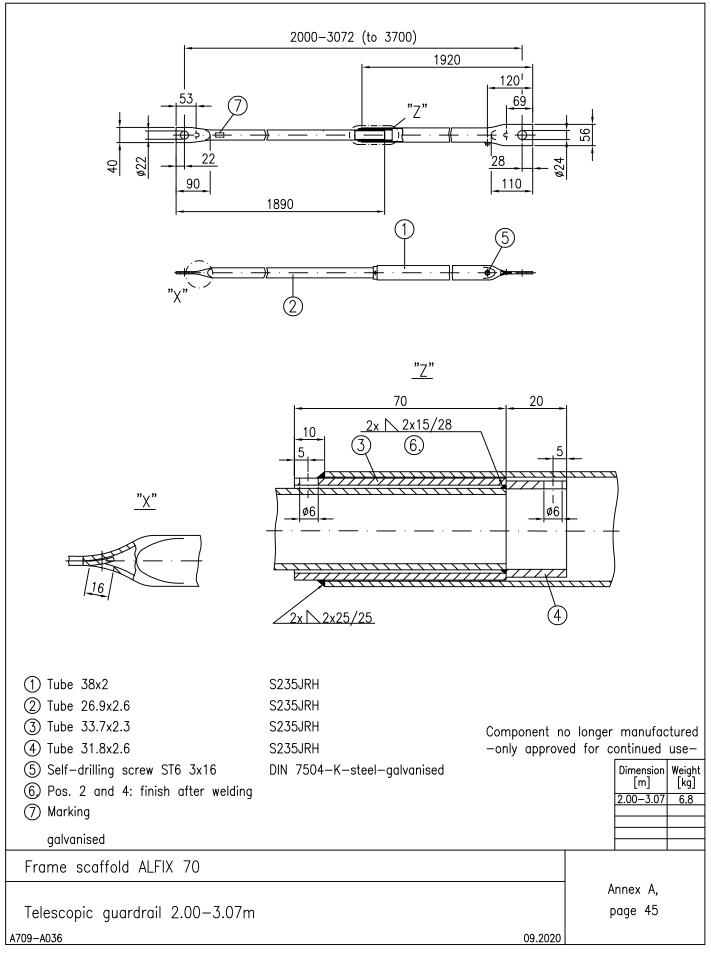


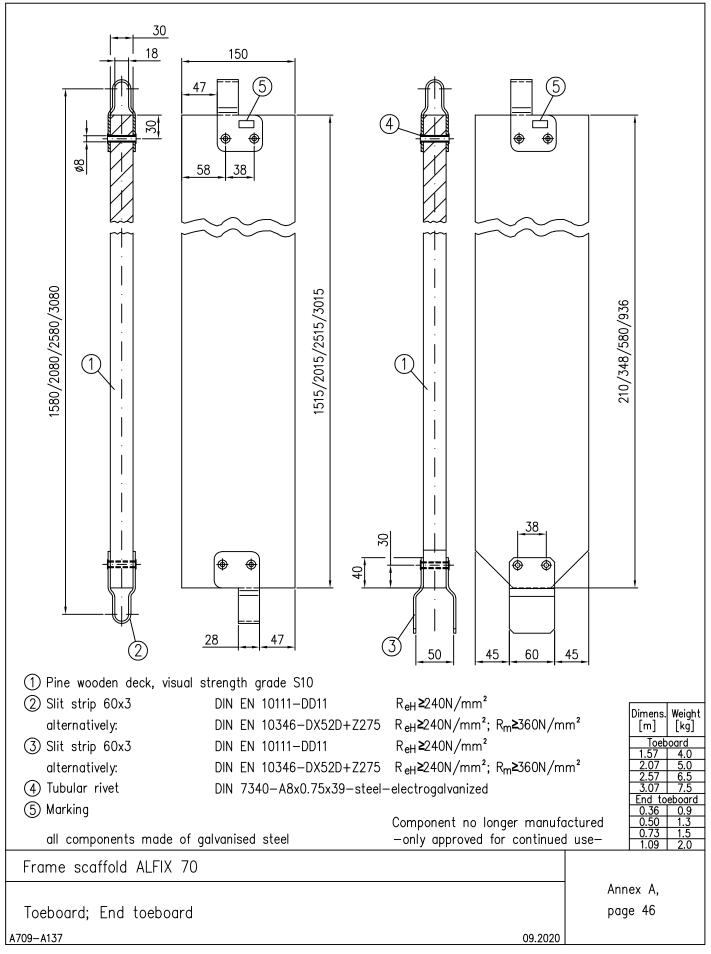


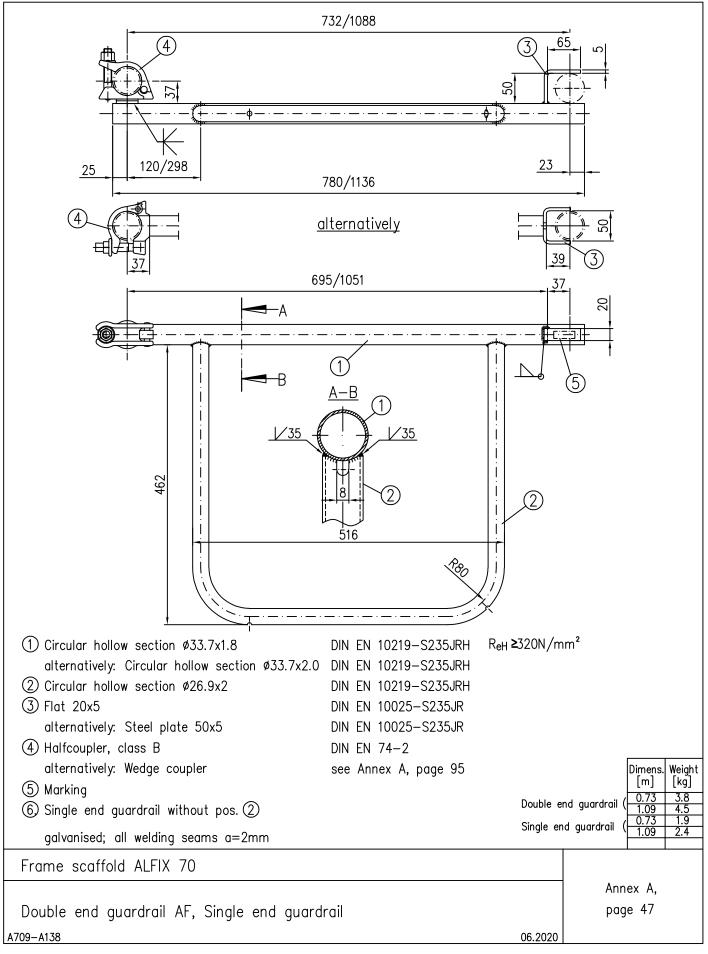


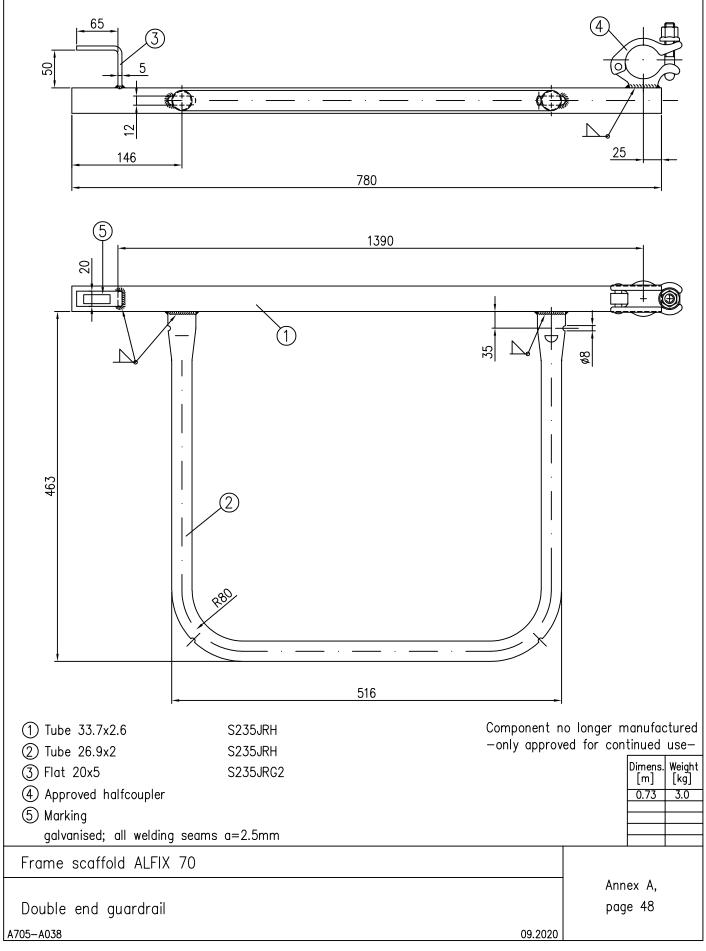


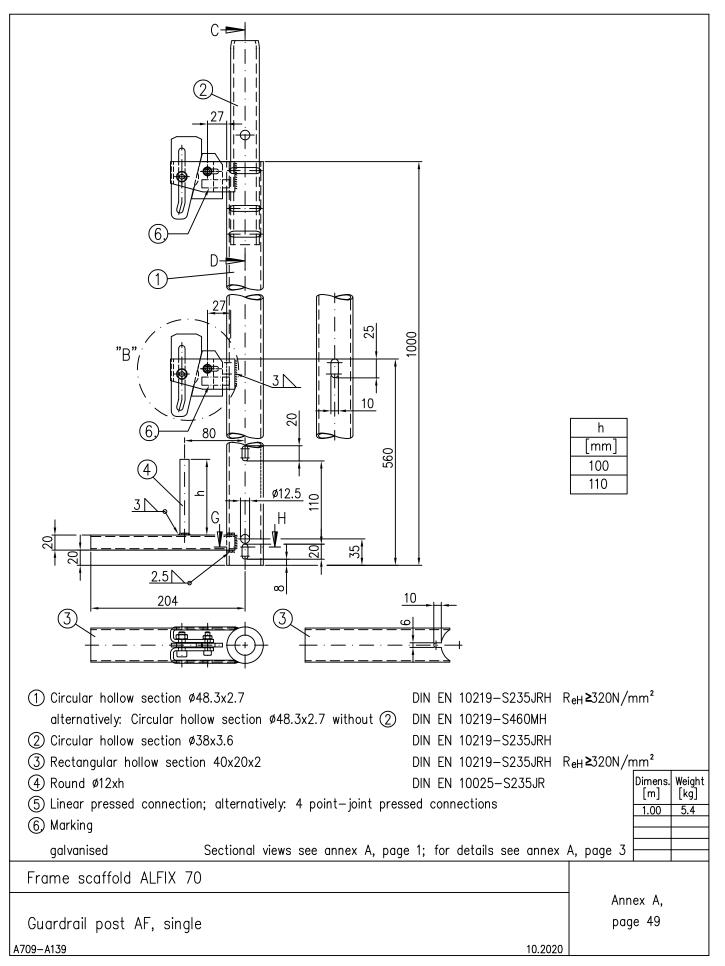


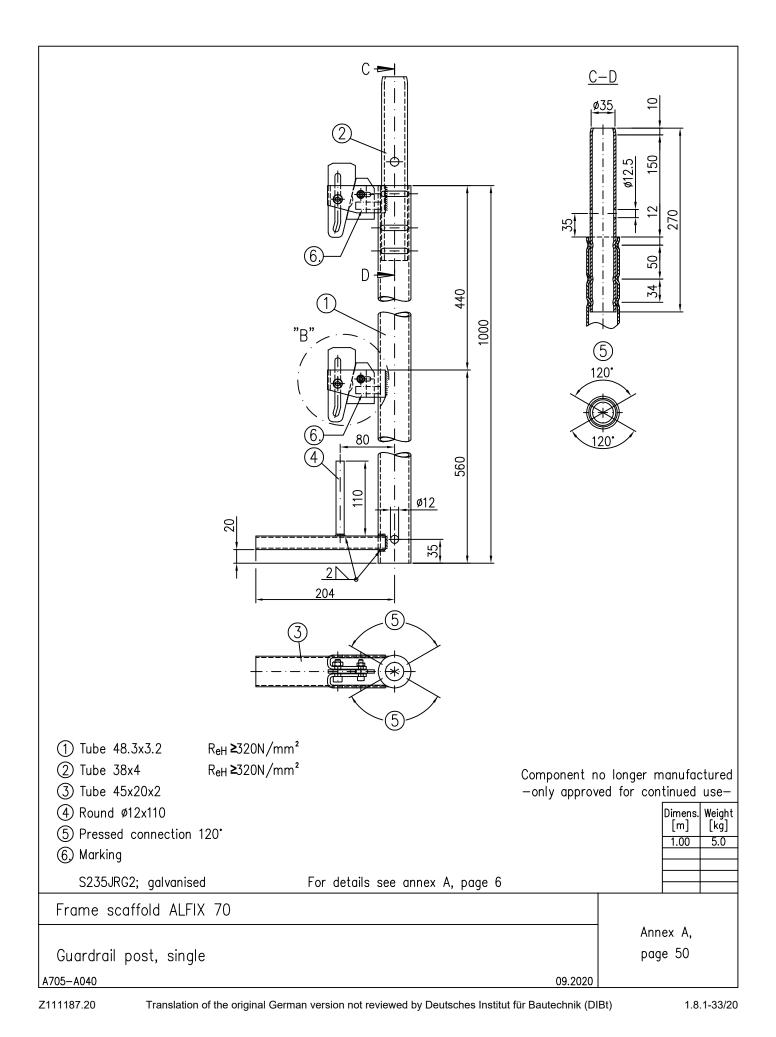


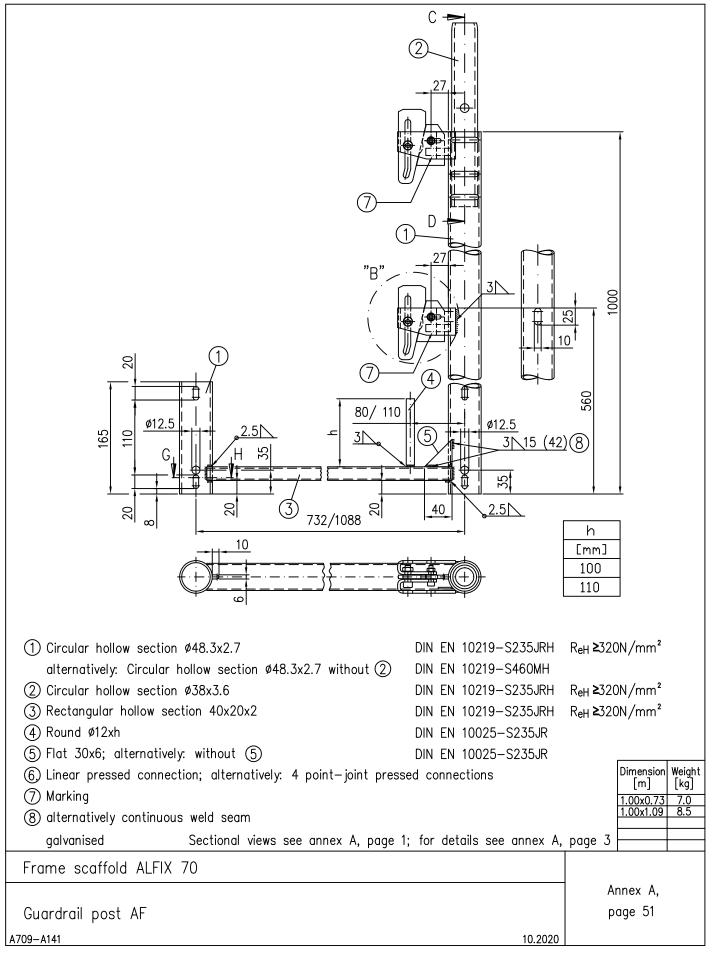


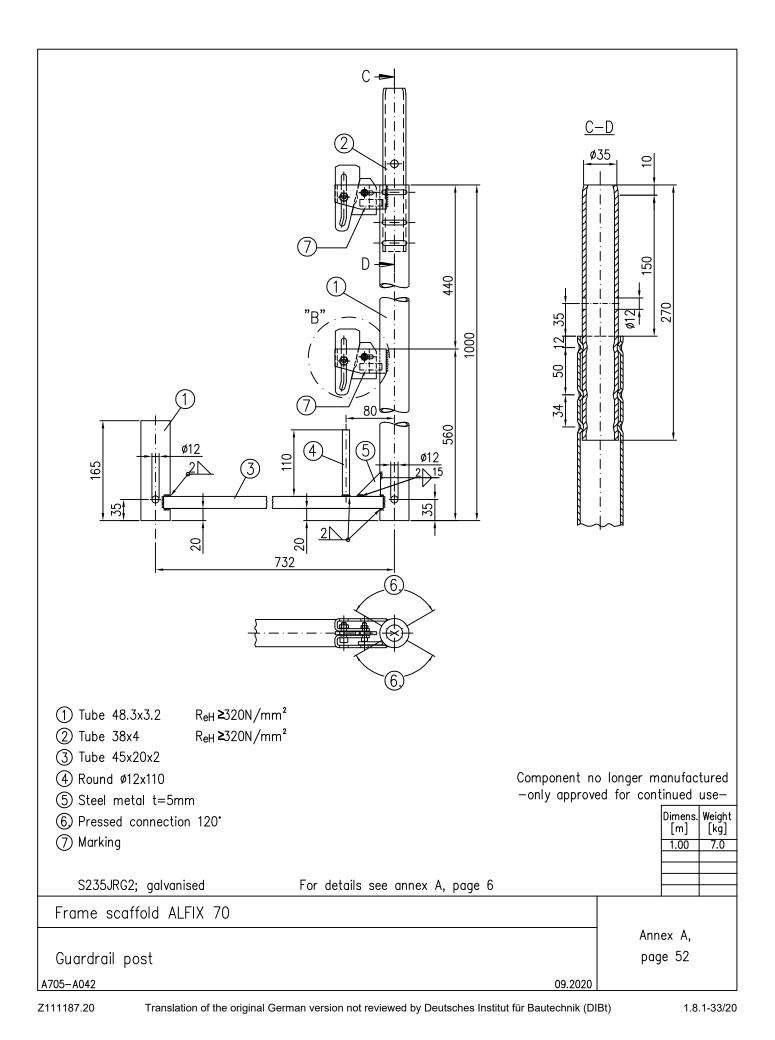


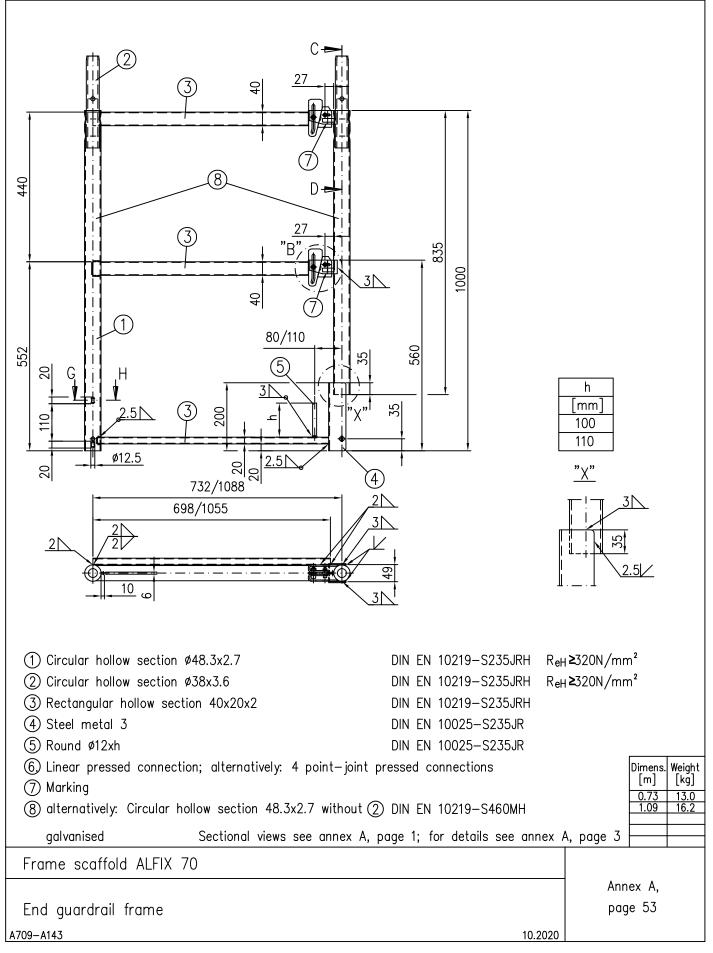


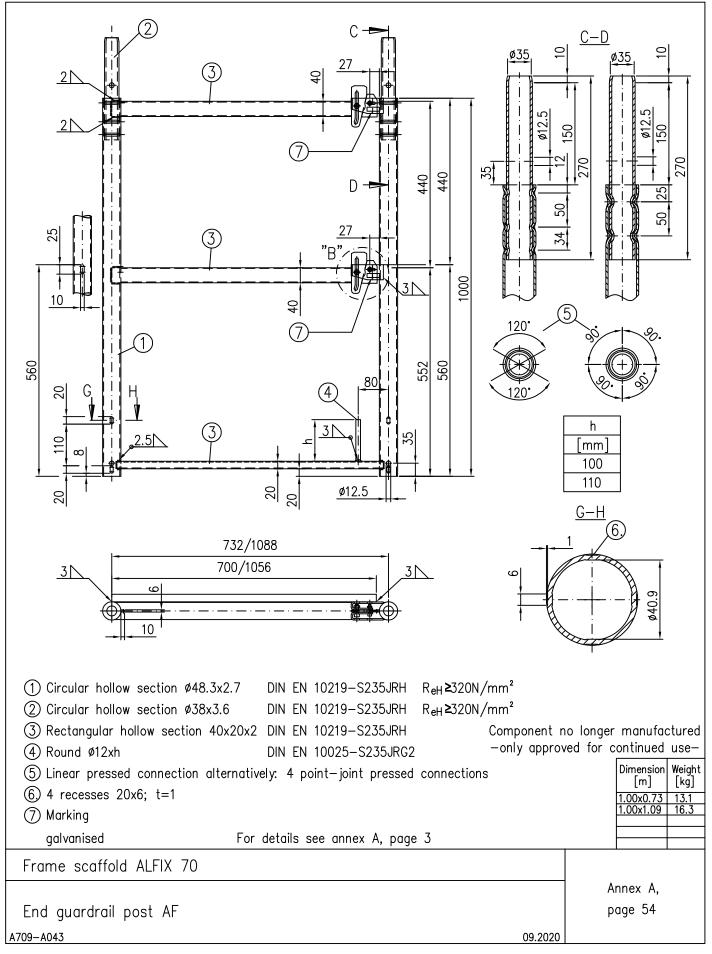


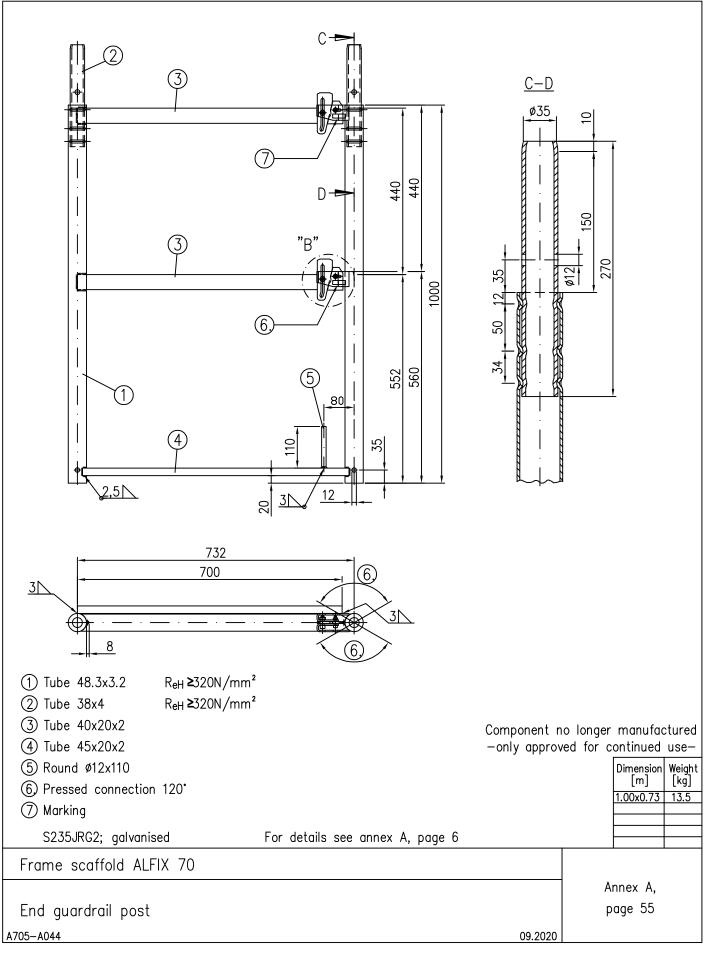


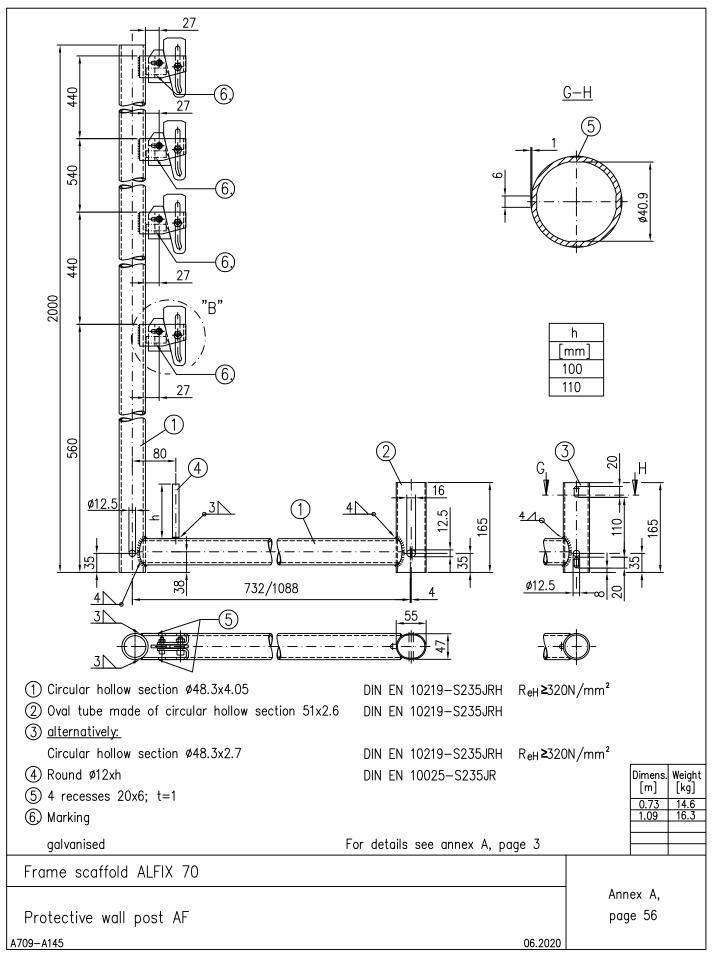


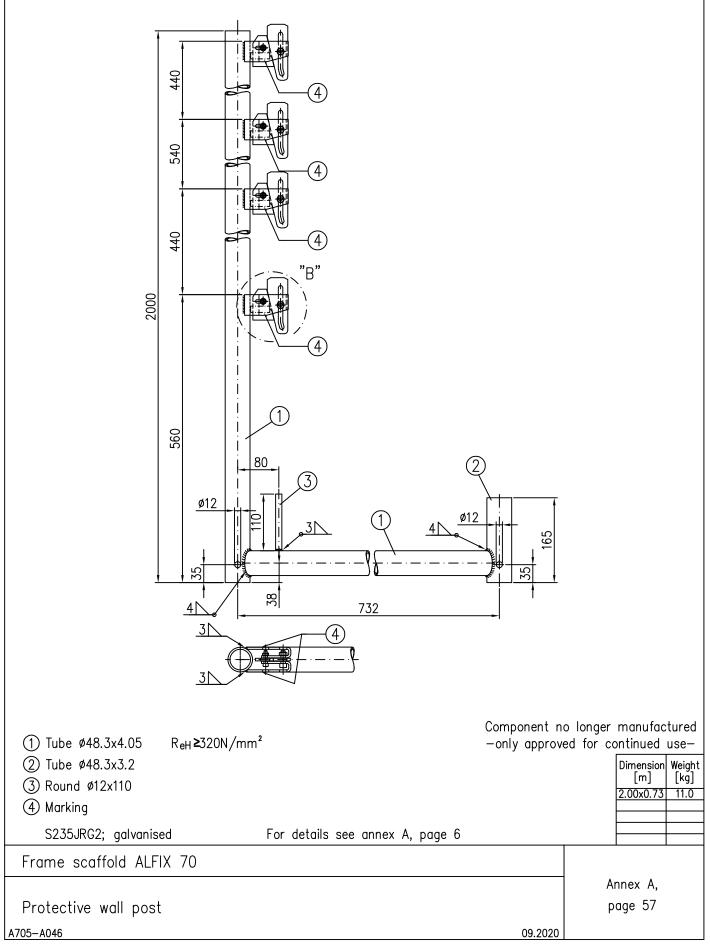




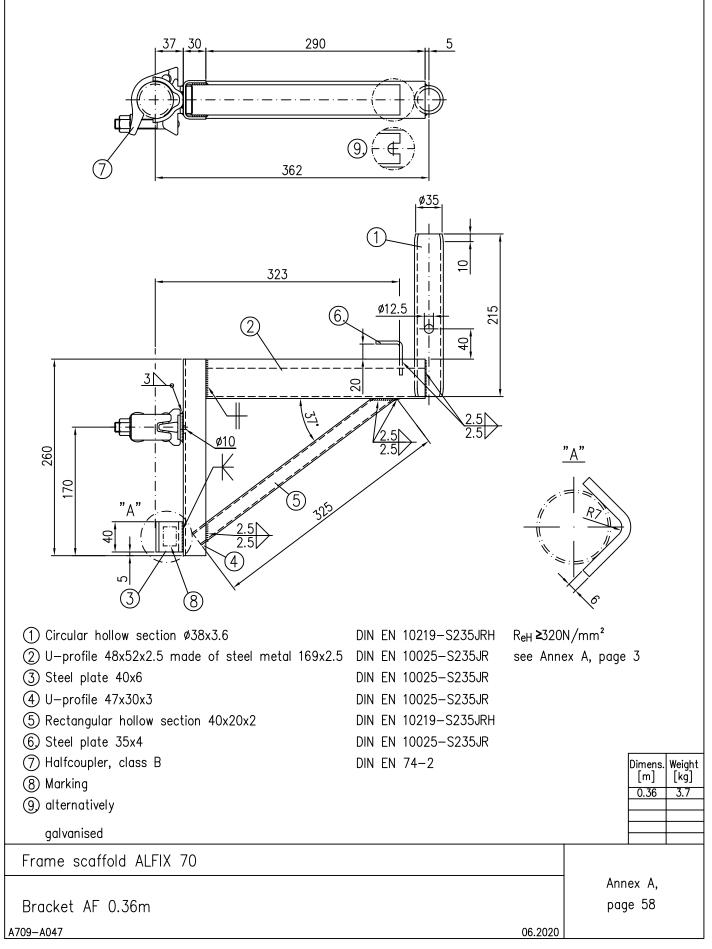


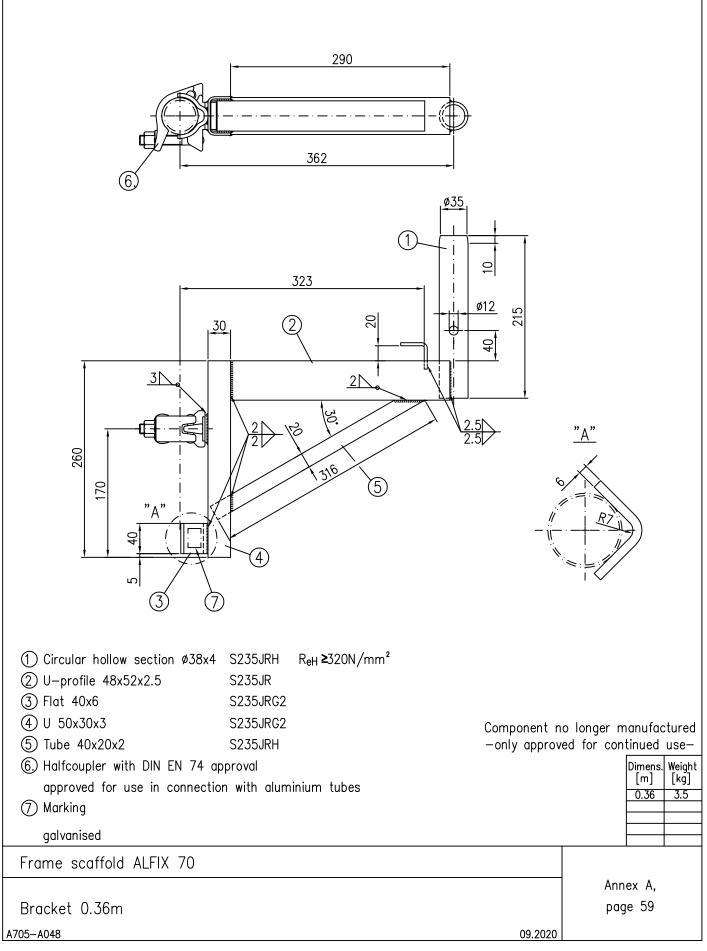


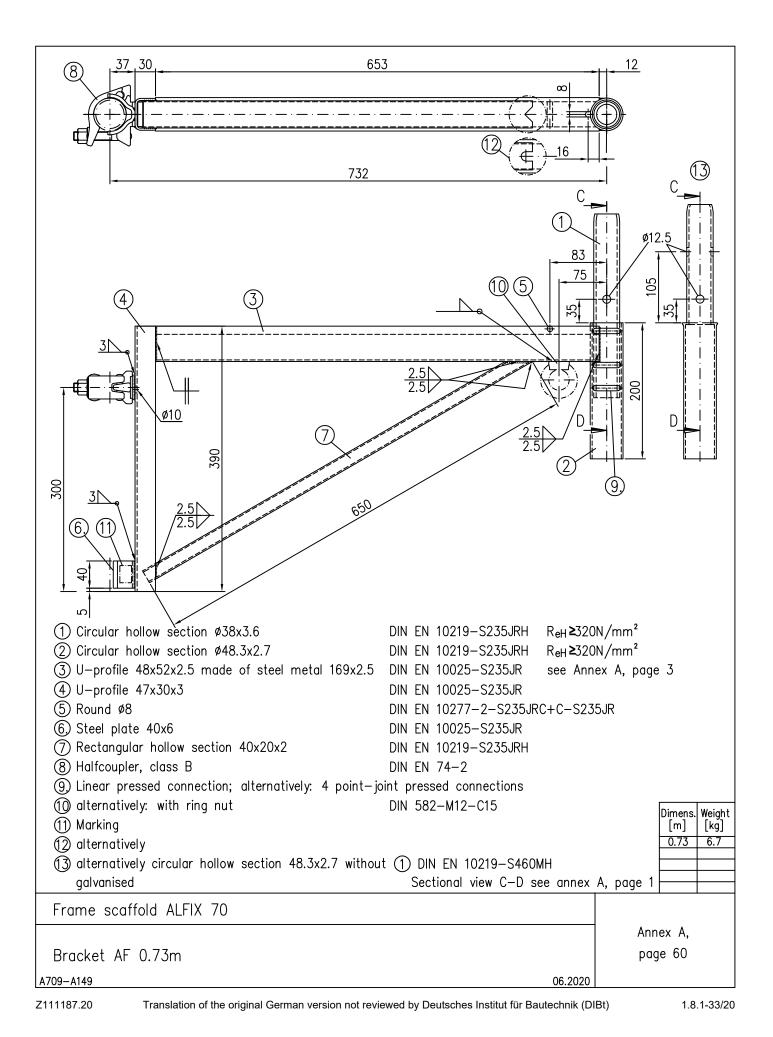


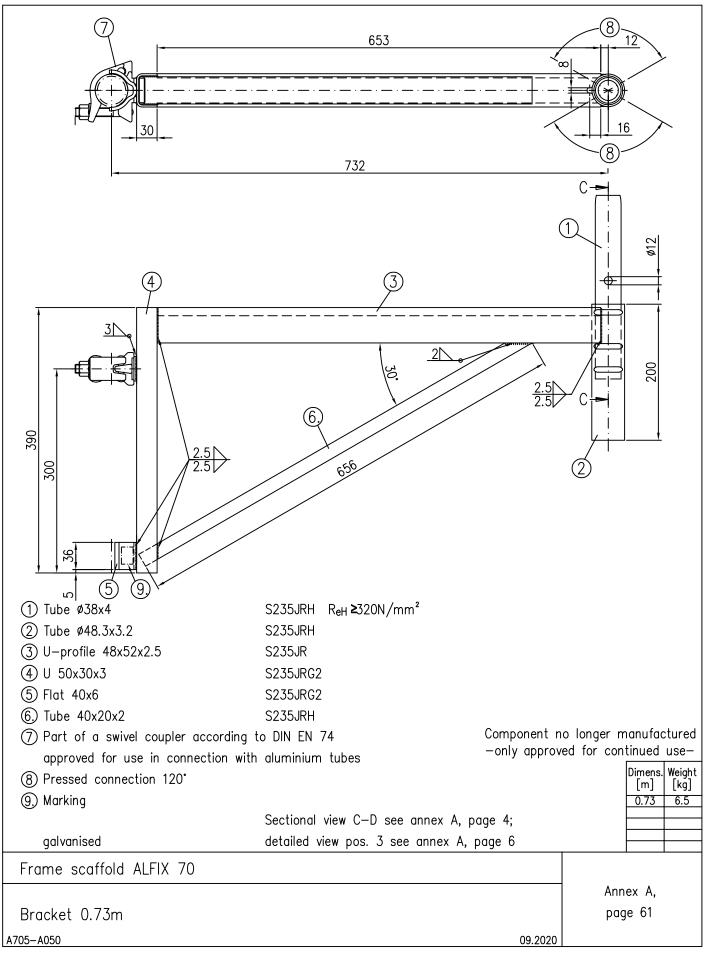


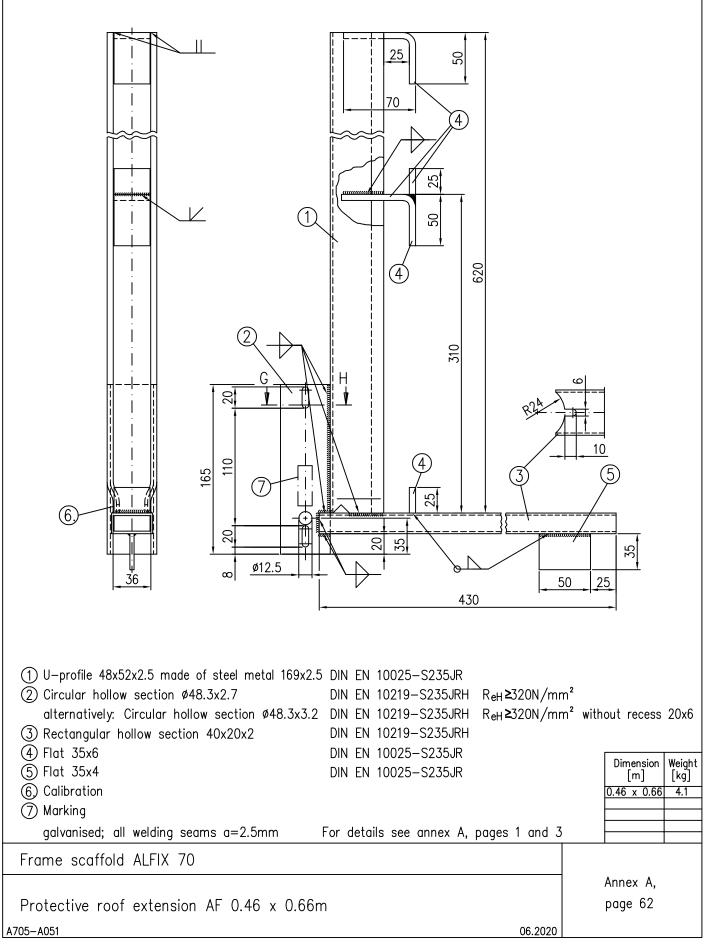
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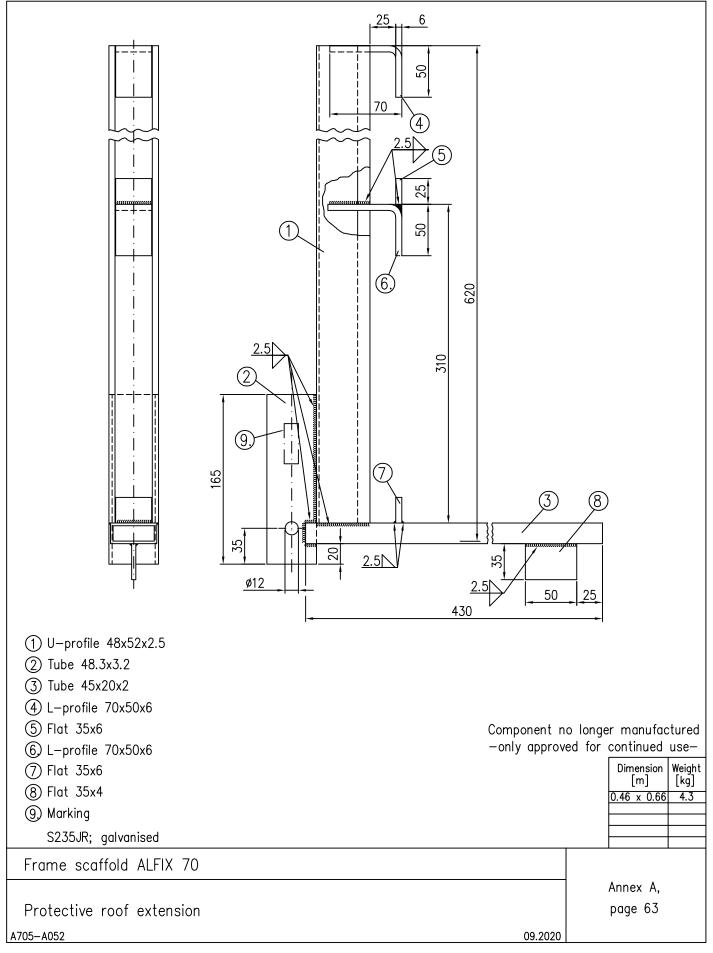




