



ALBLITZ MODUL

[Seal: Deutsches Institut für Bautechnik]

General construction technique permit

An institution under public law jointly funded by the German Federation and the federal states (Länder)

Approval Body for Construction Products & Techniques

Date: Reference number: 30 October 2023 | 37.1-1.8.22-20/17

Approval number: **Z-8.22-913**

Period of validity: from: **30 October 2023** to: **9 May 2027**

Applicant:

Alfix GmbH Langhennersdorfer Straße 15 09603 Großschirma (Germany)

Subject of approval:

Modular scaffolding system "ALBLITZ MODUL"

The above-mentioned subject matter is hereby granted general construction technique permit. This decision comprises 34 pages, as well as Annex A (pages 1 to 4), Annex B (pages 1 to 306), Annex C (pages 1 to 6) and Annex D (pages 1 to 8).

This general construction technique permit supersedes the general construction technique permit no. Z-8.22-913 dated 16 May 2022. The building authorities first approved the subject matter under review on 10 April 2007.

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I GENERAL PROVISIONS

- This general construction technique permit serves to prove the applicability of the subject matter under regulation within the meaning of the building regulations of the federal states.
- 2 This decision does not replace statutory approvals, authorisations and certifications specified for carrying out construction works.
- This decision is granted without prejudice to the rights of third parties, especially private property rights.
- The manufacturer and seller of the subject matter of the decision must, without prejudice to further regulations laid out under "Special Provisions", make copies of this decision available to the persons using or applying the subject matter of the decision and must inform these persons that the decision must be present at the place of use. Upon request, copies must be provided to the relevant authorities.
- This decision may only be reproduced in its entirety. Publication of the decision in excerpts requires the prior consent of Deutsches Institut für Bautechnik (DIBt). Text and drawings of promotional material must be consistent with this decision. Translations must include the following note: "Translation of the original German version not reviewed by Deutsches Institut für Bautechnik".
- This decision is issued on a revocable basis. The provisions of this decision may be amended or modified at a later time, particularly if new technical knowledge requires this.
- This decision relates to the information on the subject matter of the permit made available by the applicant during the approval process and the documents submitted. Any change made to the basic provisions of this approval is not covered by this decision and must be disclosed to Deutsches Institut für Bautechnik without delay.

II SPECIAL PROVISIONS

1 Subject matter and scope of application

Subject matter of the permit are the planning, dimensioning and execution of the modular scaffolding system "ALBLITZ MODUL", which consists of

- scaffolding components according to Table 1 and
- scaffolding components in accordance with MVV TB (Model Administrative Provisions Technical Building Rules), Part C 2.16 according to the respective scope of application.

The modular scaffolding system is erected using standards, ledgers, diagonal braces and decks as basic components as well as scaffolding spindles (base jacks), scaffold ties, system components for the side protection, access components and supplementary components. The standards, ledgers and diagonal braces are connected to each other by means of special scaffolding (module) nodes. The scaffold nodes are available in different versions, which can be combined with each other for two different load-bearing groups according to Table 2.

The scaffold nodes consist of a connector disc (rosette) welded onto a tubular standard and connector heads welded onto tube ledgers or U-ledgers or hinged to vertical diagonal braces. The connector heads enclose the connector disc (rosette) and are wedged to the connector disc by driving a captive wedge into the connector disc (rosette) with a hammer blow in a way that the connector heads are pressed against the tubular standard.

Each connector disc (rosette) has 8 openings, allowing to connect up to 8 members.

The modular scaffolding system "ALBLITZ MODUL" may be used as service and working scaffold according to DIN EN 12811-1:2004-03 in conjunction with the "Application Guideline for Working Scaffolds in accordance with DIN EN 12811-1" and DIN 4420-1:2004-03, as supporting structure in accordance with DIN EN 12812:2008-12 in conjunction with the "Application Guideline for Supporting Structures in accordance with DIN EN 12812" or as any other temporary structure.

2 Provisions for planning, dimensioning and execution

2.1 Planning

2.1.1 General provisions

The Technical Building Rules [Technische Baubestimmungen] apply to the planning of scaffolds using components of the "ALBLITZ MODUL" modular system, in particular those for working and service scaffolds according to DIN EN 12811-1:2004-03 in conjunction with the "Application Guideline for Working Scaffolds in accordance with DIN EN12811-1"¹, DIN 4420-1:2004-03, the "Approval Principles for Working and Service Scaffolds – Requirements, Calculation Assumptions, Tests, Certificate of Conformity"³; for supporting structures DIN EN 12812:2008-12 in conjunction with the "Application Guideline for Supporting Structures in accordance with DIN EN 12812"². is applicable, as well as the provisions set out below.

When using the modular scaffolding system as a temporary structure that does not fall within the scope of temporary auxiliary construction aids, application-specific requirements may need to be taken into account in the design.

The scaffolds shall be planned in accordance with engineering standards. Verifiable calculations must be prepared in accordance with the Technical Rules and Regulations [Technisches Regelwerk] and the design drawings.

see DIBt-Mitteilungen (notifications of the DIBt), issue 2/2006, p. 61 et seq.

see DIBt-Mitteilungen (notifications of the DIBt), issue 6/2009, p. 227 et seq.

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The modular scaffolding system "ALBLITZ MODUL" comprises the scaffold components listed in section 1. The structural differences between the scaffold nodes and components are shown as follows:

"MODUL MULTI": Annex B, pages 2 to 7 according to Z-8.22-906
"MODUL MULTI 4.0": Annex B, pages 151 to 154 according to Z-8.22-906
"Allround K2000+ design": Annex B, pages 171 to 176 according to Z-8.22-64
"Allround lightweight design": Annex B, pages 165 to 170 according to Z-8.22-939

Please refer to Annex B, pages 1 and 161 to 164 for further illustrations of the modular connector node.

Depending on the construction techniques used, a differentiation is made between the designs and load-bearing groups listed in Table 2.

Table 1: Scaffolding components for use in the "ALBLITZ MODUL" modular scaffolding system

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity |
|---|------------------|---|--|
| Vertical diagonal braces | 8 | 3, 6 | |
| Horizontal diagonal braces | 9 | 7 | |
| Vertical starter piece | 10 | 2 | |
| Standard with tube connector 200 | 11 | 2 | |
| Standard with screwed-in tube connector 520 | 12 | 2 | according to |
| Standard with screwed-in tube connector 500 | 13 | 2 | Z-8.22-906 |
| Standard 0.50 m with screwed-in tube connector 520, s=4.05 mm | 14 | 2, 12 | |
| Vertical starter standard | 15 | 2 | |
| Top standard | 16 | 2 | |
| Base jack | 17 | | |
| Base jack AB | 18 | | according to Z-8.1-862 |
| Base jack AF with swivel base | 19 | | 2-0.1-002 |
| Base jack with swivel base | 20 | | |
| U-head jack | 21 | | |
| Spindle coupler | 22 | | |
| Suspended scaffolding connector | 23 | 3, 4, 151 | |
| Locking device for base jack | 24 | 3, 4, 151 | |
| Tube ledger | 25 | 3, 4 | |
| Horizontal diagonal ledger | 26 | 3, 4 | according to |
| Tube ledger reinforced | 27 | 3, 4 | Z-8.22-906 |
| Double tube ledger 1.57 m | 28 | 3, 4 | |
| Double tube ledger 2.07 m | 29 | 3, 4, 28 | |
| Double tube ledger 2.57 m | 30 | 3, 4, 28 | |
| Double tube ledger 3.07 m | 31 | 3, 4, 28 | |
| U-ledger 0.37 m; 0.39 m; 0.45 m; 0.73 m | 32 | 3, 5, 152 | |
| U-ledger reinforced 1.09 m and 1.40 m | 33 | 3, 5, 27, 32 | |

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<u>Table 1:</u> (continued)

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity |
|---|------------------|---|---|
| U-2-deck bearer 1.57 m | 34 | 3, 5, 32 | |
| U-2-deck bearer 2.07 m | 35 | 3, 5, 32, 34 | |
| U-2-deck bearer 2.57 m | 36 | 3, 5, 32, 34 | |
| U-2-deck bearer 3.07 m | 37 | 3, 5, 32, 34 | |
| Support ledger with tube fixture | 40 | 3 | |
| | | | |
| Support ledger | 43 | 3, 32 | |
| U-transom lattice girder 0.73 m / 1.09 m V | 44 | 27, 32, 127 | |
| Tube-transom lattice girder 0.73 m / 1.09 m V | 45 | 27, 144 | |
| MODUL lattice girder 6.14 m | 46 | 3, 4, 151 | |
| MODUL lattice girder 4.14 m / 5.14 m | 47 | 3, 4, 46, 151 | |
| MODUL lattice girder with tube fixture 6.14 m | 48 | 3, 4, 46, 151 | |
| MODUL lattice girder with tube fixture 4.14 m / 5.14 m | 49 | 3, 4, 46, 48, 151 | |
| MODUL lift-off preventer | 50 | | |
| Aluminium frame platform with tube fixture 1.57 m; 2.07 m | 51 | 53 | |
| Aluminium frame platform with tube fixture 2.57 m; 3.07 m | 52 | 53 | |
| Aluminium access frame platform with tube fixture 3.07 m | 54 | 53, 56, 60 | according to |
| Aluminium access frame platform with tube fixture 2.57 m | 55 | 53, 56, 60 | Z-8.22-906 |
| Aluminium access frame platform with tube fixture 1.57 m – 3.07 m without ladder | 57 | 53, 56 | |
| Aluminium access frame platform with tube fixture 2.57 m; 3.07 m with aluminium chequer plate | 58 | 59, 60 | |
| Steel deck AF with tube fixture 0.32 m | 61 | | |
| Steel deck AF with tube fixture 0.30 m; 0.34 m | 62 | | |
| Intermediate deck AF with tube suspension | | | |
| 0.16 m, 0.19 m | 63 | | |
| Steel deck with tube fixture | 64 | | |
| Intermediate deck with tube suspension | 65 | | |
| Aluminium frame platform with plywood 0.50 m – 2.07 m | 66 | 68 | |
| Aluminium frame platform with plywood 2.57 m; 3.07 m | 67 | 68 | |
| Aluminium frame platform with internal hatch 2.57 m; 3.07m | 69 | 60, 68, 71 | |
| Aluminium frame platform with internal hatch 1.09 m – 3.07 m without ladder | 70 | 68, 71 | |
| Aluminium deck with plywood 2.57 m; 3.07 m | 72 | 74 | |
| Aluminium deck with plywood 1.57 m; 2.07 m | 73 | 74 | |
| Aluminium access deck 3.07 m with ladder | 75 | 60, 74, 77 | according to Z-8.1-862 |
| Aluminium access deck 2.57 m with ladder | 76 | 60, 74, 77 | |

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Table 1: (continued)

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity |
|--|------------------|---|--|
| Aluminium deck with plywood 3.07 m | 78 | 80 | |
| Aluminium deck with plywood 1.57 m, 2.07 m, 2.57 m | 79 | 80 | |
| Aluminium access deck 3.07 m with ladder | 81 | 60, 80, 83 | |
| Aluminium access deck 2.57 m with ladder | 82 | 60, 80, 83 | |
| Steel deck AF 0.32 m | 84 | | |
| Steel deck | 85 | | according to |
| Steel deck AF 0.30 m; 0.34 m | 86 | | Z-8.1-862 |
| Steel plank 0.30 m | 87 | | |
| Intermediate deck AF 0.16 m, 0.19 m | 88 | | |
| Intermediate deck | 89 | | |
| Lightweight aluminium deck 0.60 m | 90 | | |
| Solid wood deck 48 | 91 | | |
| Solid wood deck 45 | 92 | | |
| Wooden deck | 93 | | |
| MODUL gap cover | 94 | | according to |
| MODUL gap cover with tube fixture | 95 | | Z-8.22-906 |
| Gap cover | 96 | | according to |
| Aluminium stairway AF-0.62 m 2.57 m; 3.07 m | 97 | | Z-8.1-862 |
| Stair guardrail 2.57 m; 3.07 m | 98 | 3 | according to Z-8.22-906 |
| Inner guardrail for aluminium stairway 2.00m | 99 | | according to |
| Stair stringer fall protection 1.00 x 0.50 m | 100 | | Z-8.1-862 |
| MODUL stair guardrail holder | 101 | 3, 4, 151 | |
| MODUL swinggate | 102 | 3, 139 | |
| Bracket 0.39 m with tube fixture | 103 | 3, 4, 151 | |
| MODUL bracket 0.39 m | 104 | 3, 5, 32, 152 | |
| MODUL bracket 0.73 m | 105 | 3, 5, 32, 152 | according to Z-8.22-906 |
| Bracket with tube fixture 0.50 m | 106 | 3, 4, 151 | 2-0.22-900 |
| MODUL toeboard | 107 | | |
| MODUL toeboard 4.14 m | 108 | | |
| MODUL toeboard, aluminium | 109 | | |
| Toeboard; End-toeboard AF | 110 | | |
| Toeboard 4.14 m AF | 111 | | |
| Toeboard; End-toeboard | 112 | | |
| Toeboard 4.14 m | 113 | | according to |
| Toeboard, aluminium; End-toeboard, aluminium AF | 114 | | Z-8.1-862 |
| Toeboard, aluminium; End-toeboard, aluminium | 115 | | |

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<u>Table 1:</u> (continued)

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity |
|--|------------------|---|--|
| MODUL guard net system | 116 | 3, 4, 25, 151, 155 | according to |
| MODUL double end guardrail | 117 | 3, 4, 151 | Z-8.22-906 |
| Storey ladder 2.00 x 0.40 m, steel | 118 | | according to |
| Storey ladder 2.00 x 0.40 m, aluminium | 119 | | Z-8.1-847 |
| Scaffold retainer / wall tie | 120 | | according to |
| Quick-release wall tie | 121 | | Z-8.1-862 |
| Wedge head coupler, swivel base | 122 | 3, 139, 151 | |
| MODUL U-tube connector | 123 | | |
| MODUL tube connector | 124 | 3 | according to Z-8.22-906 |
| Wedge-head coupler, rigid | 125 | 3, 4, 151 | 2 0.22 000 |
| Support ledger | 126 | 3, 4, 151 | |
| Transom 0.73 m, 1.09 m | 127 | 32 | |
| Guardrail coupler AF | 128 | | |
| Toeboard coupler; Halfcoupler with hook | 129 | | |
| Squared timber coupler | 130 | | |
| Toeboard support | 131 | 3 | according to |
| Locking pin | 132 | | Z-8.1-862 |
| Putlog coupler | 133 | | |
| Diagonal cross brace | 134 | | |
| Advanced guardrail post 2.00 m | 135 | | |
| Telescopic guardrail 2.00 - 3.07 m | 136 | | |
| MODUL advanced guardrail post | 137 | | according to Z-8.22-906 |
| Advanced end guardrail / Aluminium telescopic guardrail | 138 | | according to Z-8.1-862 |
| AB head jack "U" | 140 | | |
| MODUL U-lattice girder 6.14 m, 7.71 m | 141 | 3, 4, 5, 32, 147, 151, 152 | |
| MODUL U-lattice girder 4.14 m; 5.14 m | 142 | 3, 4, 5, 32, 141, 147, 151, 152 | |
| Claw coupler | 143 | | according to Z-8.22-906 |
| Tube ledger, reinforced 1.09 m; 1.29 m; 1.40 m | 144 | 3, 4, 151 | 2-0.22-300 |
| Tube ledger, reinforced, 1.57 m; 2.07 m | 145 | 3, 4, 144, 151 | |
| Tube ledger, reinforced, 2.57 m, 3.07 m | 146 | 3, 4, 144, 151 | |
| U-ledger 1.04 m; 1.09 m; 1.29 m | 147 | 3, 5, 152 | |

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Table 1: (continued)

| Table 1. (Continued) | | | | |
|--|------------------|---|--|--|
| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity | |
| U-ledger with integrated joist 1.40 m -2.07 m | 148 | 3, 5, 152 | | |
| U-ledger, reinforced 1.40 m - 2.57 m | 149 | 3, 5, 144, 147, 152 | | |
| U-ledger, reinforced 3.07 m | 150 | 3, 5, 144, 147, 149, 152 | | |
| Standard 4.0 | 153 | 2 | according to | |
| Vertical starter standard 4.0 | 154 | 2 | Z-8.22-906 | |
| Tube ledger 4.0 | 155 | 3, 151 | | |
| Horizontal diagonal ledger 4.0 | 156 | 3, 151 | | |
| MODUL gap cover, T-shaped | 157 | | | |
| Standard with tube connector 200 45/5 | 158 | 2 | | |
| Platform guardrail AB 2.57; 3.07 m | 159 | 3 | | |
| Base jack 60 | 178 | | according to Z-8.1-16.2 | |
| Starter piece, lightweight | 179 | 165 | | |
| Standard with integrated tube connector, lightweight | 180 | 165, 182 | | |
| Starter standard 2.21 m, lightweight | 181 | 165, 182 | | |
| Standard without tube connector, lightweight | 183 | 165, 182 | | |
| Tube connector for standard | 184 | | | |
| O-ledger, lightweight 0.73 – 4.35 m | 185 | 166, 170 | | |
| O-ledger, lightweight, HD | 186 | 166, 170 | | |
| U-ledger, lightweight 0.73 m T14 | 187 | 167, 170, 190 | | |
| U-ledger, lightweight 1.09 – 1.40 m T14 | 188 | 167, 170, 190 | | |
| U-ledger, lightweight 1.40 – 3.07 m T14, reinforced | 189 | 167, 170, 190 | | |
| Diagonal brace "Lightweight design" | 191 | 169, 170 | | |
| U-toeboard, wood 0.73 – 3.07 m | 192 | | | |
| U-toeboard, wood 4.14 m | 193 | | according to Z-8.22-939 | |
| U-toeboard, steel 0.73 – 3.07 m T17 | 194 | | 2-0.22-333 | |
| U-toeboard, steel 0.73 – 3.07 m | 195 | | | |
| U-bracket, lightweight 0.39 m | 196 | 168, 170, 190 | | |
| U-bracket, lightweight 0.73 m | 197 | 168, 170, 190 | | |
| U-bracket, lightweight 0.28 m | 198 | 168, 170, 190 | | |
| U-bracket, lightweight 0.45 m with 2 wedge heads | 199 | 167, 170, 190 | | |
| U-bracket, lightweight 0.73 m with 2 wedge heads | 200 | 167, 170, 190 | | |
| Bracket brace 2.05 m "Lightweight design" | 201 | 169, 170 | | |
| U-deck lift-off preventer T8 0.39 – 1.57 m | 202 | | | |
| U-deck lift-off preventer T9 1.40 – 3.07 m | 203 | | | |
| Universal U-deck lift-off preventer | 204 | | | |
| O-lattice girder, lightweight 5.14; 6.14 x 0.5 m | 205 | 166, 170 | | |

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Table 1: (continued)

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity | |
|---|------------------|---|---|--|
| U-lattice girder, lightweight 2.07 – 3.07 x 0.5 m | 206 | 166, 167, 170, 190 | | |
| U-lattice girder, lightweight 4.14 – 6.14 x 0.5 m | 207 | 166, 167, 170, 190 | | |
| Plug-in tube connector for U-profile | 208 | | | |
| Tube connector for lattice girder | 209 | | | |
| U-lattice girder ledger, lightweight 0.73 m | 210 | 190 | | |
| O-lattice girder 4.14 – 7.71 x 0.4 m "Lightweight design" | 211 | 166, 170 | | |
| Double wedge head coupler "Lightweight design" | 212 | 170 | according to | |
| Side safety meshguard, lightweight 1.57– 3.07 m | 213 | 166, 170 | Z-8.22-939 | |
| U-passageway girder, lightweight 1.57 m | 214 | 166, 167, 170, 190 | | |
| Reinforcing post 2.6 m "Lightweight design" | 215 | 168, 170 | | |
| U-comfort stairway 2.57; 3.07 x 2.00 x 0.64 m | 216 | 217 | | |
| Stair guardrail 2.57; 3.07 m | 218 | 170, 176 | | |
| KK stair guardrail 2.57 m; 3.07 m "Lightweight design" | 219 | 168, 170 | | |
| Stair guardrail holder | 220 | 170, 176 | | |
| Stairway guardrail 1.0 x 0.5 m | 221 | | P 4. | |
| Scaffold retainer / wall tie 0.38 – 1.75 m | 222 | | according to Z-8.1-16.2 | |
| Locking pin, red Ø 11 mm | 223 | | | |
| U-protective roof bracket T7 "Lightweight design" | 224 | 167, 170, 190 | according to | |
| U-ledger with gap cover, lightweight 0.73 – 3.07 m | 225 | 166, 170 | Z-8.22-939 | |
| Starter piece "K2000+ design" | 226 | 171 | | |
| Standard with tube connector "K2000+ design" | 227 | 171, 182 | | |
| Standard without tube connector "K2000+ design" | 228 | 171 | | |
| O-ledger 0.73 – 4.35 m "K2000+ design" | 229 | 172, 176 | | |
| O-ledger HD "K2000+ design" | 230 | 172, 176 | | |
| U-ledger 0.73 m "K2000+ design" | 231 | 173, 176, 190, 234 | according to Z-8.22-64 | |
| U-ledger 1.09 – 1.40 m reinforced "K2000+ design" | 232 | 173, 176, 190, 234 | - | |
| U-double ledger 1.57 – 3.07 m "K2000+ design" | 233 | 173, 176, 190, 234 | | |
| Diagonal brace "K2000+ design" | 235 | 175, 176 | | |
| U-bracket 0.39 m "K2000+ design" | 236 | 174, 176, 190, 234 | | |