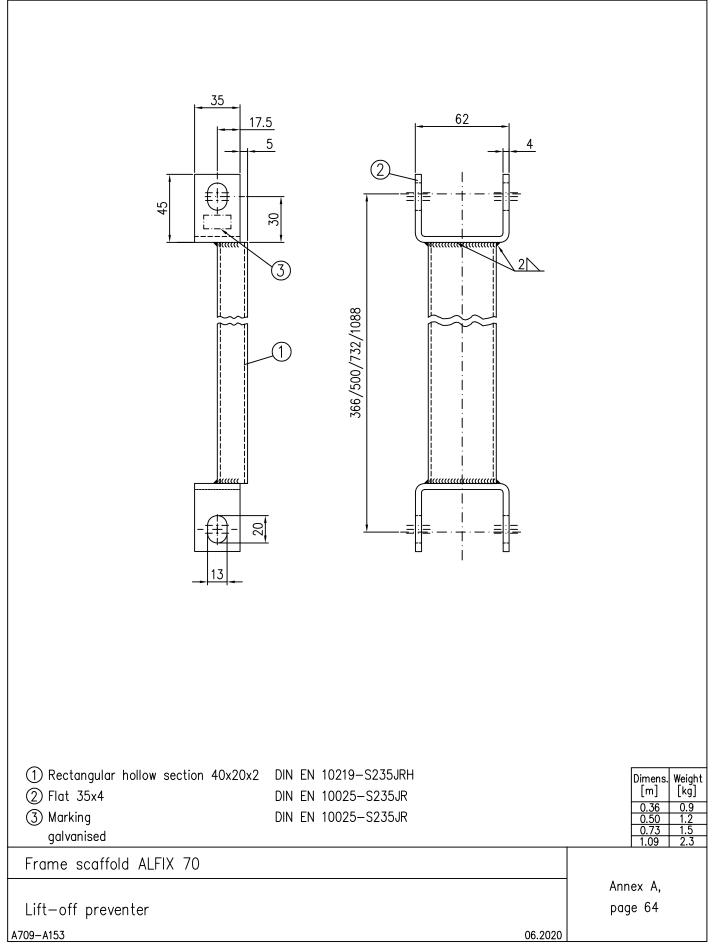


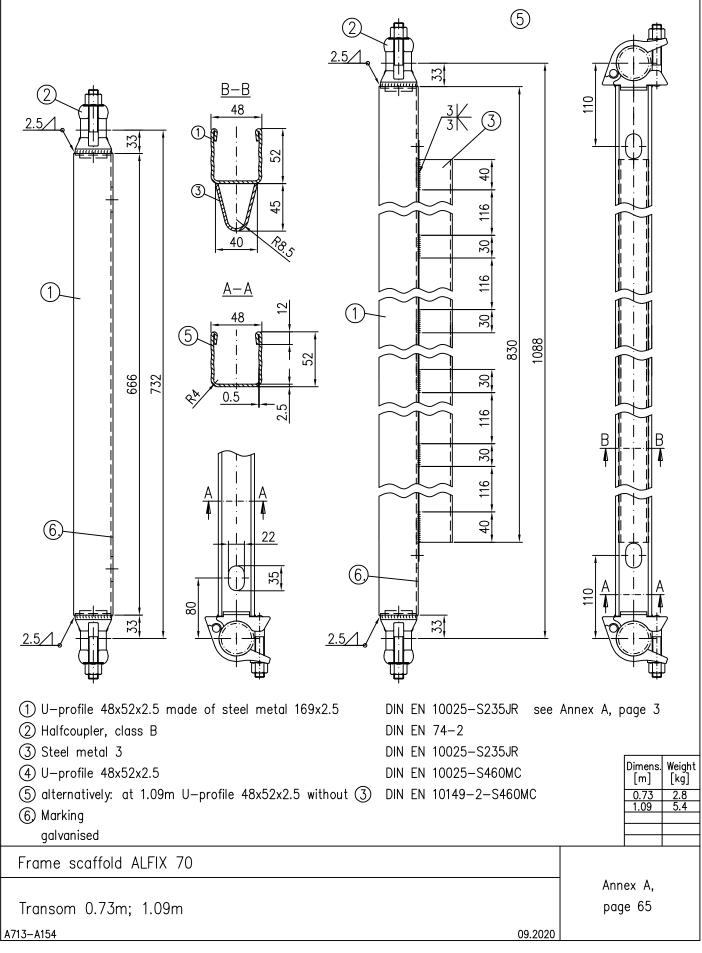


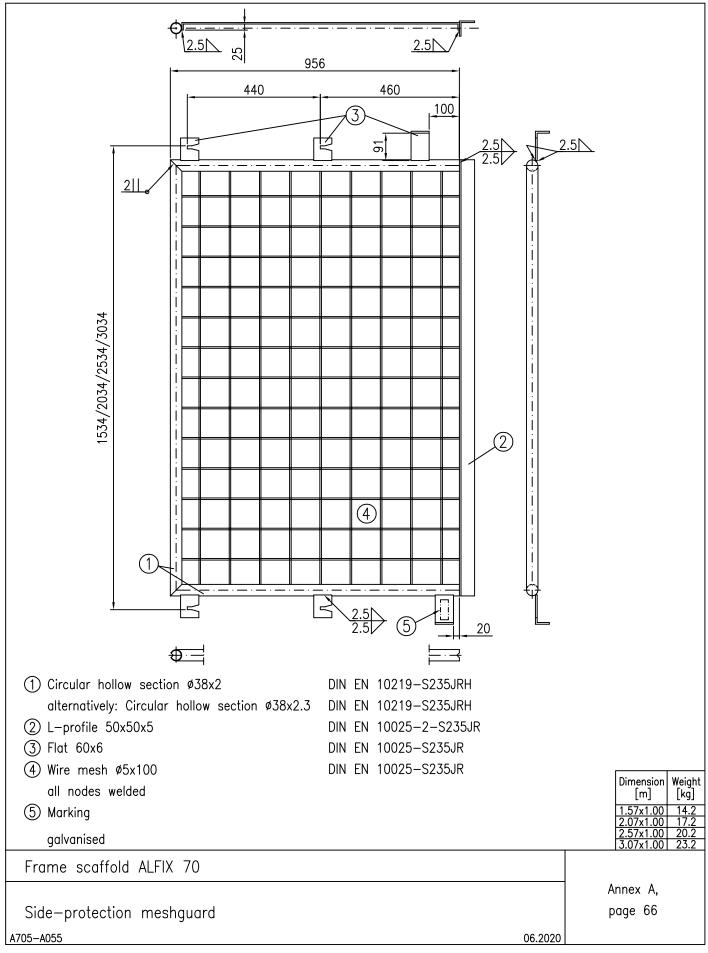


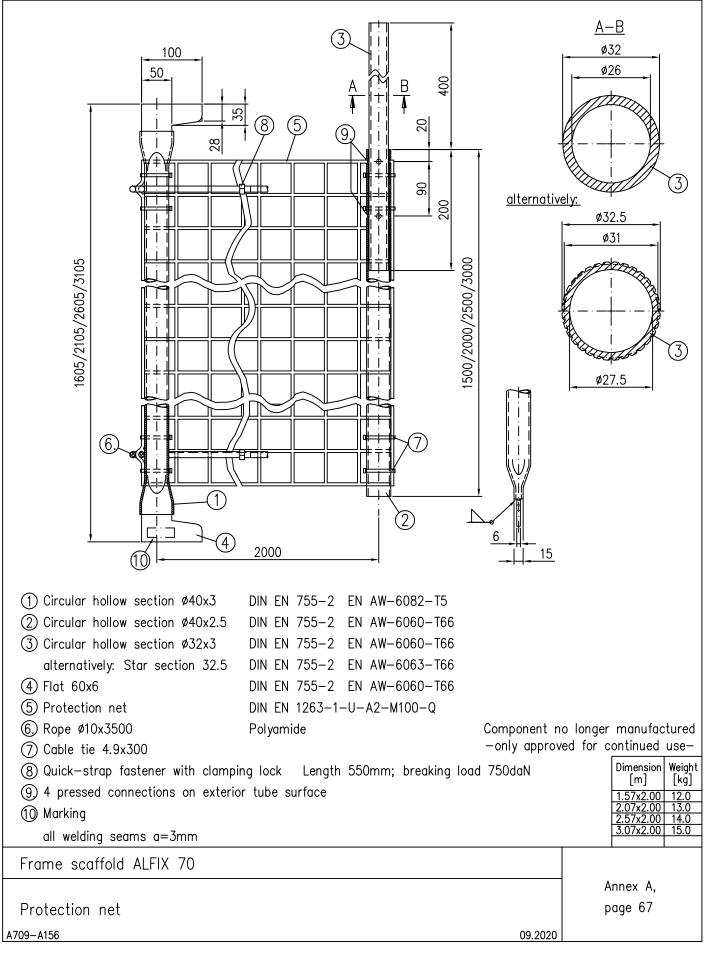


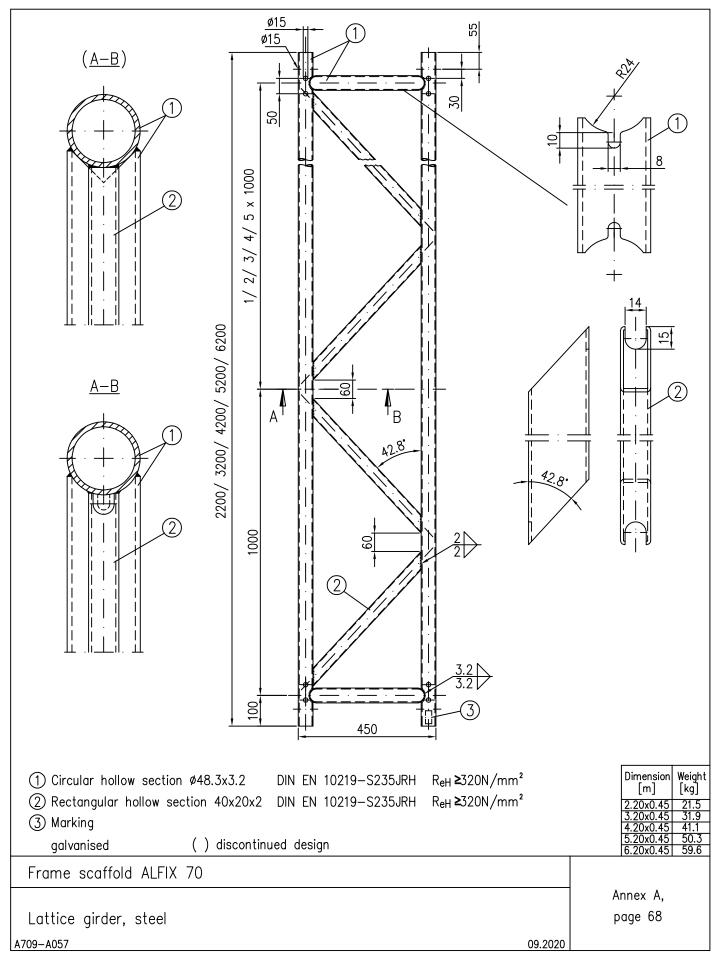
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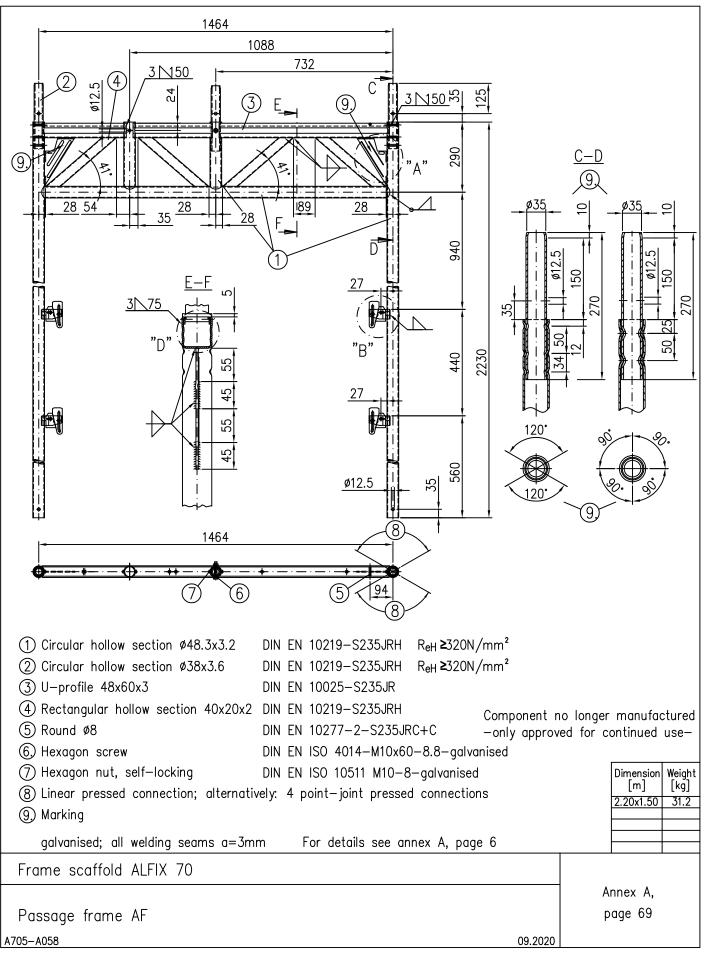