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Table 1: (continued)

| (continued) | | | |
|---|------------------|---|--|
| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity |
| U-bracket 0.73 m "K2000+ design" | 237 | 174, 176, 190, 234 | |
| U-bracket 0.28 m "K2000+ design" | 238 | 174, 176, 190, 234 | |
| U-bracket 0.45 m with two wedge-heads "K2000+ design" | 239 | 173, 176, 190, 234 | |
| U-bracket 0.73 m with two wedge-heads "K2000+ design" | 240 | 173, 176, 190, 234 | |
| Bracket brace 2.05 m "K2000+ design" | 241 | 175, 176 | according to Z-8.22-64 |
| O-lattice girder 5.14; 6.14 x 0.5 m "K2000+ design" | 242 | 172, 176 | Σ-0.22-04 |
| U-lattice girder 2.07 – 3.07 x 0.5 m "K2000+ design" | 243 | 172, 173, 176, 190, 234 | |
| U-lattice girder 4.14 – 6.14 x 0.5 m "K2000+ design" | 244 | 172, 173, 176, 190, 234 | |
| O-lattice girder 4.14 – 7.14 x 0.4 m "K2000+ design" | 245 | 172, 176 | |
| Lattice girder coupler | 246 | | according to Z-8.1-16.2 |
| Double wedge-head coupler "K2000+ design" | 247 | 176 | |
| Side safety meshguard 1.57 – 3.07 m "K2000+ design" | 248 | 172, 176 | according to Z-8.22-64 |
| U-passageway girder 1.57 m "K2000+ design" | 249 | 172, 173, 176, 190, 234 | |
| Tube connector with halfcoupler | 250 | | according to Z-8.22-939 |
| U-aluminium-platform stairway 2.57; 3.07 x 2.00 x 0.64 m | 251 | 252 | according to Z-8.1-16.2 |
| KK stair guardrail 2.57; 3.07 m "K2000+ design" | 253 | 174, 176 | according to Z-8.22-64 |
| Locking pin Ø 9 mm | 254 | | according to Z-8.1-16.2 |
| U-protective roof bracket T7 "K2000+ design" | 255 | 173, 176, 190, 234 | |
| U-ledger with gap cover 0.73 – 3.07 m "K2000+ design" | 256 | 172, 176 | according to |
| TG-60 frame 0.50 x 1.09 m "K2000+ design" | 257 | 171 | Z-8.22-64 |
| TG-60 frame 0.71 x 1.09 m "K2000+ design" | 258 | 171 | |
| TG-60 frame 1.00 x 1.09 m "K2000+ design" | 259 | 171 | |
| Assembly guardrail, aluminium 1.57 / 2.07; 2.57 / 3.07 m | 260 | | according to |
| Assembly post T5 | 261 | | Z-8.1-16.2 |
| | • | | i |

Table 1: (continued)

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity | |
|--|------------------|---|---|--|
| U-deck T4 0.73 – 3.07 x 0.32 m, steel, design: point-welded | 262 | | | |
| U-deck T4 0.73 – 3.07 x 0.32 m, steel, design: hand-welded | 263 | | | |
| U-deck 0.73 – 3.07 x 0.32 m, steel, design: point-welded | 264 | | according to | |
| U-deck 0.73 – 3.07 x 0.32 m, steel, design: hand-welded | 265 | | Z-8.1-16.2 | |
| U-robust deck 0.73 – 2.57 m x 0.61 m | 266 | | | |
| U-robust deck 3.07 x 0.61 m | 267 | | | |
| U-robust deck 0.73 – 3.07 m x 0.32 m | 268 | 266 | | |
| U-steel deck with trapdoor 2.57 x 0.64 m | 269 | | | |
| Storey ladder 7 rungs T15 | 270 | | according to Z-8.22-939 | |
| Storey ladder 7 rungs | 271 | | | |
| U-robust deck with trapdoor with ladder 2.57 – 3.07 x 0.61 m | 272 | | | |
| U-aluminium deck with trapdoor 2.07 – 3.07 x 0.61 m | 273 | | Post to | |
| U-aluminium deck with trapdoor 2.57 – 3.07 x 0.61 m, with ladder | 274 | | according to Z-8.1-16.2 | |
| U-robust deck with trapdoor 1.57 – 3.07 x 0.61 m, trapdoor arranged offset | 275 | | | |
| U-robust deck with trapdoor with ladder 2.57 – 3.07 x 0.61 m; trapdoor arranged offset | 276 | | | |
| U-gap deck, telescopic 0.73 – 3.07 m | 277 | | according to | |
| Gap cover, steel 0.73 – 3.07 x 0.32 m | 278 | | Z-8.22-939 | |
| U-deck, steel 0.73 – 3.07 x 0.19 m | 279 | | | |
| U-deck, steel 0.73 – 3.07 x 0.19 m (discontinued design) | 280 | | according to Z-8.1-16.2 | |
| U-aluminium deck with trapdoor 1.00 x 0.61 m | 281 | | according to Z-8.22-939 | |
| O-deck T9 0.73 – 3.07 x 0.32 m, steel, design: spot-welded / hand-welded | 282 | | | |
| O-deck T4 0.73 – 3.07 x 0.32 m, steel, design: spot-welded (discontinued design) | 283 | | | |
| O-deck T9 0.73 – 3.07 x 0.19 m, steel | 284 | | | |
| O-deck 0.73 – 3.07 x 0.19 m, steel (discontinued design) | 285 | | according to Z-8.1-919 | |
| O-platform stairway, aluminium 2.57; 3.07 x 2.0 x 0.64 m | 286 | | | |
| O-Komfort stairway 2.57; 3.07 x 2.0 x 0.64 m | 287 | | | |

Table 1: (continued)

| Designation | Annex B, page | Details / components in accordance with Annex B, page | Regulations for manufacturing, marking and certificate of conformity | |
|--|------------------|---|---|--|
| O-ledger with gap cover, lightweight 0.73 – 3.07 m | 288 | 166, 170 | | |
| O-ledger with gap cover 0.73 - 3.07 m "K2000+ design" | 289 | 172, 176 | according to | |
| O-ledger with halfcoupler 0.73 m "Lightweight design" | 290 | 166, 170 | Z-8.1-919 | |
| O-ledger with halfcoupler 0.73 m "K2000+ design" | 291 | 172, 176 | | |
| Base jack 80, reinforced | 292 | | according to Z-8.1-16.2 | |
| AGS standard, lightweight 2.00 m | 293 | 165, 182 | according to Z-8.22-939 | |
| STAR guardrail 0.73 – 3.07 m T18 | 294 | | according to Z-8.1-919 | |
| STAR double end guardrail 0.73 m | 295 | | according to Z-8.1-919 | |
| U-deck, lightweight 0.73 – 3.07 x 0.32 m, steel, design: spot-welded / hand-welded | 296 | | according to Z-8.1-16.2 | |
| O-deck, lightweight 0.73 – 3.07 x 0.32 m, steel, design: spot-welded / hand welded | 297 | | according to | |
| O-toeboard 0.73 – 3.07 m, steel | 298 | | Z-8.1-919 | |
| O-toeboard 0.73 – 3.07 m T18, steel | 299 | | | |
| Deck lift-off preventer 0.37 – 3.07 m, steel | 300 | | | |
| MODUL bracket brace 2.05 m | 301 | 3, 6 | according to | |
| Advanced end guardrail | 302 | 3, 4 | Z-8.22-906 | |
| MODUL transverse toeboard | 303 | | | |
| Stair stringer fall protection 1.00 x 0.50 m | 304 | | according to | |
| Inner guardrail for aluminium stairway 2.00 m | 305 | | Z-8.1-862 | |
| MODUL U-lattice girder 1.57 m – 3.14 m | 306 | 3, 4, 5, 32, 147, 151, 152 | according to Z-8.22-906 | |

2.1.2 Standard assembly configuration

The use of the scaffold components in facade scaffolding is described in the provisions of a standard system configuration. Proof of structural stability for said fully erected scaffolds in their standard system configuration has been furnished. Assembly configurations of facade scaffolds are considered to be standard system configuration when they comply with the provisions of Annex C and D. Any assembly configurations that deviate from the standard system configuration shall be assessable and verified in each individual case.

The standard system configuration applies to facade scaffolds with a structural height that does not exceed 24 m, not including the spindle extension length above the ground. In accordance with Annexes C and D, the scaffolding system may be used in its standard system configuration with system width $b=0.732\,m$ and bay widths $\ell \leq 3.07\,m$ for working scaffolds with load classes ≤ 3 according to DIN EN 12811-1:2004-03, and as a brickguard and roof edge protection scaffold with a maximum falling height of class 1 (FL1) and as brickguard and roof edge protection scaffold with protective walls of class SWD 1 in accordance with DIN 4420-1:2004-03...

2.1.3 Deviations from the Standard Assembly Configurations

If assembly configurations deviate from the standard system configuration in accordance with Annex C or Annex D, proof of structural stability of the scaffoldings shall be provided for each individual configuration or by means of a structural design calculation in accordance with the Technical Building Rules [Technische Baubestimmungen] and the provisions of this decision. The characteristic values to be used for the proof of structural stability are specified in this decision.

Other anchorage patterns are possible and other nettings or tarpaulins as scaffold cladding may be used. Any increased stresses / loads (e.g. from higher dead weights and wind loads or from increased live loads) must be taken into consideration in a scaffold up to the anchors and the erection plane (ground). The impact of building hoists or other lifting equipment must also be taken into account if they are not operated independently of the scaffold.

2.2 Dimensioning

2.2.1 General provisions and system assumptions

2.2.1.1 General provisions

For the design and calculation of scaffolds to be erected by using the modular scaffolding system, unless otherwise specified in this decision, particular attention shall be paid to the Technical Building Rules [Technische Baubestimmungen], especially those for working and service scaffolds according to DIN EN 12811-1:2004-03 in conjunction with the "Application Guideline for Working Scaffolds according to DIN EN 12811-1", DIN 4420-1:2004-03 the "Approval Principles for Working and Service Scaffolds – Requirements, Calculation Assumptions, Tests, Certificate of Conformity" and for supporting structures DIN EN 12812:2008-12 in consideration of the "Application Guideline for Supporting Structures in accordance with DIN EN 12812"^{2,4}.

When using the modular scaffolding system as a temporary structure that does not fall within the scope of temporary auxiliary construction aids, application-specific requirements may need to be taken into account in the design.

Depending on the components used, the load-bearing groups (LBGs) according to Table 2 apply. If it is not certain which construction techniques will be used in a combination mix, the load-bearing capacities and stiffness of LBG "B" must be assumed when verifying the scaffolding's structural stability for the ledger connections and for the vertical diagonal braces.

If an additional proof of structural stability for the connection between the connector heads and the standard, ledger and diagonal bracing tubes (specified in the Annexes) is required for the scaffold nodes' verification, this will be referred to in the following sections.

Table 2: Load-bearing groups (LBG) for ledger and diagonal brace connection

| Construction technique (design) | Constructio | n technique (design and vertica | n) of the connector of all standard | disc (rosette) |
|--|----------------------------|------------------------------------|-------------------------------------|----------------------------|
| of connector heads for ledgers and diagonal braces *) | "Modul Multi" | "Modul Multi 4.0" | "K2000+" | "Allround lightweight" |
| "Modul Multi" | according to Z-8.22-906 | | Load-bearing | group "B" |
| "Modul Multi 4.0" | | | | Load-bearing group "A" |
| "K2000+" | Load-beari | ng group "B" | according to Z-8.22-64 | according to Z-8.22-949 |
| "Allround lightweight" | | Load-bearing group "A" | according to Z-8.22-949 | according to Z-8.22-939 |
| *) For components with the connector head for the U-bracket ("lightweight design") in accordance with Annex B, page 168, the regulations according to Z-8.22-939 must be applied in full | | | | |

Please also take into consideration the advisory results of the "Scaffold Expert Committee", the so-called "SVA Gerüste", available on the DIBt homepage.

2.2.1.2 System assumptions

The structural systems for the calculation are to be modelled in accordance with Annex A, page 3. The short members specified there from the tubular standard axis to the connectors may be assumed to be rigid. The indices specified in the following sections refer to a local coordinate system, in which the x-axis represents the ledger axis, and the z-axis – the axis of the tubular standard (cf. Annex A, page 3).

When verifying the structural stability of the scaffolding system, it must be observed that the bending moment in the ledger-tubular standard connection is related to the standard's outer edge and that the vertical component in the vertical diagonal brace connection must be taken into account with a connection eccentricity corresponding to the information provided in Annex A, page 3. The torsional moment resulting from the horizontal component at the vertical diagonal brace connection around the axis of the tubular standard is transmitted by the connector and must be verified in the ledgers.

As specified, in the connection of a ledger, transmission of axial forces, bending moments, and shear forces in the plane between the tubular standard and the ledger and in the plane at a right angle thereto is allowed. In LBG "A", O-ledgers may also transfer torsional moments. Please refer to the respective load-bearing capacities in Table 3.

When using short ledgers with $L < 0.60 \, m$, the connections shall be assumed to be articulated. Shear forces may only be transmitted if they result from the (outer) loads, which are transmitted at the short rod.

If it is not ensured that only components of a single design are used in a scaffold or that their impact on structural stability is recorded by detailed calculation and design documents, the specifications of LBG "B" must be used for the verification procedure of the respective scaffold.

At the connection of a vertical and horizontal diagonal, only normal forces may be transferred. Structural proof of stability of the horizontal diagonal ledger according to Annex B, page 156 can be provided as ledgers in accordance with section 2.2.2 in connection with the Technical Building Rules [Technische Baubestimmungen].

Stiffness values and load-bearing capacities of direct connections apply to connection in the "large" or "small" opening of the connector disc (rosette), provided no additional information for connections in the small opening have been provided. The information for the deck level only applies for deck ledgers that are connected in the small opening.

In all equations in the following sections, the internal forces N and V must be given in [kN], while bending and torsional moments M must be quoted in [kNcm].

2.2.2 Ledger connection

2.2.2.1 Load-deformation behaviour

2.2.2.1.1 Bending in the vertical plane

When no articulated connection is assumed, also see section 3.2.1., depending on the design, proof of stability of a scaffolding shall be provided taking into account the ledger connections in the plane formed by the tubular standard and the ledger (vertical plane) with torsional spring fixation according to the following moment/rotation angle relationship (M_y/ϕ) :

- in accordance with Annex A, Figure 1 for load-bearing group "A" or
- in accordance with Annex A, Figure 2 for load-bearing group "B"

2.2.2.1.2 Bending in the horizontal plane

When no articulated connection is assumed, regardless of the designs, proof of stability of a scaffolding must be provided taking into account the ledger connections in the plane formed by the tubular standard and the ledger (horizontal plane) with torsional spring fixation according to the following moment/rotation angle relationship (M_z/ϕ) :

- in accordance with Annex A, Figure 3 for load-bearing group "A" or
- in accordance with Annex A, Figure 4 for load-bearing group "B"

2.2.2.1.3 Torsion at the tube ledger of load-bearing group "A"

When no articulated connection is assumed, proof of stability of the O-ledger of LBG "A" under torsion stresses in the ledger connection shall be calculated assuming a spring fixation that corresponds to the moment/rotation angle relationship (M_T/ϕ) in accordance with Annex A, Figure 5. As a rule, no torsion may be transmitted in the connection of U-ledgers and in LBG "B".

2.2.2.1.4 Vertical load at right angles to the axis of the ledger

For ledger lengths $> 0.7 \, m$ in conjunction with vertical shear forces $V_d \le 10 \, kN$, an additional degree of freedom applied in the direction of the shearing load may be disregarded. Otherwise, an additional degree of freedom in the shear force direction of $f_{0,d} = 0.175 \, cm$ must be applied.

2.2.2.1.5 Horizontal load at right angles to the axis of the ledger

For structures where the deformation influence of the ledger or bracket connection in the horizontal direction must be taken into account, proof of structural stability of the ledgers or bracket when subjected to horizontal loads V_y at right angles to the ledger or bracket axis in the ledger or bracket connection with a travel-limiting spring stiffness according to the following force-displacement relationship:

- in accordance with Annex A, Figure 6 for load-bearing group "A" or
- in accordance with Annex A, Figure 7 for load-bearing group "B"

2.2.2.2 Proof of structural stability

2.2.2.2.1 General verifications

For the connection of a ledger, proof shall be provided that the loads shall not exceed the load-bearing capacities given in Table 3.

Table 3: Design values of the load-bearing capacities in the ledger connection

| Internal forces and moments of connection | | Load-bearing capacity | | | |
|---|----------------------|-------------------------------|---|--|--|
| | | Load- bearing group "A" | Load-bearing group "B" | U-bracket connection with connector head according to Annex B, page 168 | |
| Bending moment M _{y,Rd} | [kNcm] | ±120.0 | ±10 | 01.0 | |
| Bending moment $M_{z,Rd}$ | [kNcm] | ±40.1 | ±37.2 | ±40.1 | |
| Vertical shear force $V_{z,Rd}$ | [kN] | ±31.7 | ±264 | | |
| Horizontal shear force $V_{y,Rd}$ | [kN] | ±16.0 | O-ledger: ± 10.0 U-ledger: ± 5.9 | ±16.0 | |
| Normal force W[kN] | at the large opening | ±35.1 | ±31.0 | ±35.1 | |
| Normal force N_{Rd} [kN] | at the small opening | ±42.1 | | ± 42.1 | |
| Torsional moment $M_{x,Rd}$ | [kNcm] only O-ledger | ±52.5 | <u>-</u> - | | |

2.2.2.2 Interaction standard / ledger connection

In accordance with the design, the following interaction relationships at the connector discs or rosettes shall be met:

<u>Table 4:</u> Interaction relationships

| | Load-bearing group "A" | Load-bearing group "B" |
|---------------------------|--------------------------------|--------------------------------|
| Interaction relationships | $0.324 \cdot I_A + I_5 \leq 1$ | $0.316 \cdot I_A + I_5 \leq 1$ |

Where:

I_A Load factor in ledger connection

$$I_A = \frac{M_{y,Ed}}{M_{y,Rd}}$$
 (equation 1)

where: $M_{v,Ed}$ Design bending moment in the ledger connection

 $M_{y,Rd}$ Design value of the load bearing capacity subjected to bending moments in the ledger connection according to Table 3

- Is Vectorial coefficient of utilization in the tubular standard at loaded connector discs (rosette)
 - − For $v_{act} \le 1/3$ the following applies:

$$I_5 = \frac{a}{b}$$
 (equation 2)

a, b see figure 1, whereby b has to be calculated using the interaction relationship according to figure 1

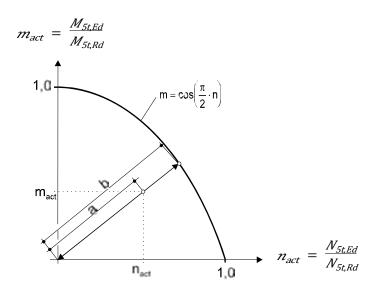


Figure 1: Vectorial coefficient of utilization in the standard

where: m_{act} is the coefficient of utilization with respect to bending moments in the standard

 $M_{St,Ed}$ Bending moment in the tubular standard

 $M_{St,Rd}$ Load-bearing capacity with respect to the bending moments in the tubular

standard

Design "A": $M_{St,Rd} = M_{pl,d} = f_{y,d} \cdot \alpha_{pl} \cdot W_{el} = 232 \text{ kNcm}$

Design "B": $M_{St,Rd} = M_{pl,d} = f_{y,d} \cdot \alpha_{pl} \cdot W_{el} = 175 \text{ kNcm}$

 n_{act} is the coefficient of utilization with respect to the normal force in the standard

 $N_{St,Ed}$ Normal force in the tubular standard

 $N_{St,Rd}$ Load-bearing capacity with respect to the bending moments in the tubular standard

Design "A": $N_{St,Rd} = N_{pl,d} = f_{v,d} \cdot A = 173 \text{ kN}$

Design "B": $N_{St,Rd} = N_{pl,d} = f_{y,d} \cdot A = 132 \text{ kN}$

- For $1/3 < v_{act} \le 0.9$ the vectorial coefficient of utilization must be determined under consideration of the interaction relationship as shown in the left side of the equation, column 4 of Table 7, DIN 4420-1:1990-12.

where:

v_{act} is the coefficient of utilization with respect to the shear force in the standard

$$v_{act} = \frac{v_{St,Ed}}{v_{St,Rd}}$$
 (equation 3)

where $V_{St,Ed}$ Design shear force in the tubular standard

 $V_{St,Rd}$ Design resistance in relation to the shear force in the tubular standard

Design "A": $V_{St,Rd} = V_{pl,d} = 63.6 \text{ kN}$ Design "B": $V_{St,Rd} = V_{pl,d} = 48.5 \text{ kN}$

2.2.2.3 Combinations of internal forces and moments

For combinations of internal forces and moments in the connection of a ledger, the following requirements shall be met depending on the design, whereby the torsion term with MT shall only be taken into account in load-bearing group "A" with tube ledger connections:

Load-bearing group "A"

Proof of structural stability of the welding seam between the connector head and the ledger tube (O-ledger) shall also be provided:

$$\left(\frac{|N_{W,Ed}|}{117 \, kN} + \frac{\sqrt{(M_{y,W,Ed})^2 + (M_{z,W,Ed})^2}}{170 \, kNcm}\right)^2 + \left(\frac{|M_{T,W,Ed}|}{152 \, kNcm} + \frac{\sqrt{(V_{y,W,Ed})^2 + (V_{z,W,Ed})^2}}{43.1 \, kN}\right)^2 \le 1$$
(equation 5)

For load-bearing group "A", proof of structural stability of the welding seam between the connector head and the U-ledger is not required.

Load-bearing group "B"

For proof of structural stability of load-bearing group "B", only the load-bearing capacities of load-bearing group "B" according to Table 3 must be applied, irrespective of the components used.

$$\frac{N_{Ed}^{(+)}}{N_{Rd}} + \frac{\mid M_{y,Ed} \mid}{M_{y,Rd}} + \frac{max(\mid V_{z,Ed} \mid -2.1 \text{ kN}; 0 \text{ kN})}{V_{z,Rd}} + \frac{\mid V_{y,Ed} \mid}{16.0 \text{ kN}} + \frac{\mid M_{z,Ed} \mid}{M_{z,Rd}} \le 1$$
 (equation 6)

Additionally, verification for the welding seam between ledger tube (O-ledger) and the connector head (in accordance with eq. 7). Verification of the welding seam is not required if the following conditions are met:

- a) If the components used all have a tube ledger connection 4.0 in accordance with Annex B, page 151 and if shear loads in these connections fulfil $V_{z,W,Ed} \le 30,5 \ kN$ verification (eq. 7) is not required.
- b) When exclusively using components with a tube ledger connection in accordance with Annex B, page 4, verification (eq. 7) is not required.

$$\frac{max(|N_{W,Ed}| - 6.4 \, kN; 0)}{76.8 \, kN} + \frac{\sqrt{(M_{y,W,Ed})^2 + (M_{z,W,Ed})^2}}{110.3 \, kNcm} + \frac{\sqrt{(V_{z,W,Ed})^2 + (V_{y,W,Ed})^2}}{48.9 \, kN} \le 1$$
(equation 7)

For the welding seam between the U-ledger profile and the connector head, the following additional verification must be provided for the "K 2000+ design" in accordance with Annex B, page 173:

$$\frac{N_{W,Ed}}{71.0 \ kN} + \frac{\sqrt{\left(M_{y,W,Ed}\right)^2 + \left(M_{z,W,Ed}\right)^2}}{116.4 \ kN} + max\left(\frac{V_{z,W,Ed}}{58.5 \ kN}; \frac{V_{y,W,Ed}}{18.0 \ kN}\right) \le 1$$
 (equation 8)

Additional proof of stability of the welding connections between other U-ledgers and ledger heads is not required.