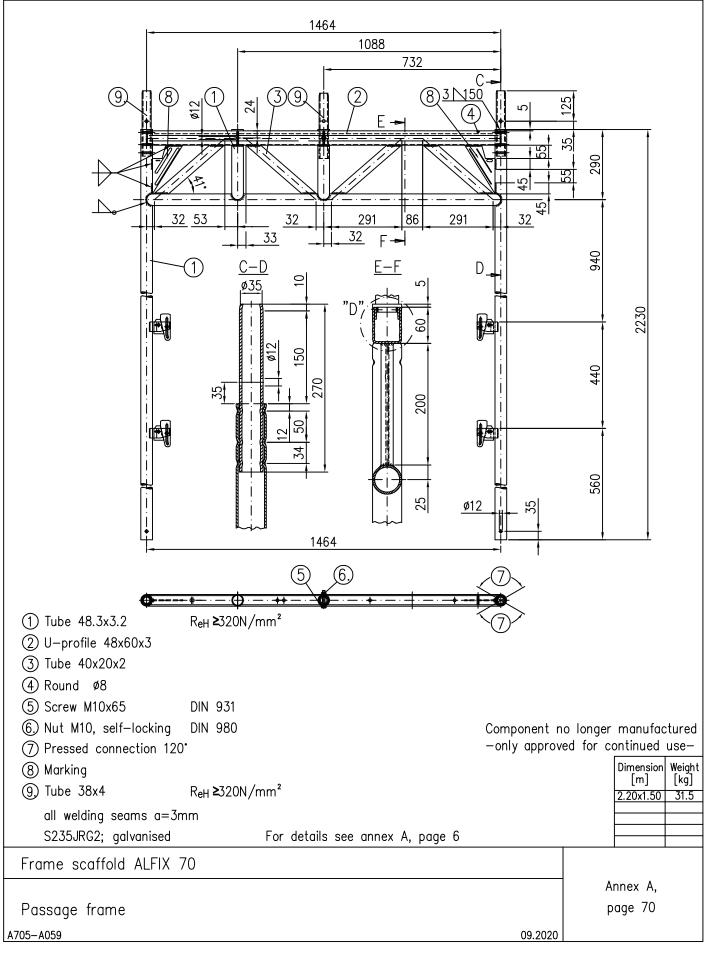


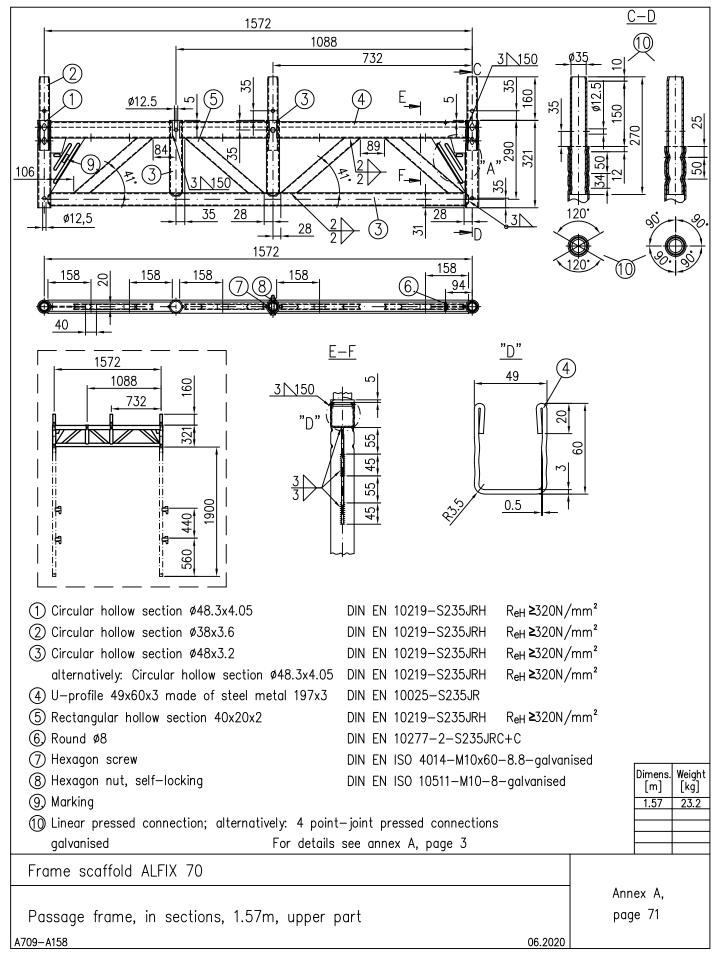


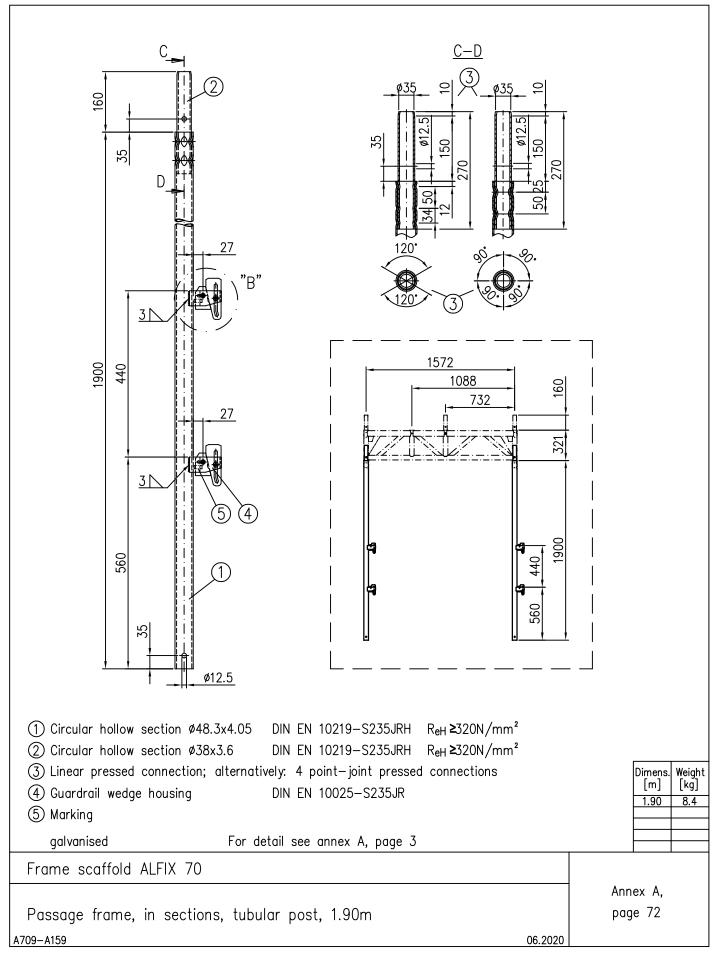


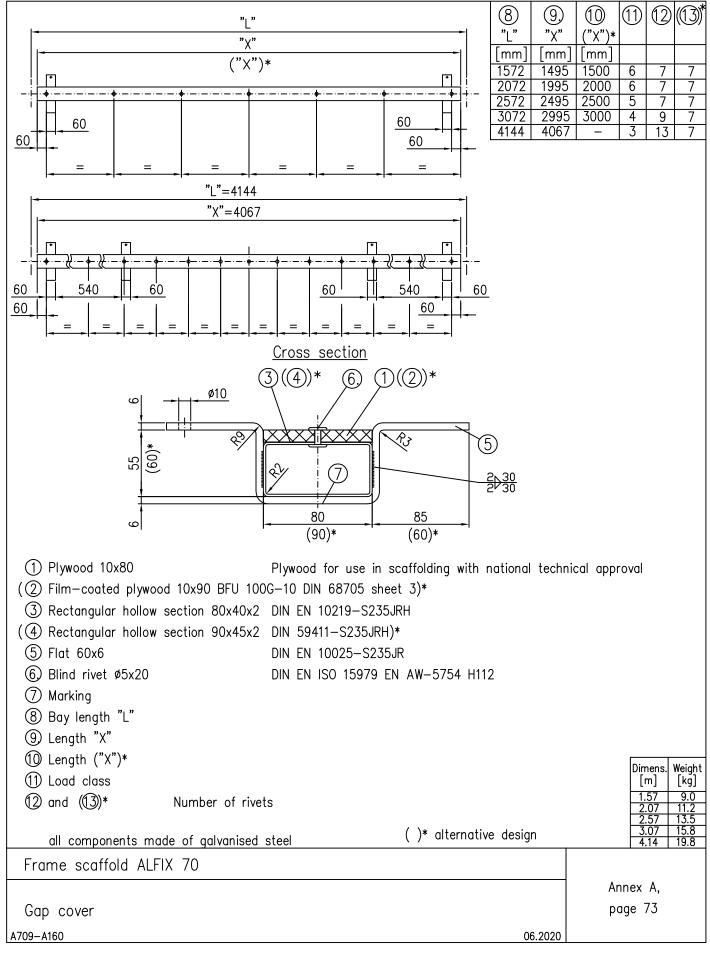


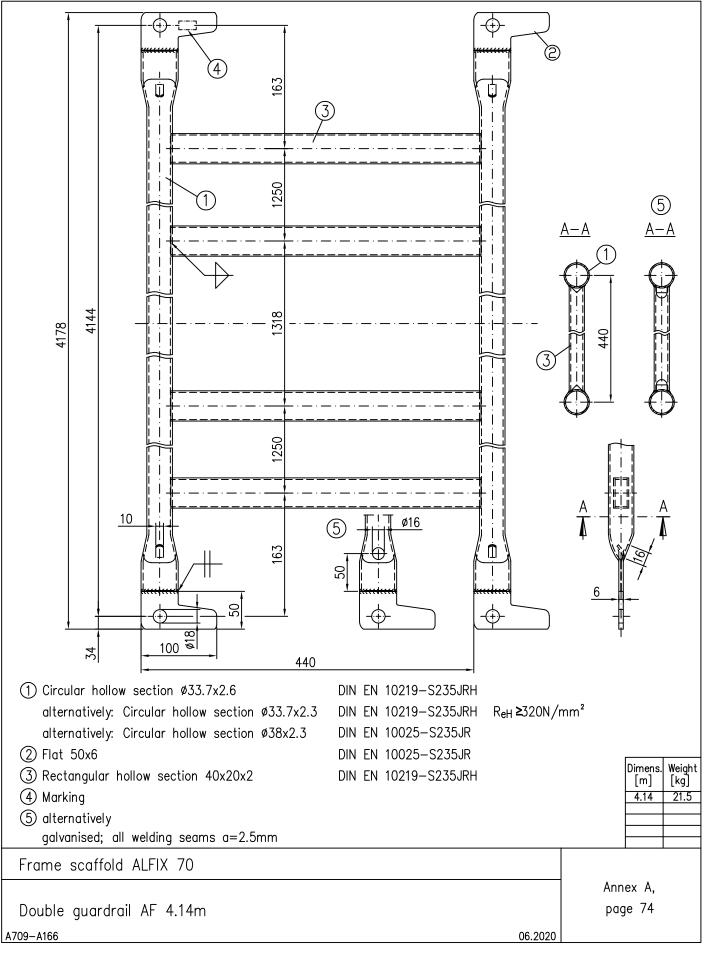


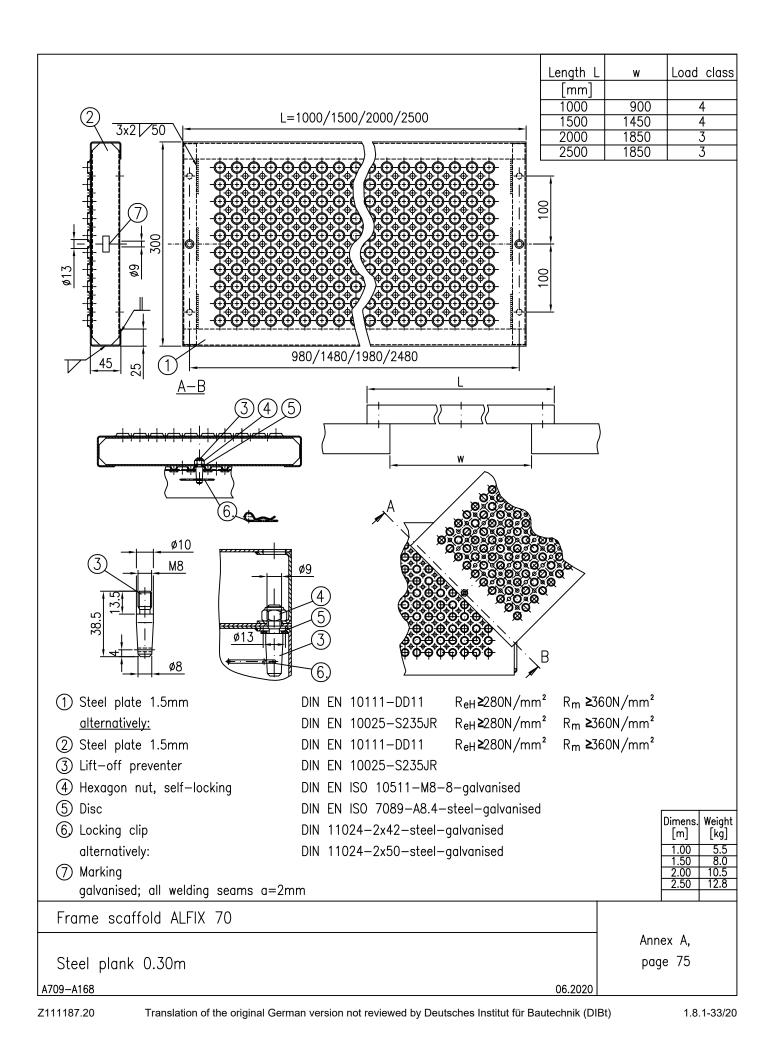


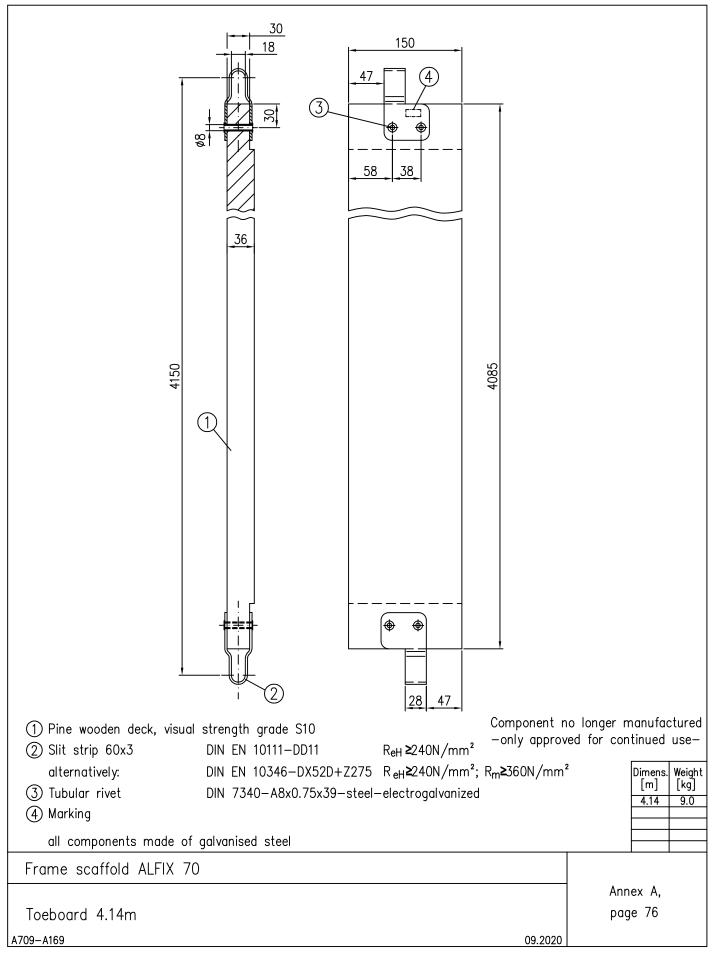


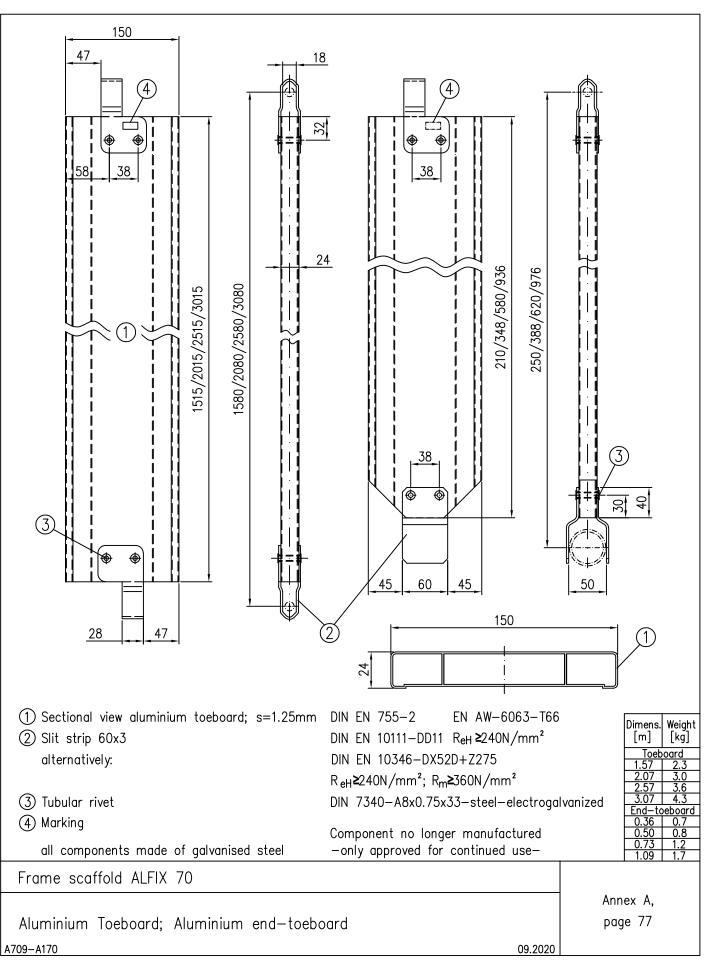


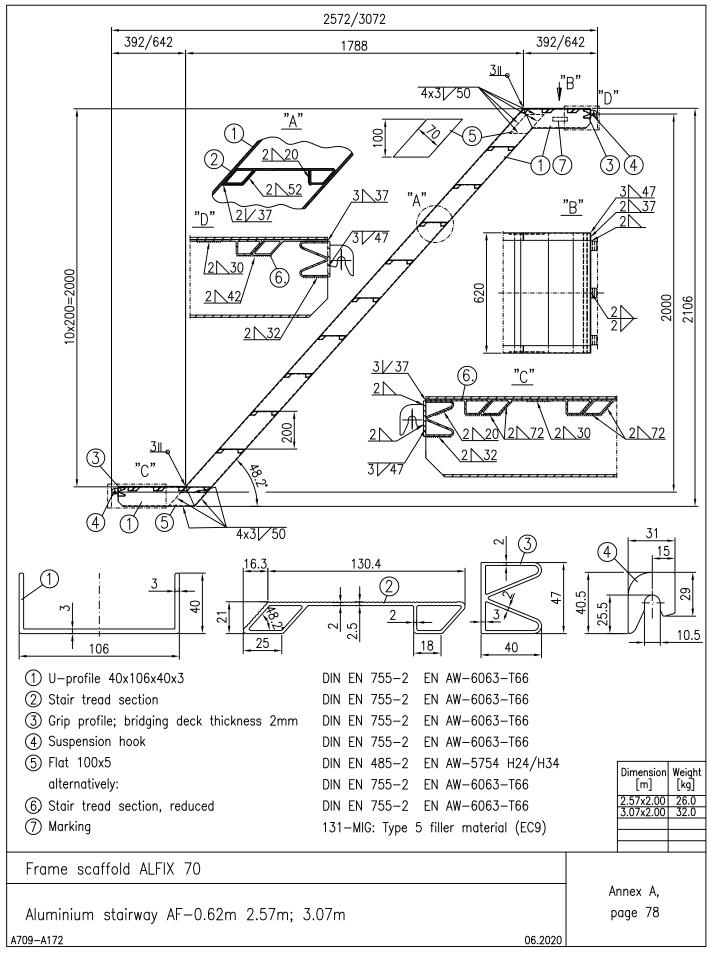


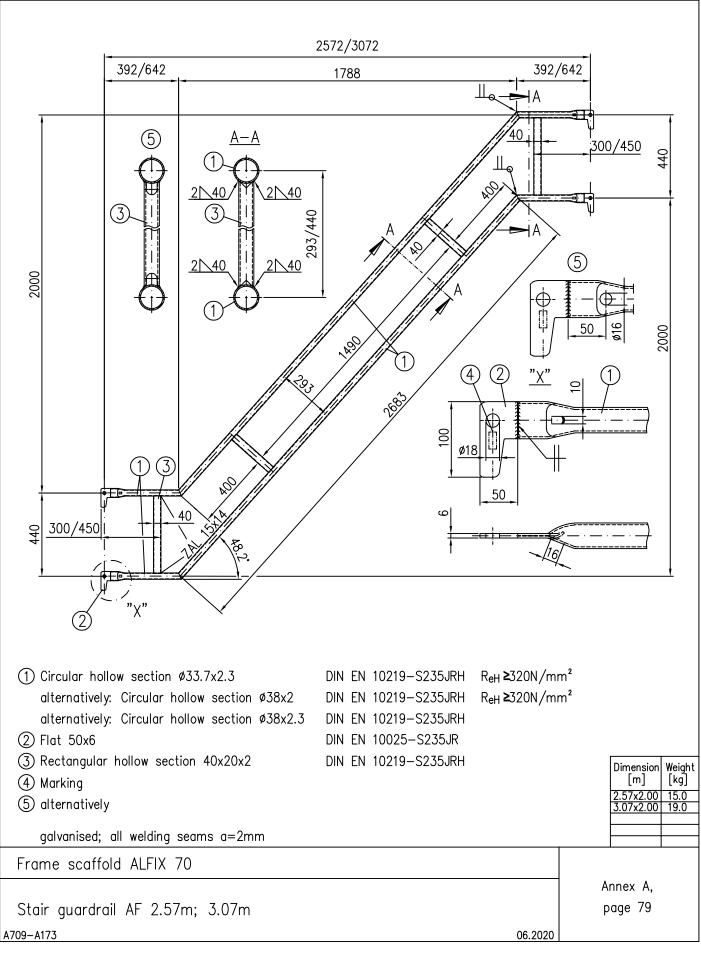


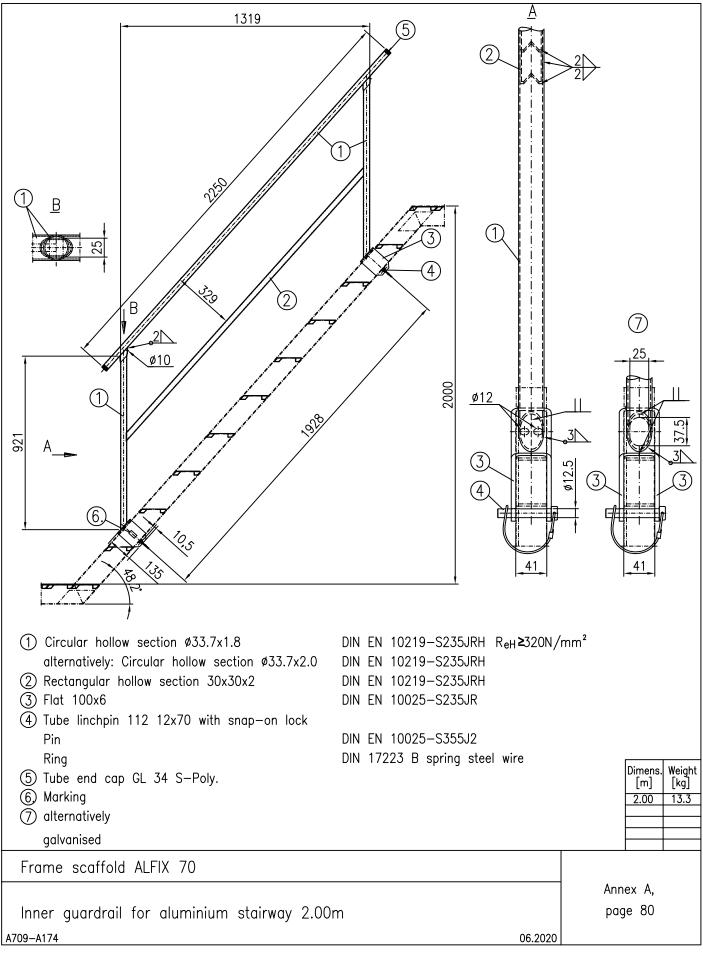


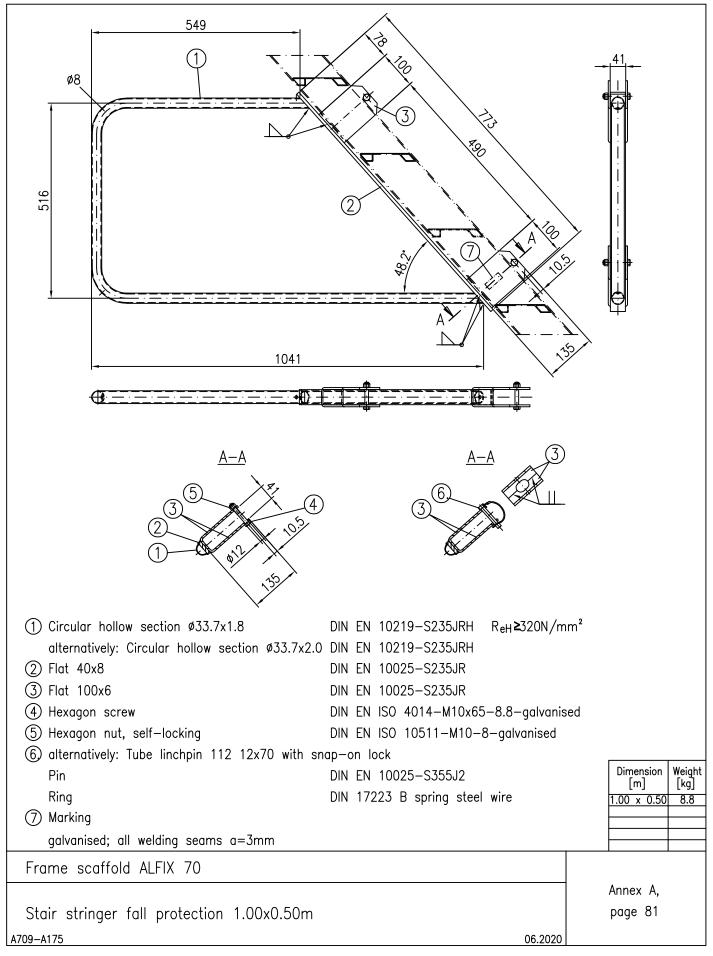


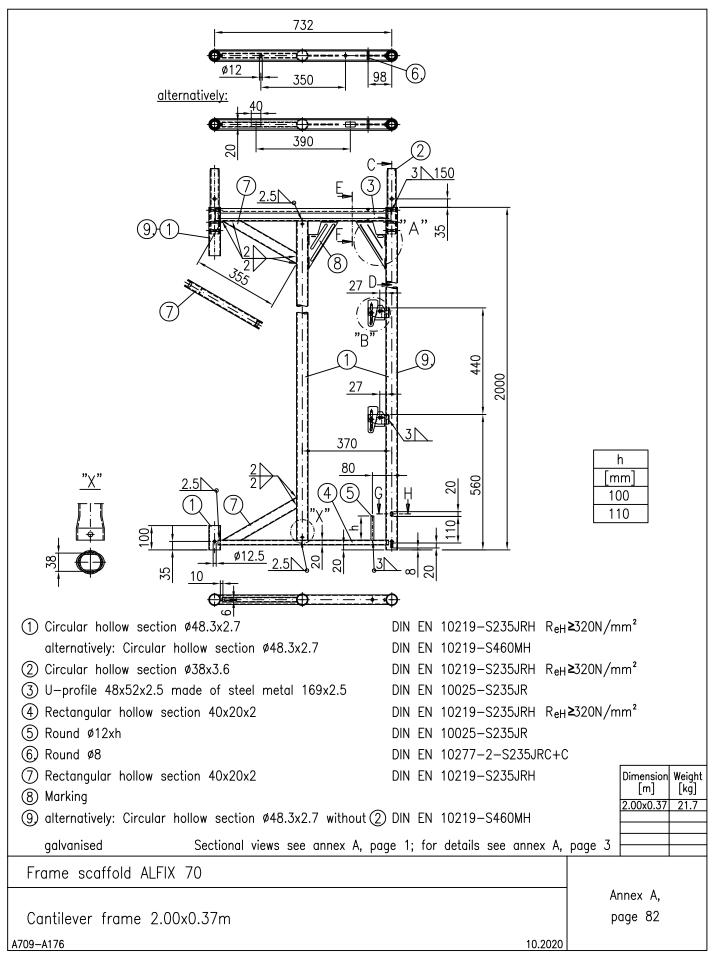


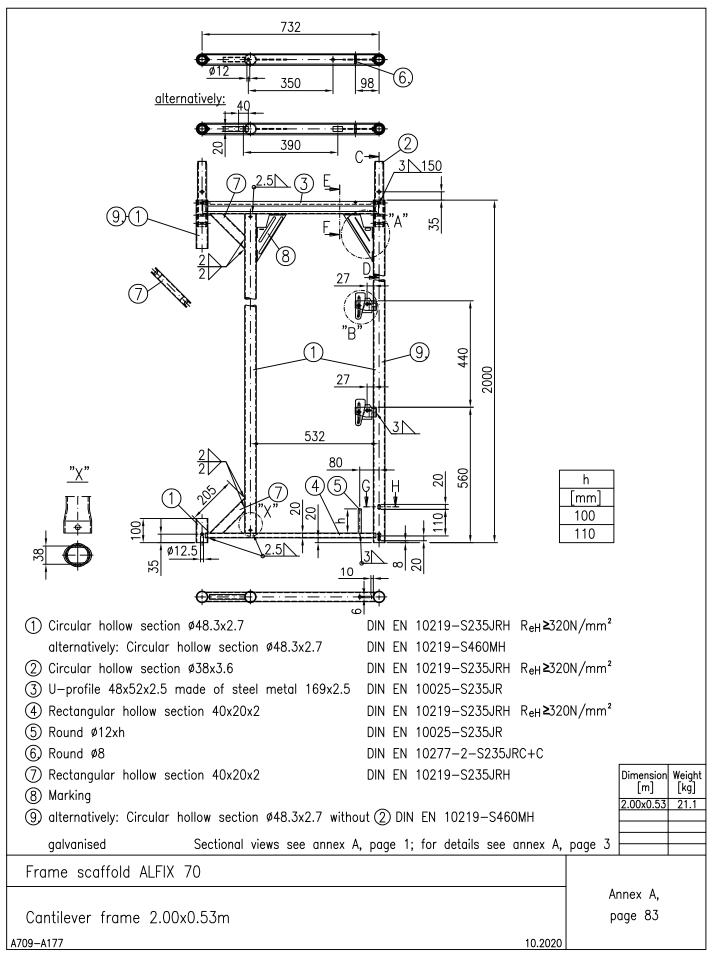


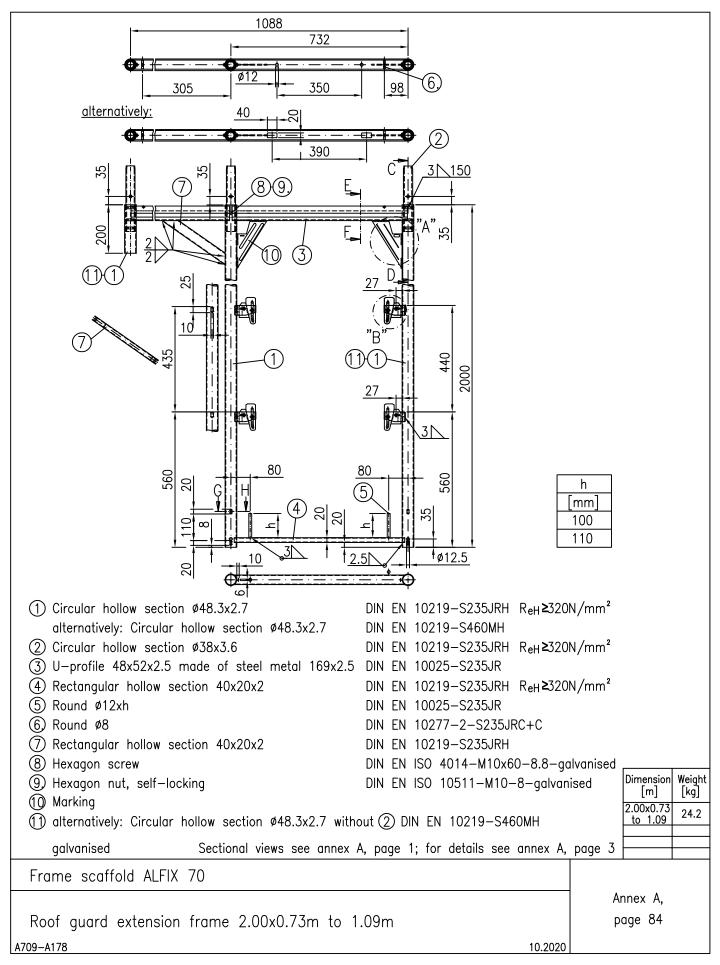


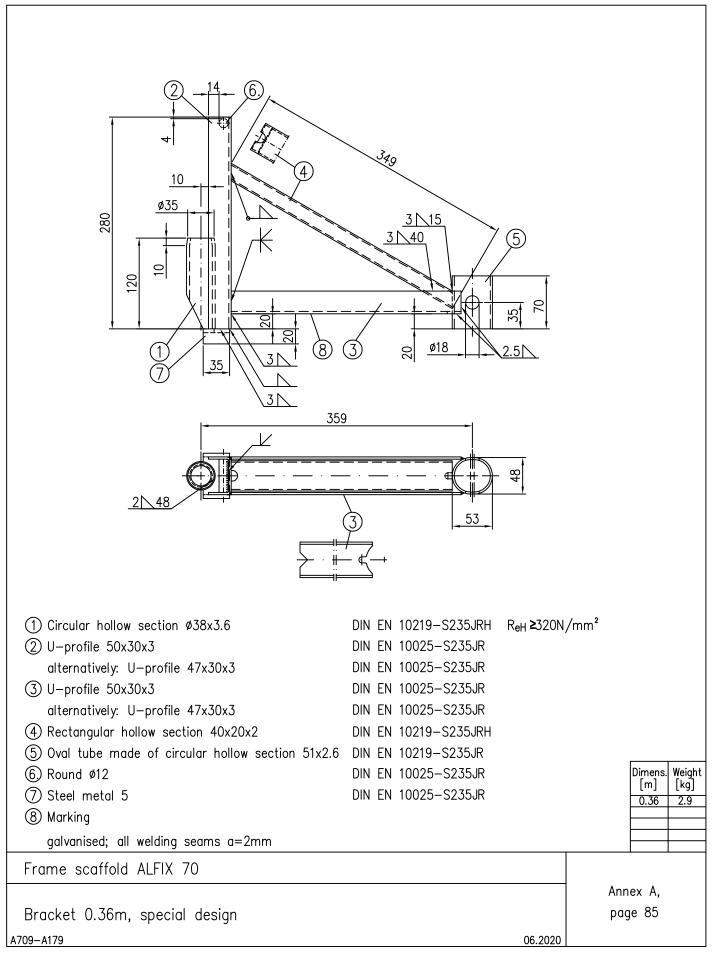


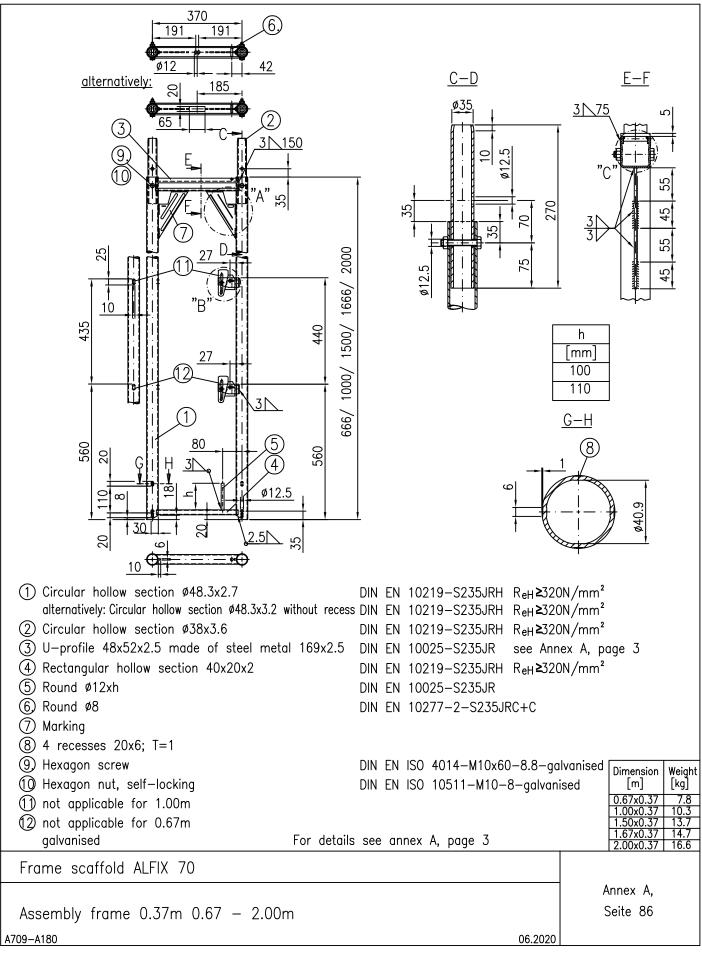


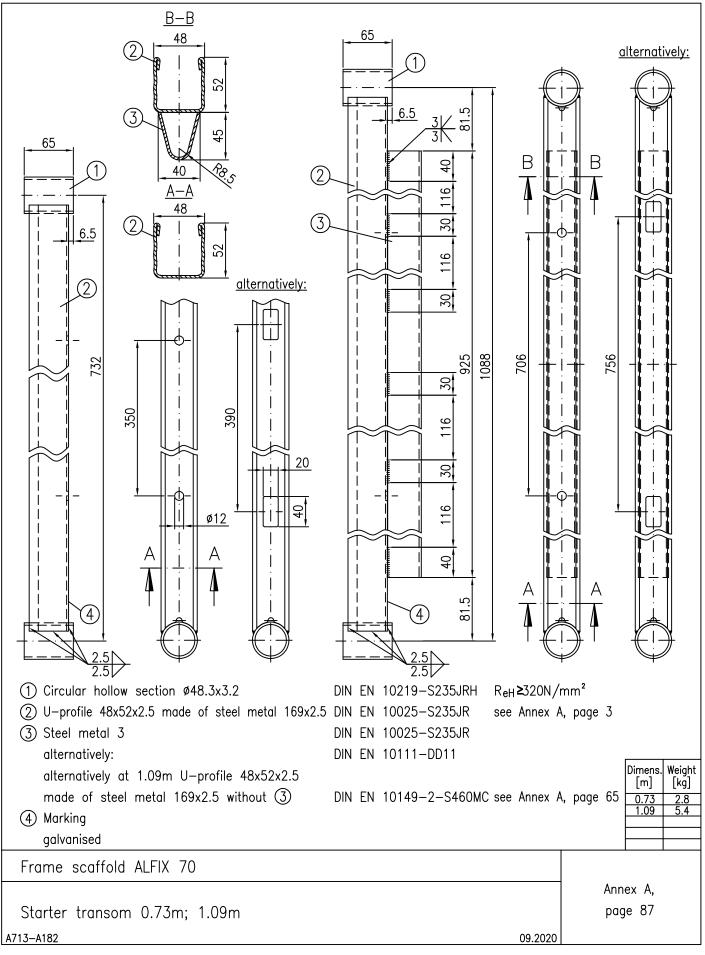


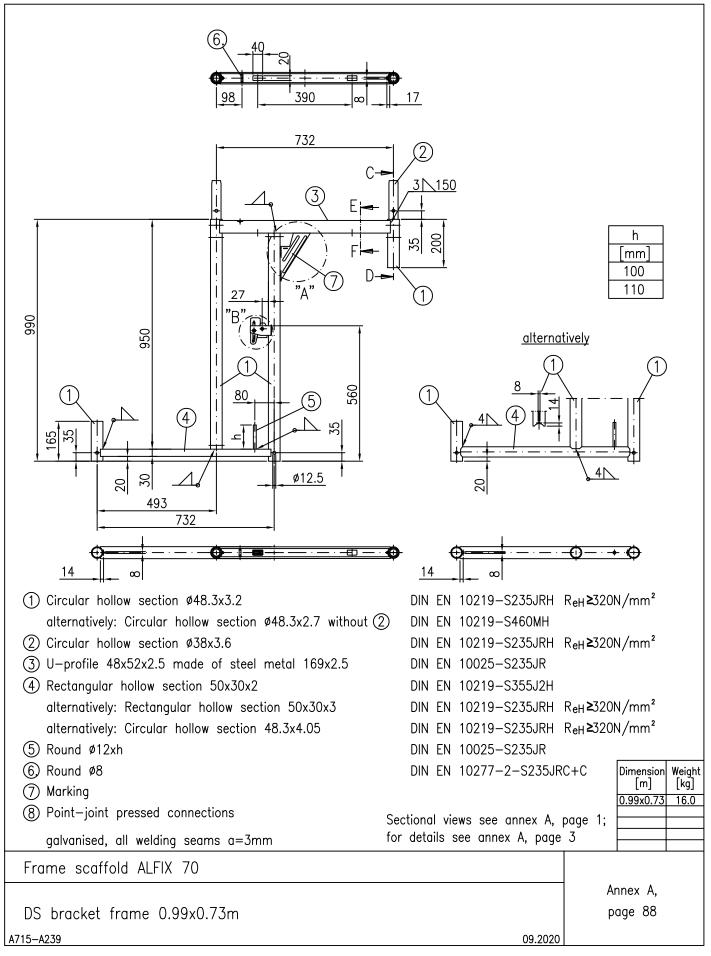


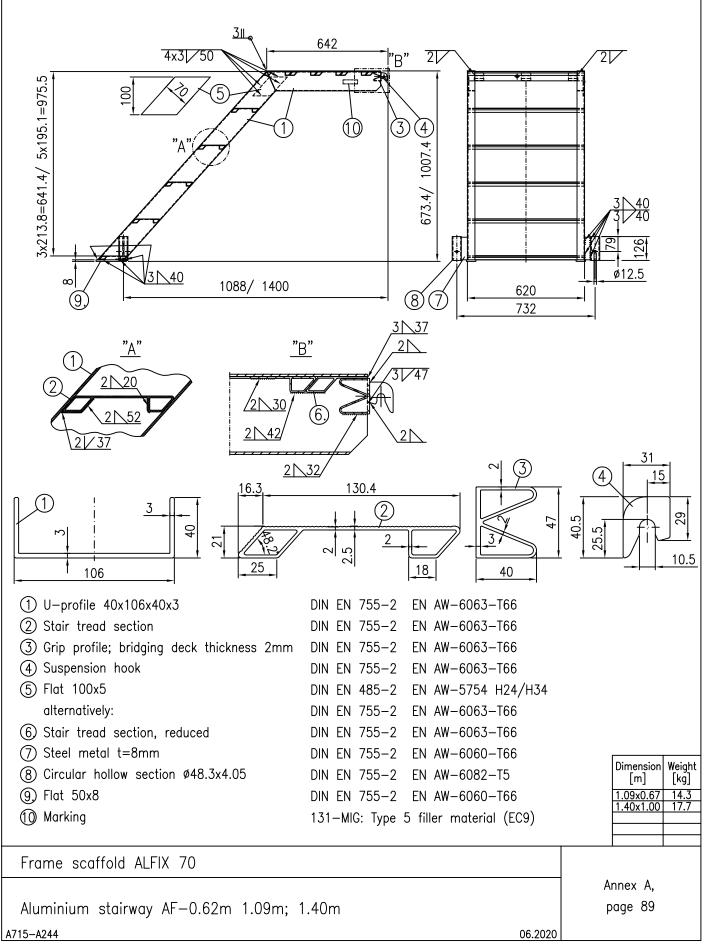




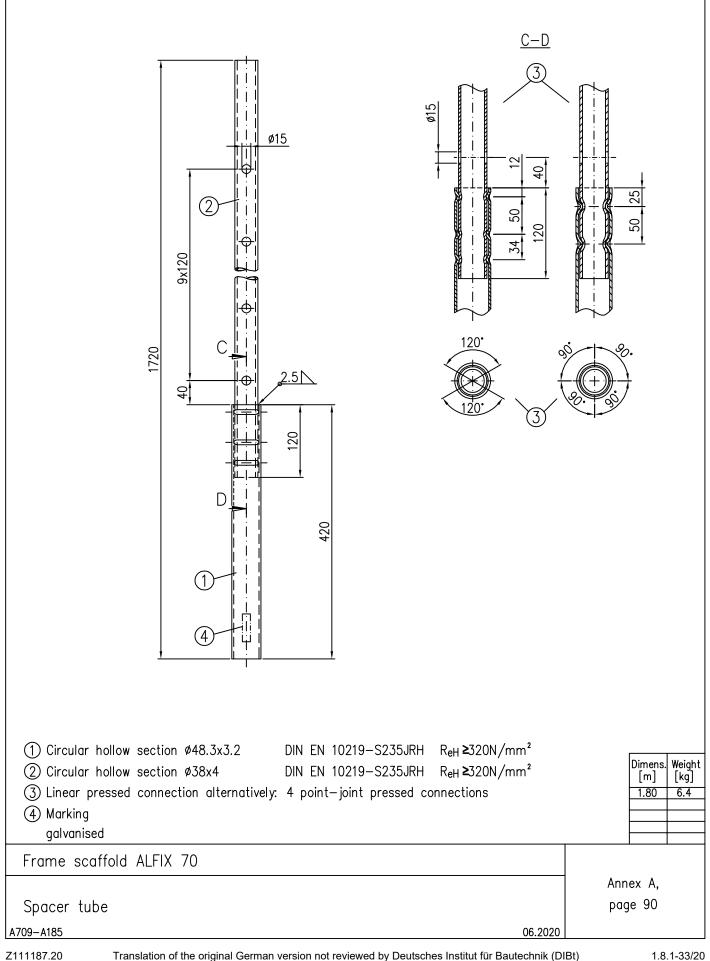


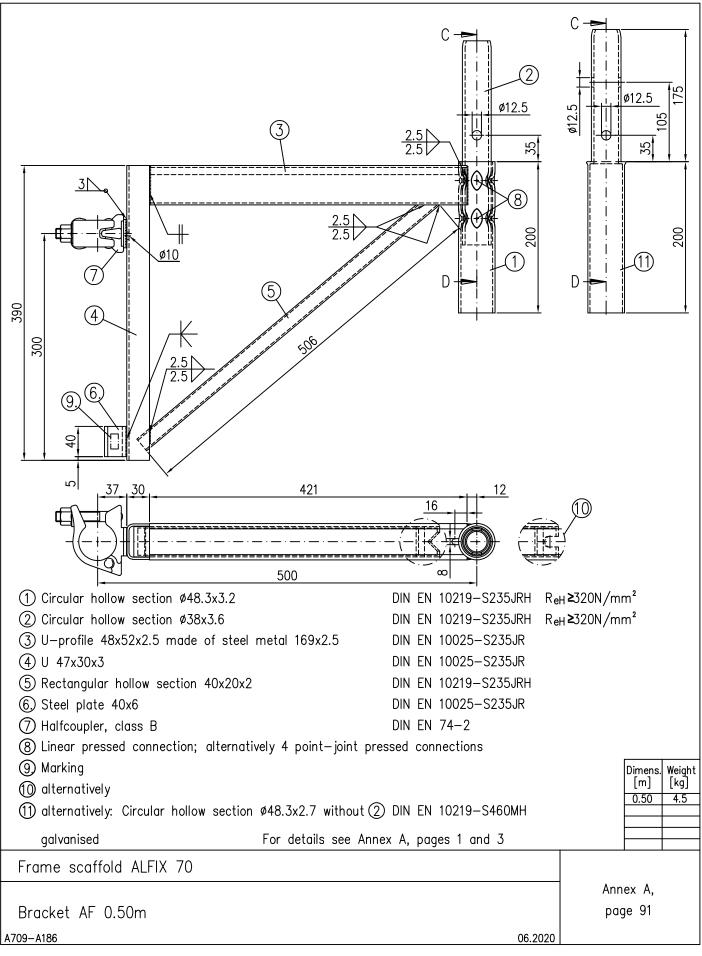


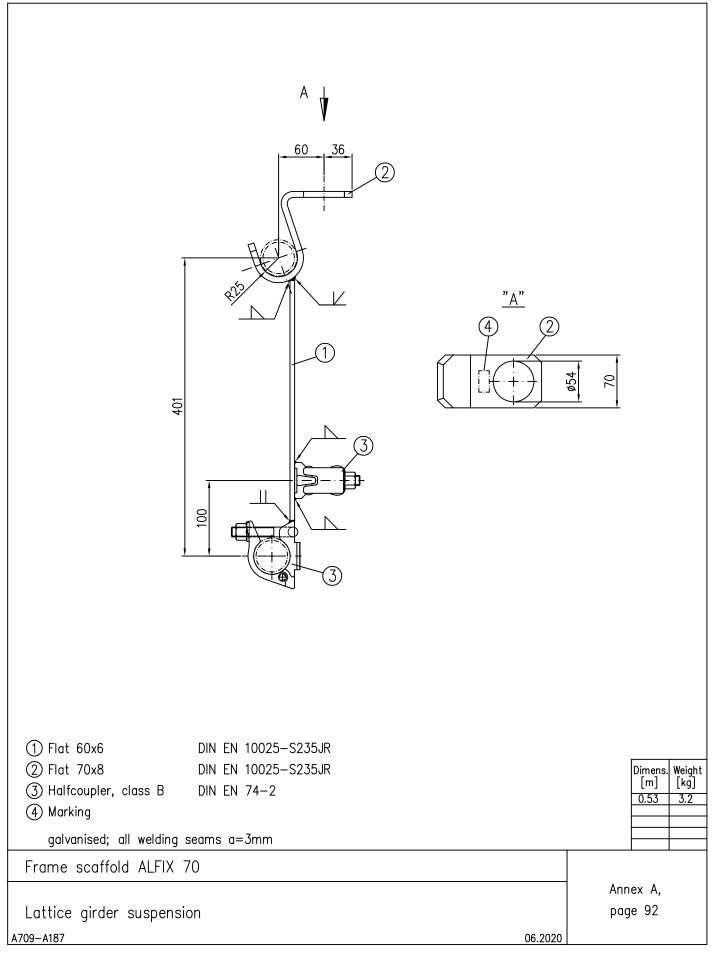


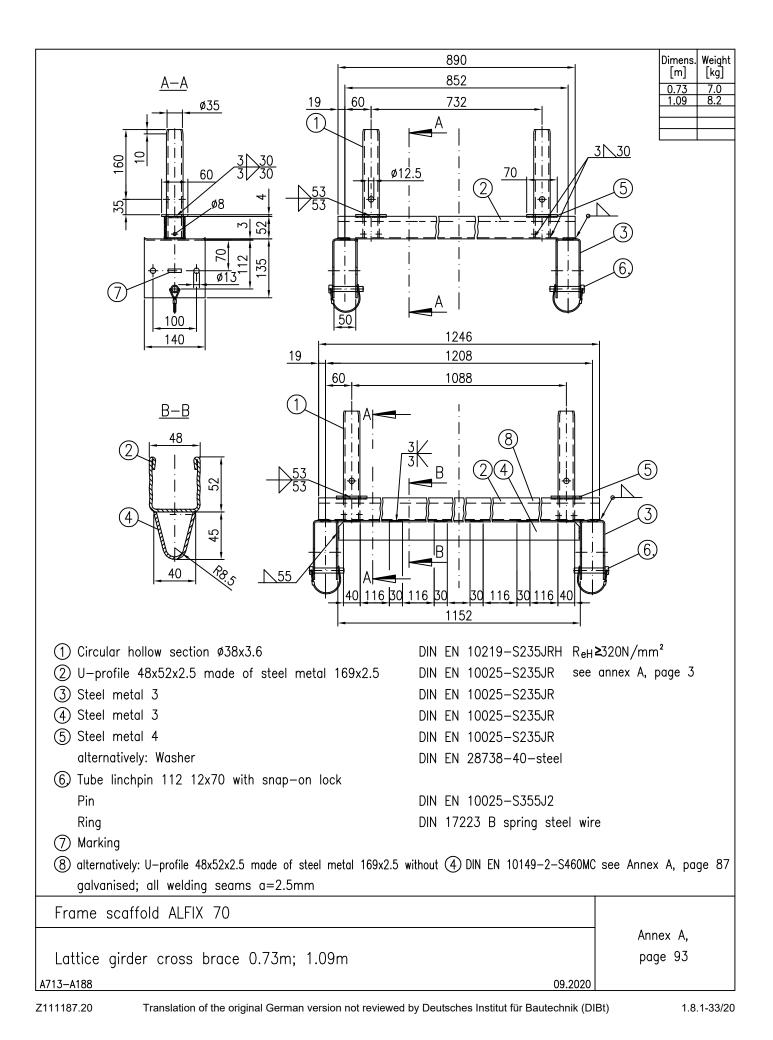


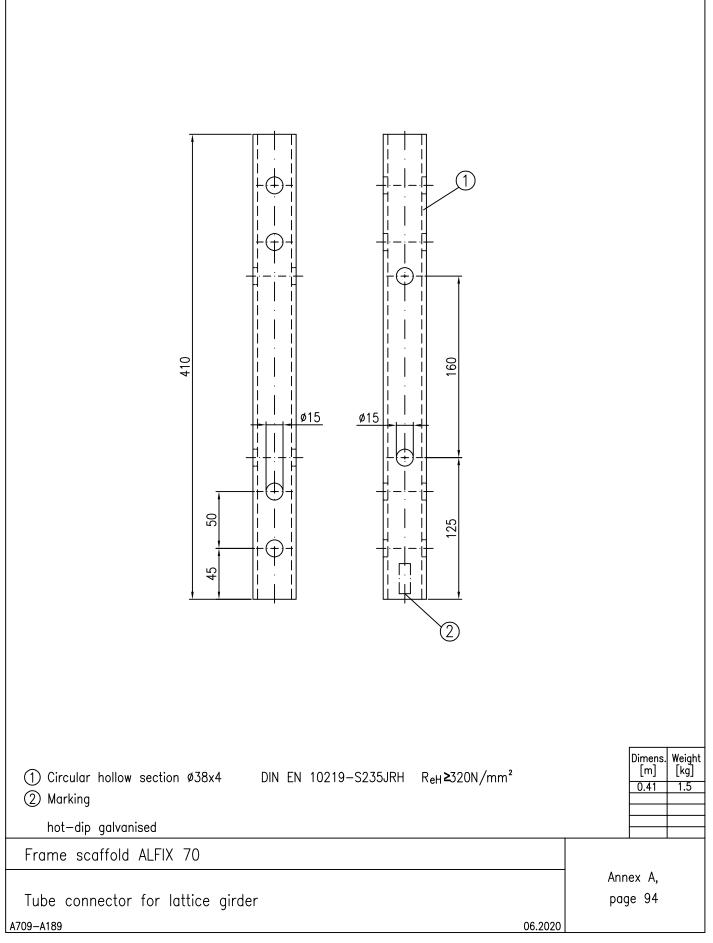
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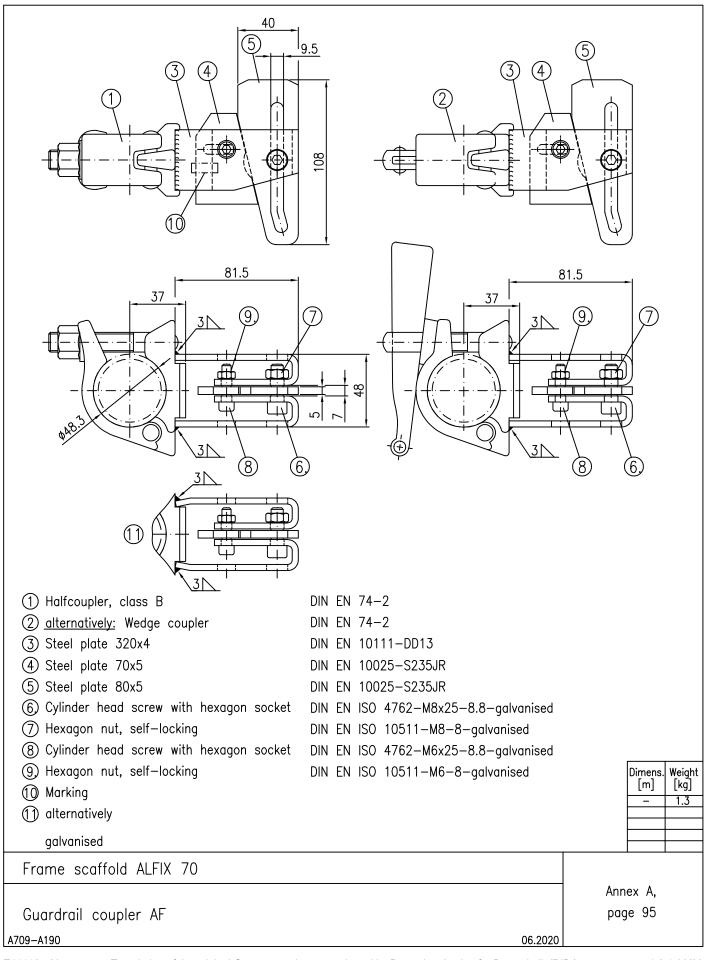