For the welding seam between the U-profile and the U-bracket connector head – "lightweight design" in accordance with Annex B, page 168, the following additional verification must be provided:

$$\frac{N_{Ed}^{(+)}}{N_{Rd}} + \frac{|M_{y,Ed}|}{M_{y,Rd}} + \frac{max(|V_{z,Ed}| - 2.1 \ kN; 0)}{V_{z,Rd}} + \frac{|M_{z,Ed}|}{M_{z,Rd}} + \frac{|V_{y,Ed}|}{27.1 \ kN} + \frac{|M_{T,Ed}|}{M_{T,Rd}} \le 1$$
 (equation 9)

For the welding seam between the bracket ledger and the U-bracket connector head – "lightweight design" in accordance with Annex B, page 168, verification in accordance with Z-8.22-939 must also be provided.

Where:

$$M_{y,Ed}$$
, $V_{y,Ed}$, $V_{z,Ed}$, $M_{z,Ed}$, $M_{T,Ed}$

Design internal forces and moments in the ledger connection in [kN] or [kNcm]

 M_{Ed}^{+}

Design action-effect due to tensile normal force in the ledger connection in [kN]

 N_{Rd} , $M_{y,Rd}$, $V_{y,Rd}$, $V_{z,Rd}$, $M_{z,Rd}$, $M_{T,Rd}$

Design load values in accordance with the load-bearing group according to Table 3 in [kN] or [kNcm]

 $N_{W,Ed}$, $M_{y,W,Ed}$, $V_{z,W,Ed}$, $V_{y,W,Ed}$, $M_{z,W,Ed}$, M

2.2.3 Connection to vertical diagonal brace

2.2.3.1 Load-deformation behaviour

When modelling the overall system in the planar model, the vertical diagonal braces – including their connections for LBG "A" and LBG "B" – must be taken into account depending on the load direction (tensile or compressive force) and the diagonal brace length with the equivalent stiffness ($E_d \cdot A_{eff}$) according to Table 5, as well as a degree of freedom in the diagonal direction of $f_{0,d} = 0.7 \, cm$ (cf. Annex A, p. 4).

The deformation contributions of standard and ledger due to eccentricity ey (cf. Annex A, page 4) are included in the data provided, which means that only ex must be taken into account in the planar static model. Proof is to be provided that the node moments M^k in accordance with Annex A, page 4, are absorbed by the longitudinal ledgers and transoms fitted to the nodes.

2.2.3.2 Proof of structural stability

Depending on the direction of loading, the following proof of structural stability must be provided for the vertical diagonal braces:

$$\frac{\left|N_{V,Ed}\right|}{N_{V,Rd}} \le 1 \tag{equation 10}$$

Where:

 $N_{V,Ed}$ Tensile or compressive forces acting in the vertical diagonal braces

 $N_{V,Rd}$ Load-bearing capacity of the vertical diagonal braces with wedge-head in relation to tensile and/or compressive force in accordance with Table 5

<u>Table 5:</u> Characteristic values of the vertical diagonal braces

| Bay length | Bay height | Compres | sion load | ad Tensile load | |
|------------|------------|-------------------------|--------------------------------------|--------------------------|-----------------------|
| L [m] | H [m] | $E_d \cdot A_{eff}[kN]$ | <i>N</i> (-) [kN] _{V,Rd} | $E_d \cdot A_{eff}$ [kN] | $N_{V,Rd}^{(+)}$ [kN] |
| 3.07 | | 2800 | 8.5 | 3153 | 22.9 |
| 2.57 | | 2610 | 10.5 | 3049 | 23.5 |
| 2.07 | | 2380 | 12.8 | 2969 | 24.3 |
| 1.57 | 2.0 | 2820 | 15.4 | 2823 | 22.6 |
| 1.40 | 2.0 | 3390 | 16.3 | 2808 | 22.0 |
| 1.29 | | 3389 | 16.9 | 2790 | 21.7 |
| 1.09 | | 3337 | 17.7 | 2763 | 21.2 |
| 0.73 | | 3334 | 16.6 | 2772 | 18.0 |
| 3.07 | | 1940 | 9.7 | 2695 | 22.0 |
| 2.57 | | 1680 | 12.3 | 2480 | 22.6 |
| 2.07 | | 1540 | 15.5 | 2342 | 23.3 |
| 1.57 | 1.5 | 1660 | 19.3 | 2254 | 24.3 |
| 1.40 | 1.5 | 2020 | 19.2 | 2204 | 23.5 |
| 1.29 | | 2330 | 19.0 | 2173 | 22.9 |
| 1.09 | | 3062 | 18.6 | 2124 | 22.0 |
| 0.73 | | 2818 | 17.6 | 2093 | 19.8 |

| Bay length | Bay height | Compres | sion load | Tensile | load |
|------------------------------------|------------|-------------------------|-----------------------|-------------------------|-----------------------|
| L [m] | H [m] | $E_d \cdot A_{eff}[kN]$ | $N_{V,Rd}^{(-)}$ [kN] | $E_d \cdot A_{eff}[kN]$ | $N_{V,Rd}^{(+)}$ [kN] |
| 3.07 | | 1540 | 10.8 | 2318 | 20.7 |
| 2.57 | | 1250 | 14.0 | 2045 | 21.2 |
| 2.07 | | 1160 | 16.9 | 1821 | 22.1 |
| 1.57 |] , [| 1160 | 17.9 | 1636 | 23.0 |
| 1.40 | 1.0 | 1160 | 18.4 | 1593 | 23.4 |
| 1.29 | | 1160 | 18.8 | 1567 | 23.7 |
| 1.09 | | 1210 | 19.4 | 1523 | 24.2 |
| 0.73 | | 2130 | 18.5 | 1436 | 21.7 |
| 3.07 | | 1330 | 11.5 | 2076 | 19.9 |
| 2.57 | | 1170 | 15.2 | 1782 | 20.0 |
| 2.07 | | 1010 | 15.4 | 1471 | 20.2 |
| 1.57 |] ,, [| 800 | 15.8 | 1162 | 20.8 |
| 1.40 | 0.5 | 730 | 16.1 | 1073 | 21.1 |
| 1.29 |] [| 680 | 16.3 | 1015 | 21.4 |
| 1.09 |] [| 610 | 16.9 | 916 | 22.1 |
| 0.73 |] [| 600 | 18.7 | 800 | 23.6 |
| when $E_d = (21.000/1.1) kN/cm^2$ | | | | | |

2.2.4 Horizontal diagonal brace connection

2.2.4.1 Horizontal diagonal brace in accordance with Annex B, page 9

2.2.4.1.1 Load-deformation behaviour

In the entire system, the horizontal diagonal braces according to Annex B, page 9, including their connections, must be taken into account dependent on the diagonal brace length and independent of the loading direction (tensile or compressive force) with the equivalent stiffness ($E_d \cdot A_{eff}$) according to Table 6 and with a travel-limiting spring in the diagonal direction according to the specifications in Annex A, figure 8.

2.2.4.1.2 Proof of structural stability

For the horizontal diagonal braces the following proof of stability is to be provided:

$$\frac{\left|N_{H,Ed}\right|}{N_{H,Rd}} \leq 1$$
 (equation 11)

Where:

 $N_{H,Ed}$ Tensile or compressive forces acting on the horizontal diagonal braces $N_{H,Rd}$ Load-bearing capacity of the horizontal diagonal ledgers according to Table 6

<u>Table 6:</u> Characteristic values of the horizontal diagonal braces according to Annex B, page 9

| Bay length L [m] | Bay width B [m] | $N_{H,Rd}$ [kN] | $E_d \cdot A_{eff}[kN]$ |
|------------------|-----------------|-----------------|-------------------------|
| 0.73 | 0.73 | 3.10 | 2760 |
| 1.09 | 1.09 | 3.07 | 2970 |
| 1.57 | 1.57 | 3.03 | 2780 |
| 2.07 | 2.07 | 2.98 | 2240 |
| 2.57 | 2.57 | 2.91 | 1530 |
| 3.07 | 3.07 | 2.81 | 830 |

| <u> Table 6:</u> | (continued) |
|------------------|-------------|
|------------------|-------------|

| Bay length L [m] | Bay width B [m] | $N_{H,Rd}$ [kN] | $E_d \cdot A_{eff}[kN]$ |
|------------------|-----------------|-----------------|-------------------------|
| 1.09 | | 3.08 | 3160 |
| 1.40 | | 3.07 | 3210 |
| 1.57 | 0.73 | 3.06 | 3200 |
| 2.07 | 0.73 | 3.03 | 3070 |
| 2.57 | | 3.00 | 2850 |
| 3.07 | | 2.96 | 2530 |
| 1.40 | | 3.06 | 3210 |
| 1.57 | | 3.05 | 3190 |
| 2.07 | 1.09 | 3.03 | 3040 |
| 2.57 | | 2.99 | 2790 |
| 3.07 | | 2.95 | 2460 |
| 1.40 | | 3.04 | 3140 |
| 2.07 | 1.57 | 3.01 | 2910 |
| 2.57 | 1.57 | 2.98 | 2650 |
| 3.07 | | 2.93 | 2330 |
| 1.40 | | 3.02 | 2970 |
| 2.57 | 2.07 | 2.95 | 2450 |
| 3.07 | | 2.90 | 2130 |
| 1.40 | 2.57 | 2.99 | 2900 |
| 3.07 | 2.01 | 2.86 | 1880 |
| 1.40 | 3.07 | 2.94 | 2380 |

- 2.2.4.2 Connection of the horizontal diagonal braces according to Table B, page 156
 Structural proof of stability of the horizontal diagonal ledgers according to Annex B, page 156, must be provided in accordance with section 2.2.2 in connection with the Technical Building Rules [Technische Baubestimmungen].
- 2.2.4.3 Connection of the lightweight HD horizontal diagonal brace O-ledger in accordance with Annex B, page 186
- 2.2.4.3.1 Load-deformation behaviour

Proof of structural stability of a scaffolding must take into account the O-ledgers HD according to Annex B, page 186 with an overall stiffness $c_{H,d}$ for the joints and the diagonal brace according to Table 7.

2.2.4.3.2 Proof of structural stability

For the O-ledgers, the following proof of stability is to be provided:

$$\frac{\left|N_{H,Ed}\right|}{N_{H,Rd}} \, \leq \, \, 1$$

(equation 12)

Where:

N_{H,Ed} Tensile or compressive force in the O-ledger HD

 $N_{H,Rd}$ Load-bearing capacity of the O-ledgers HD according to Table 7

| | | J | ,1 0 |
|--------|--------|--|--|
| L [mm] | B [mm] | Load-bearing capacity <i>N_{H,Rd}</i> [kN] | Stiffness <i>c_{H,d}</i> [kN/cm] (tensile and compressive force) |
| 1572 | 1088 | | 85.1 |
| 2072 | 732 | | 59.4 |
| 2072 | 1088 | | 67.8 |
| 2572 | 732 | ± 12.0 | 44.8 |
| 2572 | 1088 | | 49.2 |
| 3072 | 732 | | 28.1 |
| 3072 | 1088 | | 26.1 |

<u>Table 7:</u> Characteristic values of O-ledgers in accordance with Annex B, page 186

2.2.5 Diagonal cross brace

For the proof of structural stability of the entire system, the diagonal cross braces according to Annex B, page 134 must be taken into account in connection with the equivalent stiffness and load-bearing capacities given in Table 8.

<u>Table 8:</u> Characteristic values of the diagonal cross brace

| Scaffolding width [m] | System length [m] | Load | Stiffness $E_d \cdot A_{eff}$ [kN] | Load-bearing capacity N_{Rd} [kN] |
|------------------------------------|-------------------|-------------|------------------------------------|-------------------------------------|
| 0.732 | 1.05 | Compression | 2730 | -10.2 |
| 1.088 | 1.95 | Tension | 2890 | +10.2 |
| 0.732 | 4 77 | Compression | 2570 | -10.2 |
| 1.088 | 1.77 | Tension | 2670 | +10.2 |
| when $E_d = (21.000/1.1) kN/cm^2$ | | | | |

2.2.6 Connector disc (rosette)

2.2.6.1 Connection in directly adjacent openings of the connector disc

When two ledgers or one ledger and one vertical diagonal brace or one ledger and one horizontal diagonal brace are connected in adjacent openings, the following proof of structural stability must be provided:

$$(n^A + n^a)^2 + (v^A + v^a)^2 \le 1$$
 (equation 13)

where:

n, v is the interaction ratio according to Table 9

A Ledger A

Ledger a or vertical or horizontal diagonal brace between A and B according to Figure 2

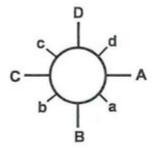


Figure 2: Allocation of the connector disc (rosette)

| <u>Table 9:</u> Interaction ratios |
|------------------------------------|
|------------------------------------|

| Inter- action ratio | Connection ledger A/ ledger a | Connection ledger A/ vertical diagonal brace a | Connection ledger A/ horizontal diagonal brace a |
|---------------------------|---|--|--|
| n^A | | $\frac{N_{Ed}^{A(+)} + M_{y,Ed}^{A} /e}{N_{Rd}^{A}}$ | |
| n^a | $\frac{N_{Ed}^{a(+)} + M_{y,Ed}^a /e}{N_{Rd}^a}$ | $\frac{0.707 \cdot N_{V,Ed}^{(+)} \cdot \sin \alpha + 1.883 \cdot N_{V,Ed} \cdot \cos \alpha}{N_{Rd}^{*}}$ | $\frac{N_{H,Ed}^{(+)}}{N_{Rd}^a}$ |
| vA | | $\frac{\left V_{z,Ed}^{A}\right }{V_{z,Rd}}$ | |
| v^a | $\frac{\left V_{z,Ed}^{a}\right }{V_{z,Rd}}$ | $\frac{ N_{V,Ed} \cdot \cos \alpha}{V_{z,Rd}}$ | 0 |

Where:

| $N_{Ed}^{A(+)}; N_{Ed}^{a(+)}$ | Tensile force in the ledger | connection | on (ledger A or led | ger a) |
|--------------------------------|--|------------|---------------------|-----------------------|
| MAy,Ed: May,Ed | Bending in the ledger connection (ledger A or ledger a) | | | |
| $V_{z,Ed}^A:V_{z,Ed}^a$ | Vertical lateral load in ledger connection (ledger A, Vertikaldiagonale a) | | | |
| Need | Normal force in the vertical | al diagona | l brace | |
| $N_{V,Ed}^{(+)}$ | Tensile force in the vertical | al diagona | l brace | |
| $N_{H,Ed}^{(+)}$ | Tensile force in the horizontal diagonal brace | | | |
| e | Lever arm tube ledger co | | | oad-bearing group "B" |
| $V_{z,Rd}$ | Load-bearing capacities | VzRd: | 31.7 kN | 26.4 kN |
| NRd, | Load-bearing capacities | NA: | 42.3 kN | 36.0 kN |
| | | Na: | 35.1 kN | 31.0 kN |
| | | Ned: | 51.1 kN | 43,7 kN |

Proof of stability shall be provided in pairs around the nodes.

2.2.6.2 Connection of ledgers and/or diagonal braces in any openings of the connector discs (rosettes)

$$\frac{\sum V_{z,Ed}}{\sum V_{z,Rd}} \leq 1$$
 (equation 14)

Where:

 $\sum V_{z,Ed}$ is the sum of all vertical design shear forces acting on the connector disc (including the vertical component of vertical diagonal braces)

 $\sum V_{z,Rd}$ Design value of the load-bearing capacity of the connector discs (rosettes) under vertical shear forces

Load-bearing group "A": $\Sigma V_{z,Rd} = 117 \text{ kN}$ Load-bearing group "B": $\Sigma V_{z,Rd} = 106 \text{ kN}$

2.2.7 U-bracket connection

For the U-bracket "lightweight design" connection in accordance with Annex B, page 168 of the U-brackets in accordance with Annex B, pages 196 to 198, the respective regulations of sections 2.2.2 and 2.2.4 may be applied to the U-ledger connection, taking into account the regulations for the design of welded malleable cast iron components in accordance with Z-8.22-939.

For the U-bracket "K2000+ design" connection in accordance with Annex B, page 174 of the U-brackets in accordance with Annex B, pages 236 to 238, the respective regulations of sections 2.2.2 and 2.2.4 may be applied to the U-ledger connection, taking into account the regulations for the design of welded malleable cast iron components in accordance with Z-8.22-64.

2.2.8 Standard joints

2.2.8.1 General provisions

Unless otherwise specified below, the joints of the standards in the "ALBLITZ MODUL" modular scaffolding system must always be designed and verified in accordance with the applicable Technical Building Rules [Technische Baubestimmungen]; see also "Mathematical treatment of standard joints with one-sided, centrally fixed joint pins for working and service scaffolds, and for supporting structures made of steel"⁵.

The decision contains various designs of tube connectors. If it is not certain which standard joint design will be used when verifying the structural stability, the most unfavourable assumptions must be applied.

If connecting means are used that deviate from the following sections, additional verification of a bolt-type connecting means for tensile force coupling (bearing of holes, bolt shearing and bolt bending) must be provided separately. When verifying the bolts' stability in tube connector joints subjected to tensile force, the bolt bending must be determined in accordance with the regulations "Mathematical treatment of standard joints with one-sided, centrically fixed butt bolts for working and service scaffolds, and for supporting structures made of steel"⁵. For the verifications of structural stability, a hole indentation of $\Delta = 5 \ mm$ must be assumed for all design versions; the wall thickness of integrated tube connectors may be uniformly assumed to be $t = 3.5 \ mm$.

2.2.8.2 Standard joints in Load-bearing group "B"

The structural stability of the standard joints in LBG "B" may be verified in the "lap joint" load-bearing model uniformly using the load-bearing capacities according to Table 10.

<u>Table 10:</u> Load-bearing capacities and load-deformation behaviour of the standard joints in load-bearing group "B"

| Internal forces and moments | Load-bearing capacity | Load-deformation behaviour |
|--|-----------------------|----------------------------|
| Tensile force Z_{Rd} | 10 kN *) **) | rigid |
| Compressive force $N_{KS,Ed}^{(-)}$ in the contact joint | 63.2 <i>kN</i> | rigid |
| Bending moment M _{DF,Rd} | 78.5 kNcm**) | see section 2.2.6.2 |
| Torsional degree of freedom | | 0.0260 rad |

^{*)} When using a Ø12-8.8 pin or larger, in which the thread is not arranged in the shear loaded joint, additional verification of structural stability of the joining means is not mandatory.

^{**)} Separate verification of the structural stability of the tube connector's net cross-section is not necessarily required.

see DIBt Newsletter 4/2017

The following verifications of structural stability must be carried out:

$$\frac{\left|Z_{Ed}\right|}{Z_{Rd}} \le 1 \tag{equation 15}$$

$$\frac{\left|N_{KS,Ed}^{(-)}\right|}{N_{KS,Rd}} \le 1 \tag{equation 16}$$

$$\frac{\left|M_{DF,Ed}\right|}{M_{DF,Rd}} \le 1$$

Where:

 Z_{Ed} , $N_{KS,Ed}^{(-)}$ $M_{DF,Ed}$ Normal force loads in the standard joint resulting bending load in the standard joint

$$M_{DF,Ed} = \sqrt{\left(M_{y,DF,Ed}\right)^2 + \left(M_{z,DF,Ed}\right)^2}$$

ZRd, NK5,Rd, MDF,Ed

Load-bearing capacities according to Table 10

For members subject to combined compressive force and bending moment or combined tensile force and bending moment, further verifications of interaction is not mandatory.

If the lower tubes are tubes with integrated tube connector, the least favourable torsional spring stiffness of the pressed-in connectors according to DIB-Newsletter⁵ or the integrated connectors according to (eq.18) must be assumed for the system calculations.

For system calculations in the "lap joint" load-bearing model, the buckling angle due to the torsional degree of freedom may be considered uniformly in accordance with Table 10.

2.2.8.3 Standard joints in Load-bearing group "A"

For the joints of standards in Load-bearing group "A" with solely integrated tube connectors, it must be verified that the loads they are subjected to do not exceed the load-bearing capacities according to Table 11. When using the "lap joint" load-bearing model, the verification of structural stability must carried out as per (eq. 15), (eq. 16), and (eq. 19).

<u>Table 11:</u> Load-bearing capacities and load-deformation behaviour of the integrated tube connector in Load-bearing group "A"

| Internal forces and moments | | Load-bearing capacity | Load-deformation behaviour | |
|---|--------------------|-----------------------|----------------------------|--|
| Tanaila faraa 7. | one bolt M12, 8.8 | 43.7 kN*) **) | rigid | |
| Tensile force Z_{Rd} | two bolts M12, 8.8 | 72.1 kN*) **) | rigiu | |
| Compressive force $N_{KS,Rd}$ in the contact joint | | 122 <i>kN</i> | rigid | |
| Bending moment M _{DF,Rd} | | 122 <i>kNcm**</i>) | according to (eq. 18) | |
| Torsional degree of freedom | | | 0.0218 rad | |
| *) Applies to the use of helts with 642.00, with threads in which the thread is not arranged in the | | | | |

Applies to the use of bolts with Ø12-8.8, with threads in which the thread is not arranged in the shear loaded joint. Additional proof of structural stability of the connecting means is not mandatory.
 Separate proof of structural stability of the net section at the tube connector is not mandatory.

In the equivalent model, the standards are assumed to be continuous up to the contact joint with a constant cross-section and in the contact joint with the following load-deformation relationship:

$$\varphi_d = \frac{M}{27000 \ kNcm - 182 \cdot |M|}$$
 with M in [kNcm] (equation 18)

This equivalent model also covers the structural behaviour of the internal joint bolt.