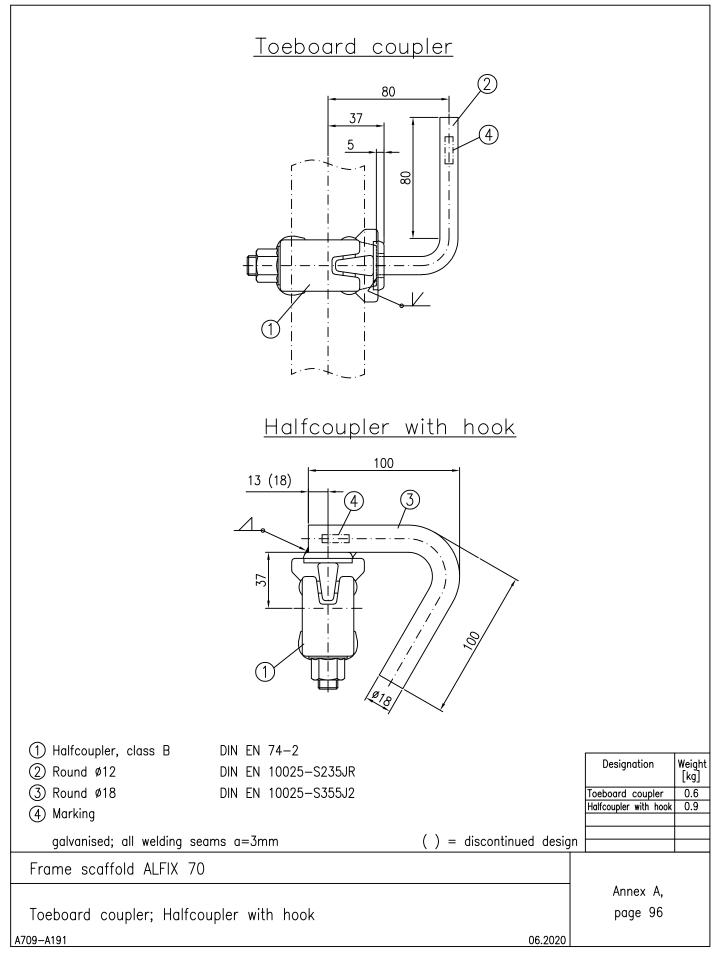


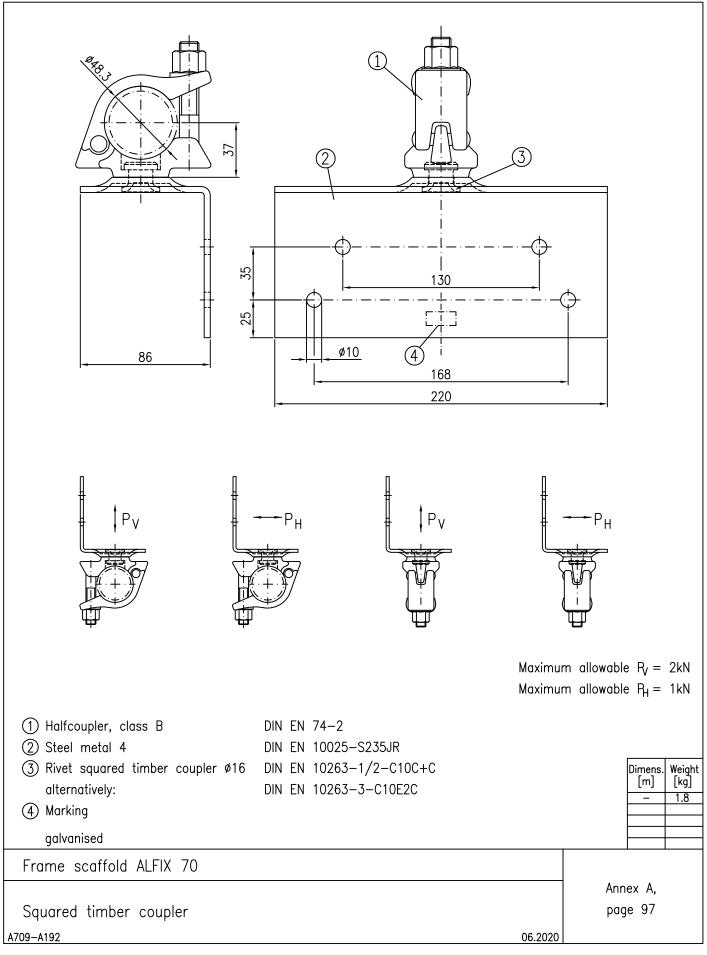


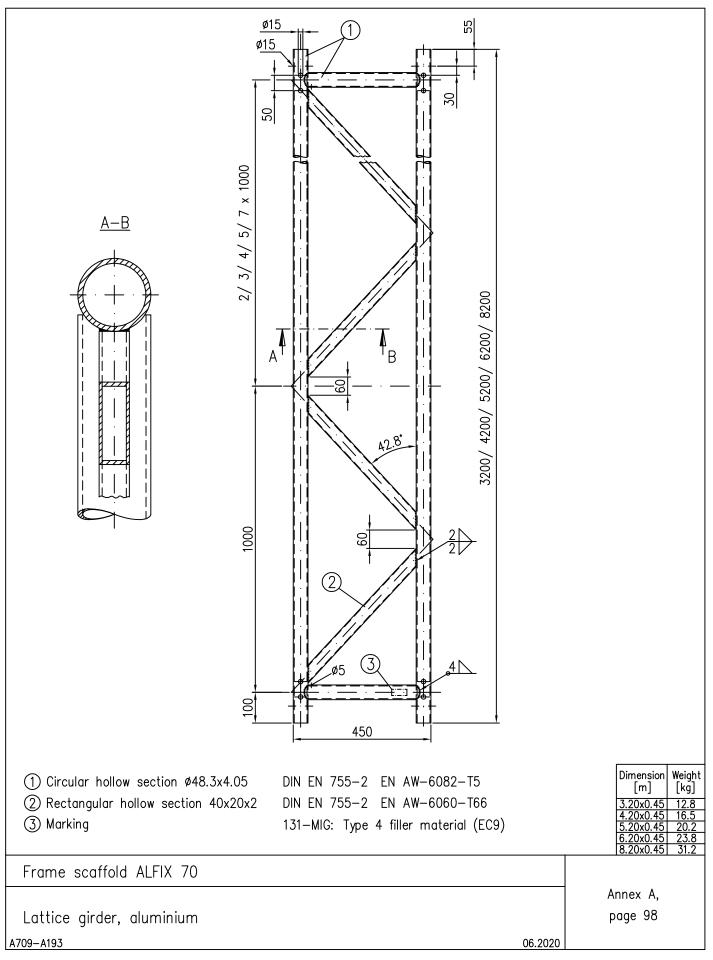




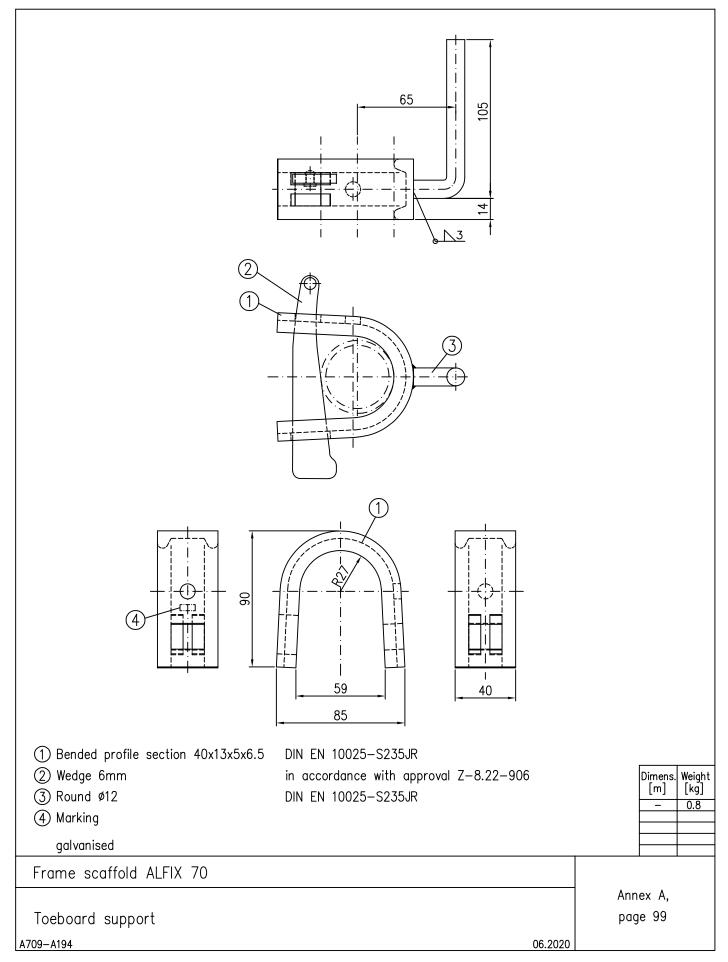


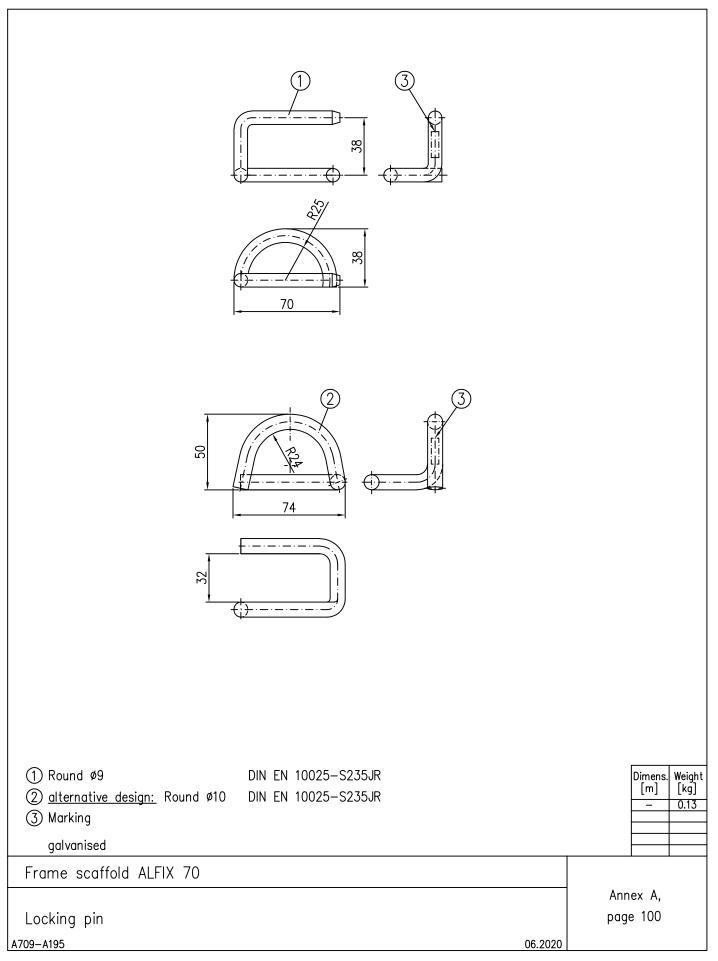


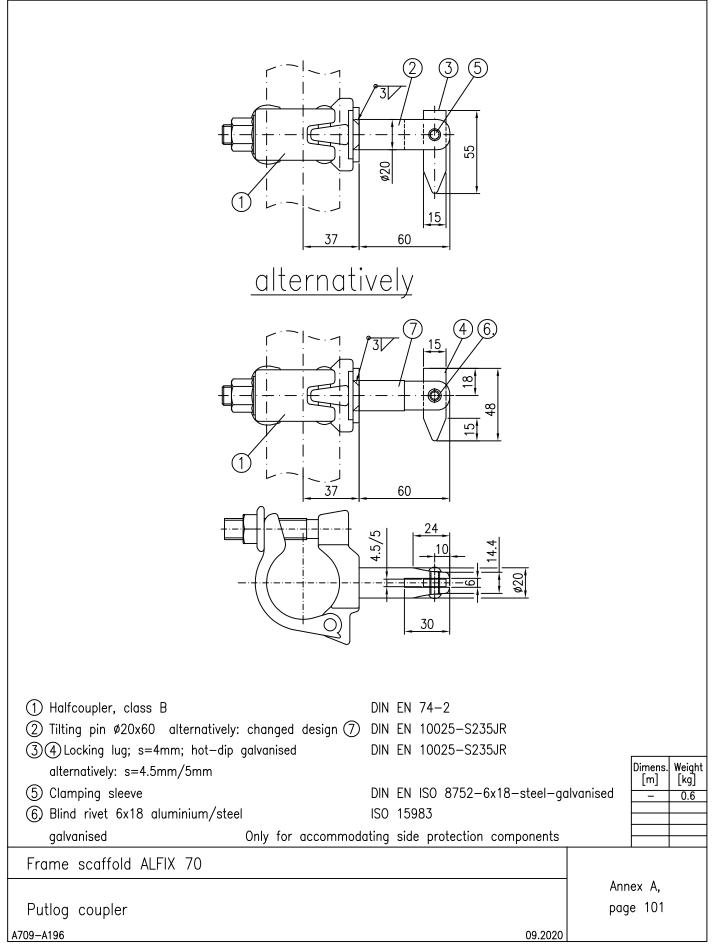


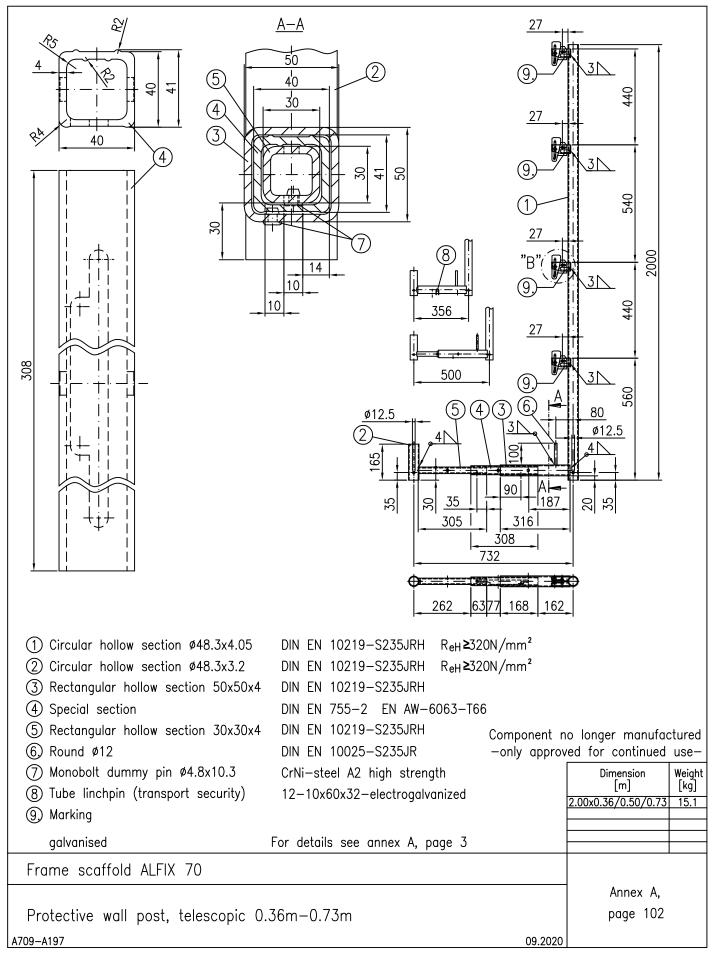


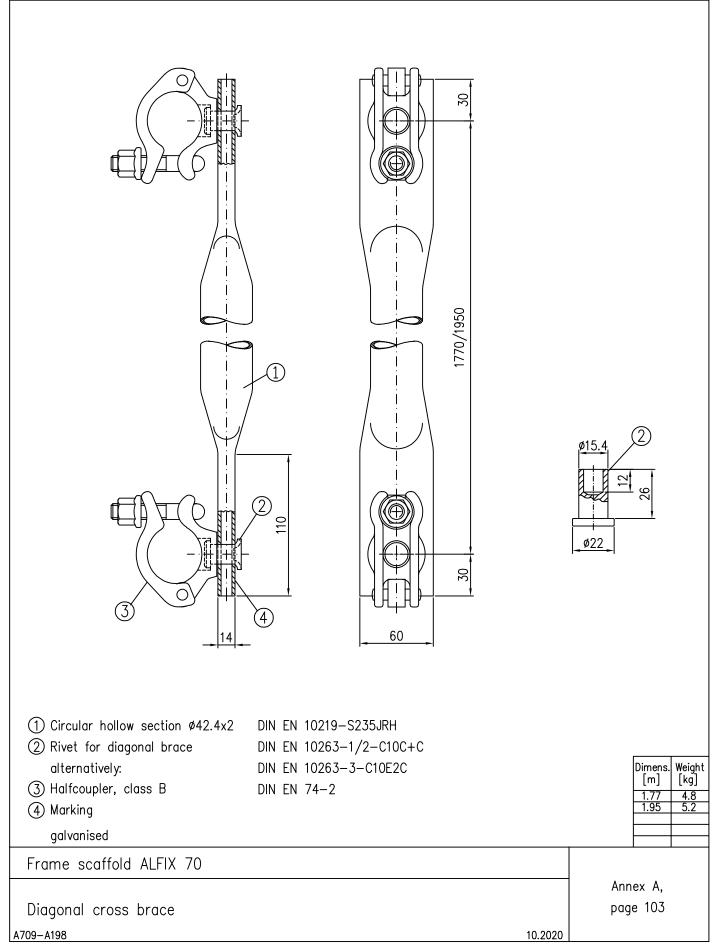
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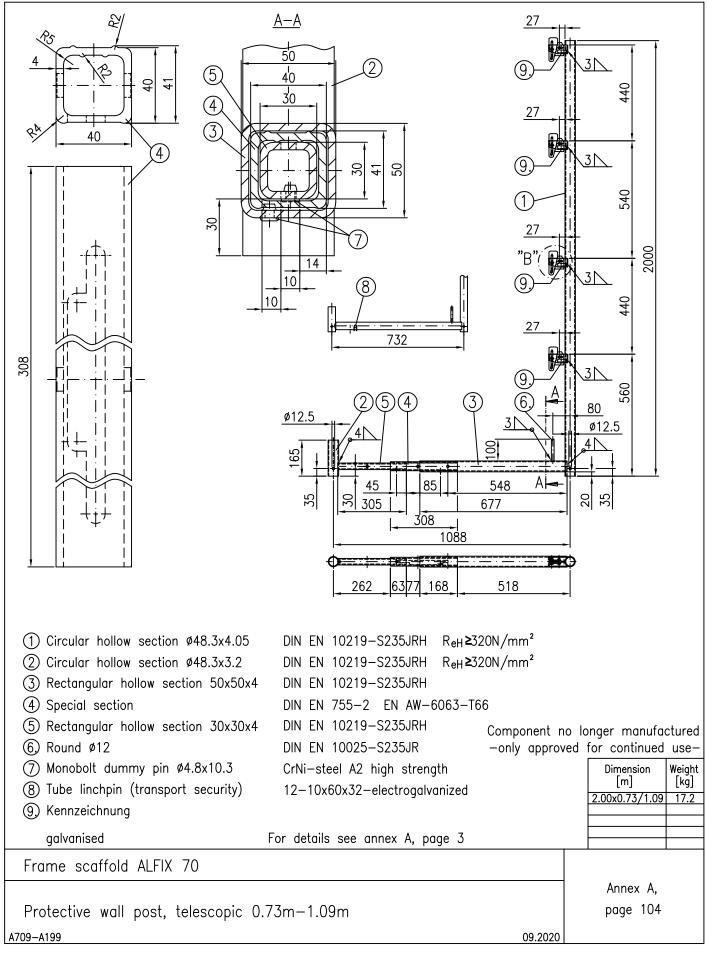


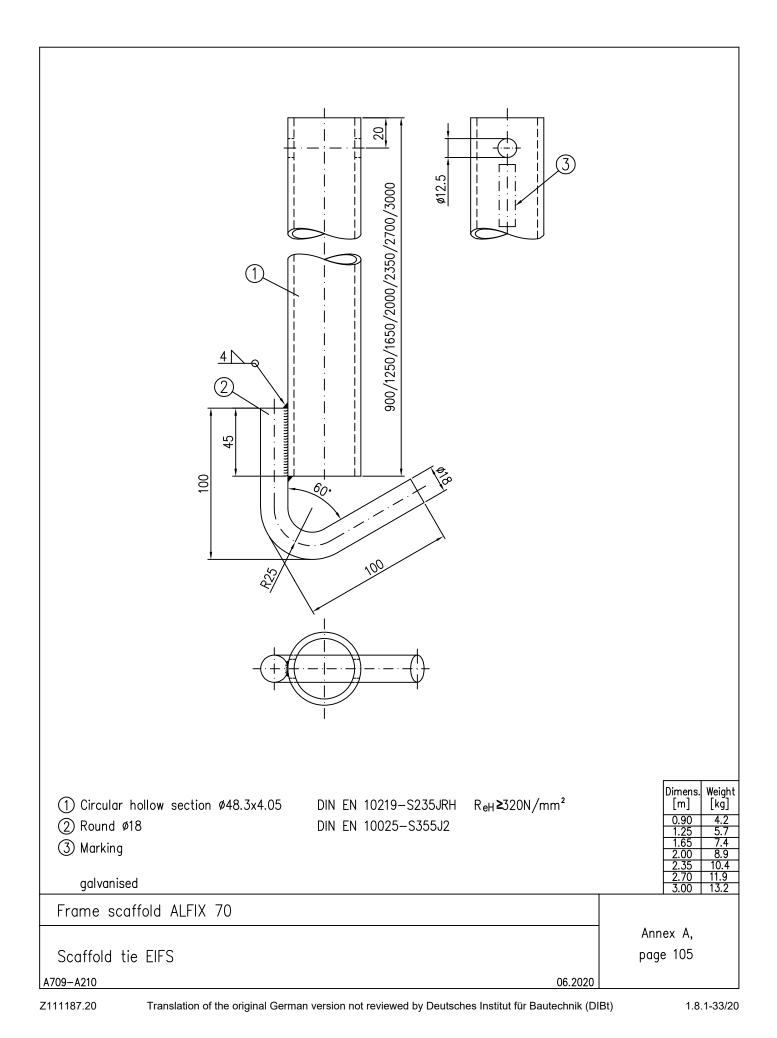


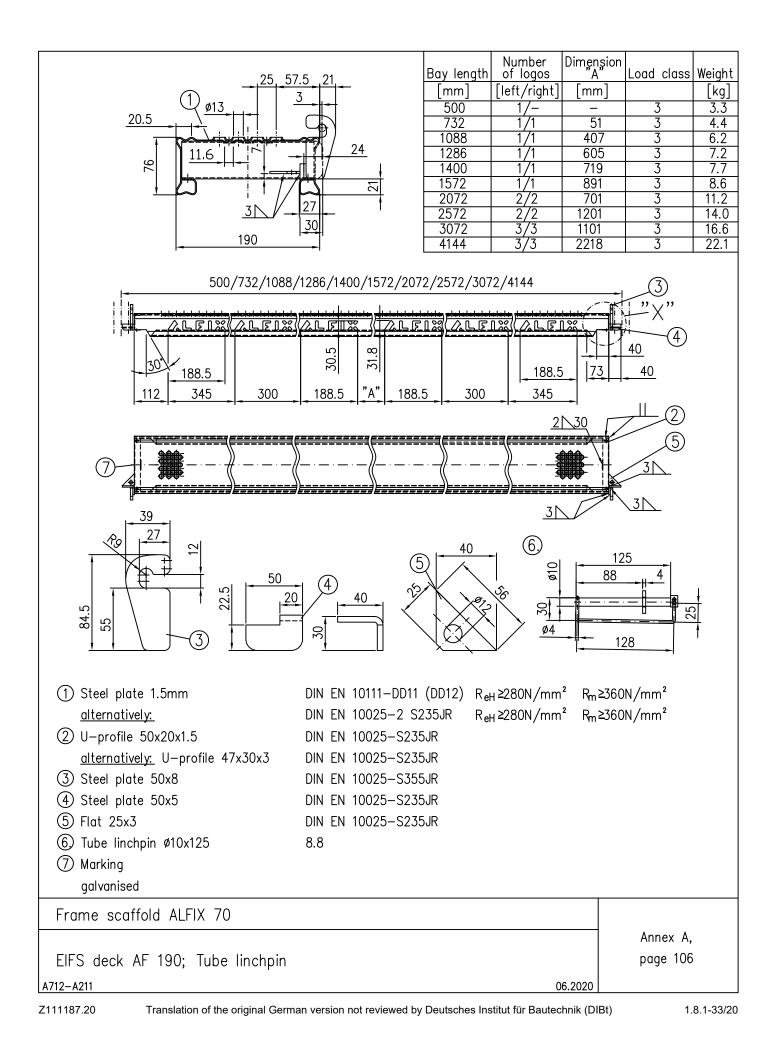


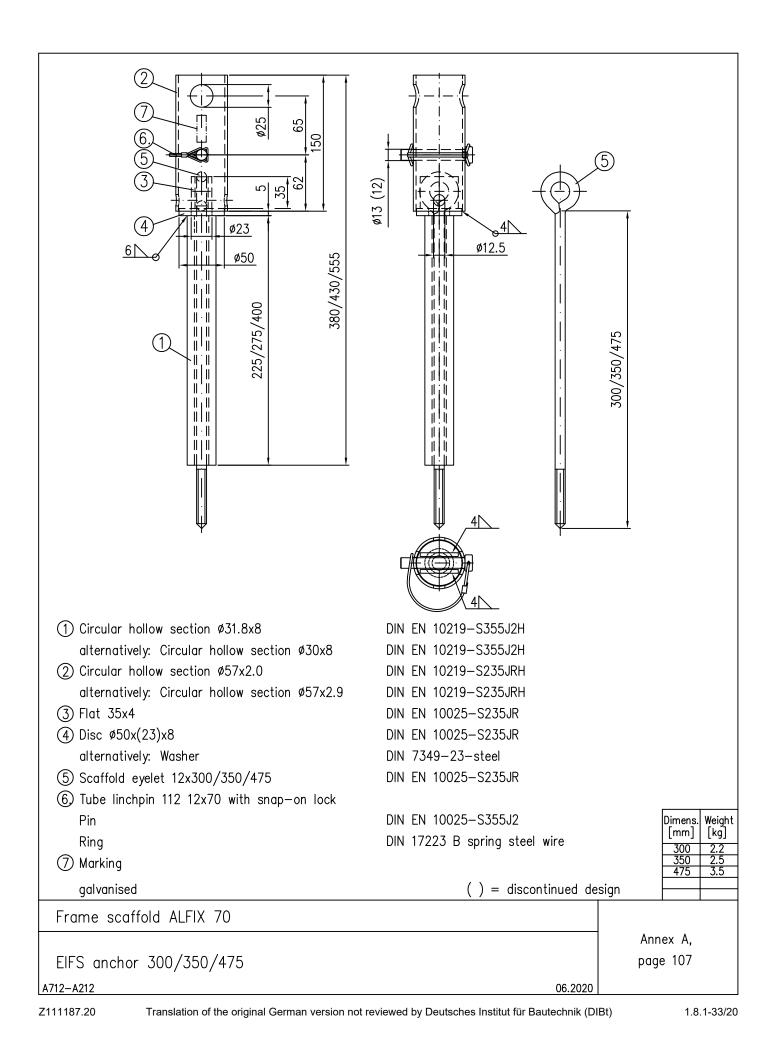


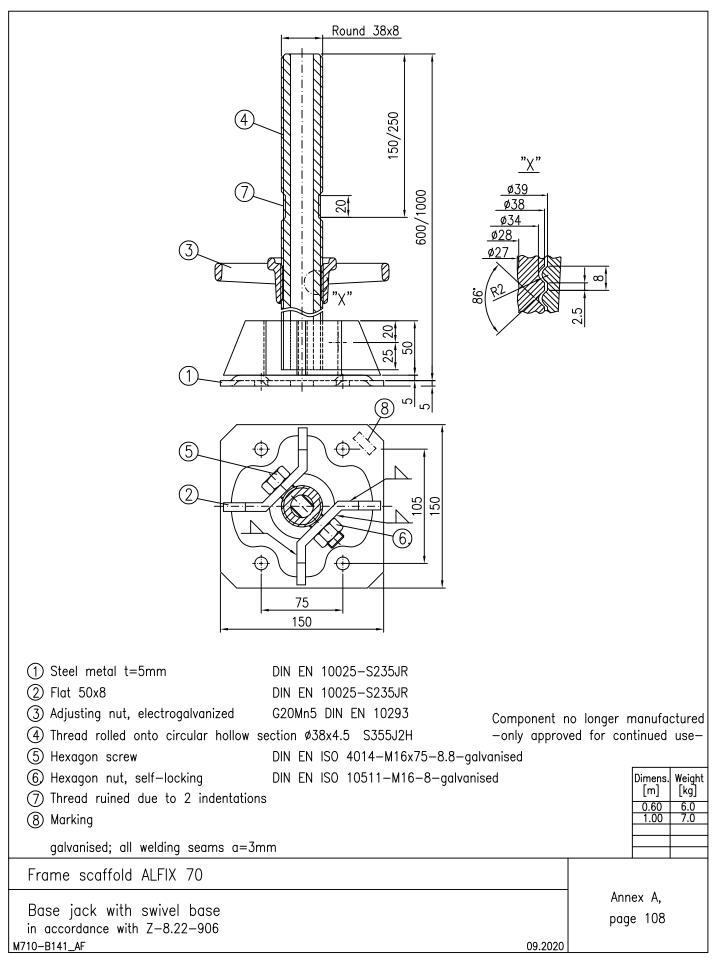






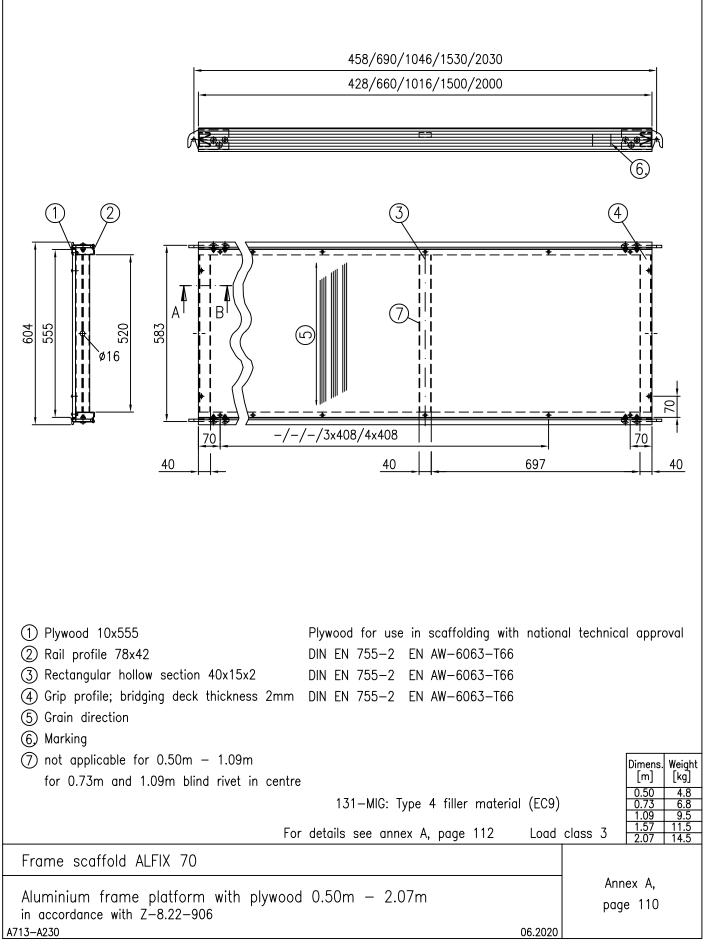




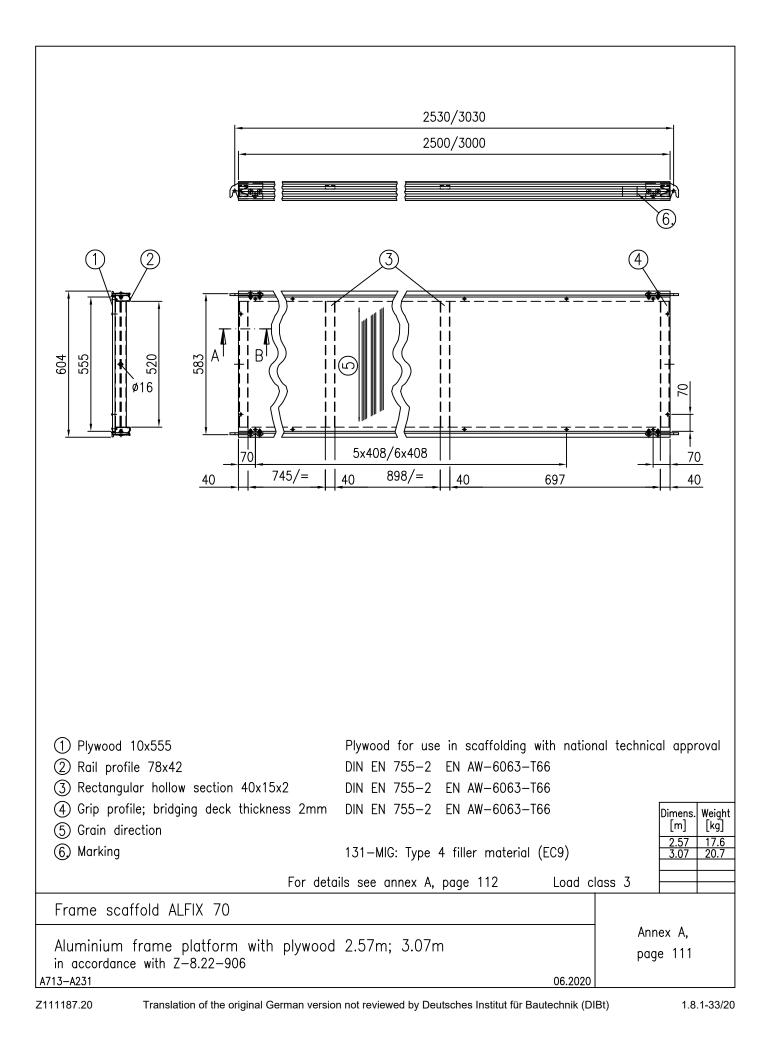


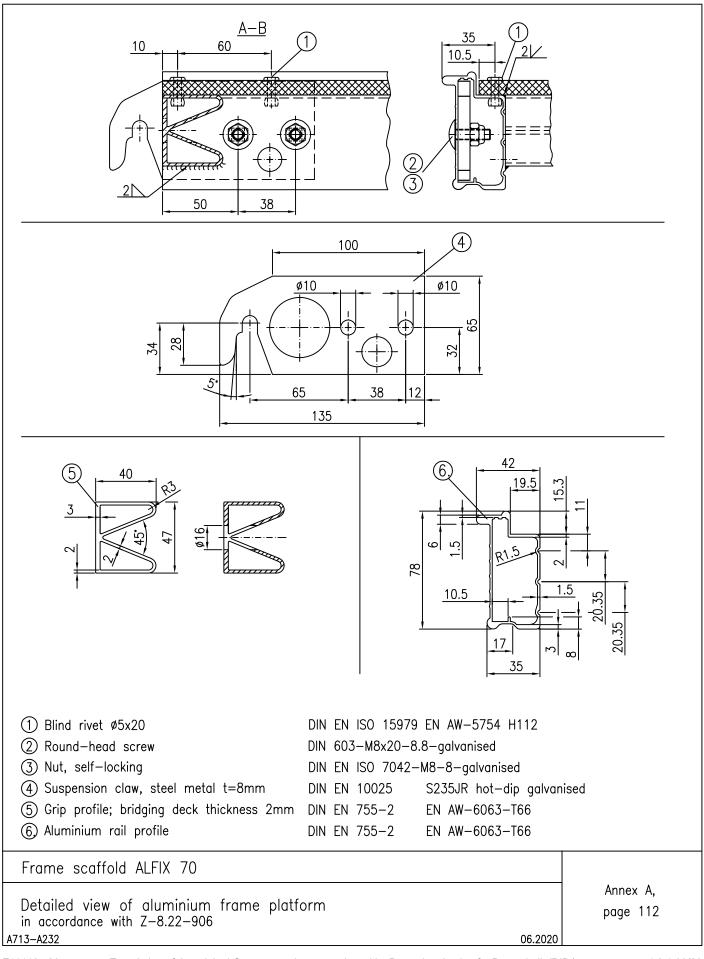
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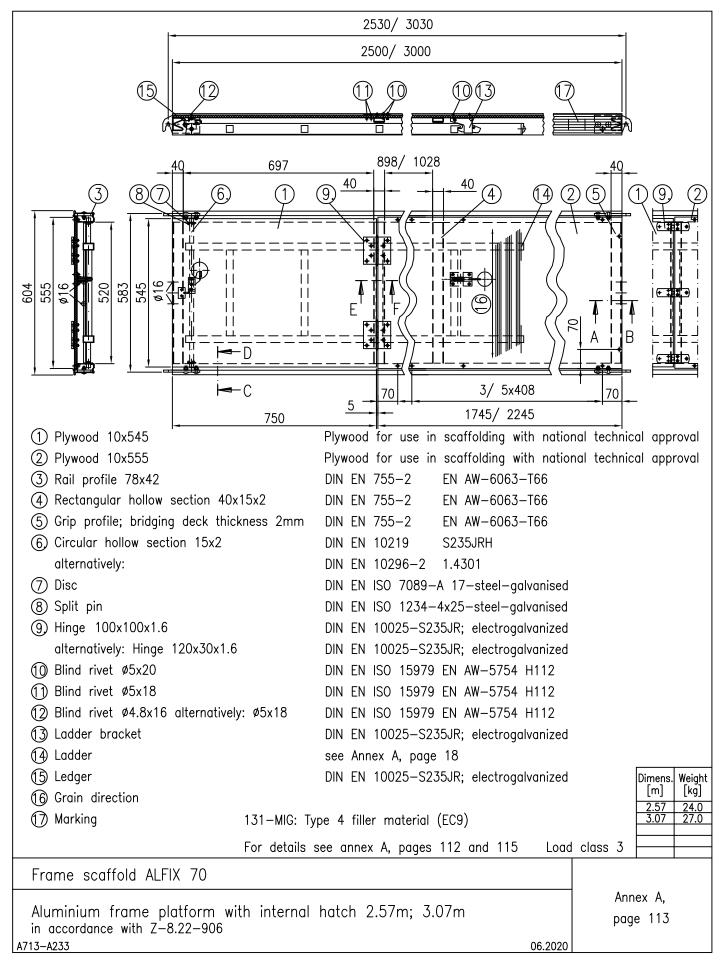
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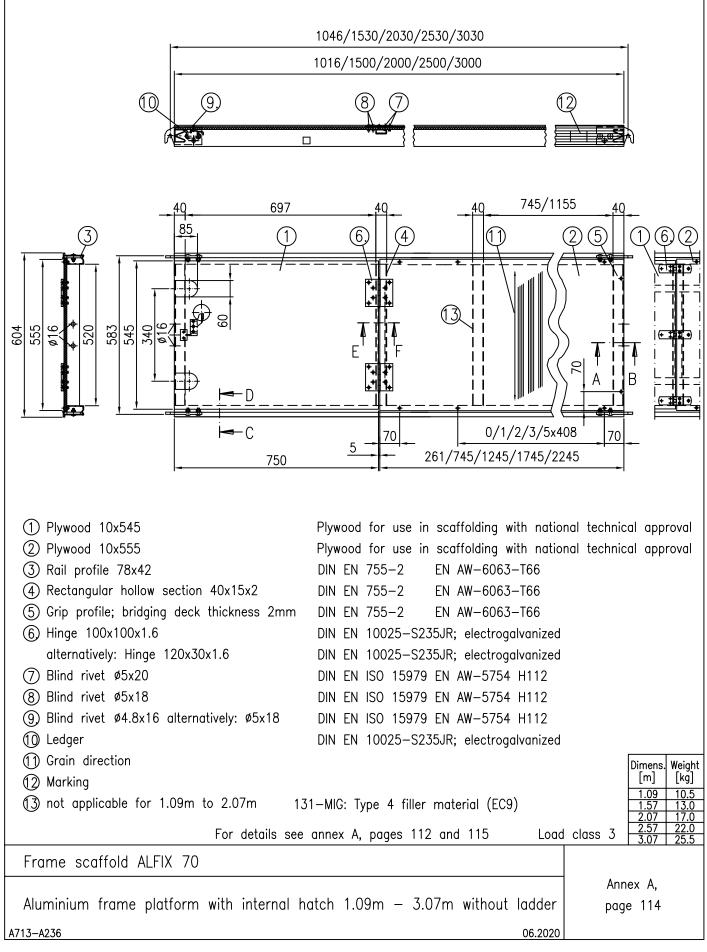