For system calculations in the "lap joint" load-bearing model, the buckling angle due to the torsional degree of freedom may be considered uniformly in accordance with Table 14.

If a tensile force and bending moment act simultaneously, the following interaction condition for the deformation range shall additionally be met:

$$\frac{M_{DF,Ed}}{M_{DF,Rd} \cdot cos\left(\frac{\pi}{2} \cdot \frac{Z_{Ed}}{72,1 \ kN}\right)} \le 1$$
 (equation 19)

Where:

 $M_{DF,Ed}$ resulting bending capacity in the standard joint:

$$M_{DF,Ed} = \sqrt{\left(M_{y,DF,Ed}\right)^2 + \left(M_{z,DF,Ed}\right)^2}$$

 $M_{DF,Rd}$ Bending capacity according to Table 14

 Z_{Ed} Tensile strength in the deformation range

 Z_{Rd} Tensile strength of the deformation range according to Table 14

Additional proof of structural stability for a combination of bending in the joint range and compressive force in the contact joint is not mandatory.

2.2.9 Wedge-head couplers

Wedge-head couplers (rigid or rotatable) according to Annex B, pages 122 and 125 are only to be used for connecting "free" scaffolding tubes \emptyset 48,3 x 3.2 mm to the standards of the scaffolding system in conjunction with the roof edge protection (e.g. see Annex D, page 7).

Depending on the standards used, a number of connection means can be used for the connection of the double wedge-head coupler "lightweight design" according to Annex B, page 212 and the double wedge-head coupler "K2000+ design" according to Annex B, page 247. The connection values according to Table 12 shall apply.

If it is not ensured that only connections of a single design will be used in a scaffold or that their impact on structural stability is taken into account by means of detailed calculation and design documents, proof of the scaffold's structural stability must be provided in accordance with Z-8.22-64.

Table 12: Designs for the connection of double wedge-head couplers

| Construction technique | | Double wedge-head coupler, lightweight design, according to Annex B, page 212 Double wedge-head coupler, "K2000+ design" according to Annex B, page 247 | | | |
|---|----------------------------|--|--------------|--|--|
| Component | according to Annex B, page | | | | |
| Standards 11 to 16, 158, 227, 228 Starter piece 10, 226 | | according to Z-8.22-64 | | | |
| | | | | | |
| Standards | 153, 154, 180, 181, 183 | according to | according to | | |
| Starter piece | 179 | Z-8.22-939 | Z-8.22-64 | | |

2.2.10 Reinforcing post 2.6 m "Allround lightweight design"

For proof of structural stability of the reinforcing post 2.6 m "lightweight design" according to Annex B, page 215 in the scaffolding system, the connections with the connector head for U-brackets "lightweight design" according to Annex B, page 168 to the connector discs shall be assumed to be articulated. Verification of structural stability of the connections shall be provided for any other stresses according to the provisions regarding the bracket connection in sections 2.2.2 and 2.2.5. of this decision.

2.2.11 TG-60 frame

For the TG-60 frame according to Annex B, pages 257 to 259, the provisions according to Z-8.22-64 shall apply.

2.2.12 Proof of structural stability of the entire scaffolding system

2.2.12.1 Vertical load-bearing capacity of decks

Proof of structural stability for the decks of the "ALBLITZ MODUL" modular system is provided in accordance with Table 13 for live loads of the scaffold load classes according to EN 12811-1:2004-03, Table 3 and for use in brickguard and roof edge protection scaffolds with fall heights of up to 2 m according to DIN 4420-1:2004-03 (class D according to DIN EN 12810-1:2004-03).

Table 13: Assignment of decking to scaffold load classes (service classes)

| Aluminium frame platform with tube fixture | Designation | Annex B, page | Bay width ℓ [m] | Use in load class (service class) |
|---|--|---------------------------------------|--------------------|--------------------------------------|
| Steel deck AF with tube fixture 61 and 62 4.14 ≤ 3 Steel deck AF 0.32 m 84 2.57 ≤ 5 Steel deck AF 0.30 m, 0.34 m 86 2.57 ≤ 5 Steel deck AF 0.30 m, 0.34 m 63 3.07 ≤ 4 Intermediate deck AF with tube fixture 0.19 m 63 3.07 ≤ 4 Intermediate deck AF 0.19 m 63 3.07 ≤ 4 Intermediate deck AF with tube fixture 0.16 m 63 2.57 ≤ 5 Intermediate deck AF 0,16 m 88 2.57 ≤ 5 Steel deck with tube fixture 64 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Steel deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 3 Aluminium frame platform 66 and 67 | Aluminium frame platform with tube fixture | 51 and 52 | ≤ 3.07 | ≤ 3 |
| Steel deck AF with tube fixture 61 and 62 Steel deck AF 0.32 m 84 Steel deck AF 0.30 m, 0.34 m 86 Intermediate deck AF 0.30 m, 0.34 m 63 Intermediate deck AF with tube fixture 0.19 m 63 Intermediate deck AF 0.19 m 88 Intermediate deck AF with tube fixture 0.16 m 63 Intermediate deck AF 0,16 m 63 Steel deck with tube fixture 64 Steel deck with tube fixture 64 Steel deck with tube fixture 64 Intermediate deck with tube fixture 65 Aluminium frame platform 66 and 67 4 2.57 5 5 2.07 5 5 5 65 3.07 89 3.07 2.57 5 5 3.07 65 3.07 <td>Aluminium access frame platform with tube fixture</td> <td>54, 55, 57, 58</td> <td>≤ 3.07</td> <td>≤ 3</td> | Aluminium access frame platform with tube fixture | 54, 55, 57, 58 | ≤ 3.07 | ≤ 3 |
| Steel deck AF 0.32 m 84 3.07 ≤ 4 Steel deck AF 0.30 m, 0.34 m 86 2.57 ≤ 5 Intermediate deck AF 0.30 m, 0.34 m 63 4.14 ≤ 3 Intermediate deck AF with tube fixture 0.19 m 88 3.07 ≤ 4 Intermediate deck AF 0.19 m 63 3.07 ≤ 4 Intermediate deck AF with tube fixture 0.16 m 63 2.57 ≤ 5 Intermediate deck AF 0,16 m 63 2.57 ≤ 5 Steel deck with tube fixture 64 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Steel deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 3 Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 3 Aluminium deck with plywood 72 and 73, 78 | 0 | 04 100 | 4.14 | ≤ 3 |
| Steel deck AF 0.30 m, 0.34 m 86 2.57 ≤ 5 ≤ 2.07 ≤ 6 Intermediate deck AF with tube fixture 0.19 m 63 3.07 ≤ 4 Intermediate deck AF 0.19 m 88 2.57 ≤ 5 Intermediate deck AF with tube fixture 0.16 m 63 3.07 ≤ 4 Intermediate deck AF 0,16 m 88 2.57 ≤ 5 Steel deck with tube fixture 64 2.57 ≤ 5 ≤ 2.07 ≤ 6 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 3 Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 3 Aluminium frame platform with internal hatch 69 and 70 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 75 and 76, 81 and 82 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 4 Lightweight alum | | • • • • • • • • • • • • • • • • • • • | 3.07 | ≤ 4 |
| Section Sec | | | 2.57 | ≤ 5 |
| Intermediate deck AF with tube fixture 0.19 m 63 3.07 ≤ 4 Intermediate deck AF 0.19 m 88 2.57 ≤ 5 Intermediate deck AF with tube fixture 0.16 m 63 3.07 ≤ 4 Intermediate deck AF 0,16 m 63 2.57 ≤ 5 Steel deck with tube fixture 64 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck 89 2.57 ≤ 5 ≤ 2.07 ≤ 6 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 3 Aluminium deck with plywood 72 and 73, 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 85 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 3 Lightweight aluminium deck 0.60 m 90 3.07 ≤ 3 Solid wood deck 48 91 2.57 ≤ 4 <td< td=""><td>oteer deck Ar 0.30 m, 0.34 m</td><td></td><td>≤ 2.07</td><td>≤ 6</td></td<> | oteer deck Ar 0.30 m, 0.34 m | | ≤ 2.07 | ≤ 6 |
| Intermediate deck AF 0.19 m 88 2.57 ≤ 5 ≤ 2.07 ≤ 6 ≤ 2.07 ≤ 6 Intermediate deck AF with tube fixture 0.16 m 63 3.07 ≤ 4 Intermediate deck AF 0,16 m 88 2.57 ≤ 5 ≤ 2.07 ≤ 6 ≤ 2.07 ≤ 6 Steel deck with tube fixture 64 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck 89 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 3 Aluminium deck with plywood 72 and 73, 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 75 and 76, 81 and 82 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 3 Lightweight aluminium deck 0.60 m 90 3.07 ≤ 3 Solid wood deck 48 91 2.57 ≤ 4 Wooden deck <td></td> <td></td> <td>4.14</td> <td>≤ 3</td> | | | 4.14 | ≤ 3 |
| | Intermediate deck AF with tube fixture 0.19 m | 63 | 3.07 | ≤ 4 |
| Intermediate deck AF with tube fixture 0.16 m 10 | Intermediate deck AF 0.19 m | 88 | 2.57 | ≤ 5 |
| Intermediate deck AF with tube fixture 0.16 m 63 2.57 ≤ 5 Intermediate deck AF 0,16 m 3.07 ≤ 4 Steel deck with tube fixture 64 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck 89 2.57 ≤ 5 ≤ 2.07 ≤ 6 2.57 ≤ 5 ≤ 2.07 ≤ 6 2.57 ≤ 5 ≤ 2.07 ≤ 6 3.07 ≤ 3 Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 3 Aluminium deck with plywood 72 and 73, 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 85 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 4 Lightweight aluminium deck 0.60 m 90 3.07 ≤ 3 Solid wood deck 48 91 2.57 ≤ 4 Wooden deck 93 2.57 ≤ 4 | | | ≤ 2.07 | ≤ 6 |
| | Literatura Control de la Africa de la Control de la Contro | 00 | 3.07 | ≤ 4 |
| Steel deck with tube fixture 64 3.07 ≤ 4 Intermediate deck with tube fixture 65 3.07 ≤ 4 Intermediate deck 89 3.07 ≤ 4 Intermediate deck 89 3.07 ≤ 4 Intermediate deck 89 3.07 ≤ 5 Intermediate deck 3.07 ≤ 3 Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 3 Aluminium frame platform with internal hatch 69 and 70 ≤ 3.07 ≤ 3 Aluminium deck with plywood 72 and 73 , 78 and 79 , 78 a | | | 2.57 | ≤ 5 |
| Steel deck with tube fixture 64 $\frac{2.57}{4.00}$ <td< td=""><td> Internediate deck AF 0, 10 III</td><td>00</td><td>≤ 2.07</td><td>≤ 6</td></td<> | Internediate deck AF 0, 10 III | 00 | ≤ 2.07 | ≤ 6 |
| | | | 3.07 | ≤ 4 |
| $ \begin{array}{c ccccccccccccccccccccccccccccccccccc$ | Steel deck with tube fixture | 64 | 2.57 | ≤ 5 |
| $ \begin{array}{c ccccccccccccccccccccccccccccccccccc$ | | | ≤ 2.07 | ≤ 6 |
| Intermediate deck 89 $ \begin{array}{r} 2.57 & \leq 5 \\ \hline 42.07 & \leq 6 \\ \hline \hline $ | landa mara ali ada ali ali ali ali da da da Cada ma | 0.5 | 3.07 | ≤ 4 |
| Aluminium frame platform 66 and 67 ≤ 3.07 ≤ 6 Aluminium frame platform with internal hatch 69 and 70 ≤ 3.07 ≤ 3 Aluminium deck with plywood 72 and 73, 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 75 and 76, 81 and 82 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 4 Lightweight aluminium deck 0.60 m 90 $\frac{3.07}{\leq 2.57}$ ≤ 4 Solid wood deck 48 91 $\frac{3.07}{\leq 2.57}$ ≤ 4 Wooden deck 93 $\frac{3.07}{\leq 2.57}$ ≤ 4 | | | 2.57 | ≤ 5 |
| Aluminium frame platform with internal hatch69 and 70 ≤ 3.07 ≤ 3 Aluminium deck with plywood72 and 73, 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder75 and 76, 81 and 82 ≤ 3.07 ≤ 3 Steel deck85 ≤ 3.07 ≤ 4 Lightweight aluminium deck 0.60 m90 $\frac{3.07}{\leq 2.57}$ ≤ 3 Solid wood deck 4891 $\frac{3.07}{\leq 2.57}$ ≤ 4 Wooden deck93 $\frac{2.57}{\leq 2.07}$ ≤ 5 | | 09 | ≤ 2.07 | ≤ 6 |
| Aluminium deck with plywood 72 and 73, 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 75 and 76, 81 and 82 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 4 Lightweight aluminium deck 0.60 m 90 $\frac{3.07}{\leq 2.57}$ ≤ 4 Solid wood deck 48 91 $\frac{3.07}{\leq 2.57}$ ≤ 4 Wooden deck 93 $\frac{3.07}{\leq 2.07}$ ≤ 5 | Aluminium frame platform | 66 and 67 | ≤ 3.07 | ≤ 3 |
| Aluminium deck with plywood 78 and 79 ≤ 3.07 ≤ 3 Aluminium access deck with ladder 75 and 76, 81 and 82 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 4 Lightweight aluminium deck 0.60 m 90 3.07 ≤ 3 ≤ 2.57 ≤ 4 Solid wood deck 48 91 2.57 ≤ 4 Wooden deck 93 2.07 ≤ 5 | Aluminium frame platform with internal hatch | 69 and 70 | ≤ 3.07 | ≤ 3 |
| Aluminium access deck with ladder 81 and 82 ≤ 3.07 ≤ 3 Steel deck 85 ≤ 3.07 ≤ 4 Lightweight aluminium deck 0.60 m 90 3.07 ≤ 3 ≤ 2.57 ≤ 4 Solid wood deck 48 91 2.57 ≤ 4 Wooden deck 93 2.07 ≤ 5 | Aluminium deck with plywood | 1 | ≤ 3.07 | ≤ 3 |
| Lightweight aluminium deck 0.60 m 90 3.07 ≤ 3 ≤ 2.57 ≤ 4 3.07 ≤ 3 Solid wood deck 48 91 2.57 ≤ 4 Wooden deck 93 2.07 ≤ 5 | Aluminium access deck with ladder | / | ≤ 3.07 | ≤ 3 |
| Lightweight aluminium deck 0.60 m $ 90 $ | Steel deck | 85 | ≤ 3.07 | <u>≤ 4</u> |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | Lightweight aluminium dook 0.60 m | 00 | 3.07 | ≤ 3 |
| Solid wood deck 48 91 2.57 \leq 4 Wooden deck 93 2.07 \leq 5 | Lightweight aluminium deck 0.60 m | 90 | ≤ 2.57 | ≤ 4 |
| Wooden deck 93 2.07 ≤ 5 | | | 3.07 | ≤ 3 |
| | Solid wood deck 48 | 91 | 2.57 | ≤ 4 |
| ≤ 1.57 ≤ 6 | Wooden deck | 93 | 2.07 | ≤ 5 |
| | | | ≤ 1.57 | <u>≤</u> 6 |

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Table 13: (continued)

| Designation | Annex B, page | Bay width ℓ [m] | Use in load class (service class) | | |
|--|-----------------|--------------------|-----------------------------------|--|--|
| | | 2.57 | ≤ 3 | | |
| Solid wood deck 45 | 92 | 2.07 | ≤ 4 | | |
| | | ≤ 1.57 | ≤ 5 | | |
| U-deck T4 0.32 m, steel | 262 and 263 | ≤ 2.07 | ≤ 6 | | |
| U-deck 0.32 m, steel | 264 and 265 | 2.57 | ≤ 5 | | |
| U-deck, lightweight 0.32 m, steel | 296 | 3.07 | ≤ 4 | | |
| U-robust deck 0.61 m | 266 and 267 | ≤ 3.07 | ≤ 3 | | |
| | | ≤ 1.57 | ≤ 6 | | |
| III mahasat da ale 0.20 ma | 000 | 2.07 | ≤ 5 | | |
| U-robust deck 0.32 m | 268 | 2.57 | ≤ 4 | | |
| | | 3.07 | ≤ 3 | | |
| U-trapdoor for steel deck 0.64 m | 269 | 2.57 | ≤ 4 | | |
| U-robust deck with trapdoor with ladder | 272, 275, 276 | ≤ 3.07 | ≤ 3 | | |
| U-aluminium deck with trapdoor 0.61 m | 273, 274 | ≤ 3.07 | ≤ 3 | | |
| II wan daak talaaania | 277 | ≤ 2.07 | ≤ 6 | | |
| U-gap deck, telescopic O-deck 0.19 m, steel | 277 279, 280 | 2.57 | ≤ 5 | | |
| O-deck 0.13 III, Steel | 279, 200 | 3.07 | ≤ 4 | | |
| U-Alu-Durchstieg 1,00 x 0,61 m | 281 | ≤ 3.07 *) | ≤ 3 | | |
| O-deck T9 0.32 m, steel | 282 | ≤ 2.07 | ≤ 6 | | |
| O-deck T4 0.32 m, steel | 283 | 2.57 | ≤ 5 | | |
| O-deck, lightweight 0.32 m, steel | 297 | 3.07 | ≤ 4 | | |
| O dook T0.0.10 m. stool | 00.4 | ≤ 2.07 | ≤ 6 | | |
| O-deck T9 0.19 m, steel O-deck 0.32 m, steel | 284 285 | 2.57 | ≤ 5 | | |
| 0-400K 0.02 III, 3166I | 200 | 3.07 | ≤ 4 | | |
| in combination with U-support brackets and U-decks | | | | | |

2.2.12.2 Elastic support of the vertical frame sections

Non-anchored nodes of standard joints in the plane perpendicular to the span direction of the decks (in case of facade scaffolding perpendicular to the facade), whereby the ledgers are connected to the small openings of the connector disc (rosette), may be assumed to be elastically supported by the horizontal level (decking), provided that the neighbouring horizontal nodes are anchored. For load classes ≤ 3 this elastic support may be taken into account by assuming a travel-limiting spring according to figure 2 in connection with the design values given in Table 14 in accordance with the system width.

Decks that are not listed may only be used as bracket deck or for applications that do not serve any bracing purpose.

Table 14: Design values of the horizontal travel springs

| | £L. | Ħ. | _ | m] | Stiffness <i>c⊥,d</i> [kN/cm] | | Load- bearing capacity of the spring load $F_{\perp,Rd}$ [kN] |
|--|---|------|-----------------|--------------------------------------|-------------------------------|--|---|
| Deck | in accordance with Annex B, page Scaffolding width b [m] | | Bay width ℓ [m] | Clearance $f_{o,d} \left[cm ight]$ | $0 < F_{\perp} \le 1.50$ [KN] | $1.50 < F_{\perp} \le F_{\perp,Rd}$ [kN] | |
| Steel deck AF with tube fixture 0.32 m | 61 | | | | | | |
| Steel deck with tube fixture | 64 | | | 3.96 | 0.58 | 0.46 | 2.50 |
| Steel deck AF 0.32 m | 84 | | | | | | |
| Steel deck | 85 | | | | | | |
| Aluminium frame platform with tube fixture | 51, 52 | 0.73 | ≤ 3.07 | | | | |
| Aluminium frame platform | 66, 67 | | | 3.40 | 0.78 | 0.78 | 1.71 |
| Aluminium deck with plywood | 72, 73, 78, 79 | | | | | | |
| U-deck T4, steel | 262, 263 | | | 4.41 | 0.54 | 0.54 | 2.33 |
| U-deck, steel | 264, 265 | | | 4.41 | 0.54 | 0.54 | 2.55 |
| U-robust deck 0.61 m | 266, 267 | | | 4.90 | 0.58 | 0.48 | 2.50 |
| Steel deck AF with tube fixture 0.32 m | 61 | | | | | | |
| Steel deck with tube fixture | 64 | 1.09 | ≤ 3.07 | 4.39 | 0.79 | 0.79 | 2.46 |
| Steel deck AF 0.32 m | 84 | | | | | | |
| Steel deck | 85 | | | | | | |

2.2.12.3 Elastic coupling of the vertical levels

The inner and outer vertical plane of a scaffolding may be assumed to be elastically coupled to each other by the decking in the direction of these planes (in the case of facade scaffolding parallel to the facade) when connected through the small opening of the connector disc (rosette). This elastic coupling may be taken into account for load classes \leq 3 depending on the system width by assuming coupling springs according to figure 2 with the characteristic values specified in Table 15.

Decks that are not listed may only be used as bracket deck or for applications that do not serve any bracing purpose.

<u>Table 15:</u> Design values of the horizontal coupling springs

| | vith | tt (|] | cm] | Stiffness $c_{\perp,d}$ [kN/cm] | | Load- bearing capacity of the spring load F _{L,Rd} [kN] | | | | | | | | | | | | |
|--|---|-----------------|-------------------------|----------------------------------|-----------------------------------|------|--|--|--|--|--|--|--|--|--------|--|--|--|--|
| Deck | in accordance with Annex B, page Scaffolding width b [m] | Bay width ℓ [m] | Clearance f_{od} [cm] | $0 < F_{\parallel} \le 2.5$ [kN] | $2.5 < F_{ } \le F_{ ,Rd}$ [KN] | | | | | | | | | | | | | | |
| Steel deck AF with tube fixture 0.32 m | 61 | | | | | | | | | | | | | | | | | | |
| Steel deck with tube fixture | 64 | | | 1.40 | 2.58 | 2.58 | 4.50 | | | | | | | | | | | | |
| Steel deck AF 0.32 m | 84 | | ≤ 3.07 | | | | | | | | | | | | | | | | |
| Steel deck | 85 | | | | | | | | | | | | | | | | | | |
| Aluminium frame platform with tube fixture | 51, 52 | 0.73 | | 0.50 | 1.86 | 1.12 | 2.22 | | | | | | | | | | | | |
| Aluminium frame platform | 66, 67 | | | | | | 3.86 | | | | | | | | | | | | |
| Aluminium deck with plywood | 72, 73, 78, 79 | | | | | | | | | | | | | | ≥ 3.07 | | | | |
| U-deck T4, steel | 262, 263 | | | 1.00 | 2.59 | 2.53 | 5.00 | | | | | | | | | | | | |
| U-deck, steel | 264, 265 | | | 1.00 | 2.59 | 2.55 | 5.00 | | | | | | | | | | | | |
| U-robust deck 0.61 m | 266, 267 | | | 0.71 | 1.56 | 1.26 | 5.00 | | | | | | | | | | | | |
| Steel deck AF with tube fixture 0.32 m | 61 | | | | | | | | | | | | | | | | | | |
| Steel deck with tube fixture | 64 | | | 1.95 | 1.67 | 1.67 | 3.94 | | | | | | | | | | | | |
| Steel deck AF 0.32 m | 84 | | | | | | | | | | | | | | | | | | |
| Steel deck | 85 | 1.09 | | | | | | | | | | | | | | | | | |
| Steel deck AF with tube fixture 0.32 m | 61 | | | 1.95 | 5 1.39 | 1.39 | | | | | | | | | | | | | |
| Steel deck with tube fixture | 64 | | ≤ 2.57 | | | | 3.28 | | | | | | | | | | | | |
| Steel deck AF 0.32 m | 84 | | | | | | | | | | | | | | | | | | |
| Steel deck | 85 | | | | | | | | | | | | | | | | | | |

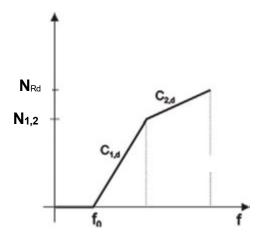


Figure 2: Representation of spring stiffness

2.2.12.4 Material parameters

For components made of S235JRH or S275J0H steel with an increased yield point $(R_{eH} \ge 320 \, \text{N/mm}^2)$

- any such components are marked accordingly in the drawings in Annex A – the design value of the yield point $f_{y,d} = 291 \ N/mm^2$ may be used for the calculation. All other characteristic values must be applied according to the basic material.