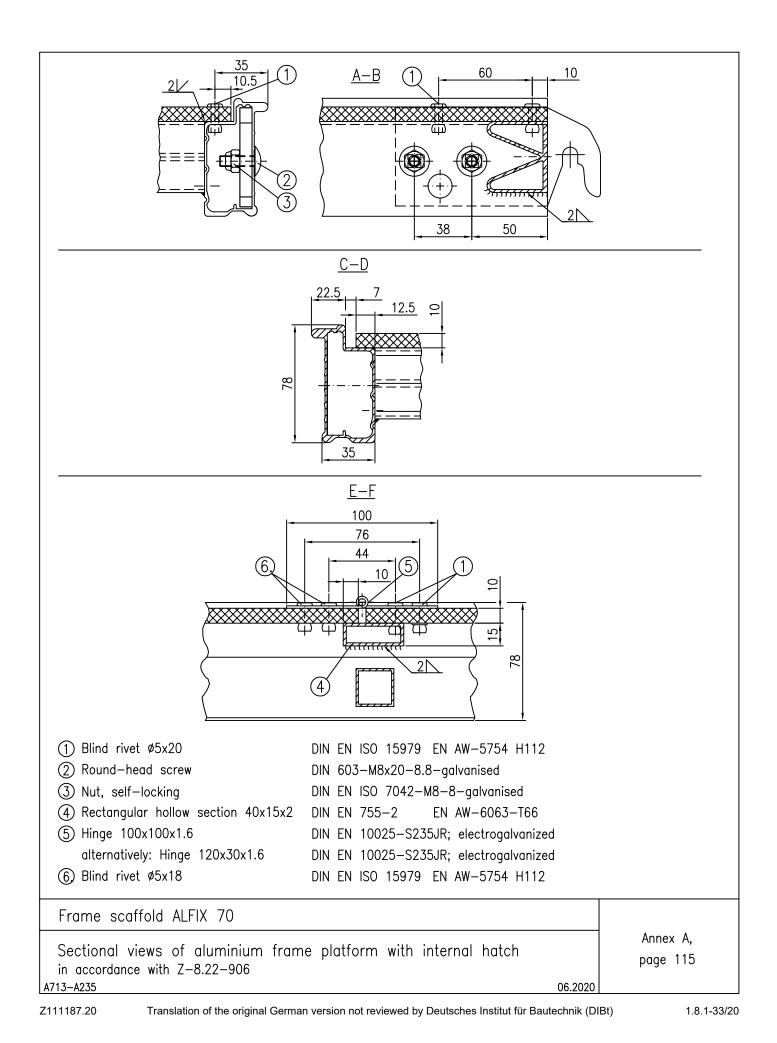


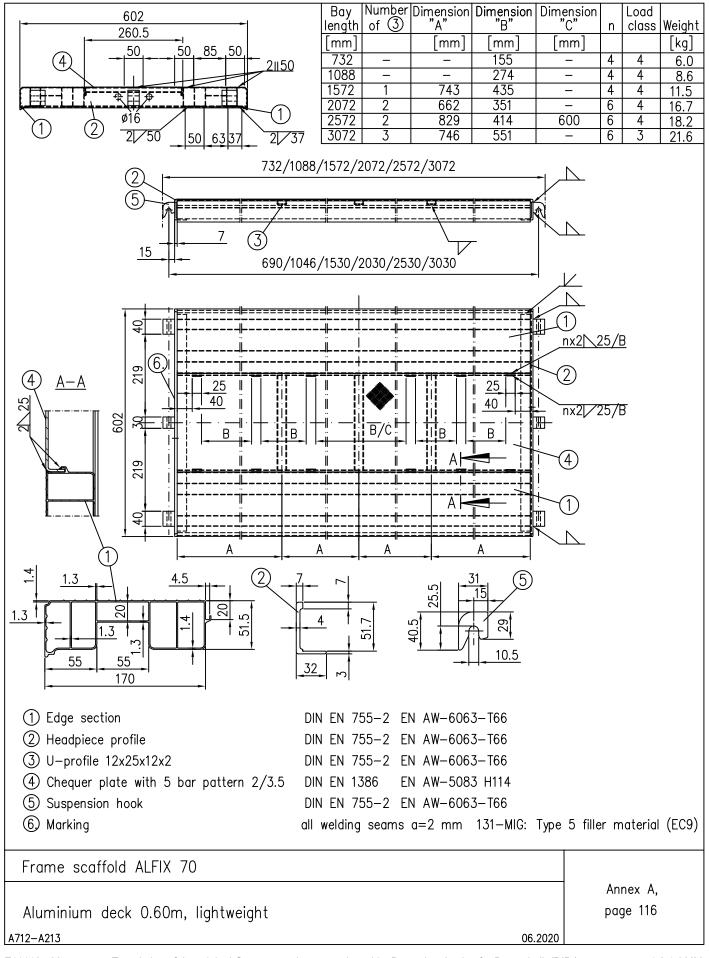


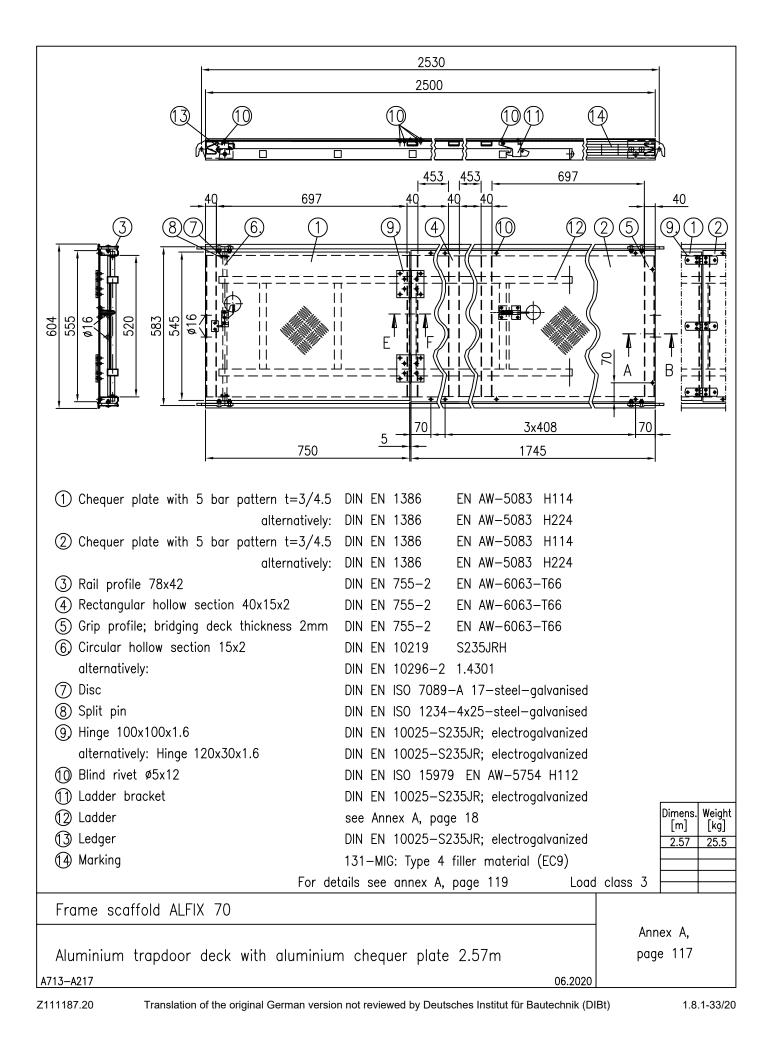


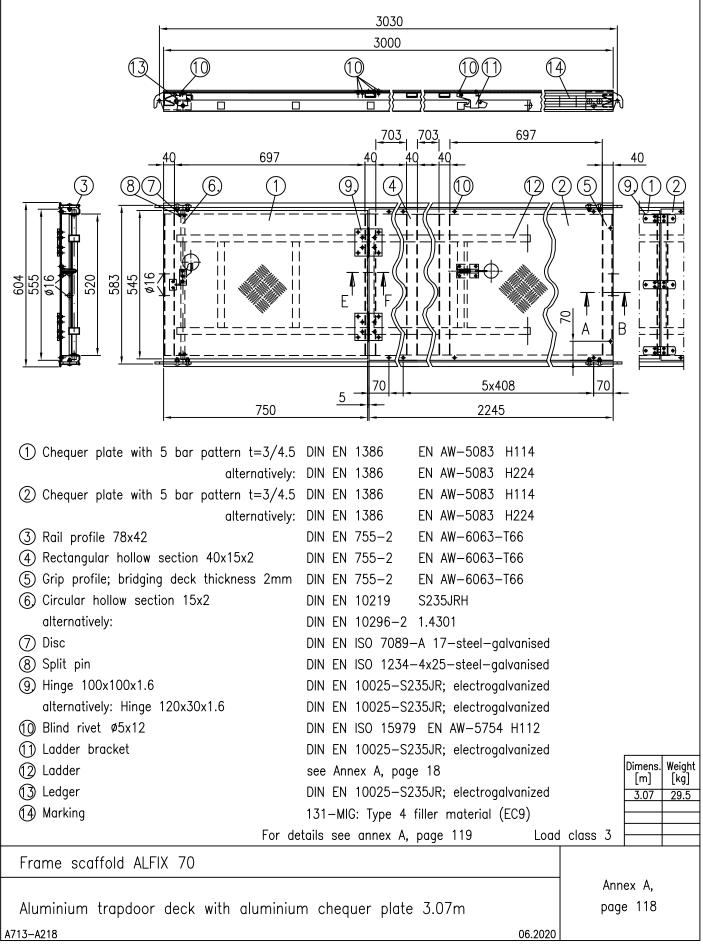


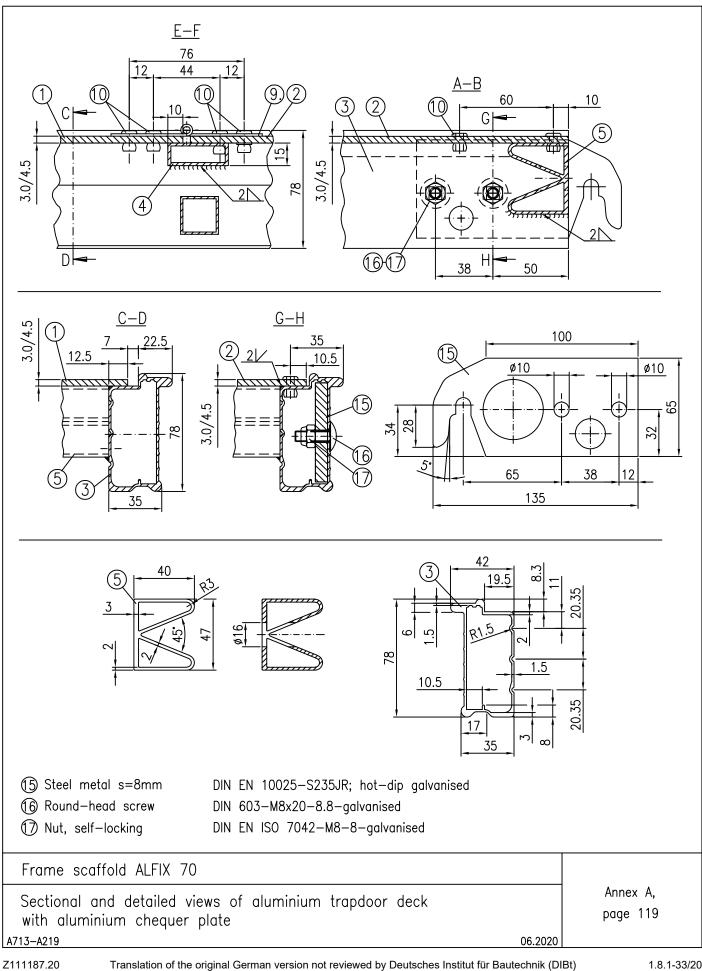


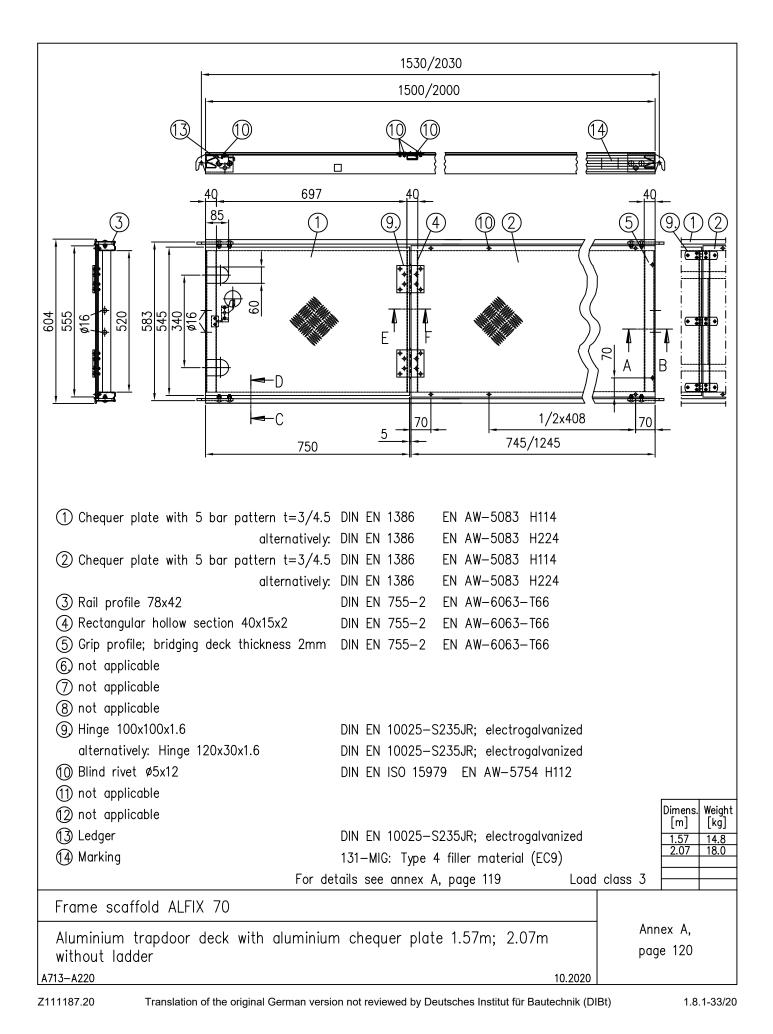


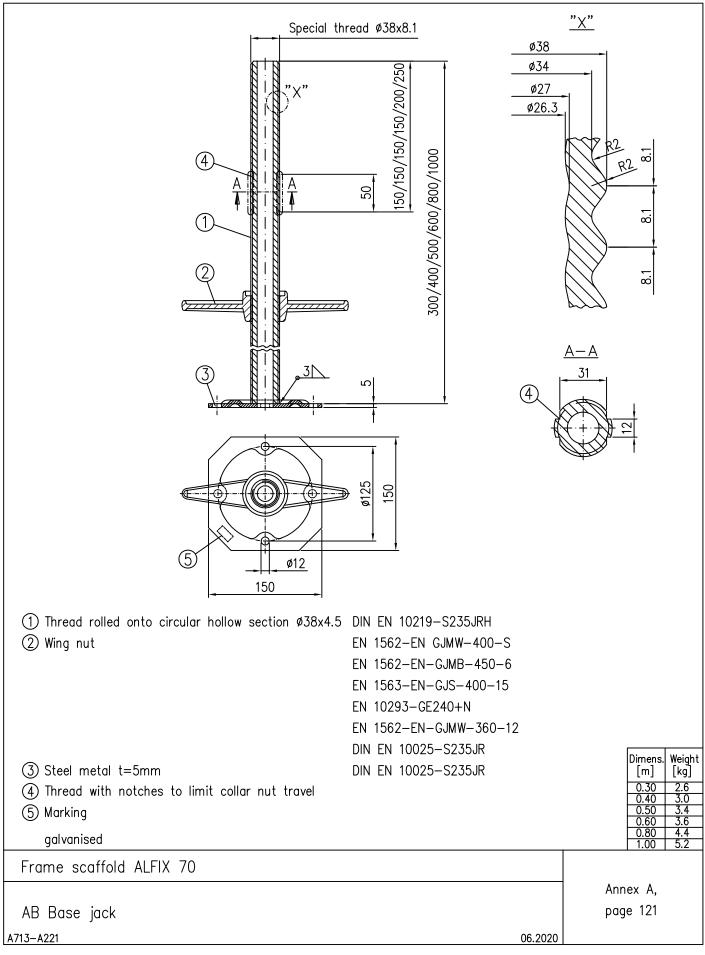


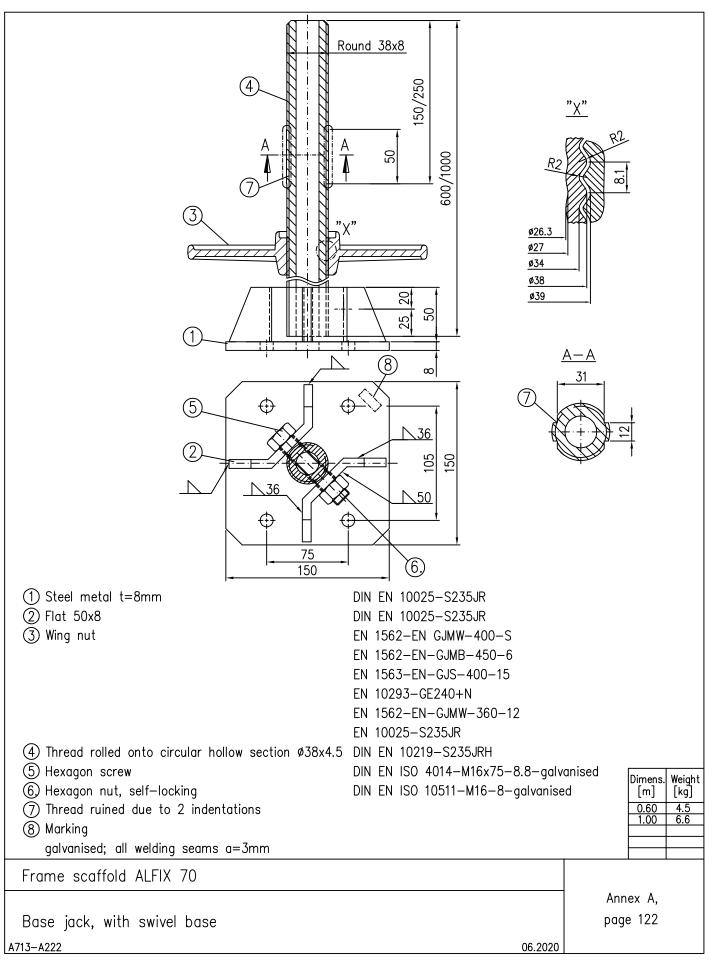




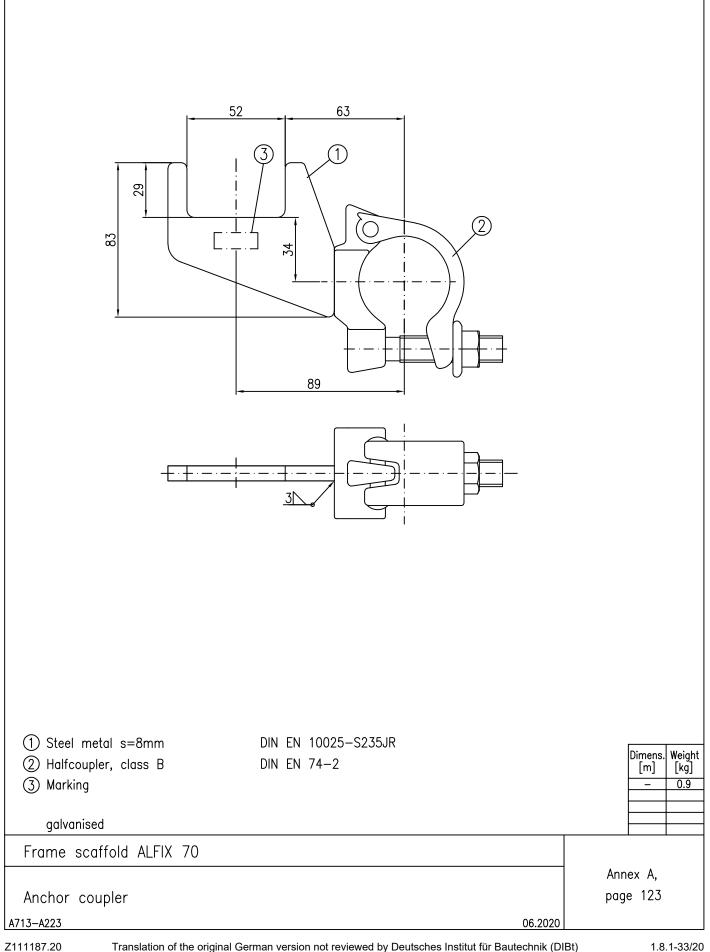


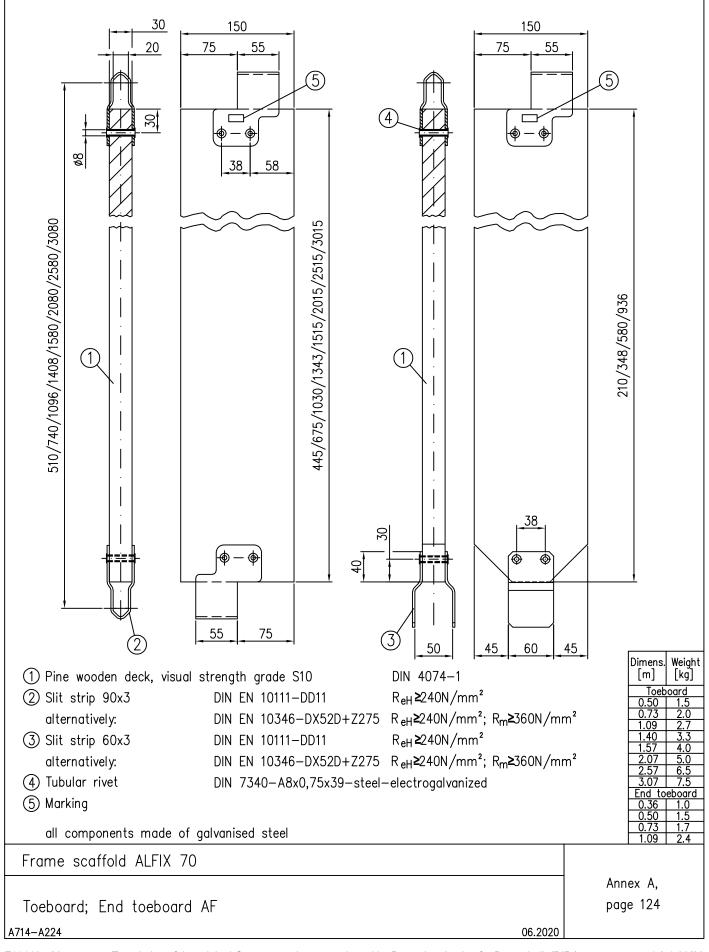




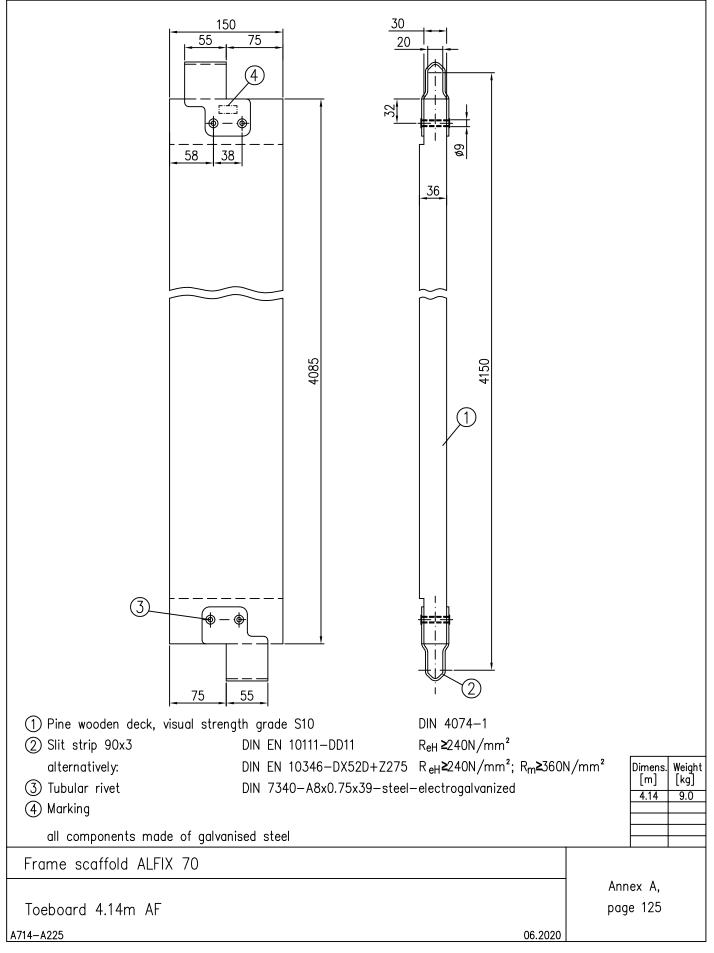


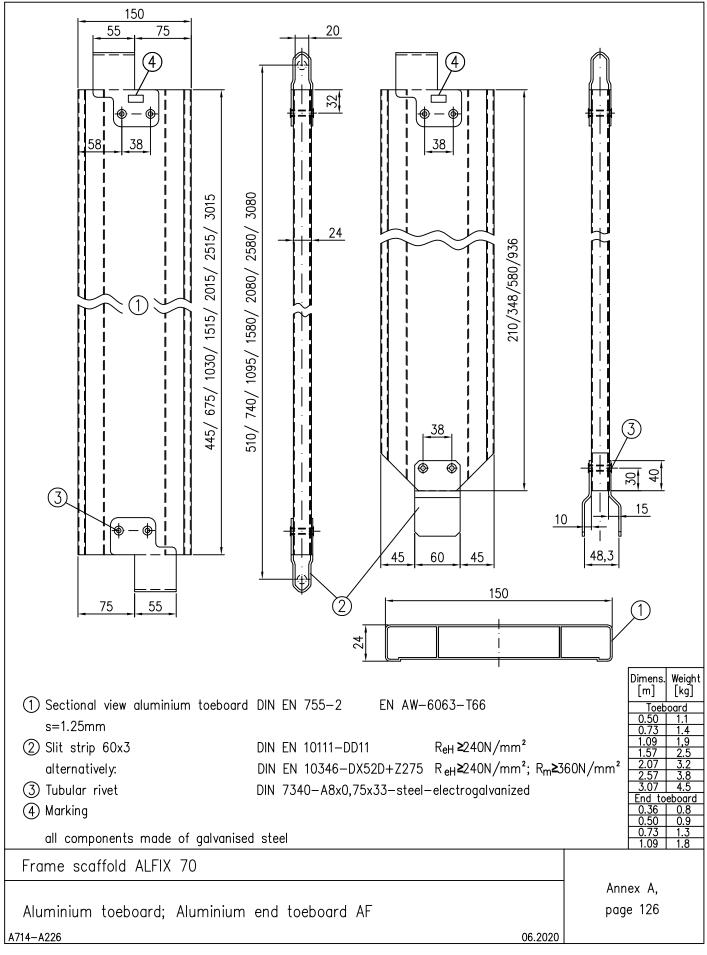
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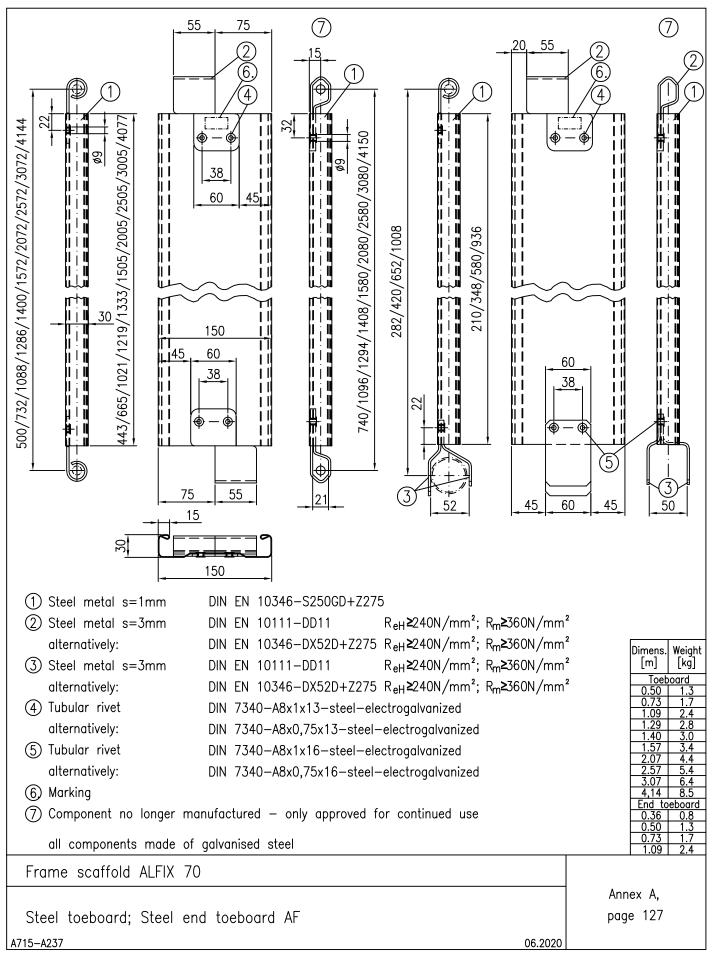


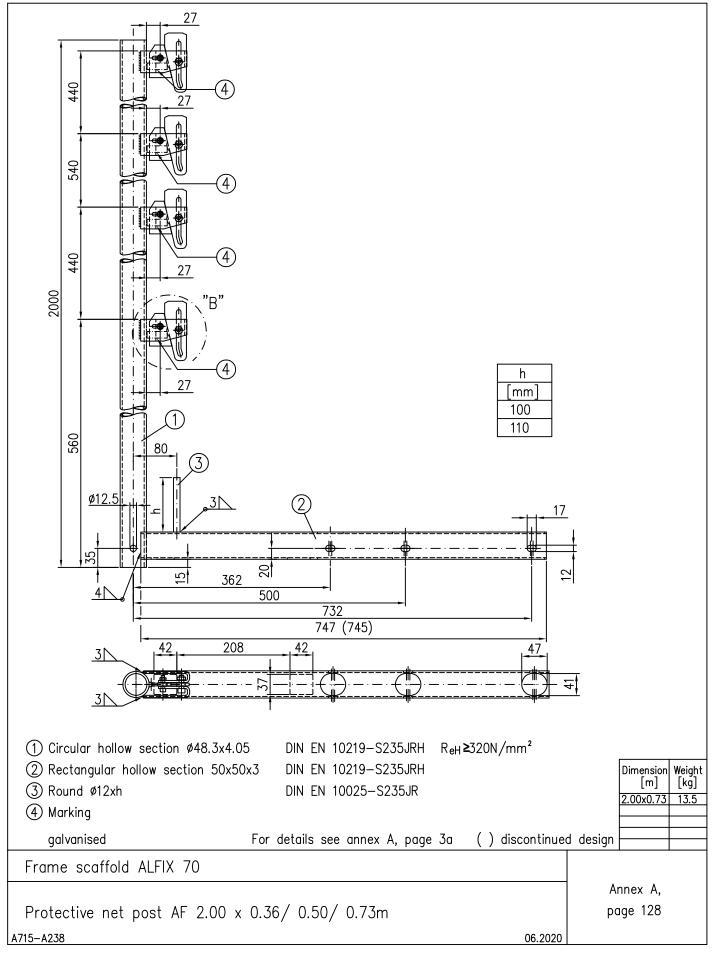


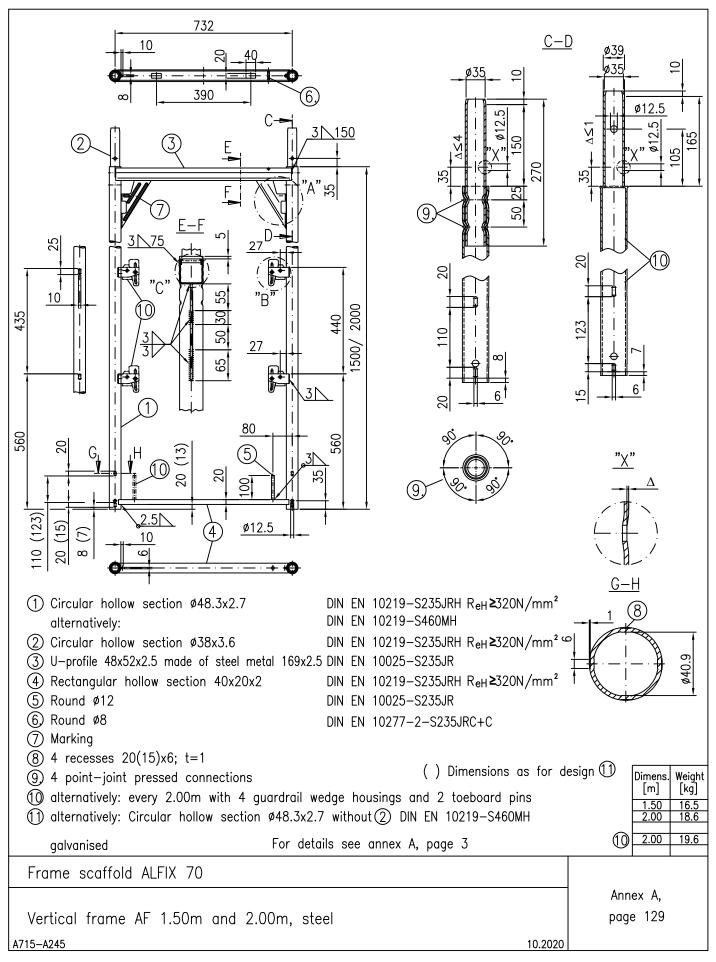
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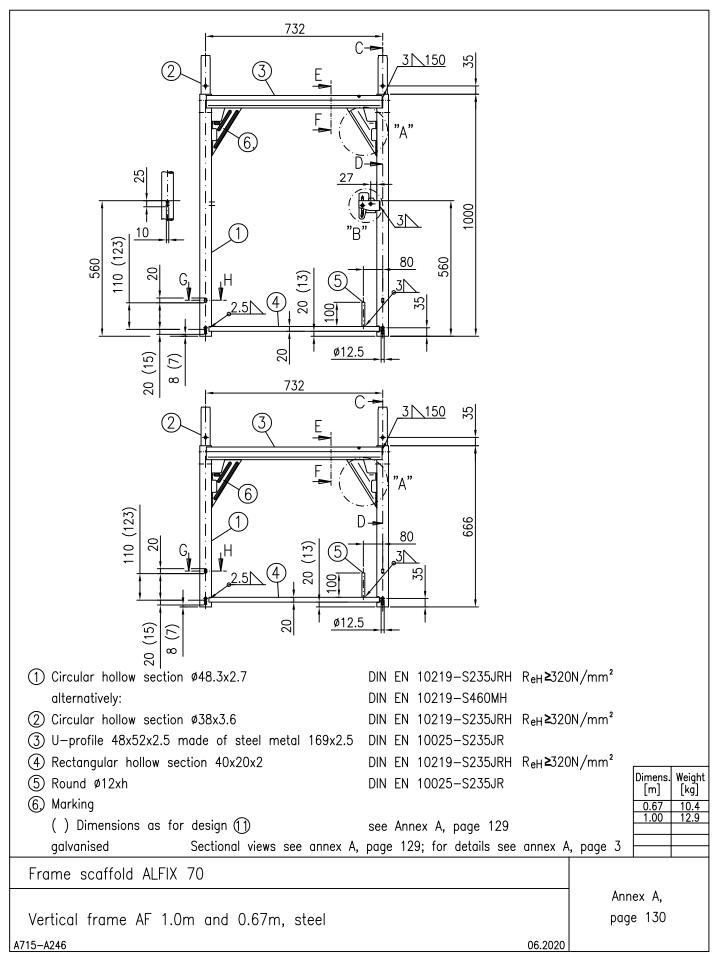


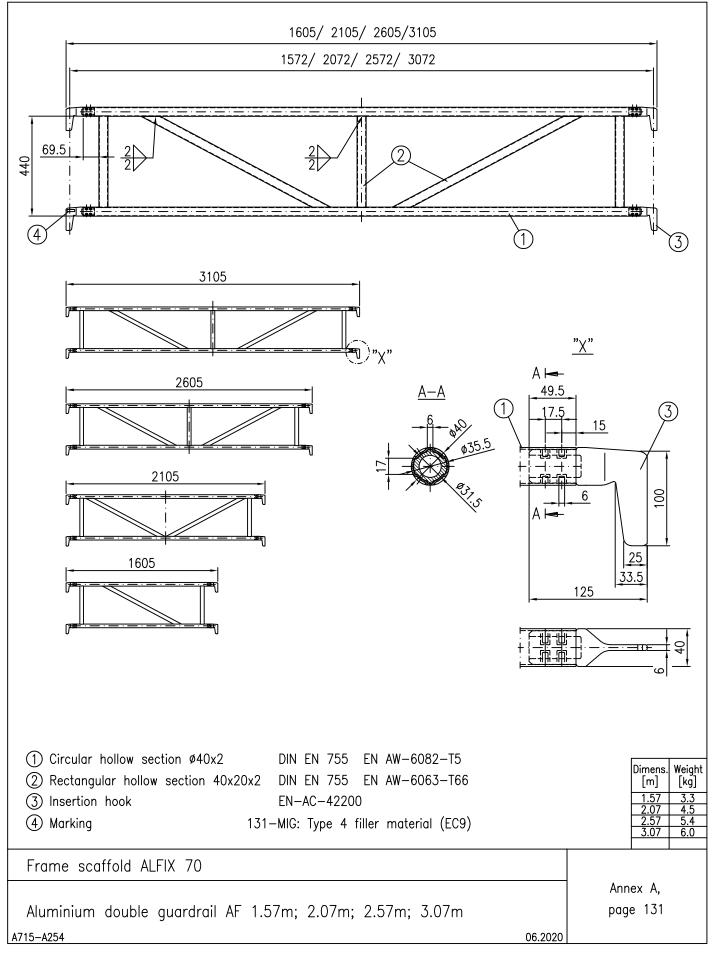


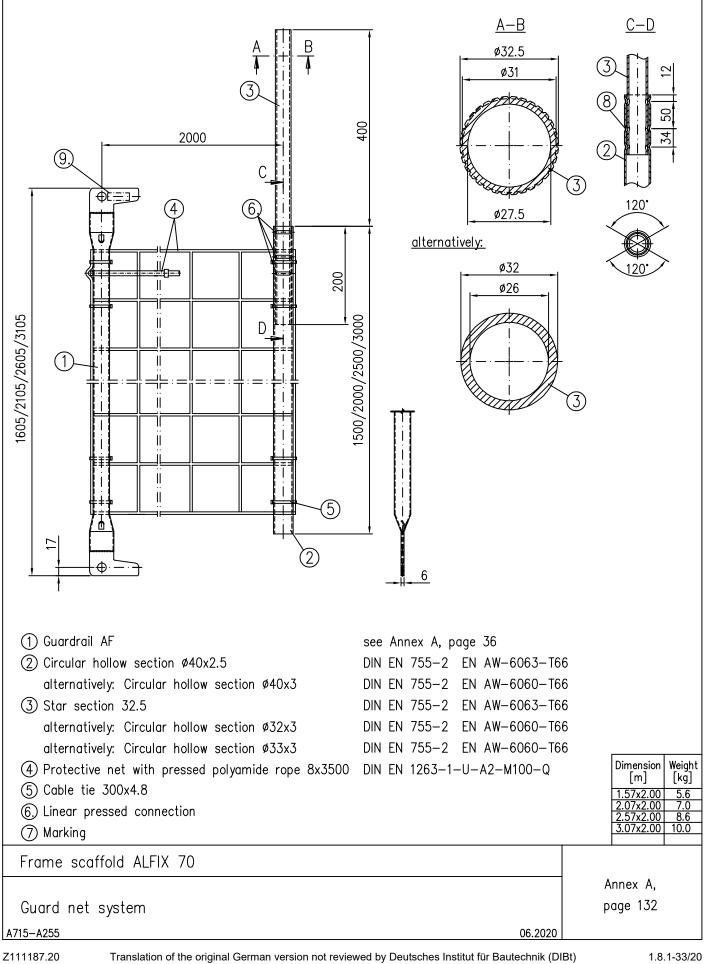


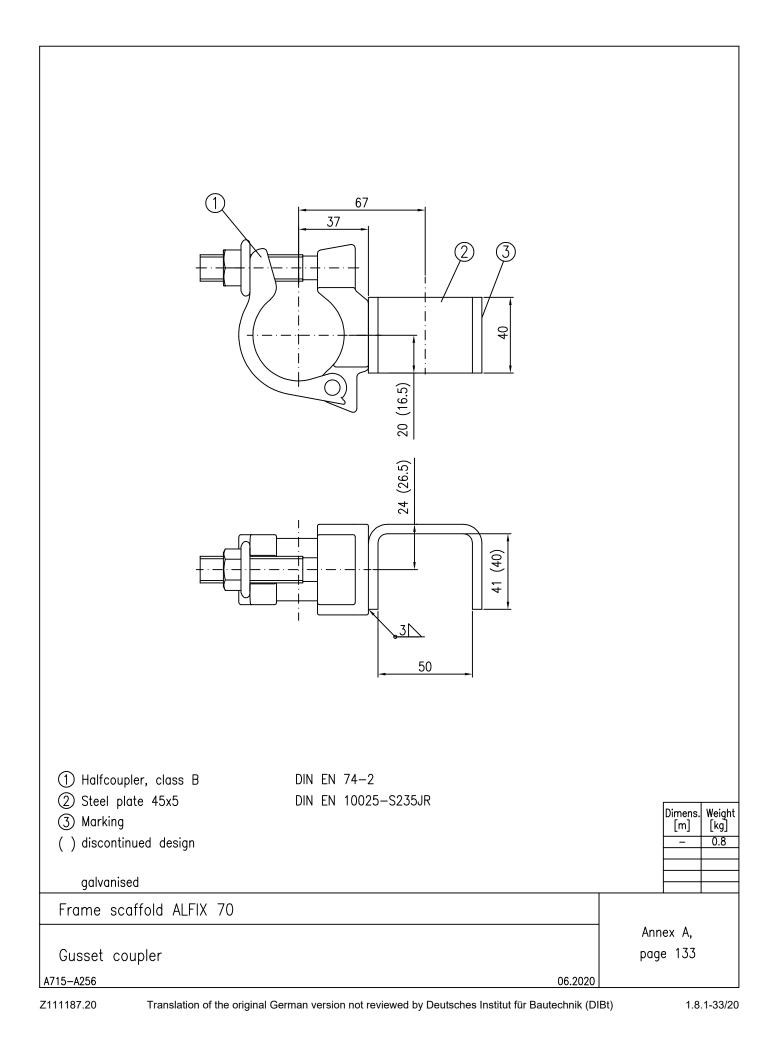


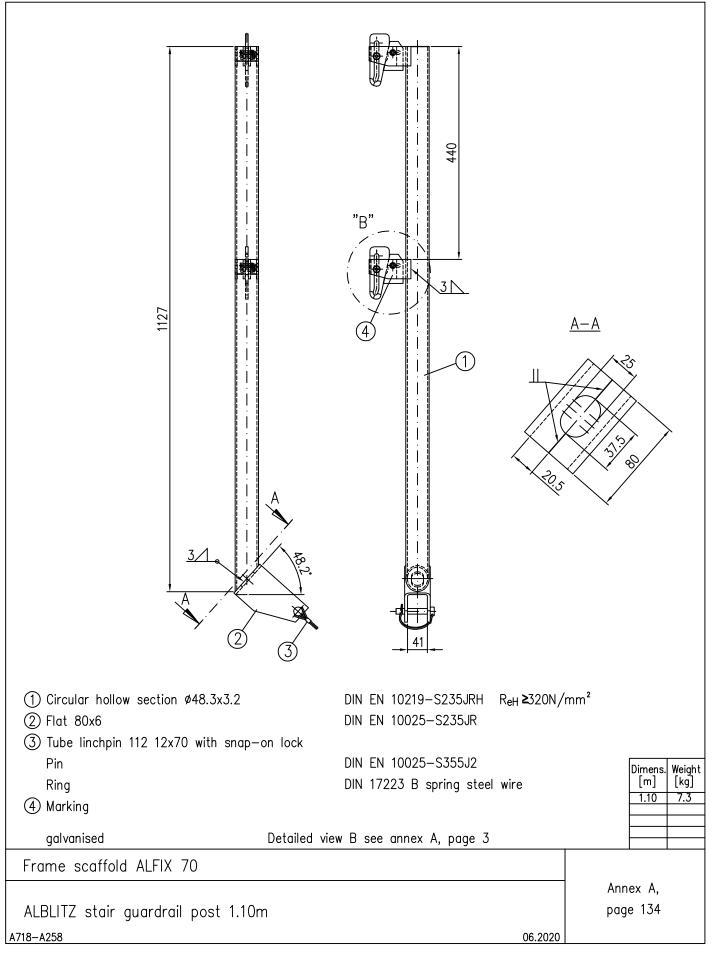




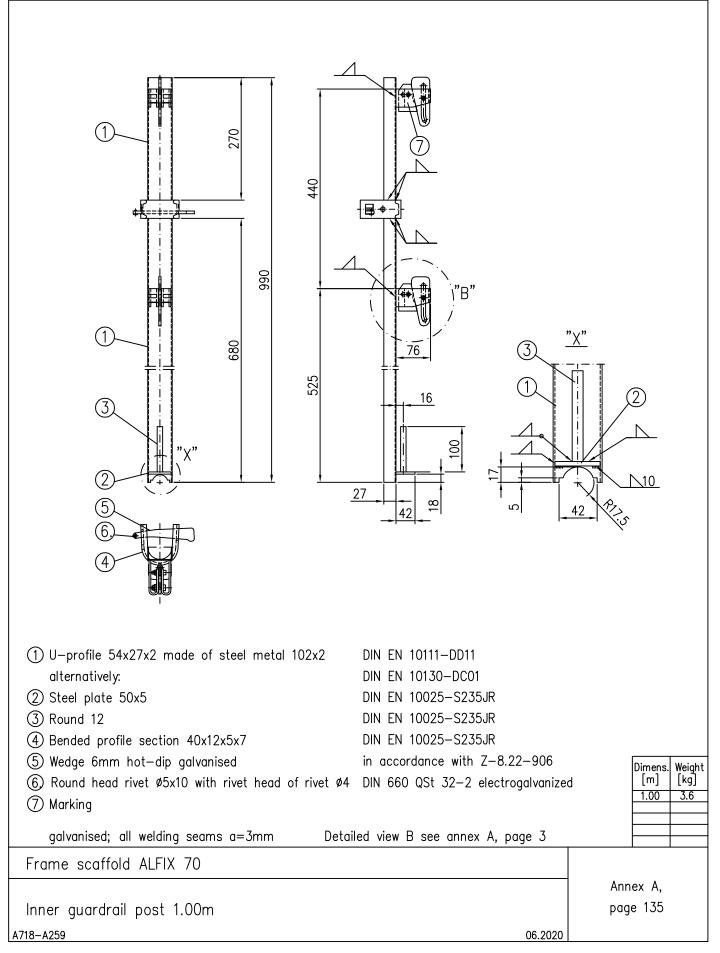


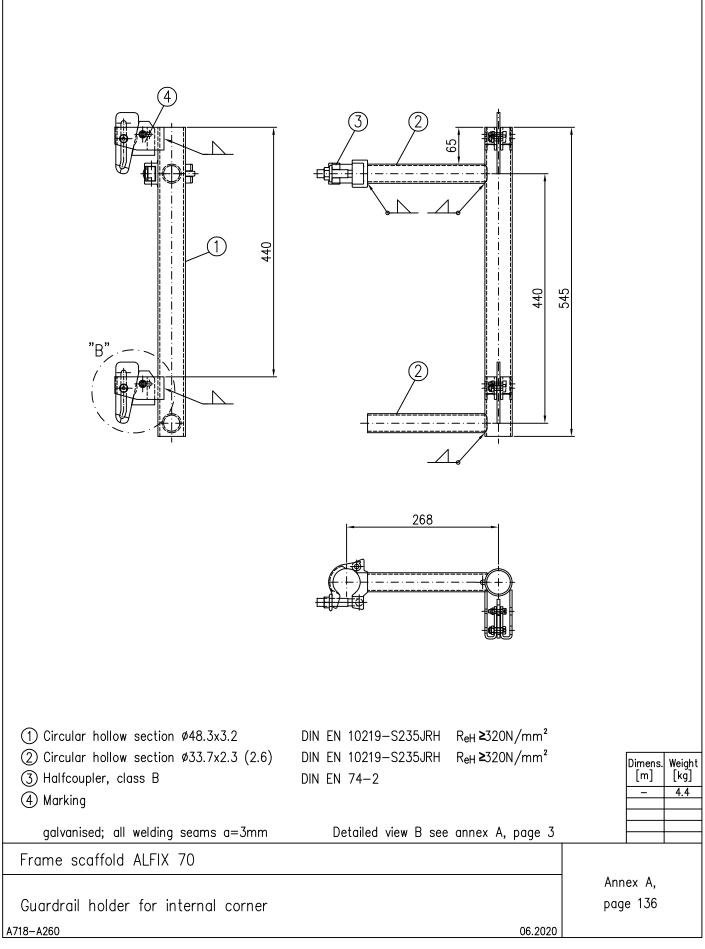




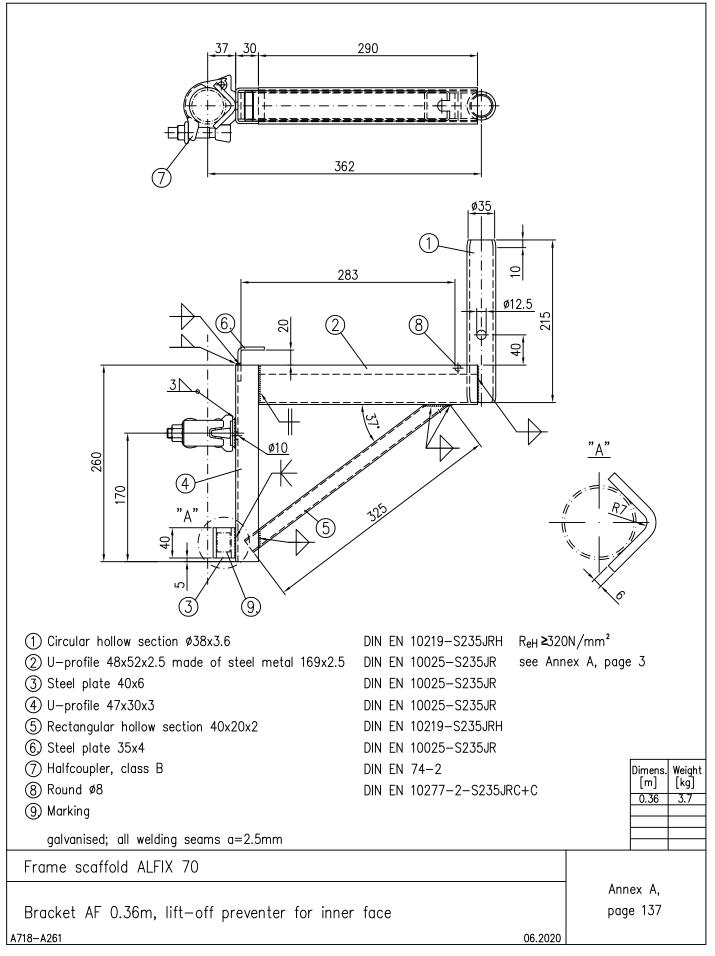


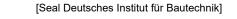
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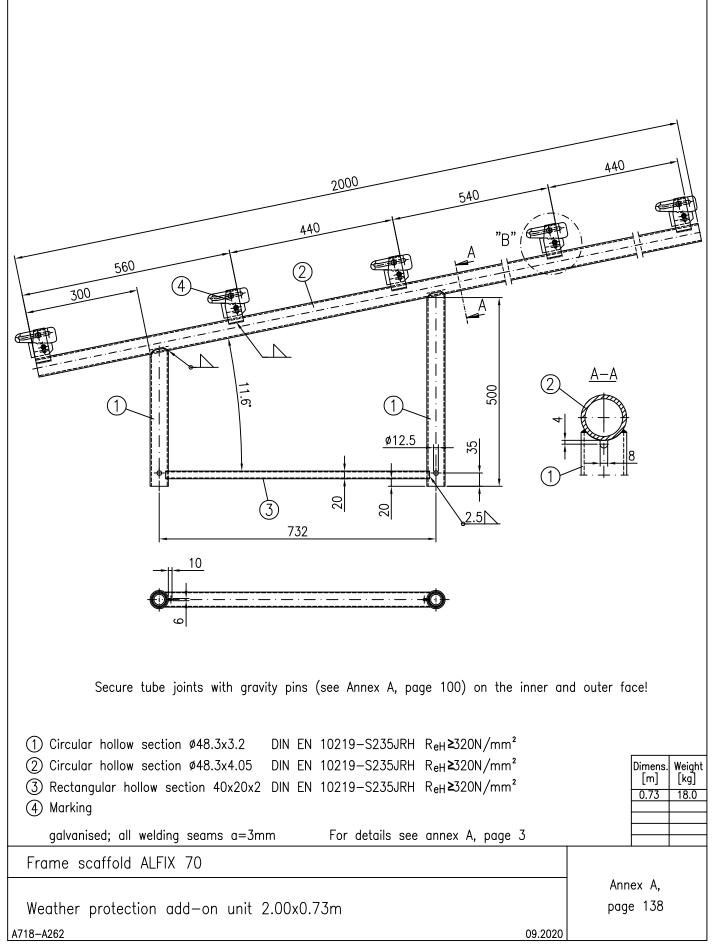


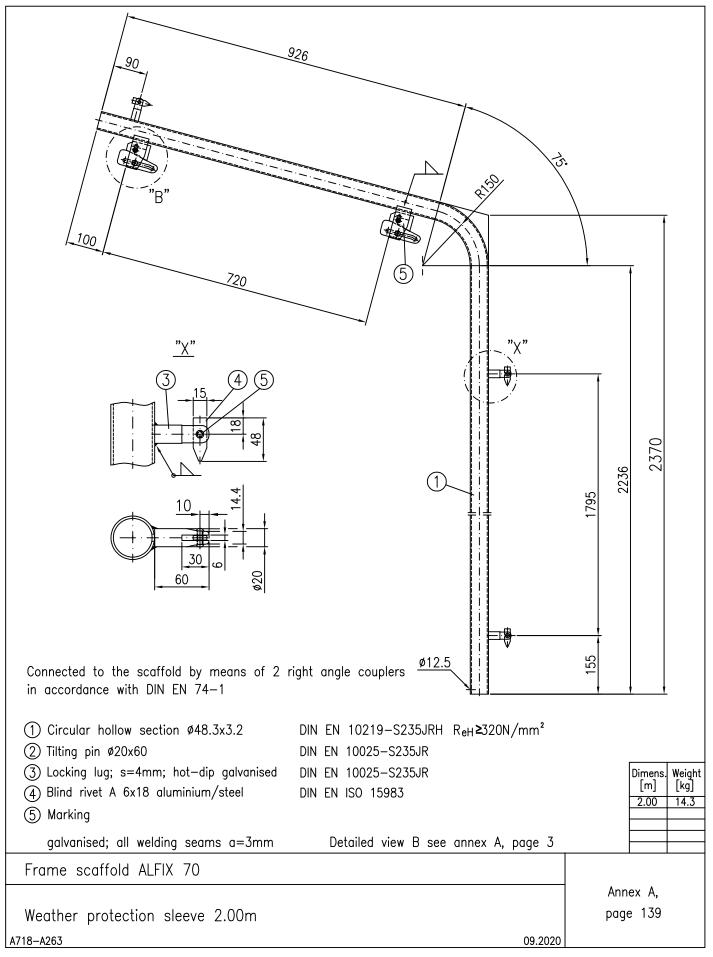


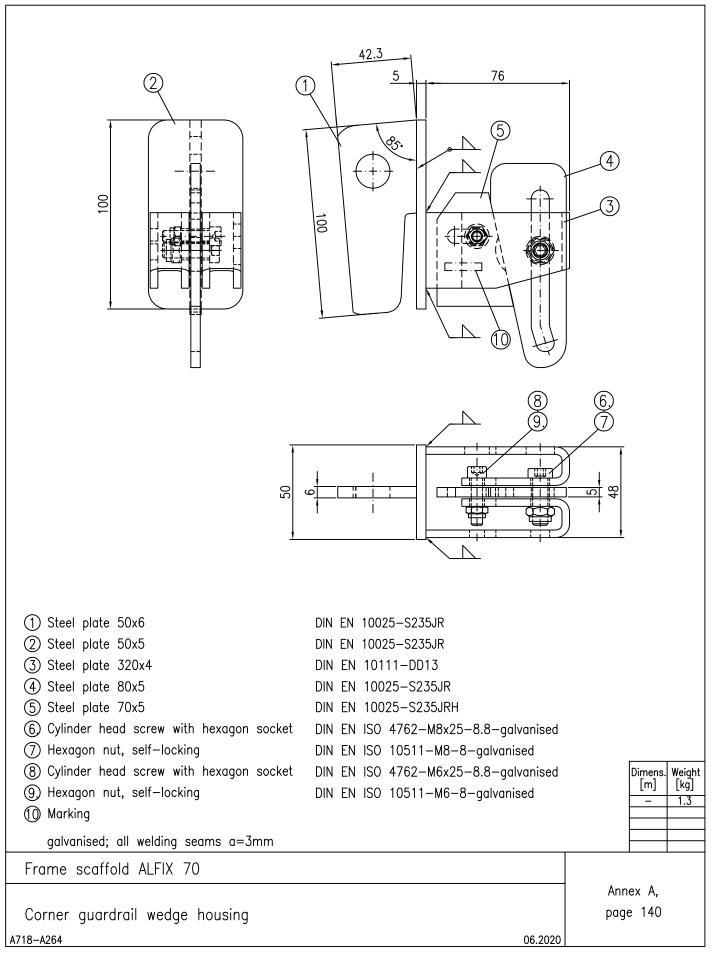
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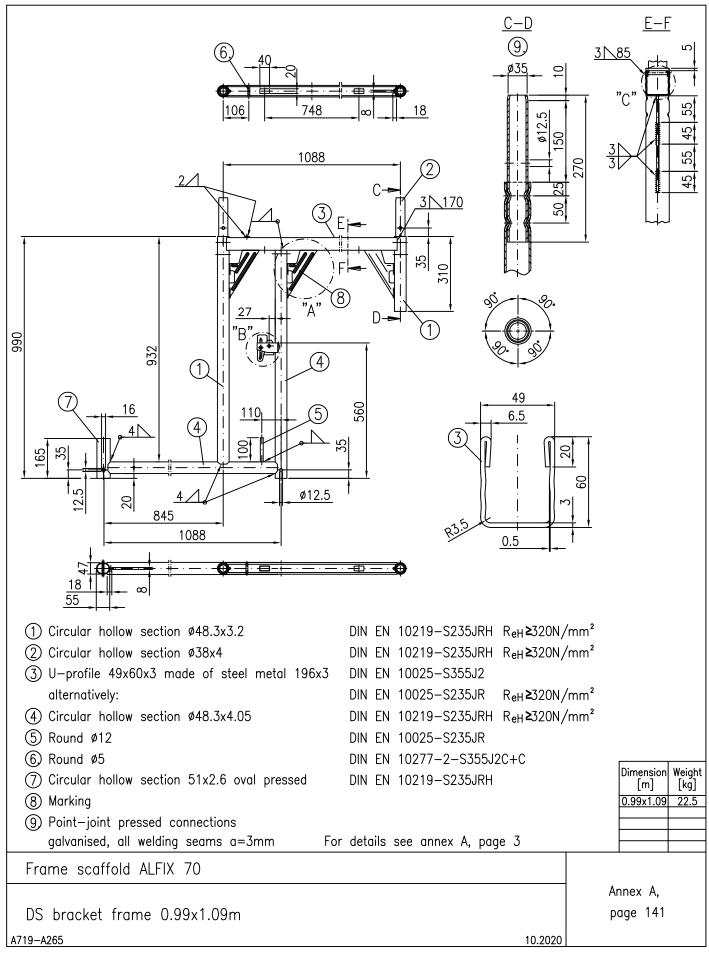


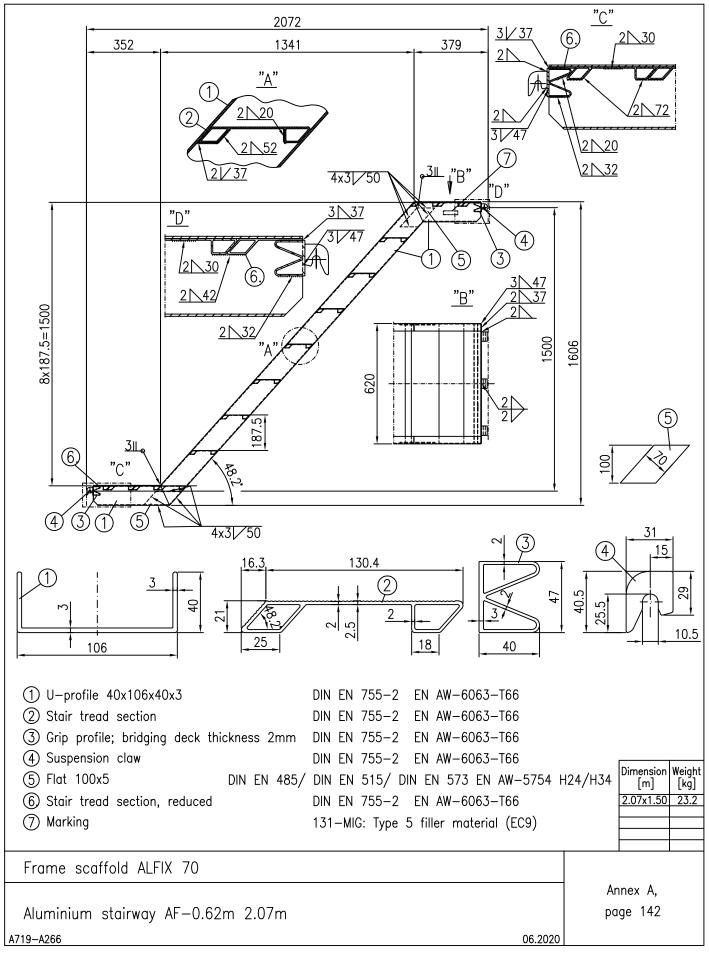


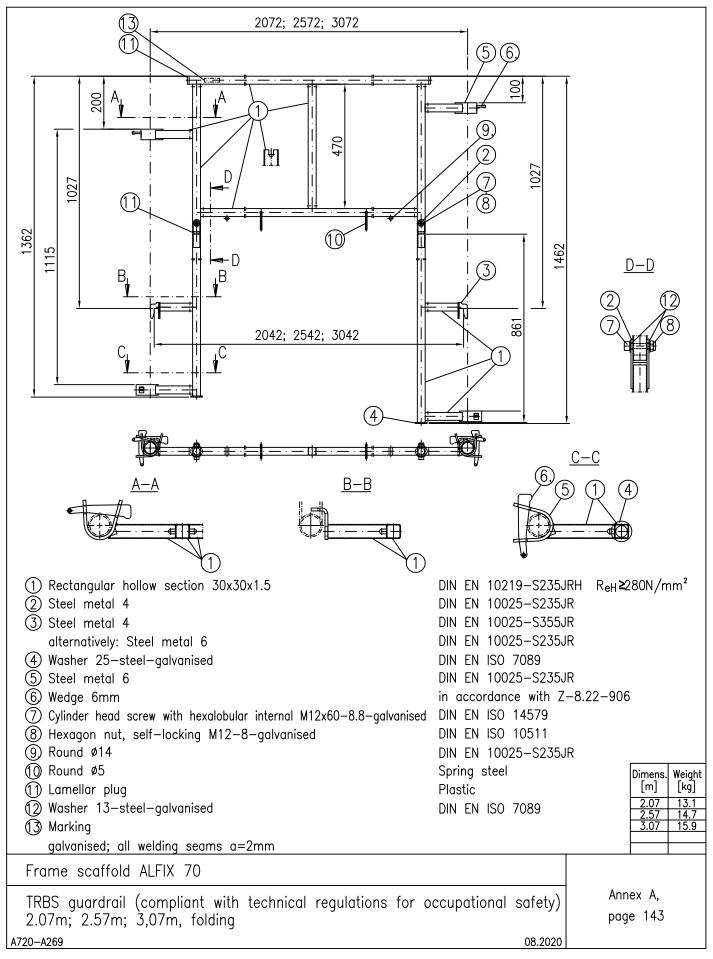


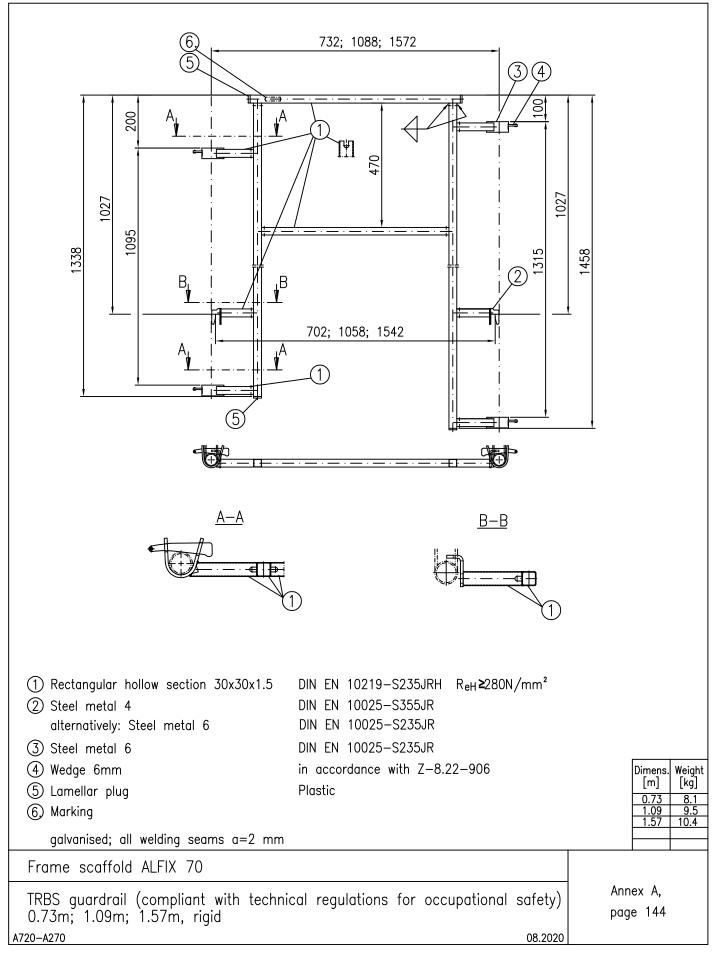


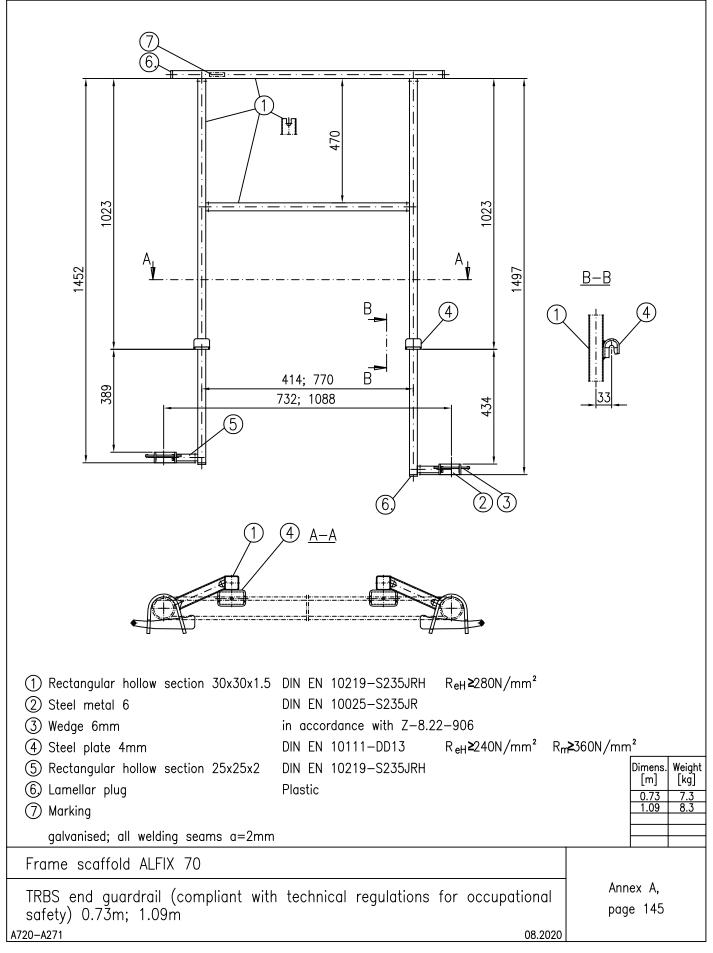












Product marking code key

AF XX Ü 862 XX

AF = ALFIX manufacturer's logo

XX = Year of manufacture

 $\ddot{U} = Mark$ of conformity

- 862 = Abbreviated approval number
- XX = Supplier number or supplier's company logo in case of third-party manufacturing

Year	ХХ
2015	15
2016	16
2017	17
2018	18
2019	19
2020	20
2021	21
2022	22
etc.	etc.

Frame scaffold ALFIX 70	
	Annex A,
Product marking code key AF	page 146
A717-A257 02.:	2023

B.1 General provisions

In its standard system configuration, the scaffolding system may be used as a working scaffold of load classes ≤ 3 with bay lengths I ≤ 3.07 m in accordance with DIN EN 12811-1:2004-03, and as a brickguard and roof edge protection scaffold in accordance with the regulations stipulated in Section B.2. The use of a protective roof in accordance with section B.7 has been verified in the standard system configuration.

The topmost horizontal level (working area) must not exceed 24m above ground level, not including the spindle extension length (bottom edge of end plate up to upper edge of spindle nut). The standard system configuration of the scaffolding system is designed for use on a working area in accordance with the regulations of the DIN EN 12811-1:2004-03 standard, section 6.2.9.2 in front of a "partially open" facade with an open proportion of no more than 60%, and in front of closed facades. The standard system configuration for cladded scaffolds applies to cladding with nets with an aerodynamic force coefficient no greater than $c_{f\perp,total} = 0.6$ and $c_{f\parallel,total} = 0.2$ and for cladding with tarpaulins. In case of cladded scaffold, the end sides of the scaffold must always be covered and the net or tarpaulin must be installed as close to the facade as possible. When determining the wind load, a service life factor of χ =0,7, assuming a maximum service life of 2 years, has been taken into account.

Without any further structural proof, the standard system configuration shall only be used if the loads of the bays will carry do not exceed the respective live loads in accordance with

DIN EN 12811-1:2004-03, table 3.

For the standard system configuration of the ALFIX 70 scaffolding system, the following designation in accordance with DIN EN 12810-1:2004-03 shall be used subject to the anchorage used:

Short scaffold ties and V-type anchor

o Long scaffold ties, compression-resistant bracing and half V-type scaffold tie

The standard system configuration distinguishes between the following assembly variants (see table B.1):

- Basic assembly variant 0:

Facade scaffold consisting only of basic components and side protection units.

- Basic assembly variant 1:

Facade scaffold consisting of basic components, side protection units, and brackets 0.36m on the inner face of the scaffold on each working level.

- Basic assembly variant 2:

Standard system configuration - General instructions

Facade scaffold consisting of basic components, side protection units, brackets 0.36m on the inner face of the scaffold on each working level, and brackets 0.73m on the outer face of the scaffold on the topmost working level.

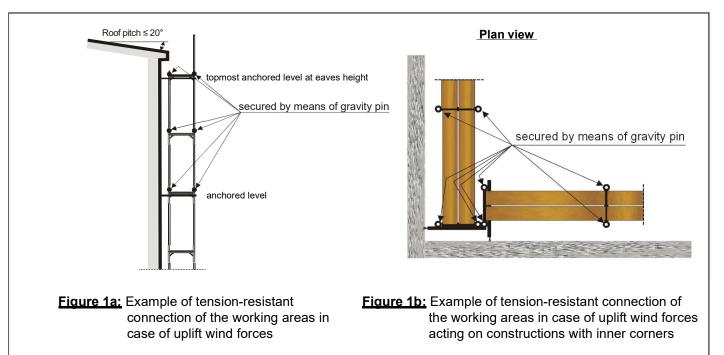
To secure the scaffold against uplifting wind forces, the topmost working levels of buildings with roof pitches $\leq 20^{\circ}$ shall be connected in a tension-resistant manner up to the next anchored level below the topmost anchored level, e.g. using gravity pins as shown in Fig. 1a; on buildings with inner corners, the tension-resistant connection shall be carried out as shown in Fig. 1b.

Scaffolding system ALFIX 70

Designs in accordance with annex C, pages 19, 22, 23, 24, 25, 26, 27 and annex C, page		annex C, pages 19, 22, 23, 24, 25, 26, 27			without brackets	with inner brackets	on each working area	with outer bracket on the	topmost working area	with inner brackets on	outer bracket on the topmost working area	
Scaffold cladding	Supplementary components		te of the	e facade bartially open	in front of	partially open	the sca	partially open	s closed	Detailed view is seen and the second		
	no supplementary components	2, 5		3, 5	1, 5	5, 6		4, 5, 6				
	with bridging girder	7		8		7		8		2		
ad	with passage frame	9		9		9		9		2		
unclad	with protective roof	2, 5, 6, 7		3, 5, 6, 8, 10	1, 5, 3, 6, 8, 10	3, 10 2, 5, 6, 7		4, 5, 6, 8		1		
	Protection scaffold and roof edge protection scaffold	2, 5, 6, 7, 9		3, 5, 6, 8, 9	1, 5, 3, 6, 8, 9	2, 5,	6, 7, 9	4, 5,	6, 8, 9	1		
	Topmost working level not anchored				10							
	no supplementary components		2, 5, 6		5, 6, 11		2, 5, 6	12	4, 5, 6, 12			
	with bridging girder		7		8		7		8	2		
ŝts	with passage frame		9		9		9		9	2		
with nets	with protective roof		2, 5, 6, 7		3, 5, 6, 8, 10, 11		2, 5, 6, 7	12	4, 5, 6, 8, 12	1		
>	Protection scaffold and roof edge protection scaffold		2, 5, 6, 7, 9		3, 5, 6, 8, 9, 11		2, 5, 6, 7, 9	12	4, 5, 6, 8, 9, 12	1		
	Topmost working level not anchored				10							
SL	no supplementary components			14	13			14	13	-		
aulir	with protective roof			14	13			14	13	1		
with tarpaulins	Protection scaffold and roof edge protection scaffold			14	13			14	13	1		
Add on	ladder / stairway access				15, 1	6						

Scaffolding system ALFIX 70

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B.2 Protection scaffold and roof edge protection scaffold

In its standard system configuration, the scaffolding system may be used as a protection scaffold and roof edge protection scaffold with a top fall arresting layer of class FL 1 and as a roof edge protection scaffold with protective walls of class SWD 1 according to DIN 4420:2004-03. Trapdoor decks must not be fitted into brackets.

The structural design as a roof edge protection scaffold shall be carried out in accordance with annex C, page 17. The protective net shall have a mesh size of 100 mm and a rope diameter of 5 mm in accordance with DIN EN 1263-1:2015-03.

B.3 Components

The components intended for use are listed in table B.2. In addition to these components, steel tubes and couplers in accordance with DIN EN 12811-1:2004-03 may also be used in the exceptional cases below:

- Connection of the add-on access stairway to the facade scaffold in accordance with annex C, pages 15 16 (tubes and couplers),
- Bracing of the bridging girders in accordance with annex C, page 20 (tubes and couplers),
- Bracing of the passage frames in accordance with annex C, page 21 (tubes and couplers),
- Connection of the scaffold ties to the standards in accordance with annex C, pages 23 to 26 (couplers),
- Corner formation in accordance with annex C, page 28 (tubes and couplers) and
- End side protection above the inner brackets and on the topmost working area (tubes and couplers).

Apart from the base jacks listed in table B.2, other lightweight class B base jacks according to DIN 4425:2017-04 with an outer diameter of d = 38 mm may also be used.