2.2.12.5 Tubes Ø48.3 mm made of S460MH steel in Load-bearing group "A"

Scaffold tubes $\emptyset 48.3 \times 2.9 \text{ } \text{mm}$ and $\emptyset 48.3 \times 2.7 \text{ } \text{mm}$ made of S460MH steel may be assigned to buckling curve "a" in LBG "A".

For the proof of structural stability, the plastic shape coefficient shall be limited to $\alpha_{pl} = 1.25$. If for the tubes $\emptyset 48.3 \ mm$ made from S460MH a calculation according to the second order theory of elasticity is carried out, the next value may be assumed as the design value of the initial curvature:

$$\nu_{\mathcal{O}} = \frac{\ell}{300}$$
 (equation 19)

The cosine interaction may be used for the pressure-with-bending interaction verification.

2.2.12.6 Cross-section properties of the U-profiles

2.2.12.6.1 U-profiles without openings

The structural stability of U-profiles used with a number of components may uniformly be verified assuming the characteristic values according to figure 3:

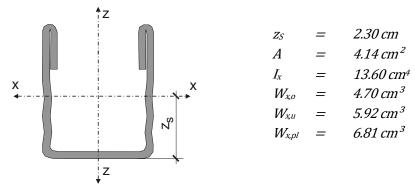


Figure 3: Characteristic values of the U-profiles without openings

2.2.12.6.2 U-profile with openings

The structural stability of U-profiles used for various components may be verified uniformly using the characteristic values according to Figure 4:

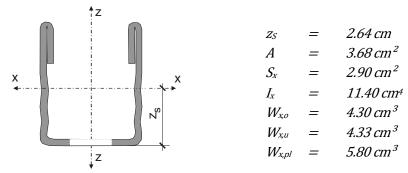


Figure 4: Characteristic values of U-profiles without holes

2.2.12.7 Scaffolding spindles / base jacks

The substitute section properties for the base jacks for the stress and / or interaction analyses and calculations of deformation according to DIN 4425:2017-04 (Annex B of DIN EN 12811-1:2004-03) shall be assumed as follows:

for the base jacks according to Annex B, pages 17 and 20:

```
A = A_S = 3.52 \text{ cm}^2
I = 4.00 \text{ cm}^4
W_{el} = 2.68 \text{ cm}^3
W_{pl} = 1.25 \cdot 2.68 = 3.35 \text{ cm}^3
```

for the base jacks according to Annex B, pages 18 and 19:

```
A = A_S = 3.85 \text{ cm}^2
I = 4.27 \text{ cm}^4
W_{el} = 2.83 \text{ cm}^3
W_{pl} = 1.25 \cdot 2.83 = 3.54 \text{ cm}^3
```

for the scaffolding spindles according to Annex B, page 178:

```
A = A_S = 3.84 \text{ cm}^2
I = 3.74 \text{ cm}^4
W_{el} = 2.61 \text{ cm}^3
W_{pl} = 1.25 \cdot 2.61 = 3.26 \text{ cm}^3
```

for the scaffolding spindles according to Annex B, page 292:

```
A = As = 4.71 \text{ cm}^2
I = 4.29 \text{ cm}^4
W_{el} = 2.97 \text{ cm}^3
W_{pl} = 1.25 \cdot 2.97 = 3.71 \text{ cm}^3
```

The cosine interaction in accordance with DIN 4425:2017-04, section 7.1 may be used to provide proof of stability for the load-bearing capacity of the scaffolding spindles / base jacks.

2.2.12.8 Couplers

For the proof of stability of the halfcouplers attached to the various components, the load-bearing capacities and stiffness values for halfcouplers of class B shall be applied in accordance with the information provided in DIN EN 74-2:2022-09.

2.3 Design

2.3.1 General provisions

The Technical Building Rules [Technische Baubestimmungen] apply to the designing of scaffolds using components of the "ALBLITZ MODUL" modular system, in particular those for working and service scaffolds according to DIN EN 12811-1:2004-03 in conjunction with the "Application Guideline for Working Scaffolds in accordance with DIN EN12811-1"¹, DIN 4420-1:2004-03, the "Approval Principles for Working and Service Scaffolds – Requirements, Calculation Assumptions, Tests, Certificate of Conformity"³; for supporting structures DIN EN 12812:2008-12 in conjunction with the "Application Guideline for Supporting Structures in accordance with DIN EN 12812"² is applicable, as well as the provisions set out below.

When using the modular scaffolding system as a temporary structure that does not fall within the scope of temporary auxiliary construction aids, application-specific requirements may need to be taken into account in the design.

Assembly, alteration and dismantling of the scaffold must be carried out in compliance with the Instructions for Assembly and Use⁶ which are not subject of this decision.

In case of working and service scaffolds, the Instructions for Assembly and Use must comply with the requirements of the "Application guideline for working scaffolds according to DIN EN 12811-1", see DIBt-Mitteilungen (notifications of the DIBt) issue 2/2006.

2.3.2 Condition of components

All components shall be inspected for proper condition prior to assembly. Damaged components may not be used.

2.3.3 Structural design

2.3.3.1 Components

Scaffolds in accordance with this decision must be erected using the scaffold components listed in section 1. Such components must be marked in accordance with the regulations of the respective decisions.

The following applies when using scaffolding nodes:

- Each connector disc (rosette) allows for connecting up to 8 members.
- The connector head wedges must be fixed by driving the wedge from top to bottom to the end-stop with a 0.5 kg hammer.

2.3.3.2 Base area

The lower standards or vertical starter pieces are to be positioned and adjusted on the base jacks in a way that the working levels are horizontal. It must be ensured that the base plates of the base jacks are horizontal and supported over the entire area to absorb and transmit the forces resulting from the scaffolding in the supporting surface.

2.3.3.3 Scaffolding decks

Scaffolding decks must be secured to prevent them from accidental lift-off.

2.3.3.4 Side protection

The provisions of DIN EN 12811-1:2004-03 apply to the side protection. Primarily use components intended for this use and only exceptionally use components such as steel tubes and couplers according to DIN EN 12811-1:2004-03 as well as scaffold decks and planks according to DIN 4420-1:2004-03.

2.3.3.5 Bracing

Scaffolds must be braced.

The vertical planes are to be braced by means of longitudinal ledgers or longitudinal ledgers in conjunction with vertical diagonal braces.

Horizontal scaffolding levels are to be braced by means of ledgers and horizontal diagonal braces or by means of system decks in conjunction with transoms in accordance with Sections 2.2.12.2 and 2.2.12.3.

The individual bracing levels are to be erected and positioned in accordance with the structural analysis.

2.3.3.6 Anchoring

Please refer to the structural analysis for anchor forces and the anchor configuration. The anchorage of scaffold retainers/wall ties to the facade or to other parts of the building is not covered by this decision. The user must ensure that the respective forces can be securely absorbed and transmitted from scaffold retainers/wall ties. Vertical forces must not be transmitted in this process.

2.3.3.7 Couplers

Couplers with screwed connectors must be tightened with a torque of 50 Nm when connecting to the standards; tolerances of \pm 10 % are permitted. According to the manufacturer's instructions for use, bolts/screws must be easy to reposition.

Couplers with wedge-lock must be connected by driving the wedge to the end-stop with a 0.5 kg hammer (or heavier).

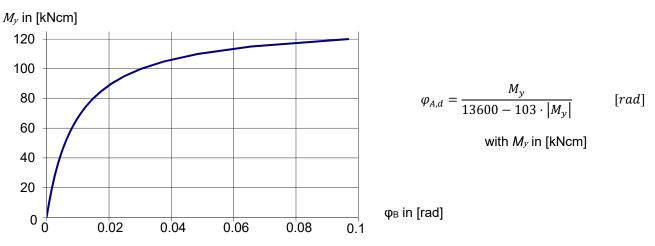


Figure 1: Load-bearing group "A": Torsional spring stiffness at the ledger connection in the vertical plane (tubular standard – ledger)

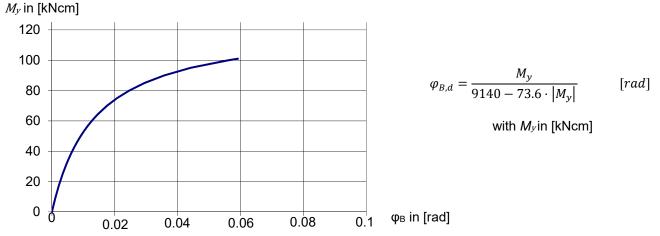


Figure 2: Load-bearing group "B": Torsional spring stiffness at the ledger connection in the vertical plane (tubular standard – ledger)

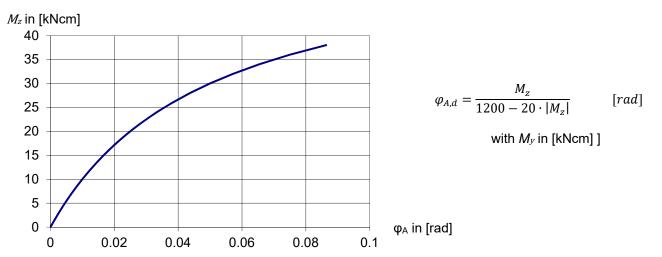


Figure 3: Load-bearing group "A": Torsional spring stiffness at the ledger connection in the horizontal plane (perpendicular to the tubular standard/ledger plane)

Modular scaffolding system "ALBLITZ MODUL"

Annex A, page 1

Moment-rotation relationships for the ledger connection

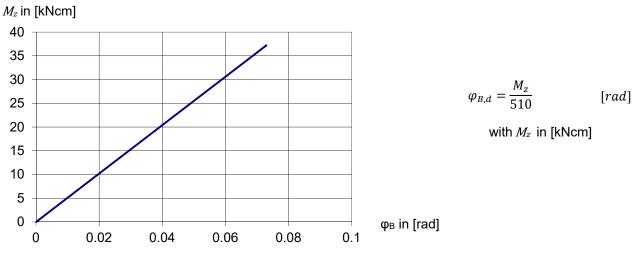


Figure 4: Load-bearing group "B": Torsional spring stiffness at the ledger connection in the horizontal plane (perpendicular to the tubular standard/ledger plane)

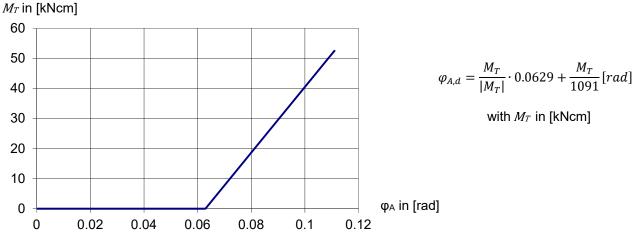


Figure 5: Load-bearing group"A": Torsional spring stiffness in the **O-ledger connection** under torsional moment at the ledger axis section.

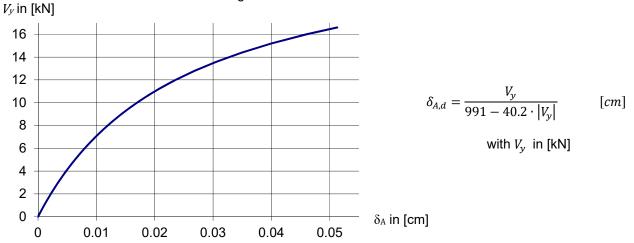


Figure 6: Load-bearing group "A": Force-displacement relationship in the ledger connection and the double wedge head coupler "lightweight design" subject to horizontal shear load

| Modular scaffolding system "ALBLITZ MODUL" | |
|---|-----------------|
| Torsional and travel-limiting spring stiffness in the ledger connection for M_z,M_T and V_y | Annex A, page 2 |

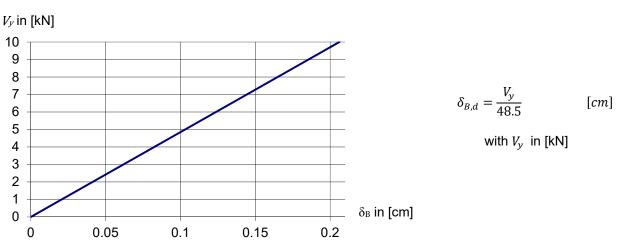


Figure 7: Load-bearing group "B": Force-displacement relationship in the ledger and bracket connection and the connection of a double wedge head coupler under horizontal shear load

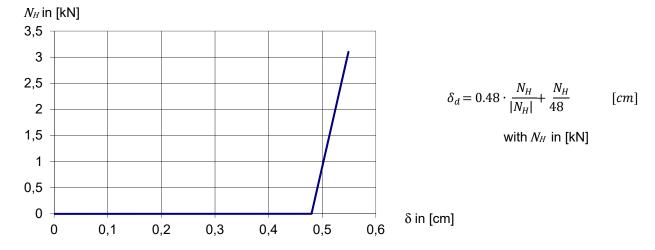
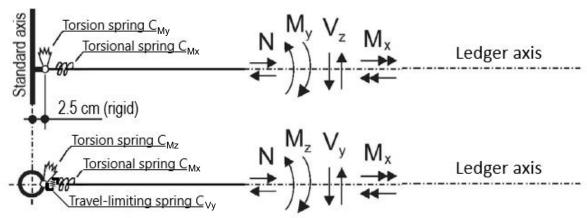


Figure 8: Travel-limiting spring stiffness in the connection of the horizontal diagonal braces according to Annex B, page 9

| Modular scaffolding system "ALBLITZ MODUL" | |
|---|-----------------|
| Travel-limiting spring stiffness in the ledger connection for V_{y} and in the connection of a horizontal diagonal brace | Annex A, page 3 |

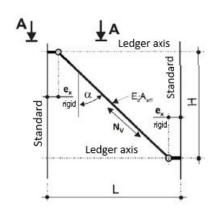
Structural system ledger connection



Note:

 M_x or. c_{Mx} only applies in case of Load-bearing group "A" in connection with O-ledgers.

Structural system vertical diagonal brace

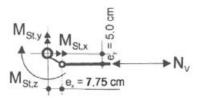


Additionally, the following degrees of freedom in the longitudinal direction of the diagonal braces are to be assumed:

Load-bearing group "A" & Load-bearing group "B":

 $f_{0,d} = 0.70 \ cm$

Sectional view A-A



The following node moments must be absorbed by the standard and the ledgers.

Node moments due to diagonal force N_V

$$M_{St,x} = N_V \cdot \cos \alpha \cdot e_y$$

$$M_{St,y} = N_V \cdot \cos \alpha \cdot e_x$$

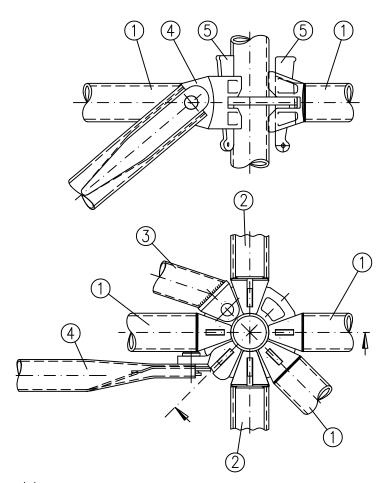
$$M_{St,z} = N_V \cdot \sin \alpha \cdot e_y$$

with
$$e_x = 7.75 cm$$
 and $e_y = 5.00 cm$

Modular scaffolding system "ALBLITZ MODUL"

Structural systems vertical diagonal brace and ledger connection

Annex A, page 4

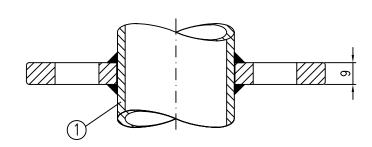


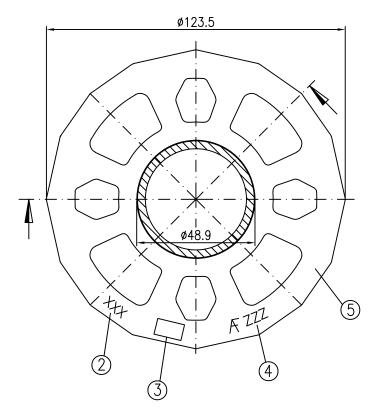
- 1 Tube ledger
- ② U-ledger
- 3 Horizontal diagonal brace
- 4 Vertical diagonal brace
- (5) Wedge 6 mm

Possible combinations of V-standards and ledgers with load-bearing group

| Ledger connection V-standards | MULTI | MULTI 4.0 | K2000+ | LW |
|-------------------------------|----------------------|----------------------|----------------------|----------------------|
| MULTI | Z-8.22-906 / | Z-8.22-906 / | Z-8.22-913 / | Z-8.22-913 / |
| | Load-bearing group B | Load-bearing group B | Load-bearing group B | Load-bearing group B |
| MULTI 4.0 | Z-8.22-906 / | Z-8.22-906 / | Z-8.22-913 / | Z-8.22-913 / |
| | Load-bearing group B | Load-bearing group A | Load-bearing group B | Load-bearing group A |
| K2000+ | Z-8.22-913 / | Z-8.22-913 / | Z-8.22-64 / | Z-8.22-949 / |
| | Load-bearing group B | Load-bearing group B | Load-bearing group B | Load-bearing group B |
| LW | Z-8.22-913 / | Z-8.22-913 / | Z-8.22-949 / | Z-8.22-939 / |
| | Load-bearing group B | Load-bearing group A | Load-bearing group B | Load-bearing group A |

| ALBLITZ MODUL | | |
|---|---------|--------------------|
| Overview of scaffolding nodes in accordance with Z-8.22-906 | | Annex B, page 1 |
| M710-B101_ABM | 05.2021 | |





(1) Circular hollow section Ø48.3x3.2 Circular hollow section Ø48.3x2.9 (Design 4.0)

② Batch number / week year

3 Company logo of the foundry

4) F abbreviated approval number

(5) Cast steel alternatively: Steel

Material thickness = 9 mm

DIN EN 10219-S235JRH R_{eH}≥320 N/mm²

DIN EN 10219-S460MH

stamped 0.4

stamped 0.4

stamped 0.4

Material in accordance with the documents filed at DIBt

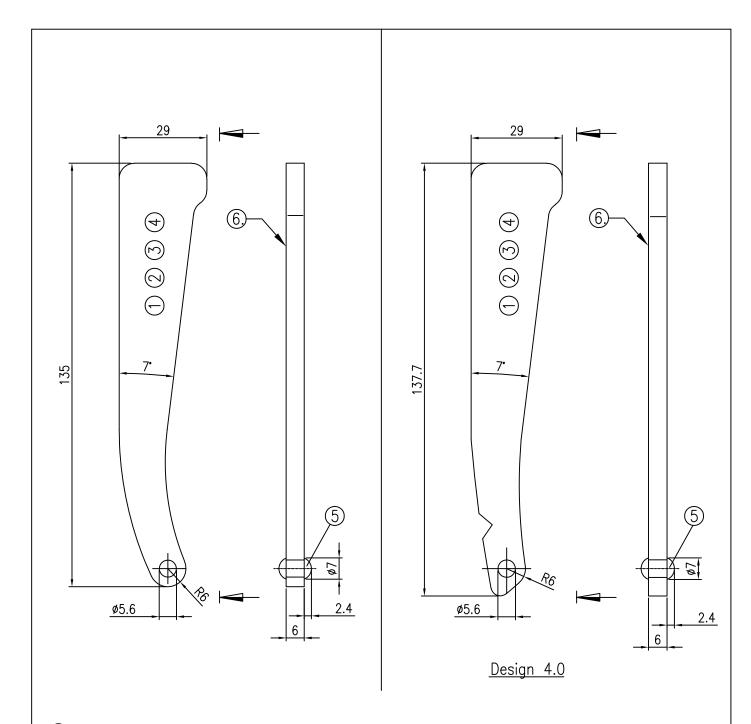
ALBLITZ MODUL

Connector disc

in accordance with Z-8.22-906

M710-B102_ABM

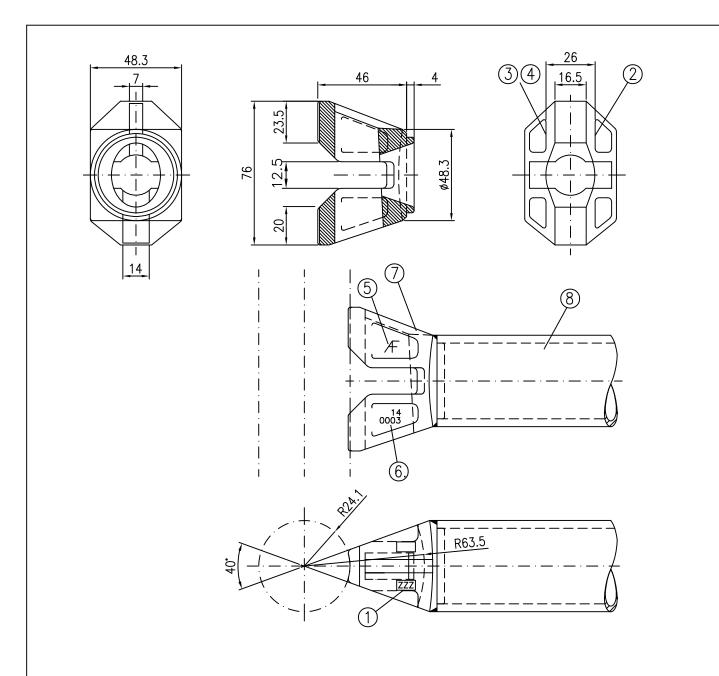
Annex B, page 2



- \bigcirc XX = Supplier number
- 2) ZZZ/ZZZ = abbreviated approval number
- ③ Æ = ALFIX manufacturer's logo
- (4) YY = Year of manufacturing (e.g. 18=2018)
- (5) Round head rivet Ø5x10 with rivet head of rivet Ø4 DIN 660 QSt 32-2 electrogalvanized
- (6) Marking

hot-dip galvanized; DIN EN 10149-S550MC

| ALBLITZ MODUL | | |
|-------------------------------|---------|----------|
| | | Annex B, |
| Wedge | | page 3 |
| in accordance with Z-8.22-906 | | 19 |
| M710-B103_ABM | 11.2022 | |