Table B.2:Components of the standard system configuration

Designation	Annex A, page
Vertical frame 18/70; 1.5m and 2.0m, steel	1
Vertical frame 18/70; 1.0m and 0.67m, steel	2
Vertical frame 70; 2.0m, steel	4

Scaffolding system ALFIX 70

Standard system configuration - General instructions

Designation	Annex A, page
Vertical frame 70; 1.0m and 0.66m, steel	5
Steel deck AF 0.32m	7
Steel deck	8
Intermediate deck AF 0.16m; 0.19m	10
Intermediate deck	11
Aluminium deck with plywood 2.57; 3.07m	12
Aluminium deck with plywood 1.57; 2.07m	13
Aluminium access deck 3.07m with ladder	15
Aluminium access deck 2.57m with ladder	16
Aluminium deck with plywood 3.07m	19
Aluminium deck with plywood 1.57m; 2.07m; 2.57m	20
Aluminium access deck 3.07m with ladder	22
Aluminium access deck 2.57m with ladder	23
Solid wood deck 45	26
Solid wood deck 48	27
Wooden deck	28
Diagonal brace 3.07m	29
Diagonal brace 2.57m	30
Diagonal brace 2.07m	31
Horizontal strut	32
Scaffold tie	33
Quick-release scaffold tie	34
Base jack	35
Guardrail AF	36
Handrail	37
Double guardrail AF	38
Double guardrail	39
Double guardrail AF, aluminium	40
Double guardrail, aluminium	41
Advanced guardrail post	42
Telescopic guardrail 2.0m – 3.07m	45
Toeboard; End toeboard	46
Double end guardrail AF, Single end guardrail	47
Double end guardrail	48
Guardrail post AF, single	49
Guardrail post, single	50
Guardrail post AF	51
Guardrail post	52
End guardrail frame	53
End guardrail post AF	54

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Standard system configuration - General instructions

Designation	Annex A, page
End guardrail post	55
Protective wall post AF	56
Protective wall post	57
Bracket AF 0.36m	58
Bracket 0.36m	59
Bracket AF 0.73m	60
Bracket 0.73m	61
Protective roof extension AF 0.46 x 0.66m	62
Protective roof extension	63
Lift-off preventer	64
Transom 0.73m	65
Side-protection meshguard	66
Protection net	67
Lattice girder, steel	68
Passage frame AF	69
Passage frame	70
Passage frame, in sections, 1.57m, upper part	71
Passage frame, in sections, tubular post, 1.90m	72
Gap cover	73
Aluminium toeboard; Aluminium end toeboard	77
Aluminium stairway AF -0.62m 2.57m; 3.07m	78
Stair guardrail AF 2.57m; 3.07m	79
Inner guardrail for aluminium stairway	80
Stair stringer fall protection 1.00 x 0.50m	81
Roof guard extension frame 2.00 x 0.73m to 1.09m	84
Starter transom 0.73m	87
Lattice girder cross brace 0.73m	93
Tube connector for lattice girder	94
Guardrail coupler AF	95
Toeboard coupler; Halfcoupler with hook	96
Toeboard support	99
Locking pin	100
Protective wall post, telescopic 0.36m – 1.73m	102
Diagonal cross brace	103
Protective wall post, telescopic 0.73m – 1.09m	104
Scaffold tie EIFS	105
EIFS deck AF 190; Tube linchpin	106
Aluminium frame platform with plywood 1.57m; 2.07m	110
Aluminium frame platform with plywood 2.57m; 3.07m	111
Aluminium frame platform with internal hatch 2.57m; 3.07m	113

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Annex B, Page 5

Standard system configuration - General instructions

Bezeichnung	Anlage A, Seite
Aluminium frame platform with internal hatch 1.57m; 2.07m	114
Aluminium deck 0.60m, lightweight	116
Aluminium trapdoor deck with aluminium chequer plate 2.57m	117
Aluminium trapdoor deck with aluminium chequer plate 3.07 m	118
Aluminium trapdoor deck with aluminium chequer plate 1.57m; 2.07m without ladder	120
AB Base jack	121
Anchor coupler	123
Toeboard, End toeboard AF	124
Aluminium toeboard; Aluminium end toeboard AF	126
Steel toeboard; Steel end toeboard AF	127
Protective net post AF 2.00 x 0.36 / 0.50 / 0.73m	128
Vertical frame AF 1.50m and 2.00m, steel	129
Vertical frame AF 1.0m and 0.67m, steel	130
Aluminium double guardrail AF 1.57m; 2.07m; 2.57m; 3.07m	131
Guard net system	132
Gusset coupler	133
ALBLITZ stair guardrail post 1.10m	134
Inner guardrail post 1.00m	135
Guardrail holder for internal corner	136
Bracket AF 0.36m, lift-off preventer for inner face	137
Corner guardrail wedge housing	140
Aluminium stairway AF-0.62m 2.07m	142
TRBS guardrail (compliant with technical regulations for occupational safety) 2.07m; 2.57m; 3.07m, folding	143
TRBS guardrail (compliant with technical regulations for occupational safety) 0.73m; 1.09m; 1.57m, rigid	144
TRBS end guardrail (compliant with technical regulations for occupational safety) 0.73m; 1.09m	145

B.4 Bracing

Scaffold decks listed in table B.3 shall be fitted continuously on all horizontal levels (working areas). All other decks may only be used as a non-bracing component in connection with brackets.

EIFS decks AF 190 in accordance with annex A, page 106 must not be used as a bracing component. Only use this deck on inner brackets.

Use the trapdoor decks listed in table B.6 in ladder access bays instead of the scaffolding decks mentioned above.

Decks and trapdoor decks shall be secured against accidental lift-off by means of guardrail posts, safety meshguard posts or lift-off preventers.

Install vertical diagonal braces (diagonals in accordance with annex A, pages 29 to 31) to brace the outer vertical plane, whereby at least one diagonal brace is required per 5 bays.

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Annex B, Page 6

Standard system configuration - General instructions

Scaffold decks	Deck width [m]	Number per bay	in accordance with annex A, page
Steel deck AF	32	2	7
Steel deck	32	2	8
Aluminium deck with plywood	61	1	12, 13, 19, 20
Wooden deck *)	32	2	28
Solid wood deck 45	32	2	26
Solid wood deck 48	32	2	27
Aluminium frame platform with plywood	61	1	110, 111
Aluminium deck 0.60m, lightweight	61	1	116

Depending on the configuration, additional vertical diagonal braces may have to be installed (e.g. annex C, page 1). Install a longitudinal ledger (horizontal strut according to annex A, page 32) in each lowermost working level, to which a diagonal brace is connected at the level of the lowermost transoms (see annex C).

Depending on the configuration, additional diagonal cross braces in accordance with annex A, page 103 may have to be installed in the lowermost vertical frames.

B.5 Anchoring

Depending on the configuration and the structural requirements, anchorage must be provided by means of scaffold ties in accordance with annex A, page 33 or by means of quick-release anchors in accordance with annex A, page 34. The quick-release anchors must additionally be locked to the transom of the vertical frame. Depending on the scaffold configuration, the scaffold ties are to be used as follows:

- A) Scaffold without inner bracket (see annex C, page 23):
 - a) Short scaffold tie, connected by means of right angle coupler to inner standard near the node.
 - b) Quick-release anchor, connected with right angle coupler to the inner standard near the node and to the transom of the vertical frame.
 - c) V-type scaffold tie: Scaffold tie 1, angularly connected with right angle coupler to the inner standard near the node; scaffold tie 2, connected with right angle coupler or class B swivel coupler to scaffold tie 1; spread angle ~90°.
- B) Scaffold with inner bracket, without outer bracket (see annex C, page 24):
 - d) Long scaffold tie, connected with right angle coupler to the outer standard near the node and additionally with an anchor coupler according to annex A, page 123 to the upper transom of the vertical frame near the node of the inner standard.
 - e) Half V-type anchor: Scaffold tie 1 as with d); Scaffold tie 2, connected with class B swivel coupler to scaffold tie 1; spread angle: ~45°.
 - f) Compression-resistant bracing: Long scaffold tie without anchorage to the building structure; free tube end in pressure contact with the building structure, connected with right angle coupler to the outer standard near the node and additionally with an anchor coupler according to Annex A, page 123 to the upper transom of the vertical frame near the node of the inner standard.
 - g) Long scaffold tie, connected to the outer standard with right angle coupler and additionally with gusset coupler in accordance with annex A, page 134 in the gusset cut-out on the inner standard

Scaffolding system ALFIX 70

Standard system configuration - General instructions

- C) Scaffold with inner bracket and outer bracket (see annex C, page 25 and 26):
 - V-type scaffold tie: Scaffold tie 1, angularly connected with right angle coupler to the inner standard directly below the gusset plate; scaffold tie 2 connected with right angle coupler or class B swivel coupler to scaffold tie 1; spread angle ~90°.
 - i) Short scaffold tie, connected with right angle coupler to inner standard, directly below the gusset plate
 - j) Half V-type scaffold tie: Scaffold tie 1 connected with right angle coupler in the gusset-plate cut-out to the outer standard; Scaffold tie 2, connected with class B swivel coupler to scaffold tie 1; spread angle: ~45°.

The scaffold ties must be attached in the immediate vicinity of the node points formed by the vertical frames and scaffold decks. Deviating from this, the scaffold ties may be fitted on an anchor level up to 0.30m below the node points (see annex C, pages 23 to 25). V-type scaffold ties and half V-type scaffold ties may not be fitted to the end sides of the scaffold.

If V-type scaffold ties and half V-type scaffold ties must be fitted adjoining an internal ladder access bay, horizontal struts according to annex A, page 32 must be installed in this access bay or the adjoining inner standards must be connected to one another by means of additional coupler tubes (scaffold tubes) with two right angle couplers directly below the V-type or half V-type scaffold ties.

The anchor forces listed in table B.4 were determined with the characteristic values of the actions. For the design analysis of the anchorage and the load transfer, the values given must be multiplied by the respective partial safety factor γ_F (generally $\gamma_F = 1.5$).

The following anchorage patterns are allowed depending on the configuration in accordance with section B.1 and table B.4:

a) 8 m anchorage pattern, offset:

Each vertical frame section is anchored at vertical intervals of 8 m; anchoring points of neighbouring vertical frames must be arranged with a vertical offset of half the spacing. Die Vertical frame sections at the edge of a scaffold and for internal ladder access bays must be anchored at vertical intervals of 4 m.

b) 4 m anchorage pattern, continuous:

Each vertical frame section is anchored at vertical intervals of 4 m. On the topmost working area, each standard must be anchored; every second anchorage may be omitted if the standard is anchored on the anchor level below the topmost level.

c) 4 m anchorage pattern, offset:

Each vertical frame section is anchored at vertical intervals of 4 m; anchoring points of neighbouring vertical frames must be arranged with a vertical offset of half the spacing. Vertical frame sections at the edge of a scaffolding must be anchored at a vertical interval of 2 m. On the topmost working area, each standard must be anchored.

d) 2 m anchorage pattern:

Each vertical frame section is anchored at vertical intervals of 2 m (each node).

Additional anchorage may be required when using e.g. outer brackets, protective walls or bridging girders and for some design variations. Ladder accesses have to be anchored in accordance with the configuration at vertical intervals of maximum 2.0m or 4.0m.

The use of wooden decks in accordance with annex A, page 28 is only possible with an anchorage pattern of \leq 4 m according to annex C, page 5.

For the intermediate state "topmost working level not anchored", the notes in section B.13 must be observed.

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Standard system configuration - General instructions

	Scaffolding in front of a									
	Anchor forces				partially open facade ¹⁾			closed facade		
Anchorage pattern	Bay length	Cladding	Fittings	A⊥-	A⊥+	Aıı	A⊥-	A⊥+	A۱	
	[m]			[kN]	[kN]	[kN]	[kN]	[kN]	[kN]	
	3.07		without	4.46	4.46	5.87	1.49	1.49	5.87	
0	2.57	no supplementary	inner bracket	3.96	3.96	5.87	1.32	1.32	5.87	
8m offset	3.07	components	with	4.50	4.50	3.53	1.50	1.50	3.53	
	2.57		inner bracket	3.99	3.99	3.53	1.33	1.33	3.53	
	3.07		without	not permitted			2.98	2.98	4.52	
8m offset	2.57	net	inner bracket				2.49	2.49	4.12	
	3.07		with inner bracket				2.98	2.98	5.30	
	2.57						2.49	2.49	4.89	
	3.07	net without inner bracket with inner bracket	4.57	4.57	4.83	1.52	1.52	2.31		
A	2.57		inner bracket	3.83	3.83	4.21	1.28	1.28	2.11	
4m offset	3.07			4.57	4.57	5.19	1.52	1.52	2.71	
	2.57			3.83	3.83	4.57	1.28	1.28	2.50	
Arra afficiat	3.07		without				6.61	3.30	4.76	
4m offset with DFA	2.57	tarpaulin	inner bracket				5.53	2.77	4.34	
(every free	3.07	tarpaulin	with	not	ot permitted		6.61	3.30	5.53	
node)	2.57		inner bracket				5.53	2.77	5.12	
	3.07		without	6.61	5.95	4.76	6.46	1.65	4.76	
	2.57	6	inner bracket	5.53	4.98	4.34	5.38	1.38	4.34	
2m	3.07	tarpaulin	with	6.61	5.95	5.53	6.46	1.65	5.53	
	2.57		inner bracket	5.53	4.98	5.12	5.38	1.38	5.12	

DFA = compression-resistant bracing

A \perp - = anchor compression forces

A \perp + = anchor tensile forces

Conversion of the anchor forces according to annex A, pages 23 to 26

B.6 Foundation loads

Depending on the design variant, the foundation loads listed in table B.5 and in annex C, page 22 must be absorbed and transferred in the supporting surface. The characteristic values given therein must be multiplied by the partial safety factor γ_F (generally $\gamma_F = 1.5$) to analyse the transfer of the loads to the supporting surface.

Scaffolding system ALFIX 70

Standard system configuration - General instructions

Standard force		Fittings	Bay length	n S	Structu	ral height		
[kN] for		-	[m]	24m	1	6m	8m	
	without		3.07	9.1	7	7.3	5.5	
Inner standard		without	2.57	7.7	6	6.2	4.7	
Fis	1	with short ner bracket	3.07	17.3	1	4.1	10.8	
	1	pracket 36)	2.57	14.5	1	1.8	9.1	
Outer standard		without	3.07	10.6	ę	9.3	8.1	
stairway access F _{AS,T}		without	2.57	8.9	7	7.8	6.8	
		without	3.07	12.7	ę	9.5	6.3	
		without	2.57	11.2	8	3.3	5.5	
				additionally				
	Protective wall		3.07		0.5			
Pro		tective wall	2.57		0.4			
Outer standard F _{AS}	0	uter bracket	3.07		5.7			
	U	lier brackel	2.57		4.9			
	Dura	otective roof	3.07		1.2			
	PIC	Diective roof	2.57			1.1		
		Add-on	3.07	4.2	2	2.9	1.6	
	stairway acces		2.57	3.5		2.4	1.3	
Special configura	tion 1	Bridging cons Fü	struction	Inner standar 1.5 • Fıs	d		standaro • F _{AS}	
Special configuration 2		Passage frame		Inner standar			standaro	

B.7 Protective roof

The protective roof may only be used on the outer face of a scaffold up to a working area of ≤ 8 m. The structural design as protective roof shall be carried out in accordance with annex C, pages 6 and 18. Additional scaffold anchoring and bracing measures must be observed.

 $F_{IS} + 0.54 \cdot F_{A}S$

Decks are to be installed up to the face of the building.

 \mathbf{F}_{D}

B.8 Passage frame

When using the passage frames, additional bracing in accordance with annex C, page 9 is required in accordance with the scaffold design.

The structural design as passage frame shall be carried out in accordance with annex C, pages 9 and 21. When constructing pedestrian passageways/underpasses, the clearance must be designed in accordance with DIN EN 12810-1:2004-03, section 7.3.6.3.

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Standard system configuration - General instructions

Annex B, Page 10

0.46 • Fas

B.9 Bridging construction

The bridging girders may be used to bridge gate entrances or similar openings when the working levels underneath the bridging construction are omitted.

The structural design of the bridging construction and the assembly variants shall be carried out in accordance with annex C, pages 7, 8 and 20. Depending on the configuration, additional bracing is required. Additional scaffold anchoring measures must be observed.

B.10 Inner ladder access / single flight stairway / add-on (external) ladder access

The use of a single flight stairway access in accordance with annex C, pages 15 and 16 is recommended. Additional scaffold anchoring and bracing measures must be observed.

Alternatively, an inner ladder access or an external ladder access may be used.

The decks listed in table B.6 must be used for both the inner and external ladder access.

Table B.6: Access decks

Access decks	Deck width [m]	Number per bay	in accordance with annex A, page
Aluminium access deck with ladder	61	1	15, 16, 22, 23
Aluminium frame platform with internal hatch	61	1	113, 114
Aluminium trapdoor deck with aluminium chequer plate	61	1	117, 118, 120

B.11 Corner formation

Outer corners are to be designed in accordance with annex C, page 27.

Observe the regulations for securing against uplifting wind forces in section B.1 for inner corners.

B.12 Widening bracket

The structural design using brackets 0.36 m in accordance with annex A, pages 58 and 59 and using brackets 0.73m in accordance with annex A, pages 60 and 61 must be carried out in accordance with annex C, page 19.

On the inner face of the scaffold, brackets 0.36m may be used on all working levels; on the outer face of the scaffold, brackets 0.36m or 0.73m may be fitted on the topmost working level only (see annex C, page 19). Bracket 0.73m must be supported by means of diagonal braces in accordance with annex A, page 103.

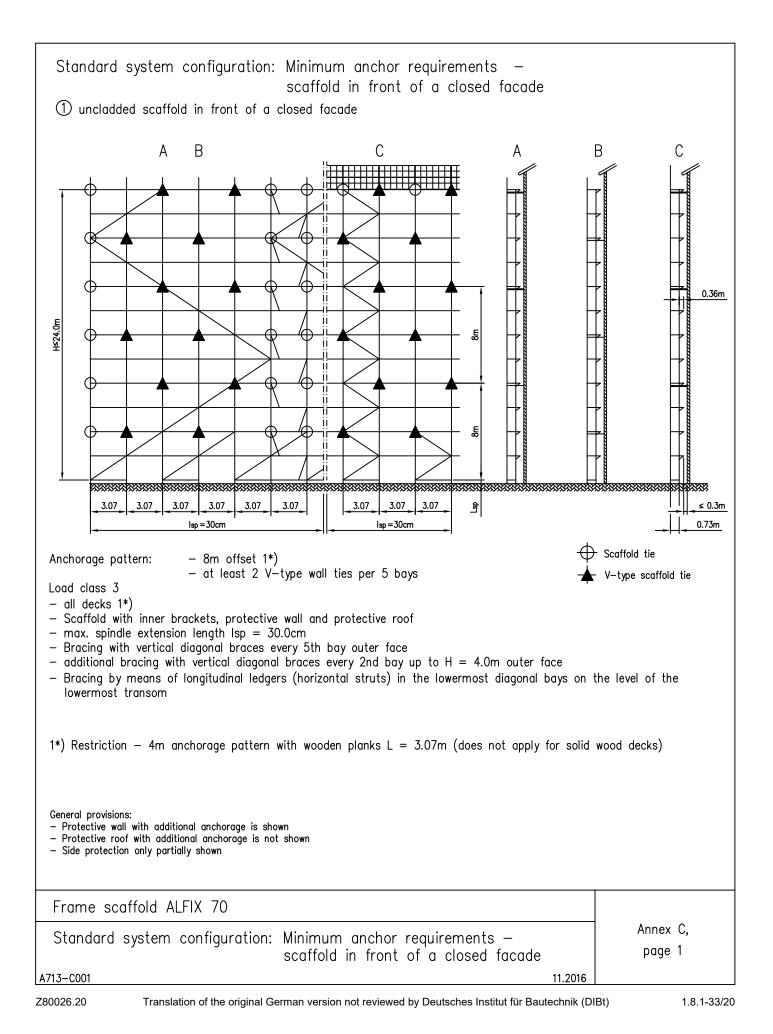
B.13 Topmost working level not anchored

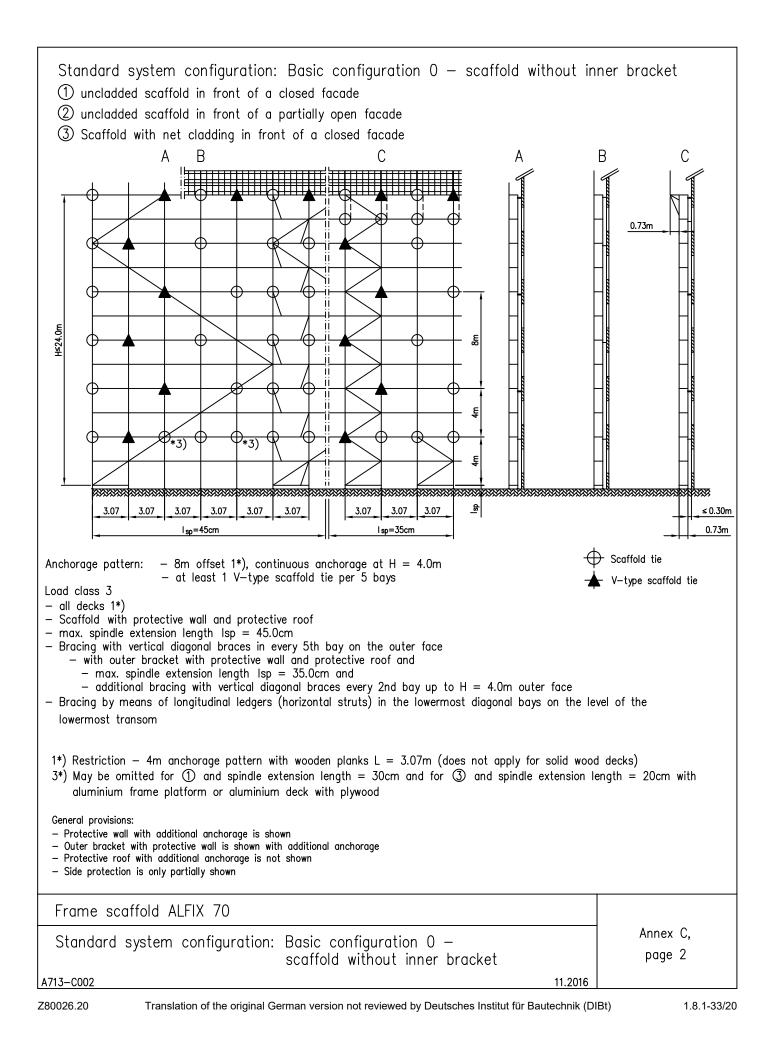
When constructing buildings, the height of the topmost working level may exceed the height of the topmost anchored level by 2 m (topmost working level not anchored), in accordance with annex C, page 10. In this intermediate state, the topmost working level must not be higher than H = 22 m (plus spindle extension length) within the scope of the verified standard system configuration. Furthermore, all joints of the standards on the three topmost levels must be secured by means of gravity pins. Additional scaffold anchoring and bracing measures must be observed.

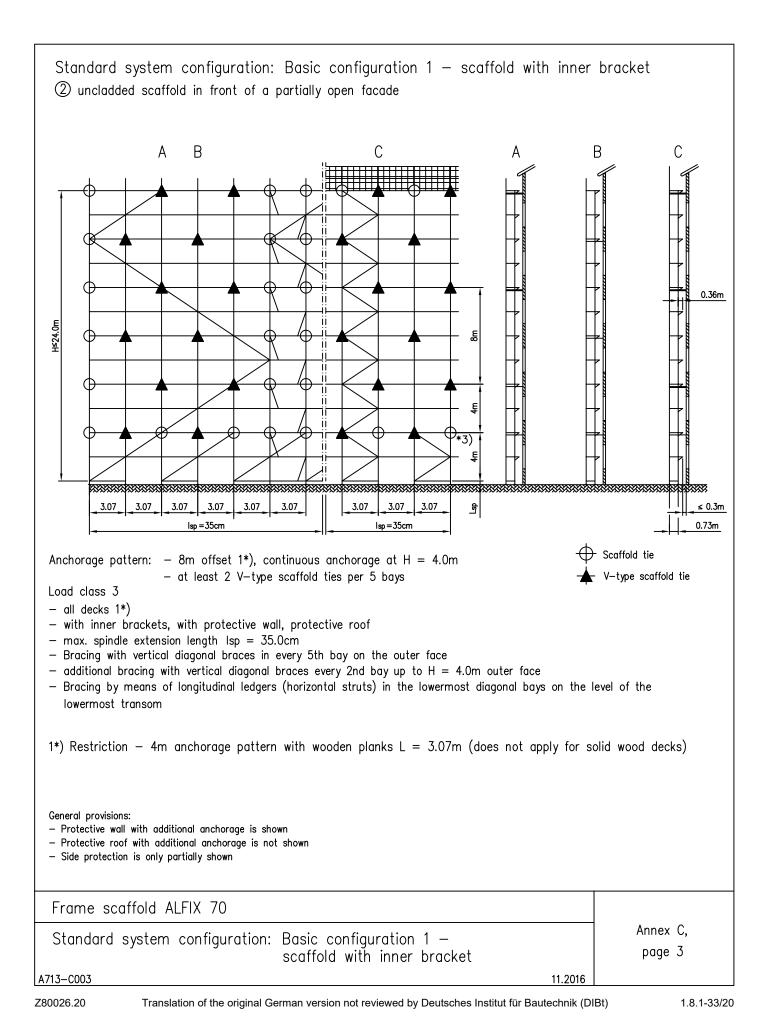
Cladding must not exceed the topmost anchoring level.

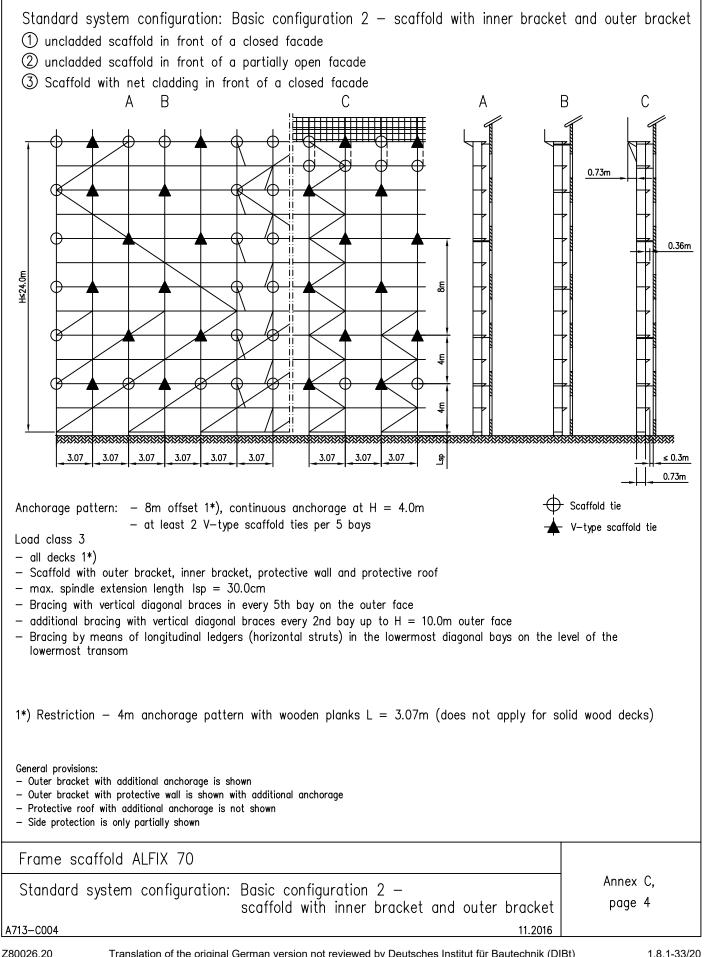
Scaffolding system ALFIX 70

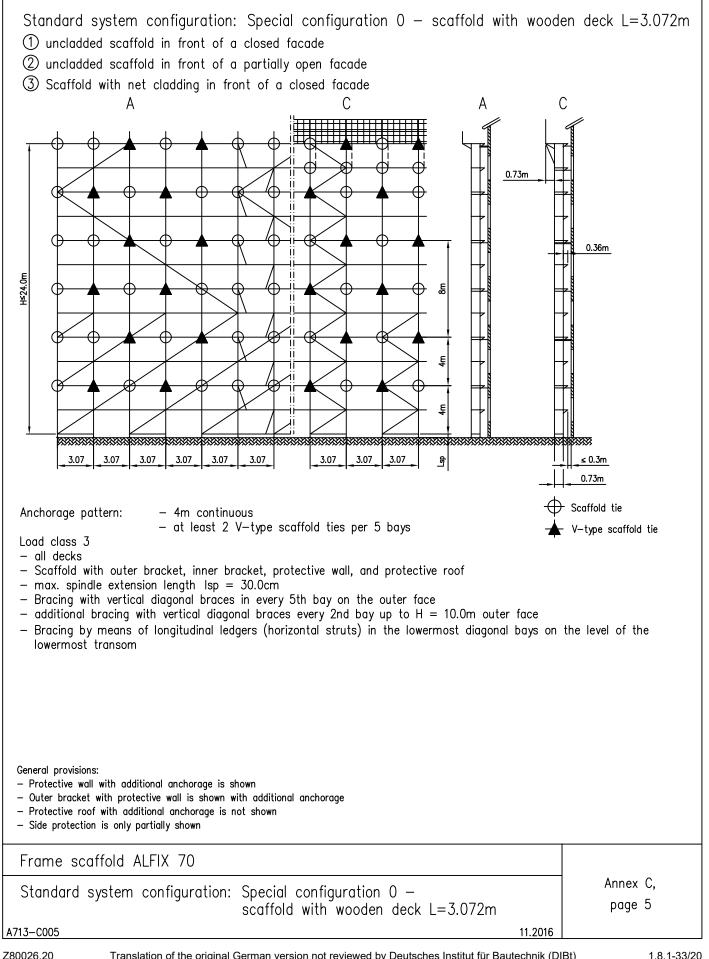
Standard system configuration - General instructions