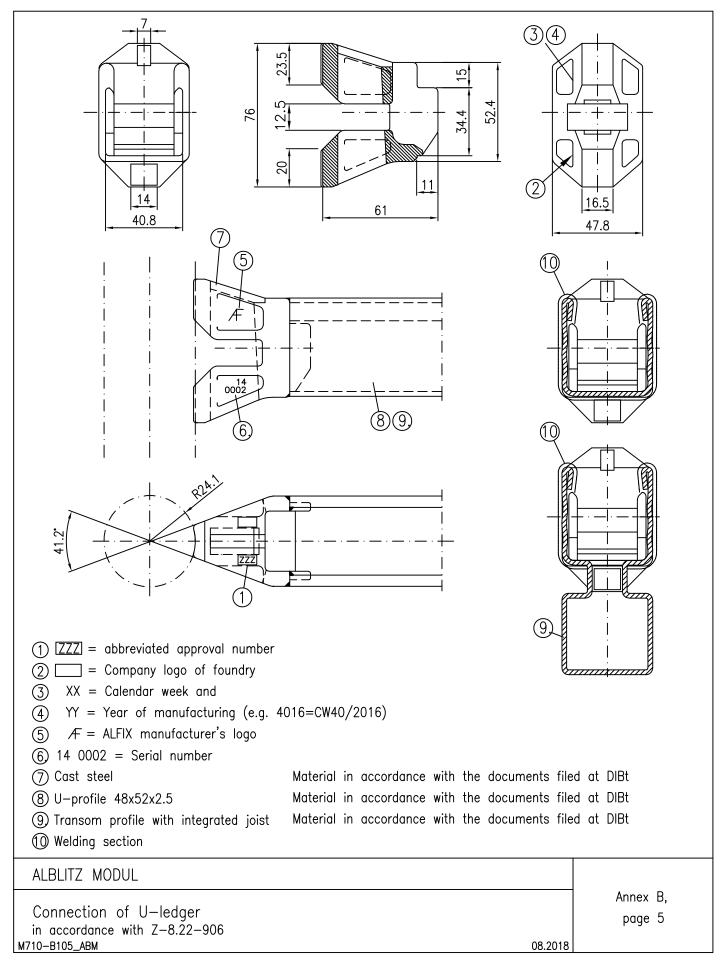
- (1) ZZZ = abbreviated approval number
- (2) = Company logo of foundry
- (3) XX = Calendar week and
- (4) YY = Year of manufacturing (e.g. 4016=CW40/2016)
- (5) Æ = ALFIX manufacturer's logo
- \bigcirc 14 0003 = Serial number
- (7) Cast steel

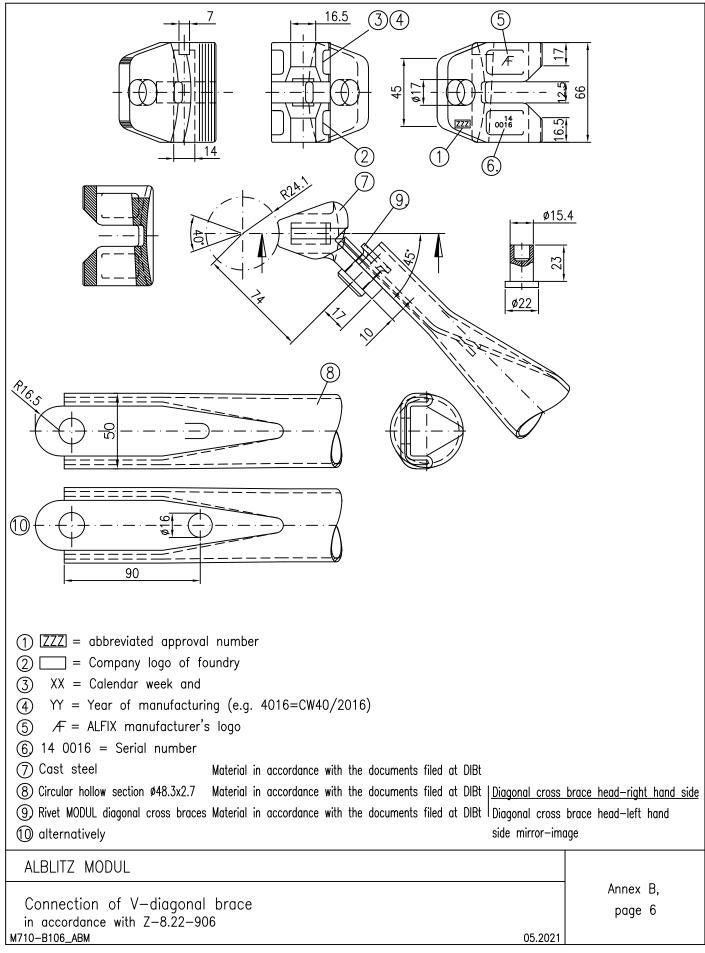
Material in accordance with the documents filed at DIBt

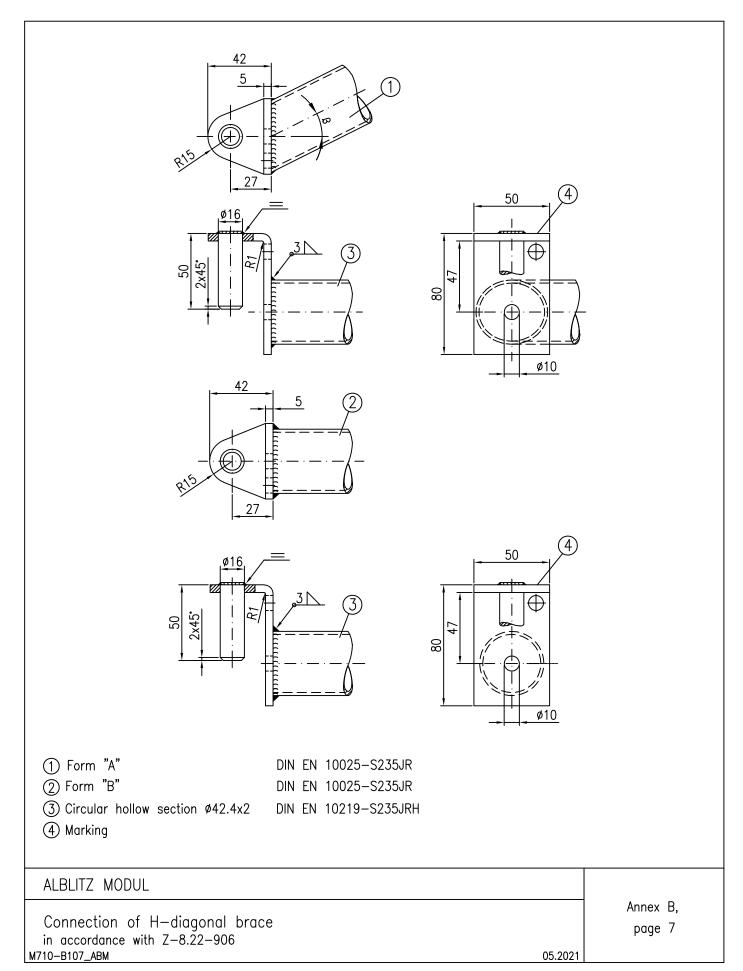
(8) Circular hollow section Ø48.3x3.2 alternatively: 48.3x2.7

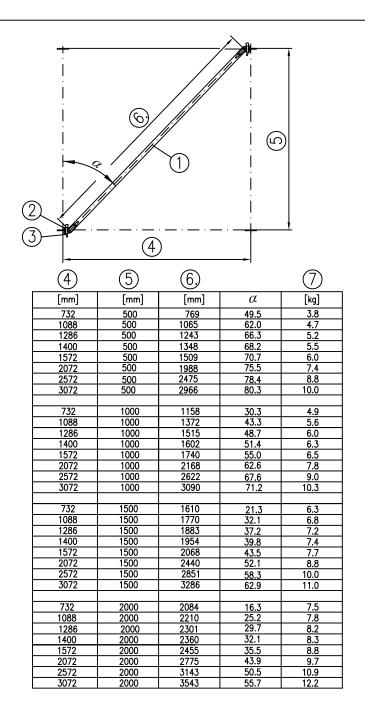
Material in accordance with the documents filed at DIBt

| ALBLITZ MODUL | | |
|-------------------------------|---------|----------|
| | | Annex B, |
| Connection of tube ledger | | page 4 |
| in accordance with Z-8.22-906 | | page . |
| M710-B104_ABM | 05.2021 | |









1 Circular hollow section Ø48.3x2.7

DIN EN 10219-S235JRH

R_eH≥320 N/mm²

(2) Connection of V-diagonal brace

see Annex B, page 6

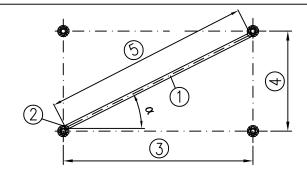
(3) Wedge 6 mm

see Annex B, page 3

- (4) Bay length L (5) Bay height H
- (6) Rivet spacing I
- (7) Weight

Vertical diagonal braces in accordance with Z-8.22-906 M710-B108_ABM

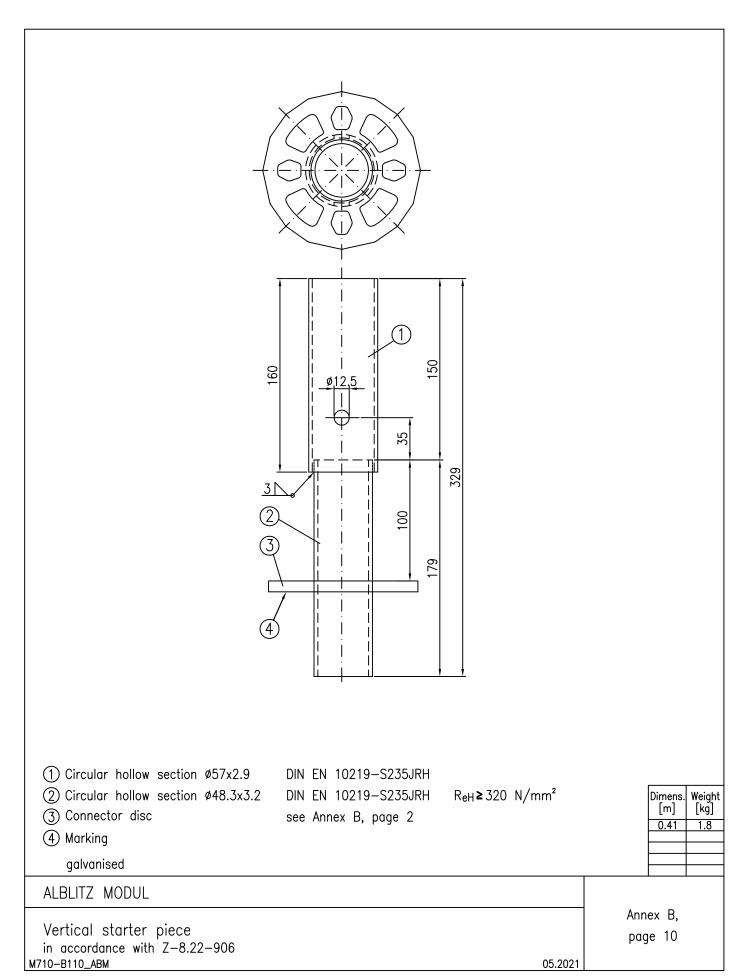
Annex B, page 8

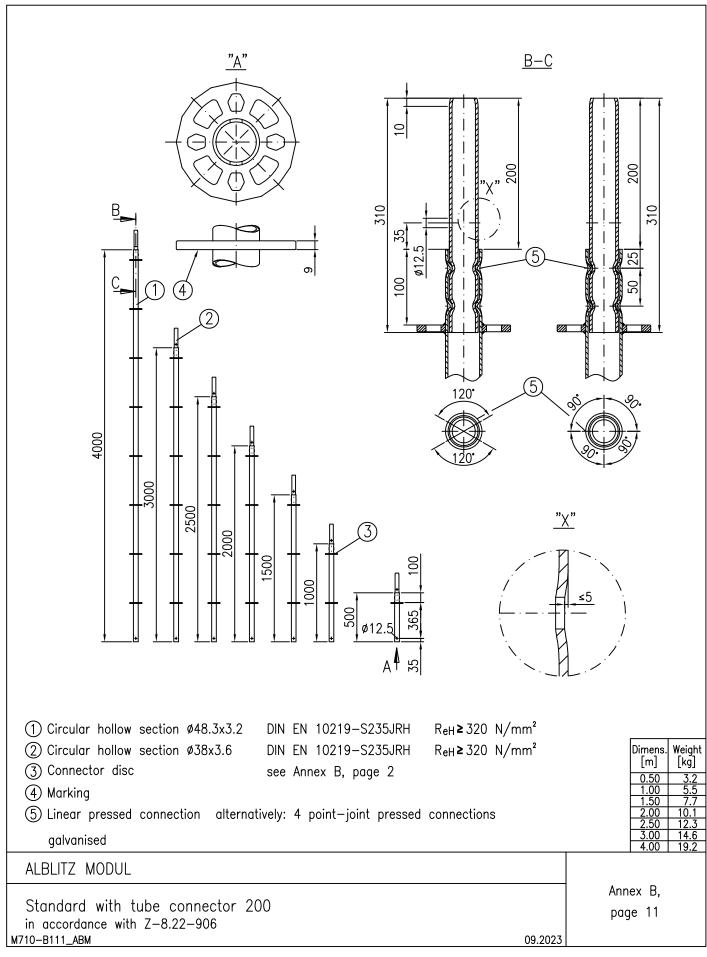


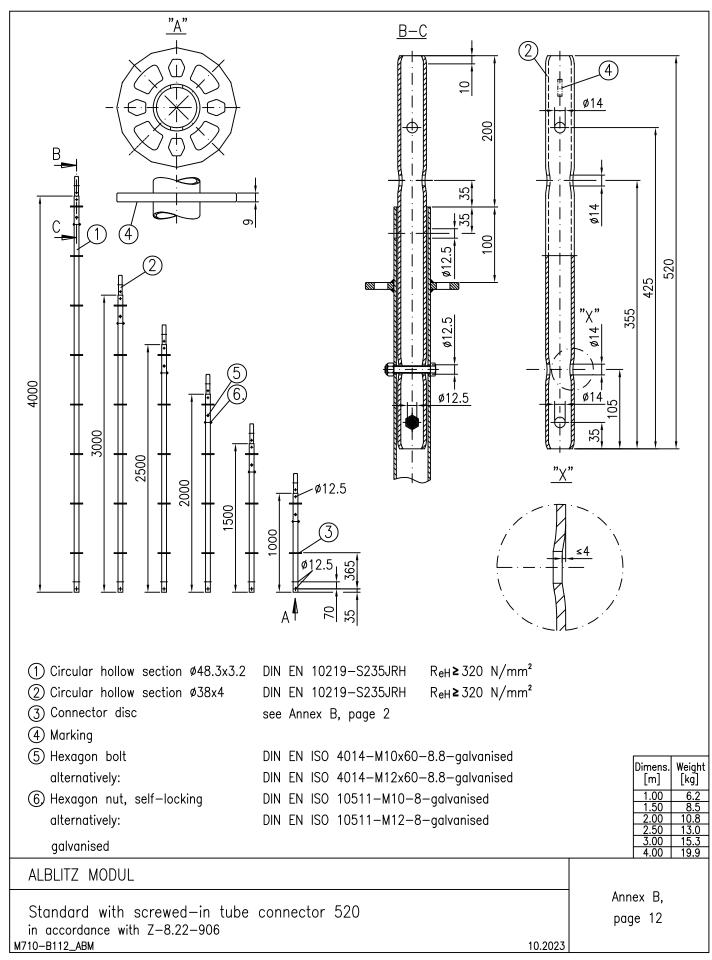
| 6 | (3) | 4 | (5) | α | 7 |
|--------------|------|--------|------|----------|------|
| | [mm] | [mm] | [mm] | [:] | [kg] |
| В | 732 | 732 | 953 | 45 | 2.5 |
| Α | 1088 | 732 | 1231 | 33.9 | 3.1 |
| Α | 1286 | 732 | 1399 | 28 | 4.8 |
| Α | 1400 | 732 | 1502 | 27.6 | 6.0 |
| Α | 1572 | 732 | 1657 | 25 | 6.6 |
| Α | 2072 | 732 | 2124 | 19.5 | 4.8 |
| Α | 2572 | 732 | 2603 | 15.9 | 6.0 |
| Α | 3072 | 732 | 3088 | 13.4 | 6.6 |
| | | | | | |
| В | 1088 | 1088 | 1457 | 45 | 3.6 |
| Α | 1286 | 1088 | 1601 | 40 | 3.7 |
| Α | 1400 | 1088 | 1692 | 37.8 | 3.9 |
| Α | 1572 | 1088 | 1831 | 34.7 | 4.4 |
| Α | 2072 | 1088 | 2262 | 27.7 | 5.0 |
| A | 2572 | 1088 | 2717 | 22.9 | 6.3 |
| A | 3072 | 1088 | 3185 | 19.5 | 7.8 |
| | | | | | |
| В | 1286 | 1286 | 1777 | 45 | 4.2 |
| A | 1400 | 1286 | 1817 | 42 | 4.4 |
| A | 1572 | 1286 | 1948 | 39 | 4.7 |
| A | 2072 | 1286 | 2357 | 31 | 5.6 |
| A | 2572 | 1286 | 2796 | 25.7 | 6.5 |
| Â | 3072 | 1286 | 3253 | 22 | 6.9 |
| | | | | | |
| В | 1400 | 1400 | 1898 | 45 | 4.3 |
| Ā | 1572 | 1400 | 2023 | 48.3 | 4.6 |
| Â | 2072 | 1400 | 2420 | 55.9 | 5.4 |
| Â | 2572 | 1400 | 2850 | 61.4 | 6.6 |
| Â | 3072 | 1400 | 3299 | 65.5 | 7.1 |
| | 0072 | .,,,,, | 0200 | 00.0 | 7 |
| В | 1572 | 1572 | 2141 | 45 | 5.0 |
| Ā | 2072 | 1572 | 2519 | 37.2 | 5.8 |
| Â | 2572 | 1572 | 2935 | 31.4 | 6.7 |
| Â | 3072 | 1572 | 3373 | 27.1 | 7.7 |
| — " — | 55.2 | 1072 | 5575 | 27.1 | ,,, |
| В | 2072 | 2072 | 2848 | 45 | 6.6 |
| Ā | 2572 | 2072 | 3221 | 38.8 | 7.3 |
| Â | 3072 | 2072 | 3625 | 34 | 8.2 |
| <u> </u> | | | | <u> </u> | |
| В | 2572 | 2572 | 3555 | 45 | 8.0 |
| Ä | 3072 | 2572 | 3925 | 39.9 | 8.6 |
| —" | 0072 | 20,2 | 0020 | 1 00.0 | 0.0 |
| В | 3072 | 3072 | 4262 | 45 | 9.6 |

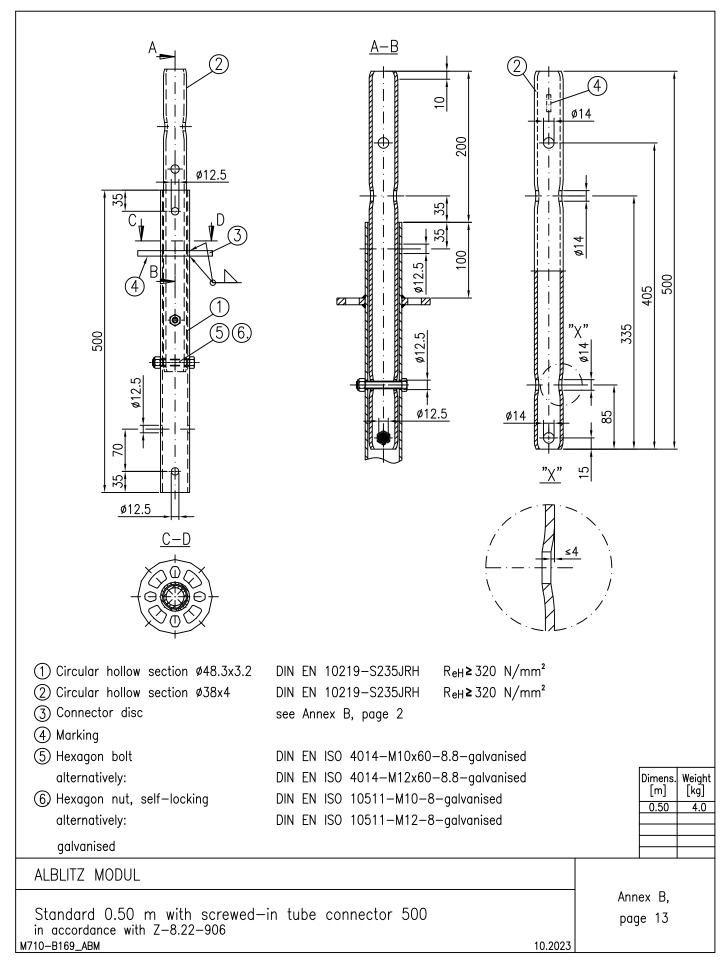
- ① Circular hollow section Ø42.4x2 DIN EN 10219-S235JRH
- ② Connection of H-diagonal brace see Annex B, page 7
- 3 Bay length L
- 4 Bay width B
- (5) Bolt spacing I
- 6 Form
- 7 Weight

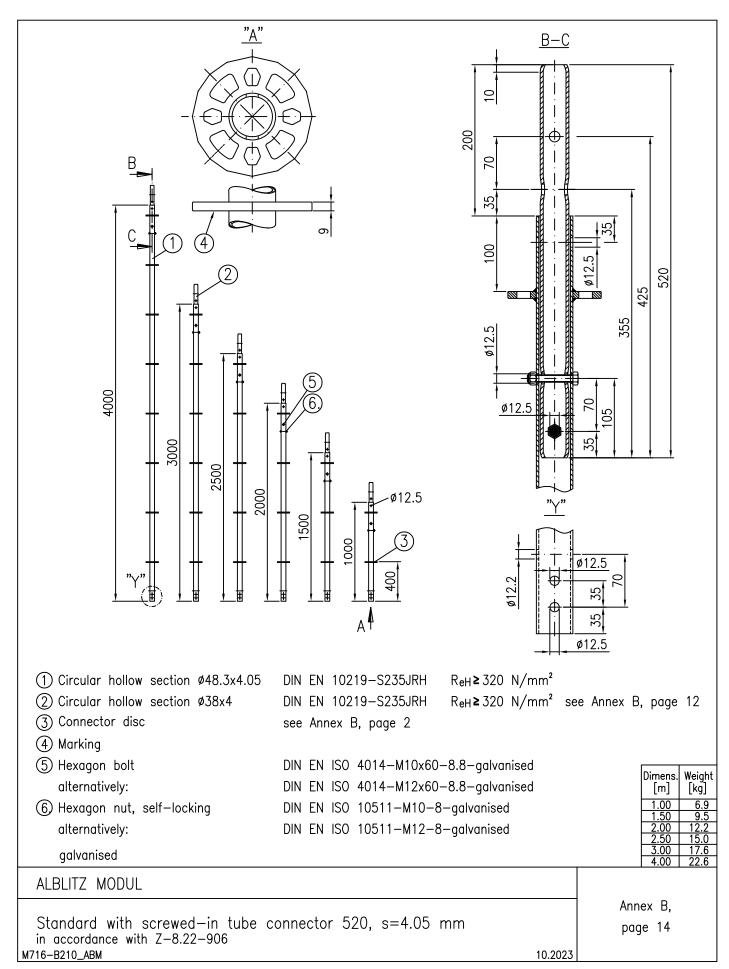
| ALBLITZ MODUL | | |
|--|---------|--------------------|
| Horizontal diagonal braces in accordance with Z-8.22-906 | | Annex B, page 9 |
| M710-B109_ABM | 05.2021 | |

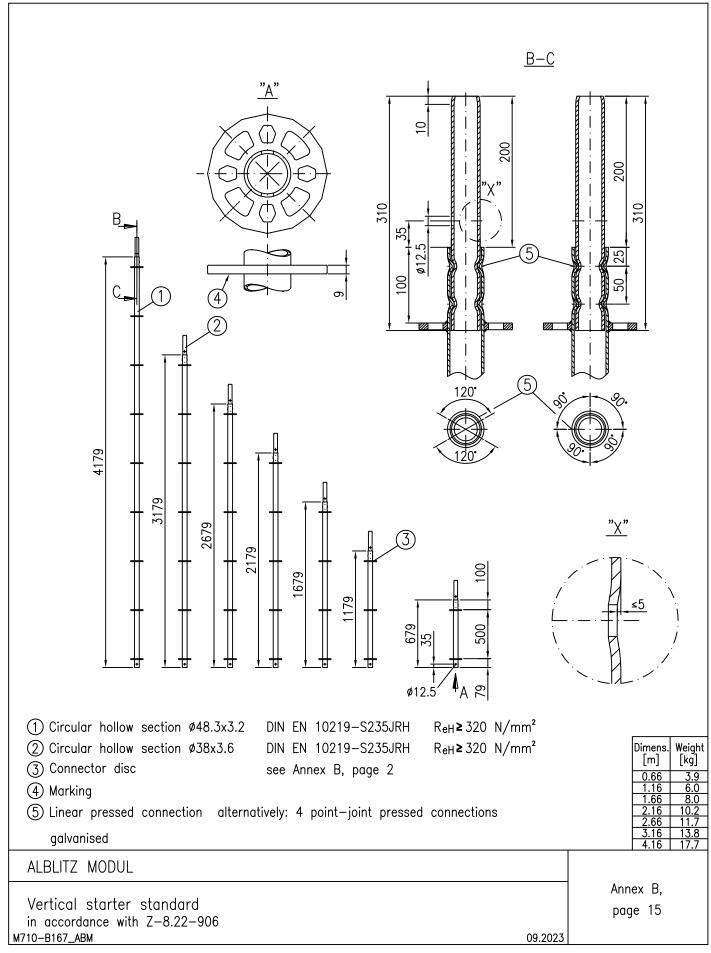


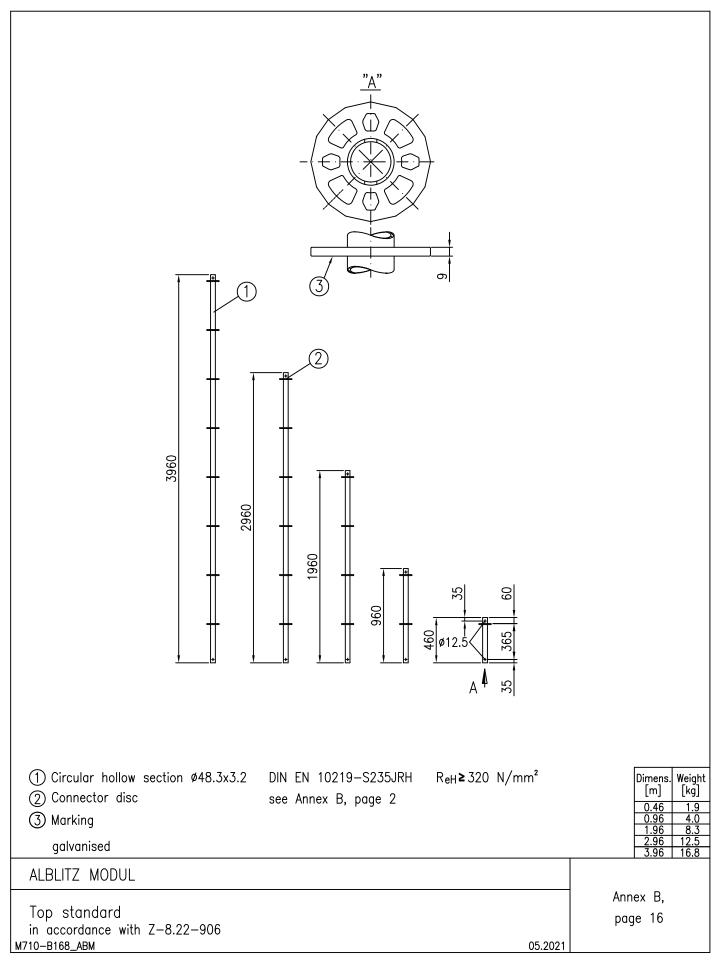


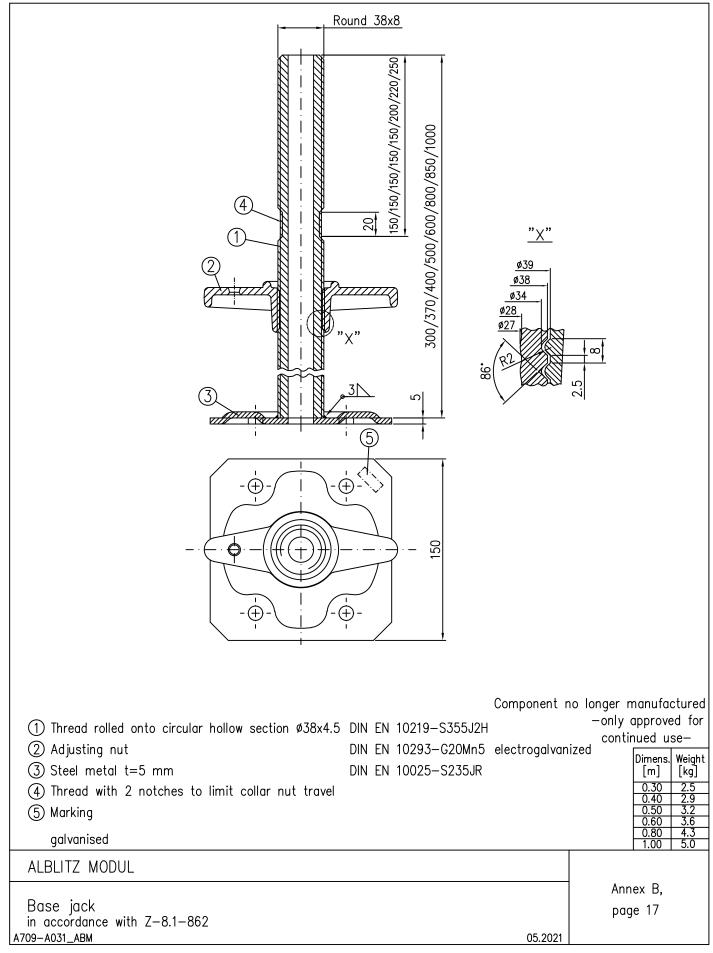


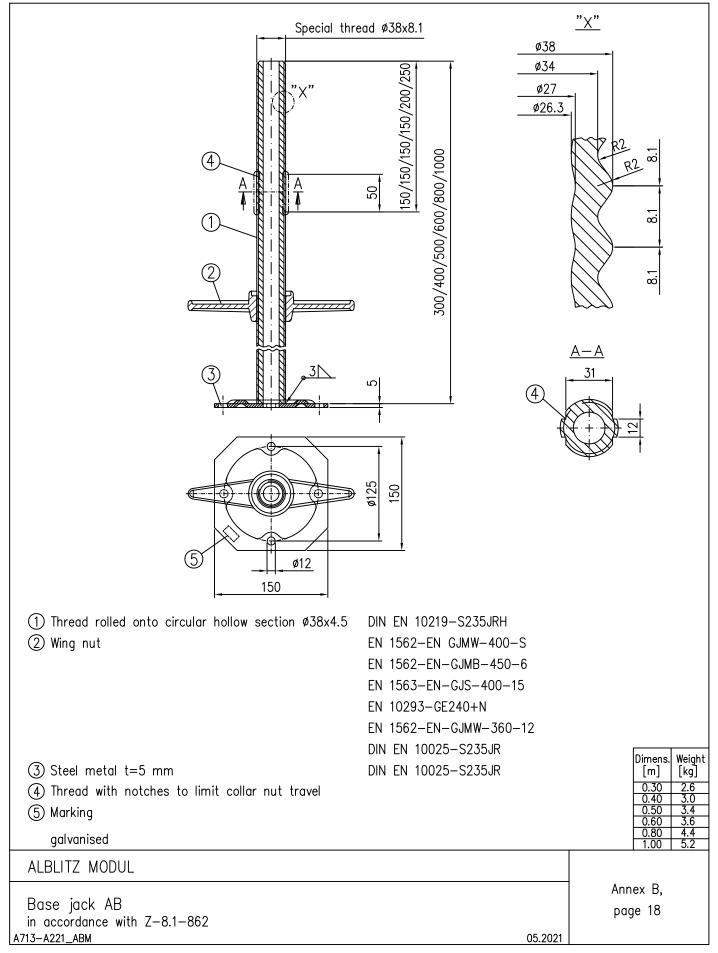


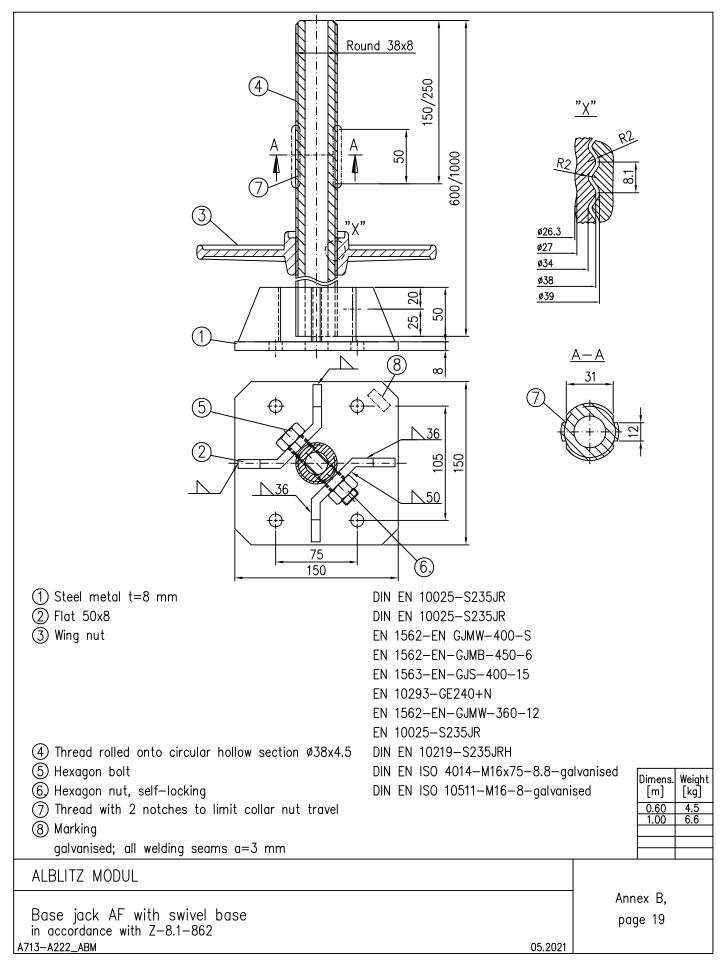


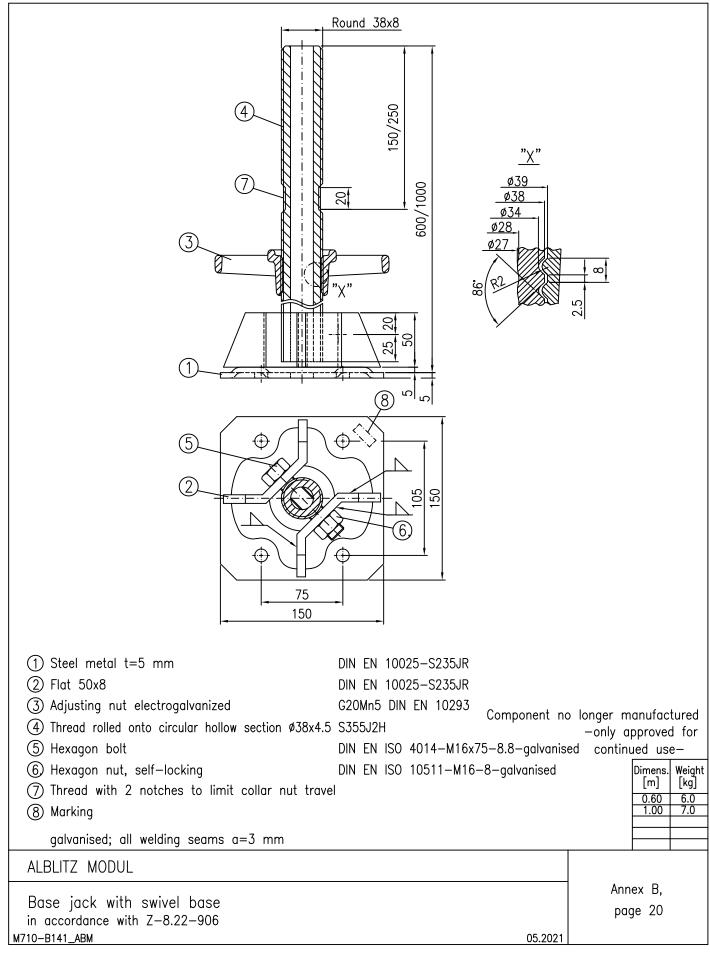


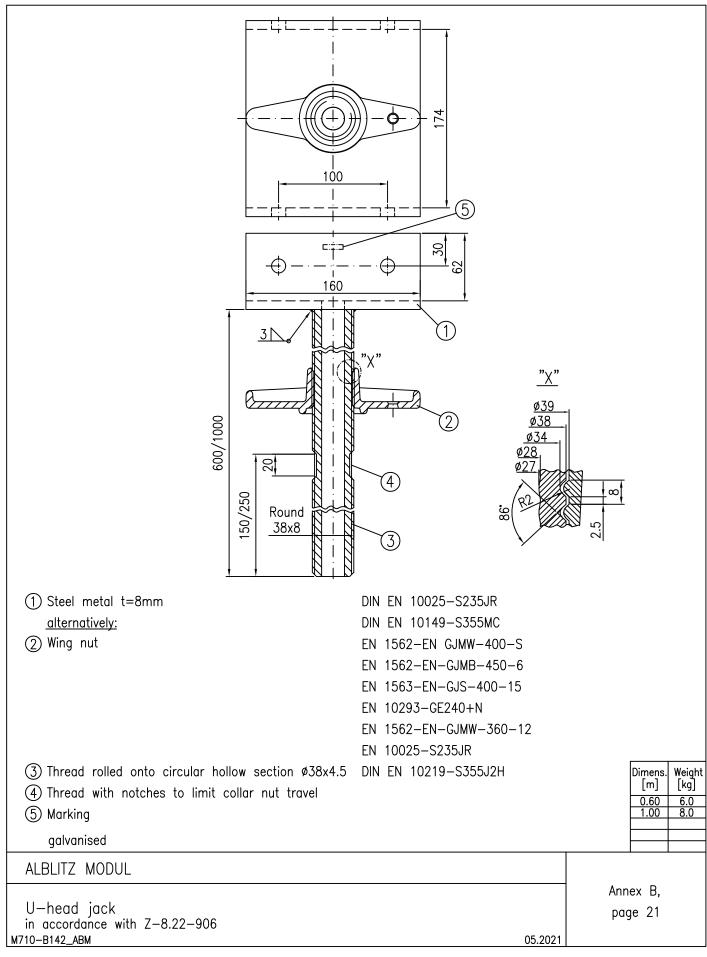


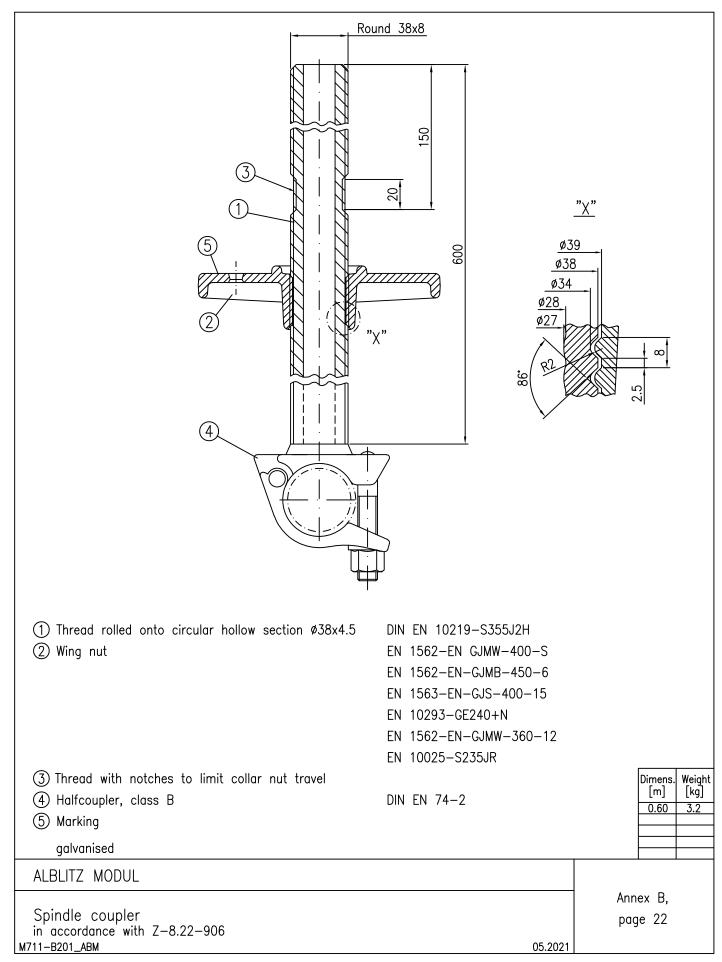


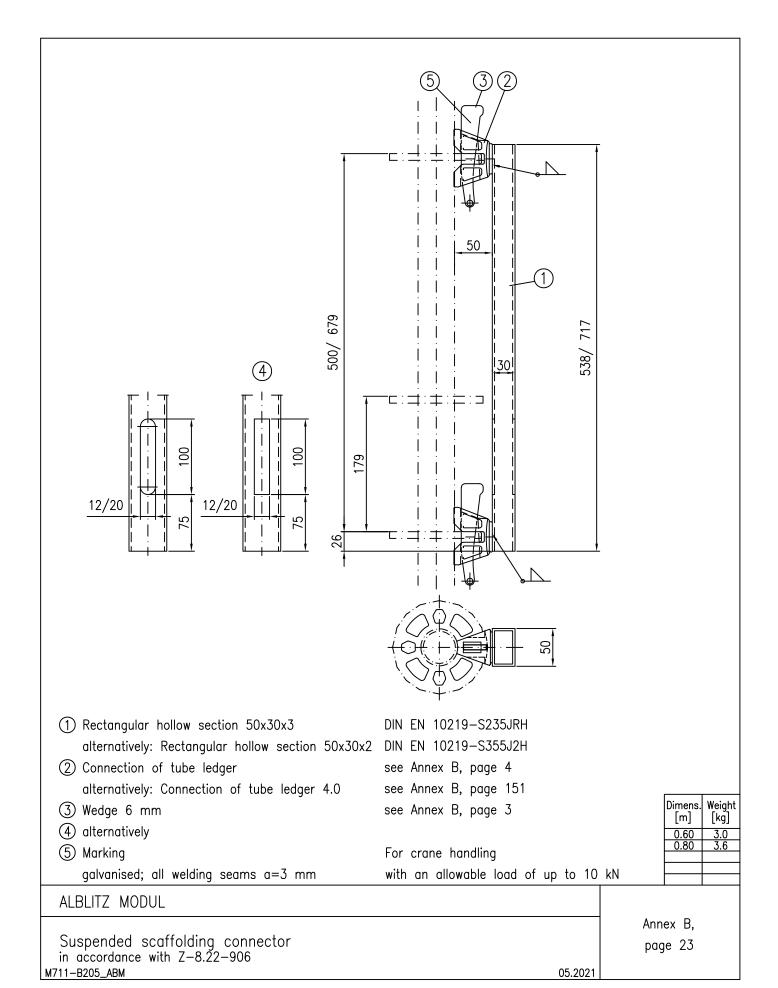


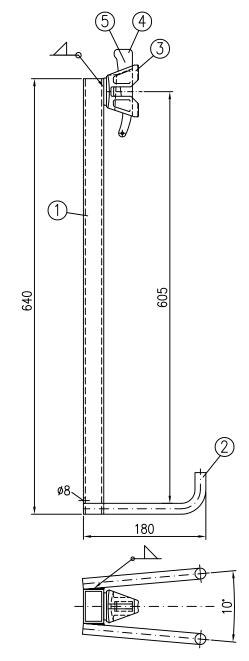












(1) Rectangular hollow section 50x30x3 alternatively: Rectangular hollow section 50x30x2