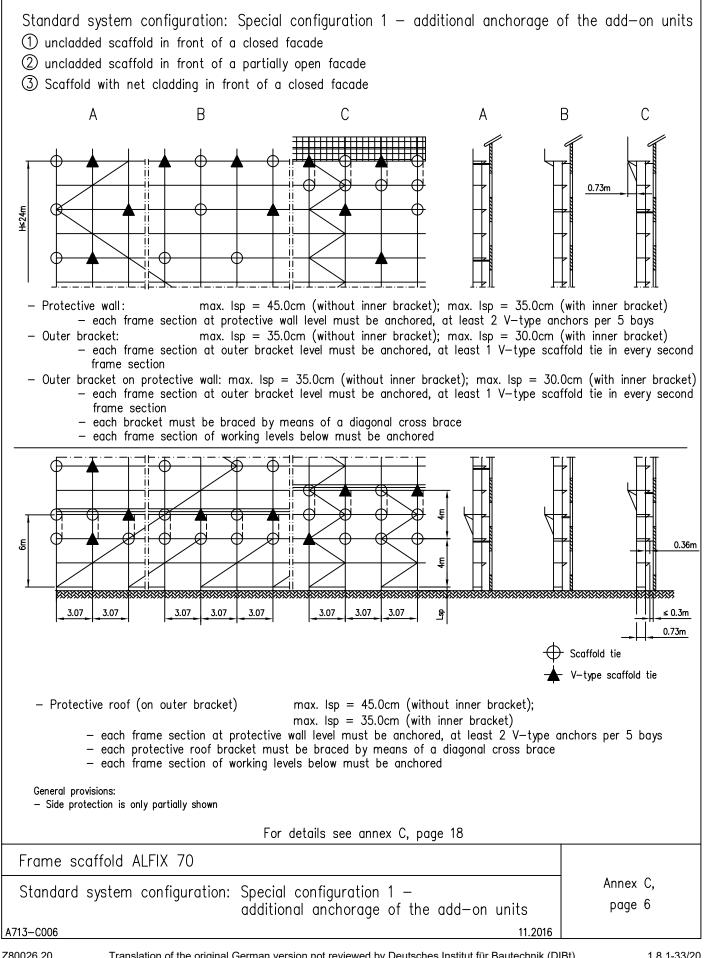


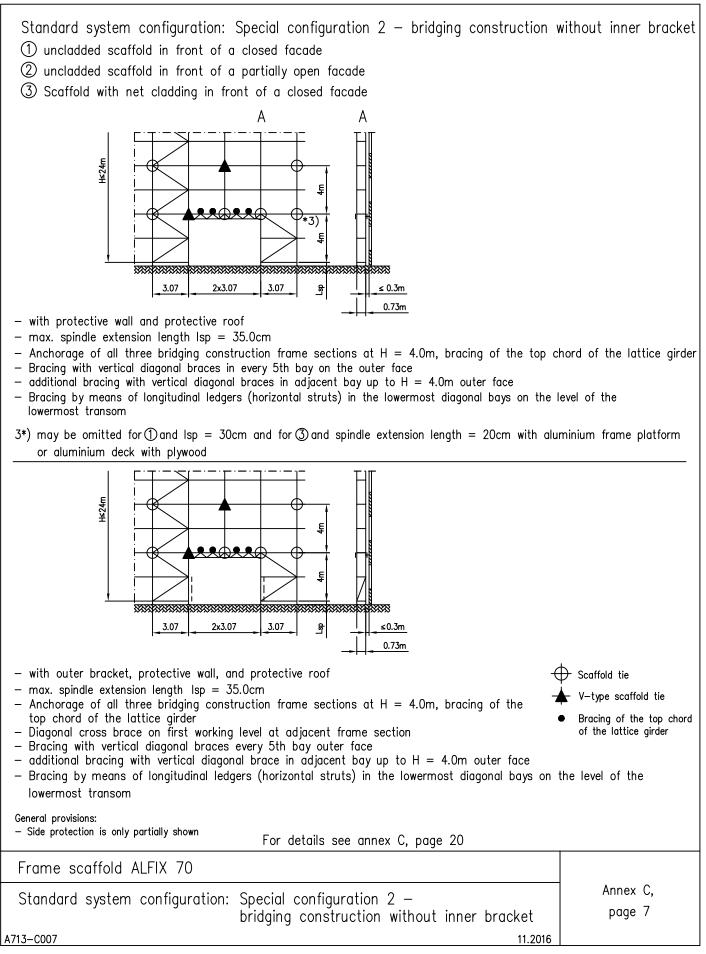


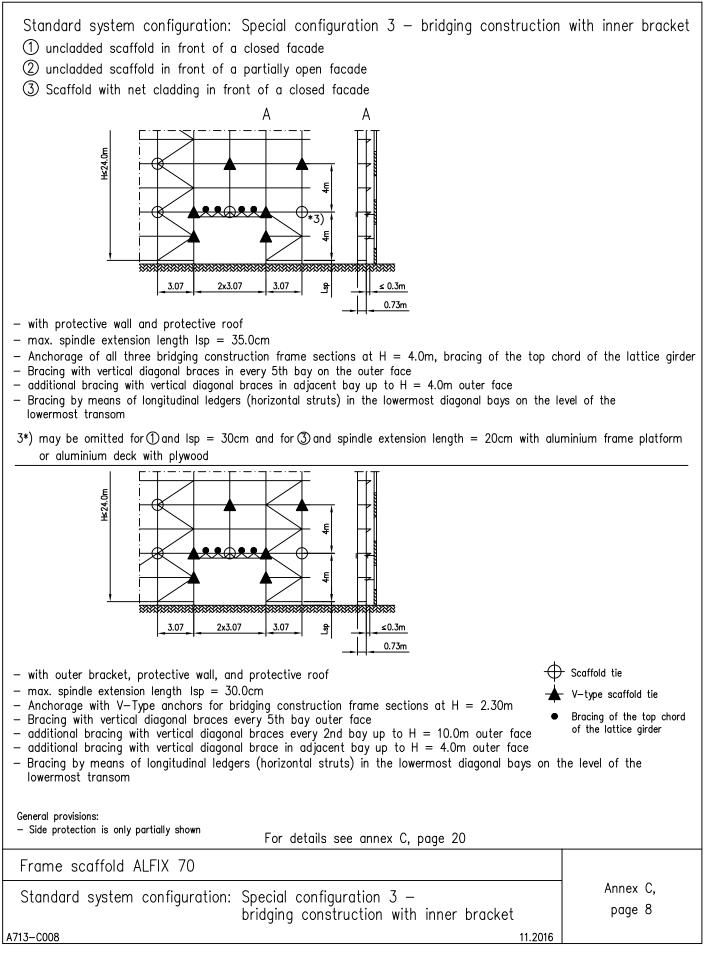


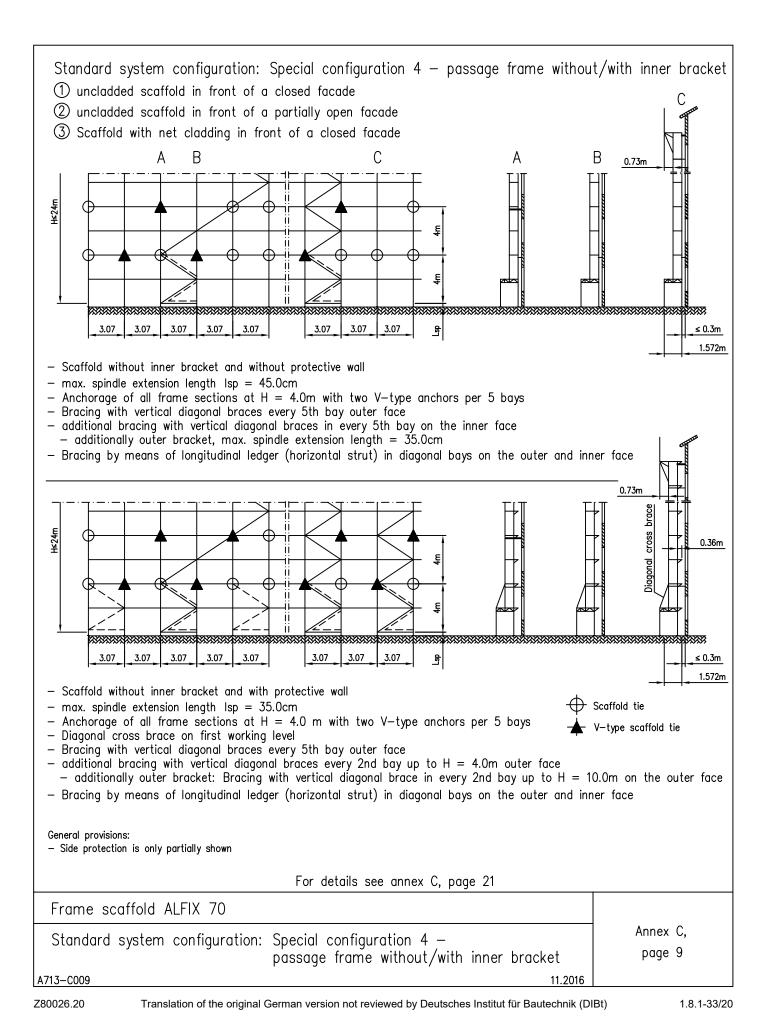


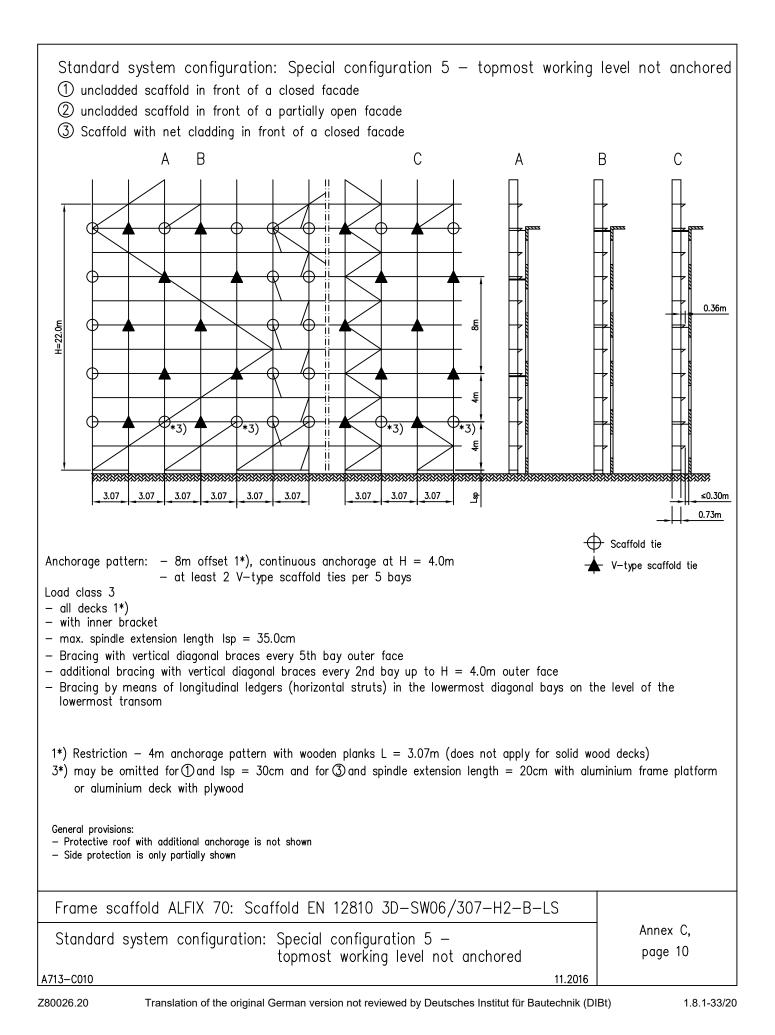


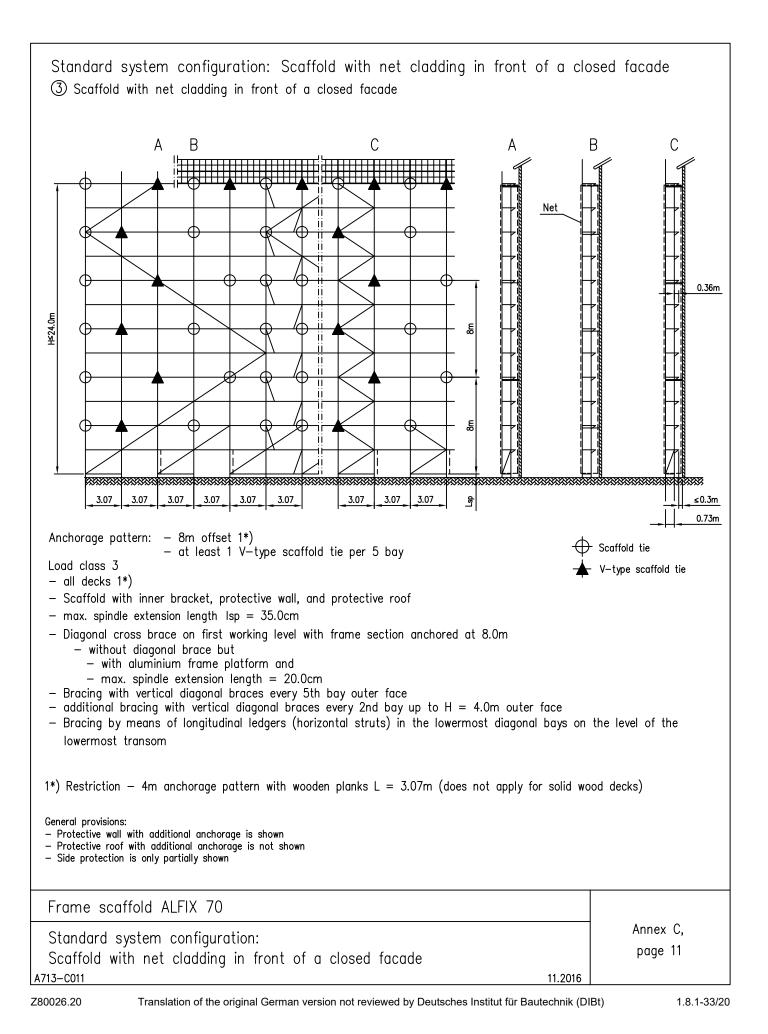


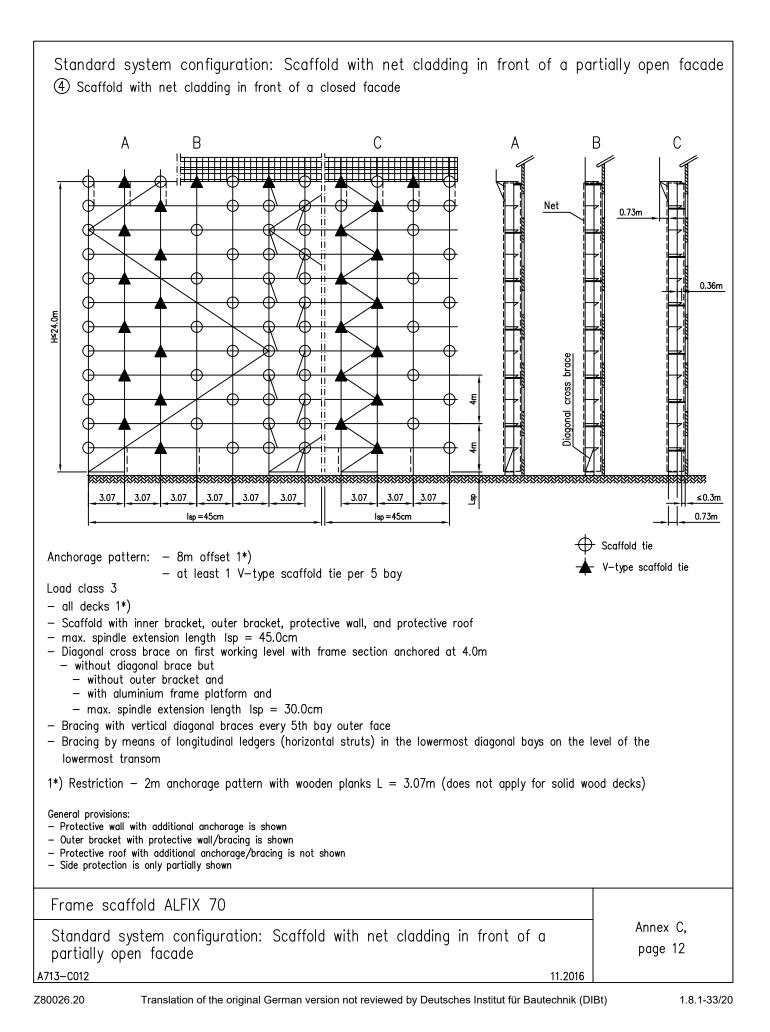


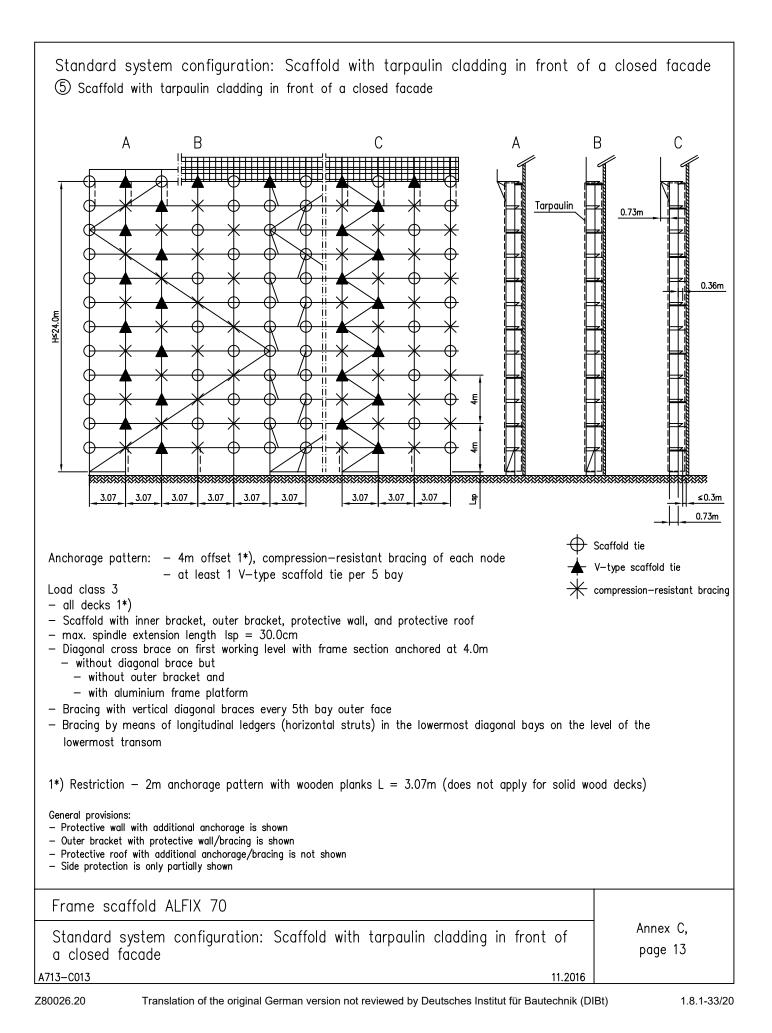


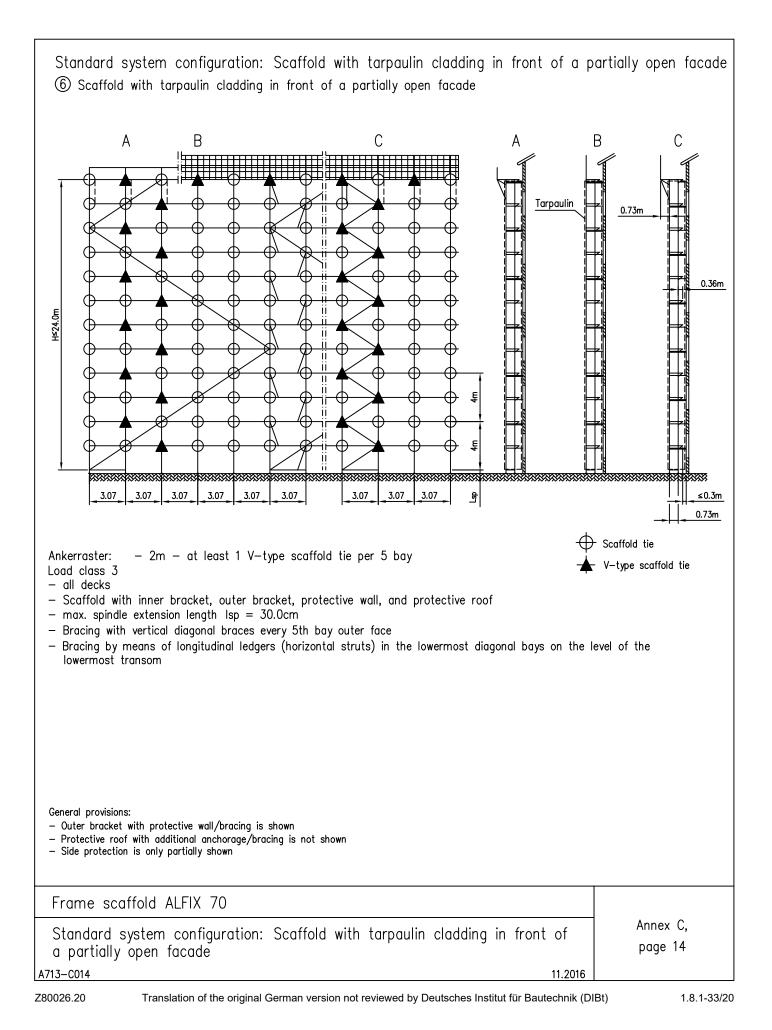


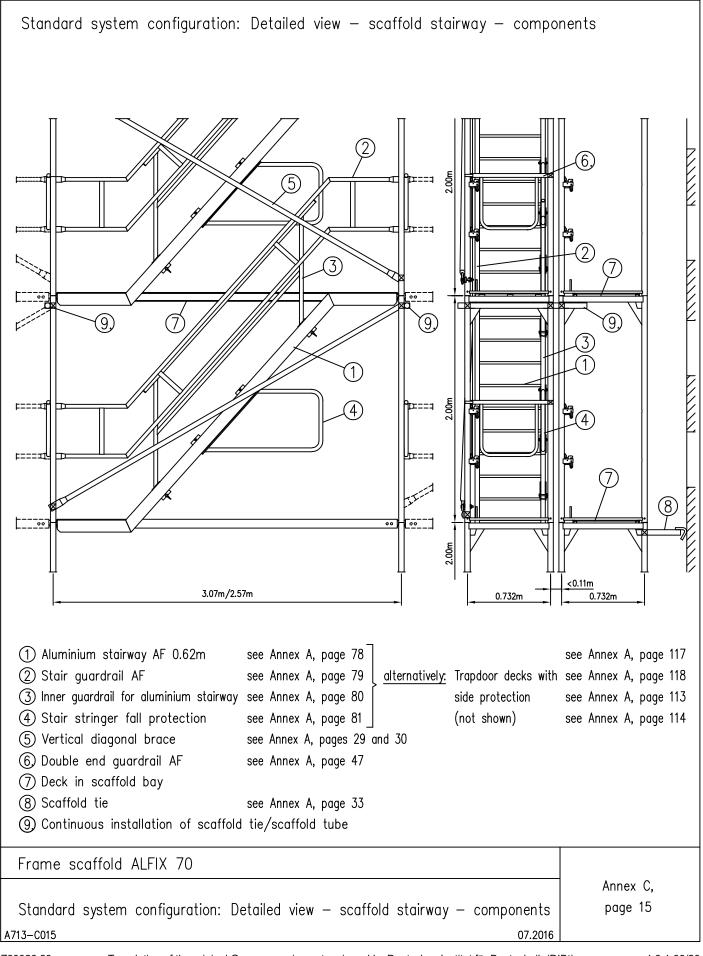


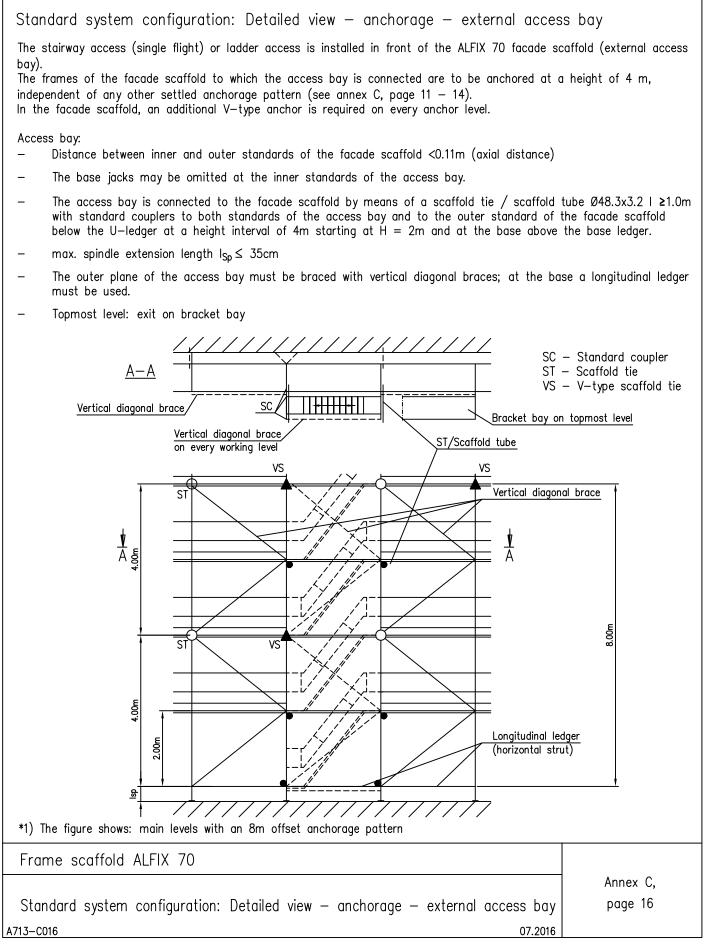


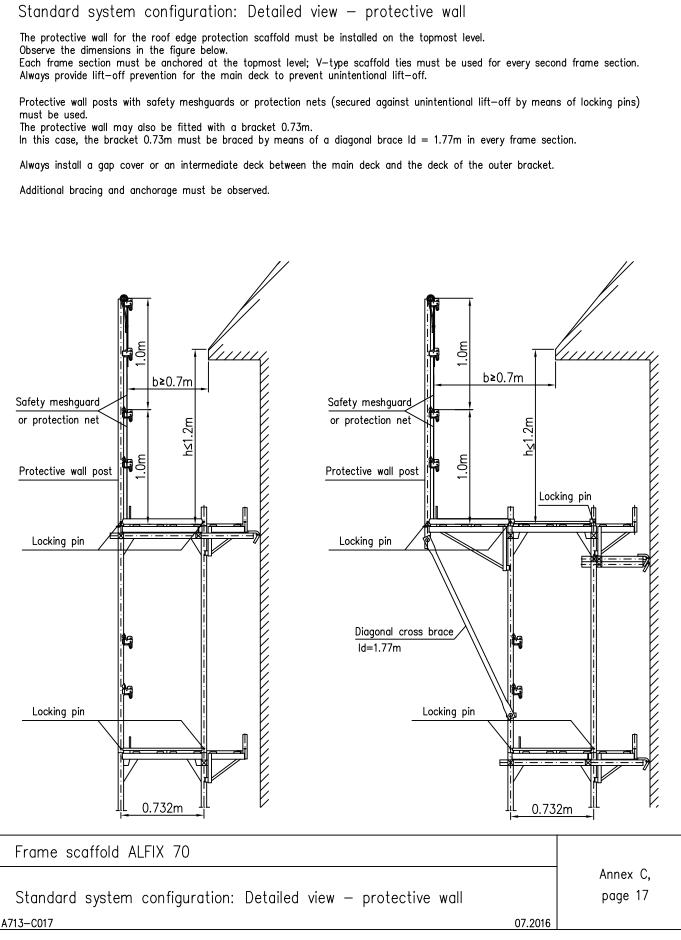












Standard system configuration: Detailed view - protective roof

The protective roof consists of a bracket 0.73m, supported by means of a diagonal cross brace, protective roof extension and decks.

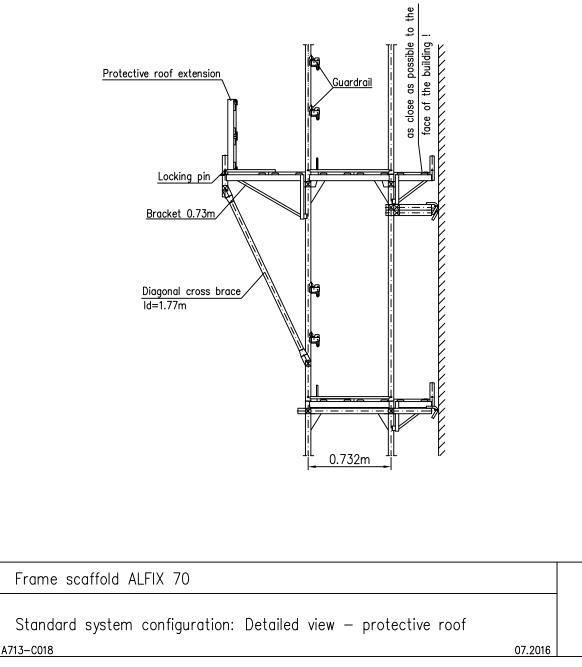
The bracket 0.73m must be assembled on the outer face of the vertical frame.

The protective roof extension is assembled on the bracket 0.73m with 2 decks (width 0.32m).

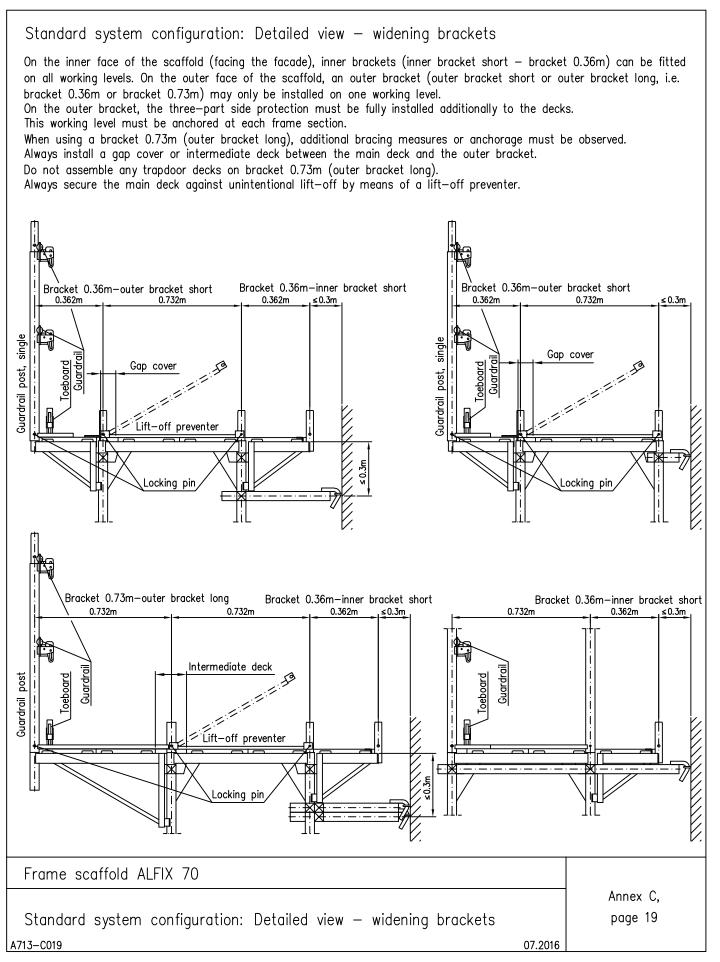
The gap between the scaffold deck and the platform of the protective roof must be covered with a gap cover. Scaffolding decks are to be installed up to the face of the building.

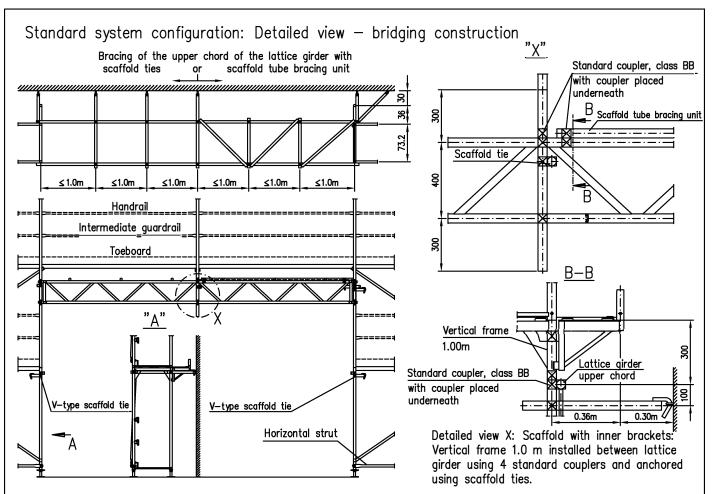
The main scaffold decking must be separated from the platform of the protective roof by means of guardrails. On the protective roof level and the working level below all frame sections must be anchored.

On the protective roof level, every second frame section must be anchored with a V-type scaffold tie.



Annex C, page 18





Two bridging girders (steel lattice girders 620) can be used for accommodating a vertical frame. Fix the upper chords of the lattice girder at intervals of a=1.0m. This can either be done by anchoring them to the facade using scaffold ties or by means of a tube and coupler bracing unit between both lattice girders. All three frame sections must be anchored at a height of approximately H=4.0m. Use at least one V-type scaffold tie within the structure of the bridging girder.

This allows the steel lattice girder 620 to transfer a central single load Fe=18.6kN (service load) in the case of 2x3.07m or Fe=22.6kN in the case of 2x2.57m bridging (this also applies to steel lattice girder 520). Other non-system lattice girders – also aluminium lattice girders – may be used as bridging girders, provided they have the required load-bearing capacity. Please refer to the tables listing the bearing reactions for the load-bearing capacity of the lattice girders in accordance with the different assembly configurations. When using outer brackets, the fame must be connected to the outer lattice girder using standard couplers, class BB, with a coupler placed underneath.

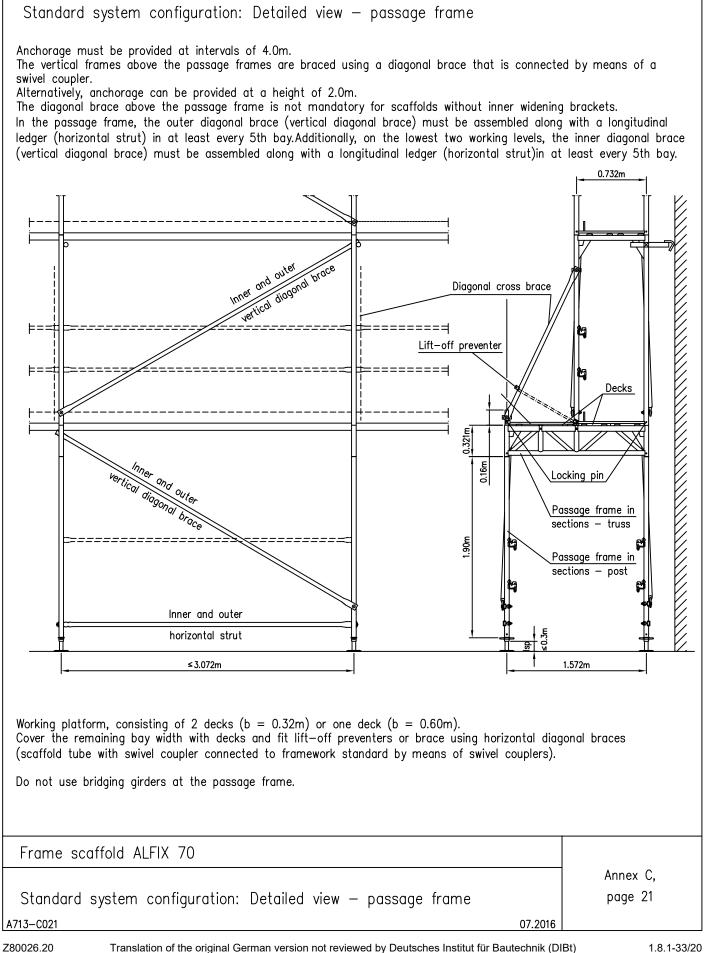
For <u>scaffolds with inner brackets</u> (shown here) the upper chords of the lattice girders are connected to the vertical frames approximately 30cm below the frame joints using standard couplers. class BB, with a coupler placed underneath. In this case, vertical frames 1.0m must be used below the frame section that needs to be accommodated. The inner standards of the lateral frame sections must be anchored using V-type scaffold ties below the bridging construction at H=2.0m.

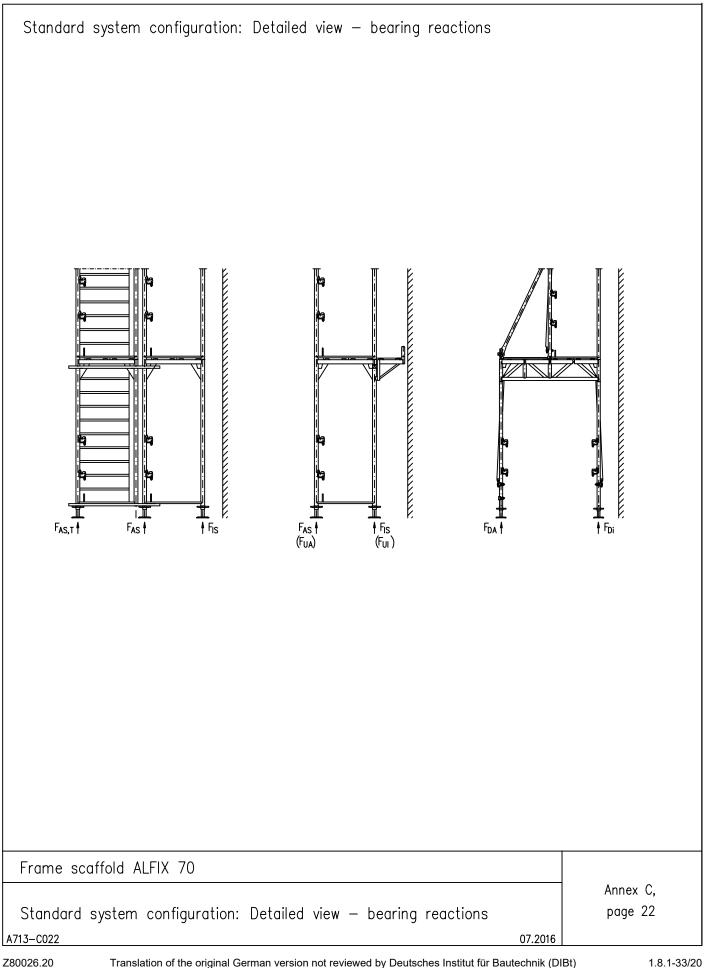
For <u>scaffolds without inner brackets</u> the upper chords of the lattice girders are connected to the vertical frame at the gusset plate using standard couplers, class BB, (couplers fitted underneath are not required in this case). In this case, vertical frames 1.00m or 0.66m must be used below the frame section that needs to be accommodated. Here, V-type scaffold ties at the lateral frame sections below the bridging construction at H=2m are not mandatory. When using outer brackets, however, both frame sections on the first working level must be braced with diagonal cross braces.

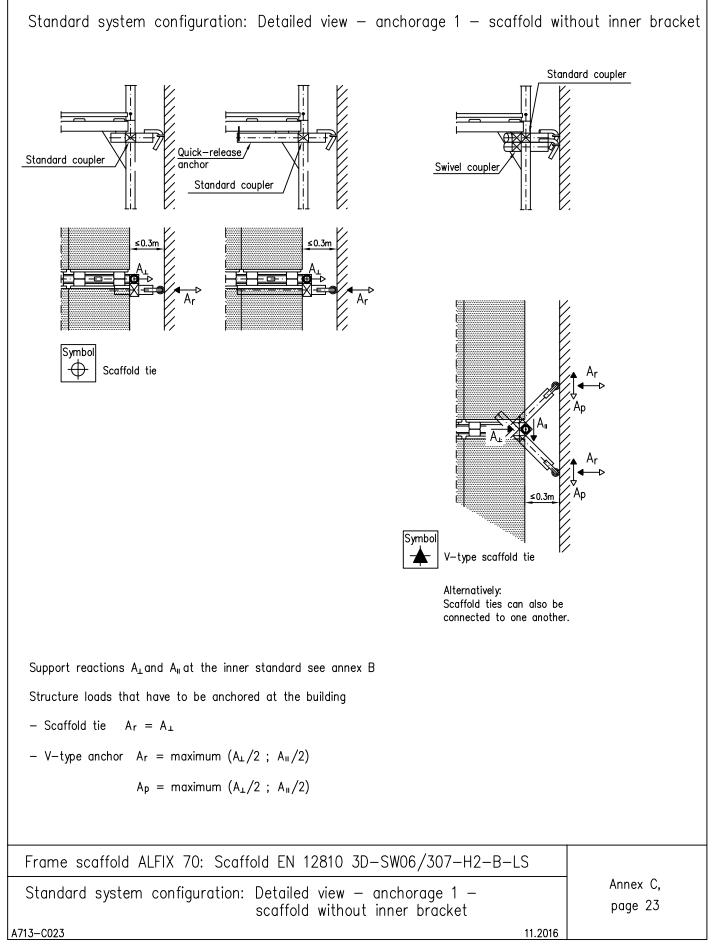
Do not use passage frames at the bridging section!

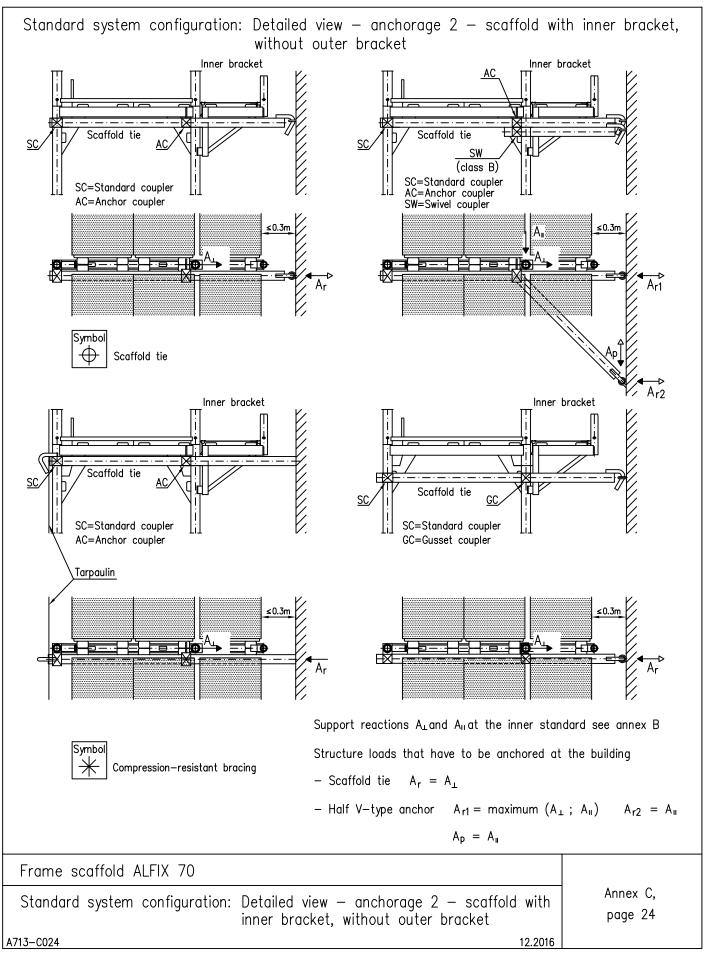
Frame scaffold ALFIX 70

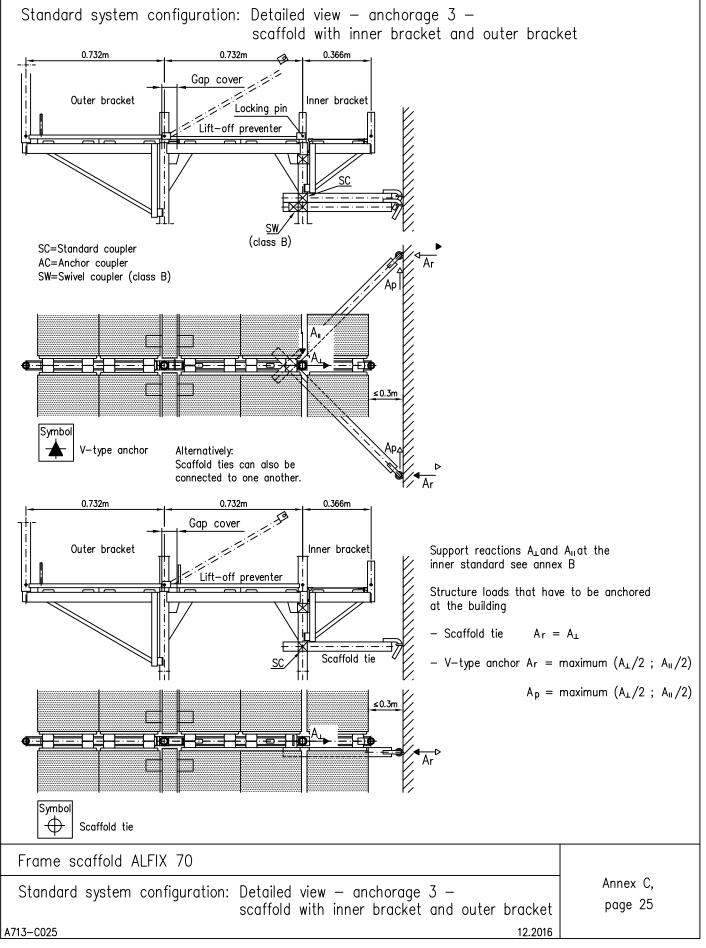
	Annex C,
Standard system configuration: Detailed view — bridging construction	page 20
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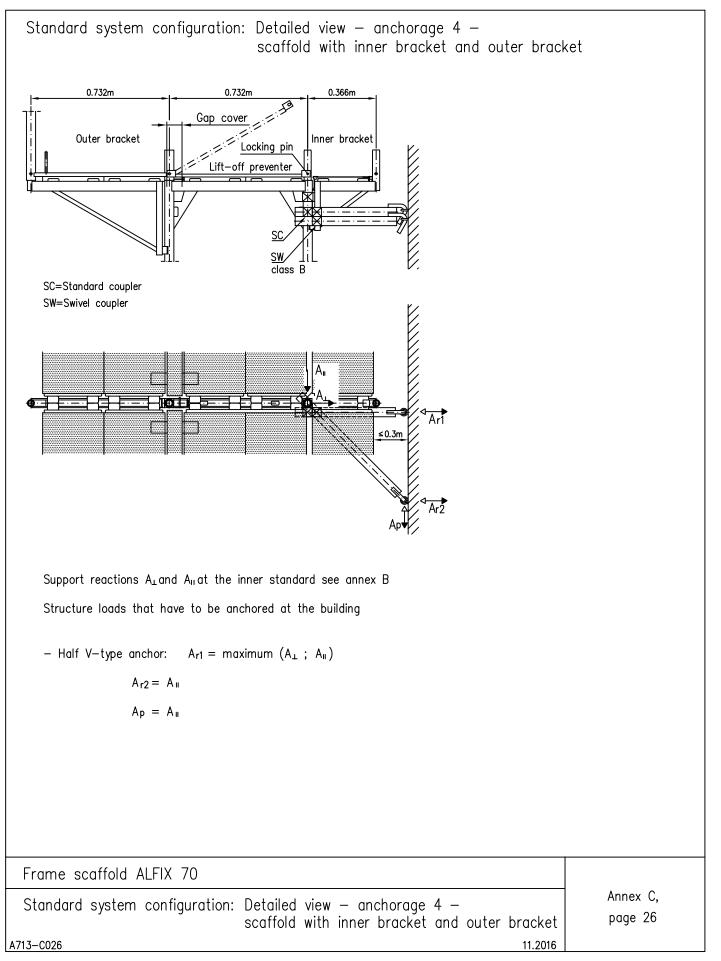


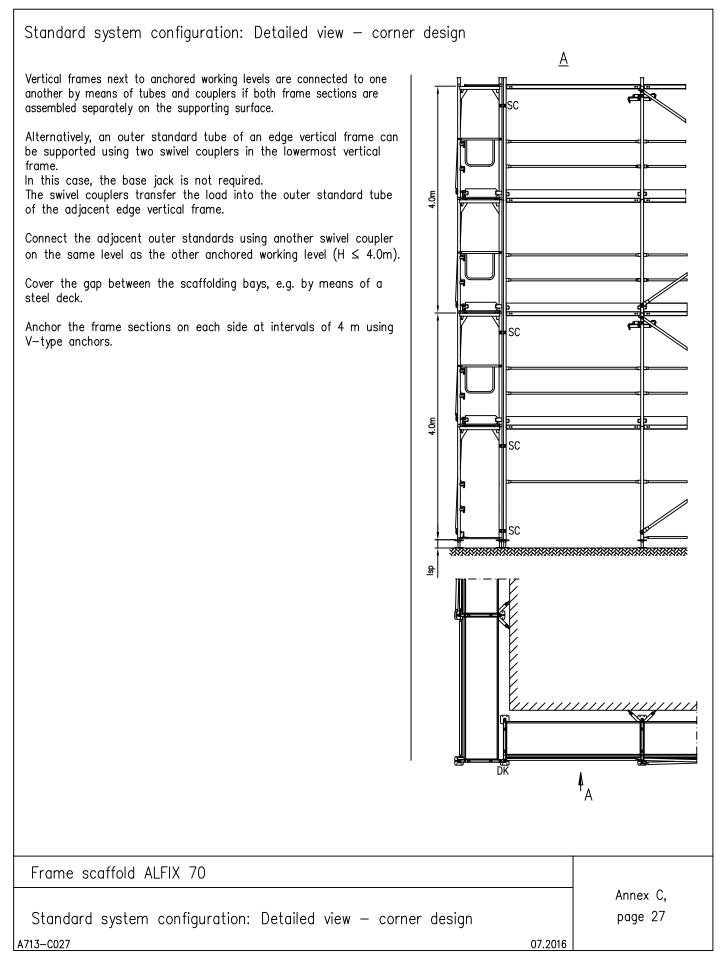












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