(2) Round Ø16

3 Connection of tube ledger alternatively: Connection of tube ledger 4.0

- 4) Wedge 6 mm
- (5) Marking

galvanised; all welding seams a=3 mm

DIN EN 10219-S235JRH

DIN EN 10219-S355J2H

DIN EN 10025-S235JR

see Annex B, page 4

see Annex B, page 151

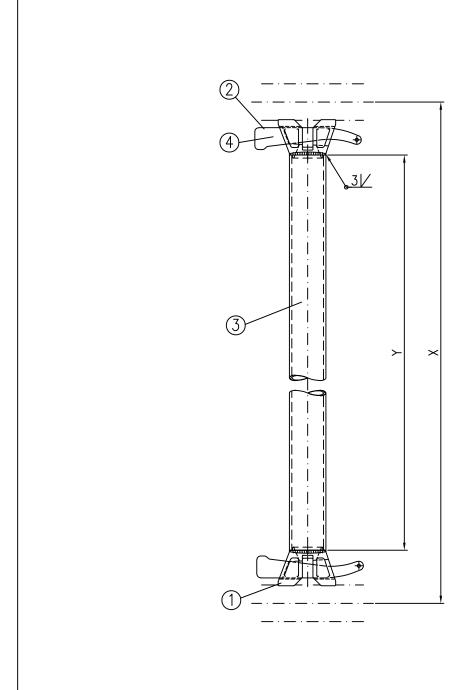
see Annex B, page 3

| Dimens. [m] | Weight [kg] | |
|----------------|----------------|--|
| 0.65 | 3.5 | |
| | | |

ALBLITZ MODUL

Locking device for base jack in accordance with Z-8.22-906 M710-B143_ABM Annex B, page 24

05.2021



| ① Connection of tube ledger | see Annex B, page 4 | |
|-------------------------------------|----------------------|--|
| ② Wedge 6 mm | see Annex B, page 3 | |
| ③ Circular hollow section Ø48.3x3.2 | DIN EN 10219-S235JRH | R _{eH} ≥320 N/mm ² |

| 4 | Marking |
|---|------------|
| | galvanised |

| 0,50 | 500 | JJ9 | 2.5 |
|------|------|------|------|
| 0,73 | 732 | 591 | 3.2 |
| 1,04 | 1036 | 895 | 4.2 |
| 1,09 | 1088 | 947 | 4.5 |
| 1,29 | 1286 | 1145 | 5.0 |
| 1,40 | 1400 | 1259 | 5.6 |
| 1,57 | 1572 | 1431 | 6.3 |
| 2,07 | 2072 | 1931 | 8.1 |
| 2,57 | 2572 | 2431 | 9.9 |
| 3,07 | 3072 | 2931 | 11.8 |
| 4,14 | 4144 | 4003 | 16.5 |

| Α | LB | П | [7 | М | 0 | D | U | l |
|---|----|---|----|---|---|---|---|---|
| | | | | | | | | |

Tube ledger in accordance with Z-8.22-906 M710-B113_ABM

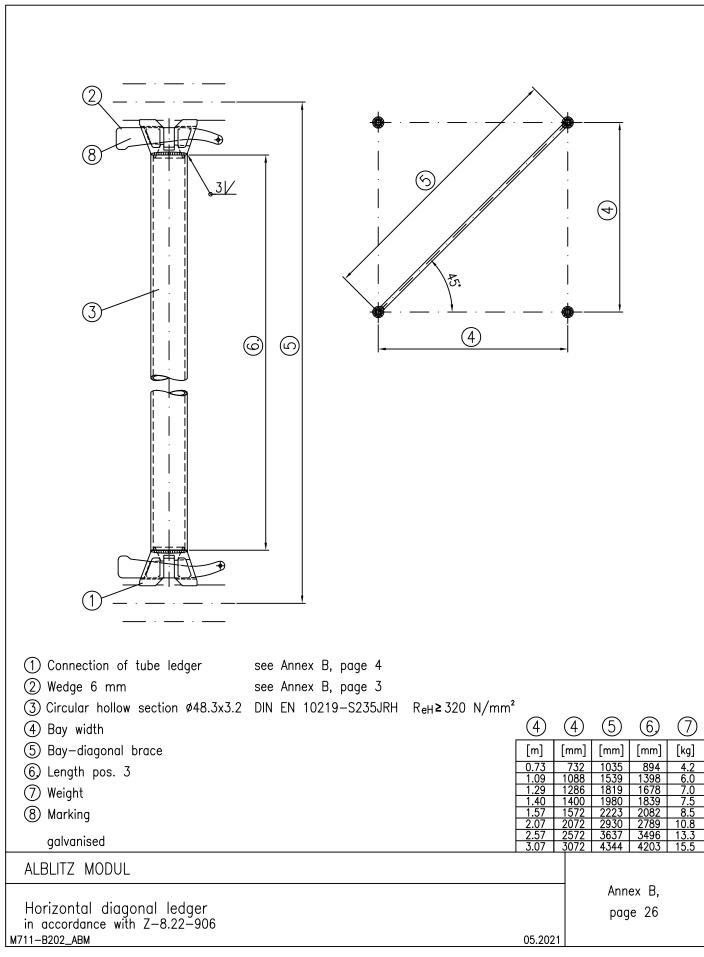
Annex B, page 25

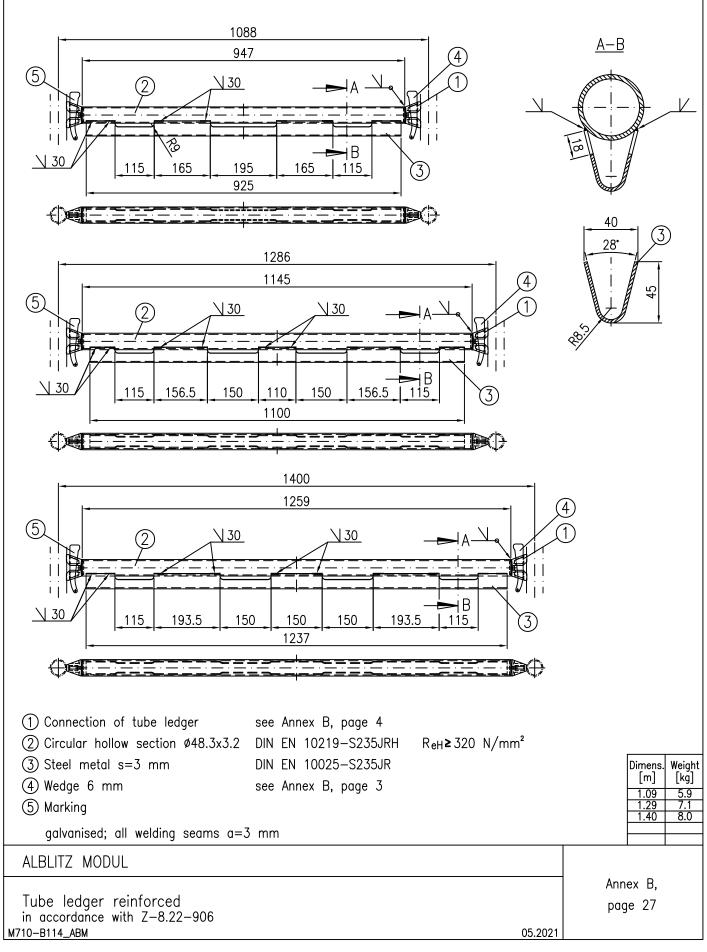
05.2021

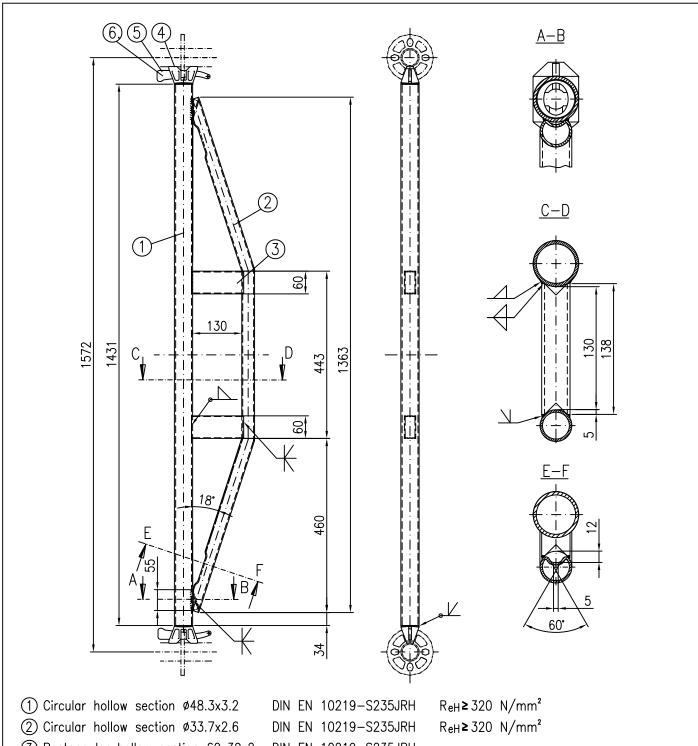
[m]

[mm]

[mm]







3 Rectangular hollow section 60x30x2

galvanised; all welding seams a=3 mm

- 4 Connection of tube ledger
- (5) Wedge 6 mm
- (6) Marking

DIN EN 10219-S235JRH

see Annex B, page 4

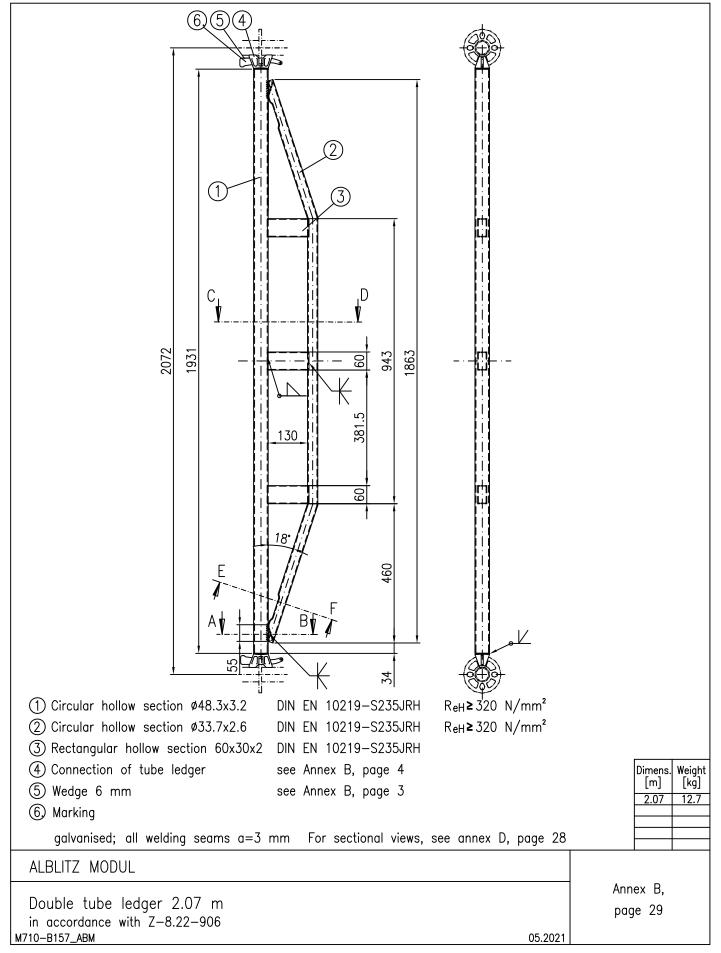
see Annex B, page 3

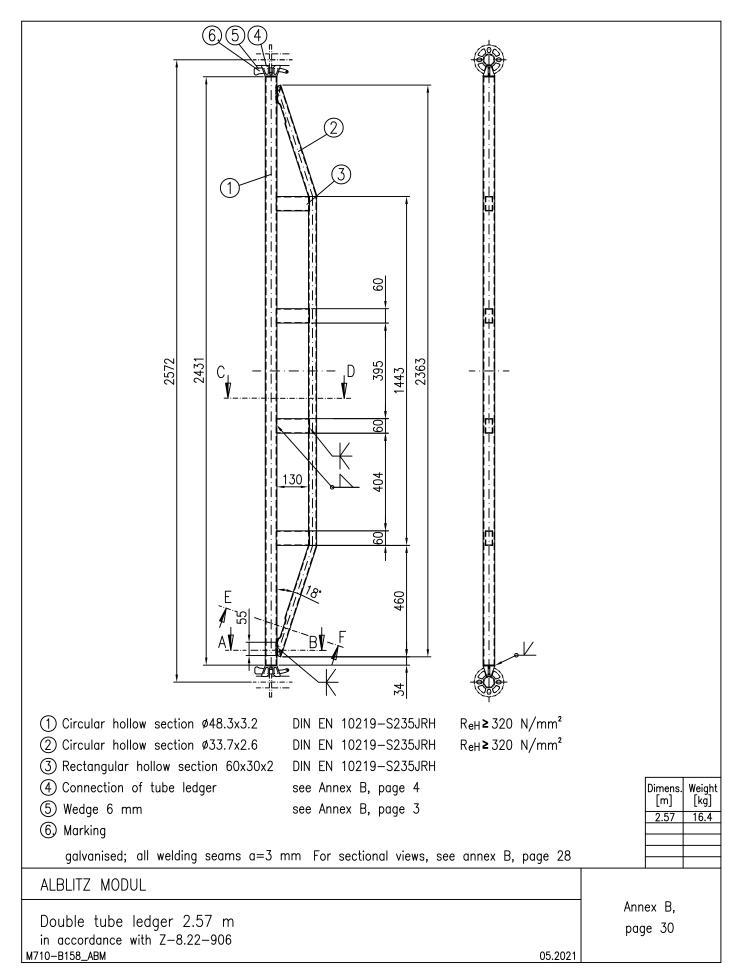
| t |
|---|
| |
| |
|] |
| |

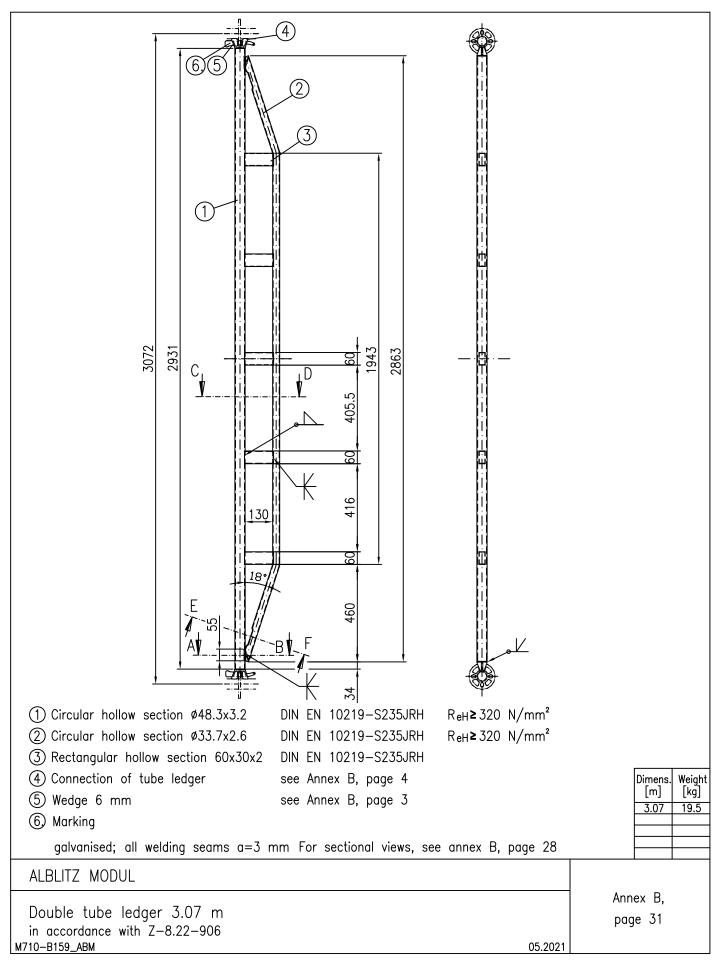
ALBLITZ MODUL

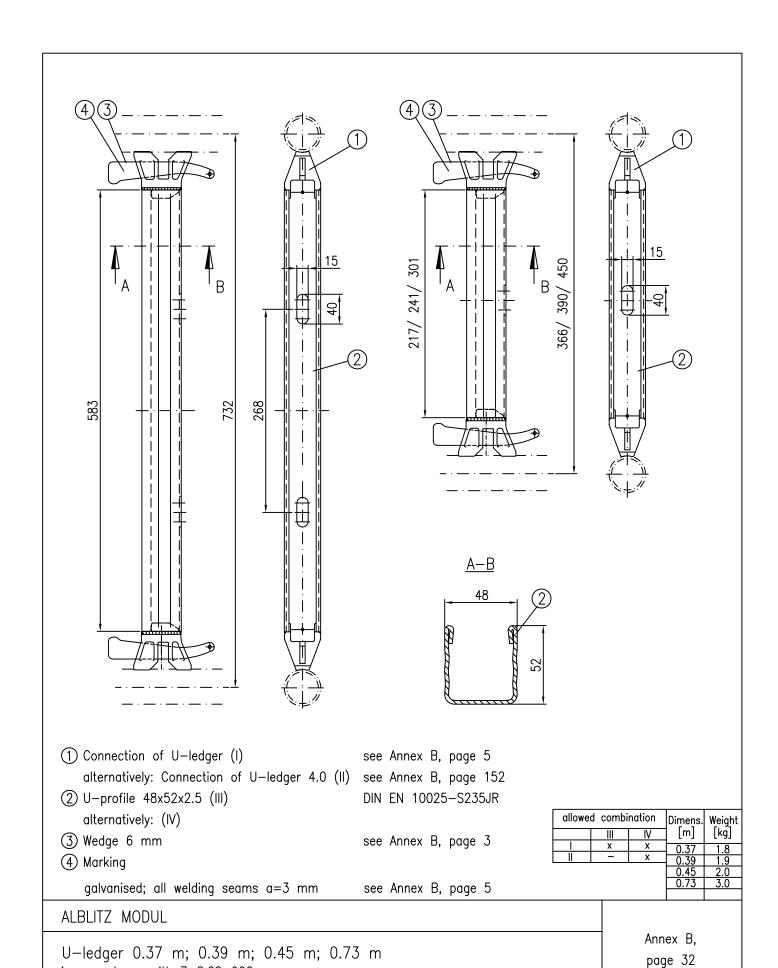
Double tube ledger 1.57 m in accordance with Z-8.22-906 M710-B156_ABM

Annex B, page 28



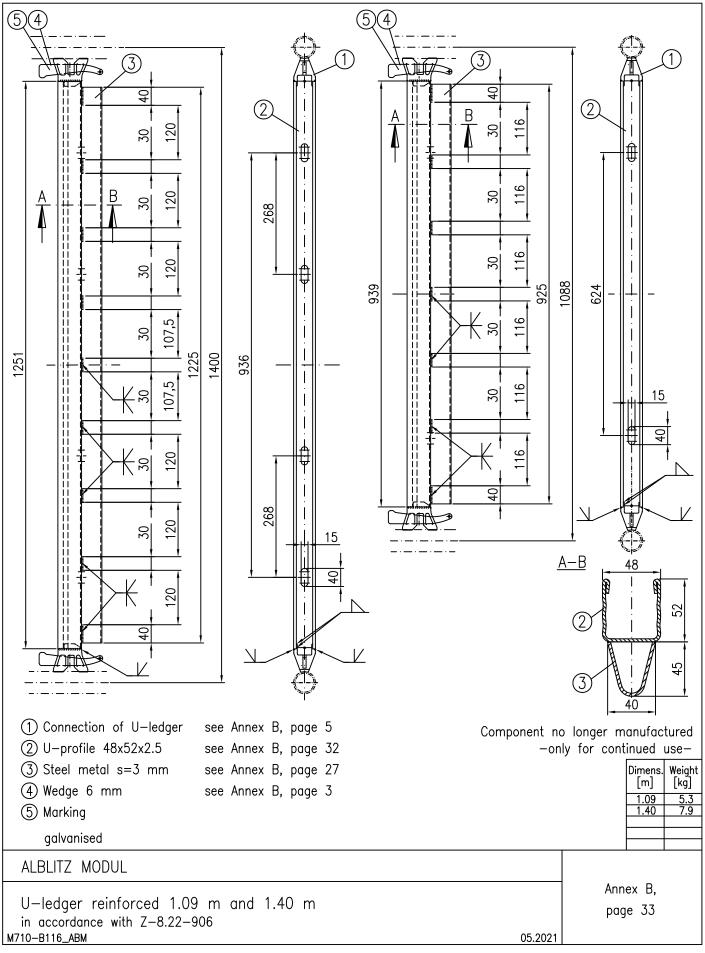


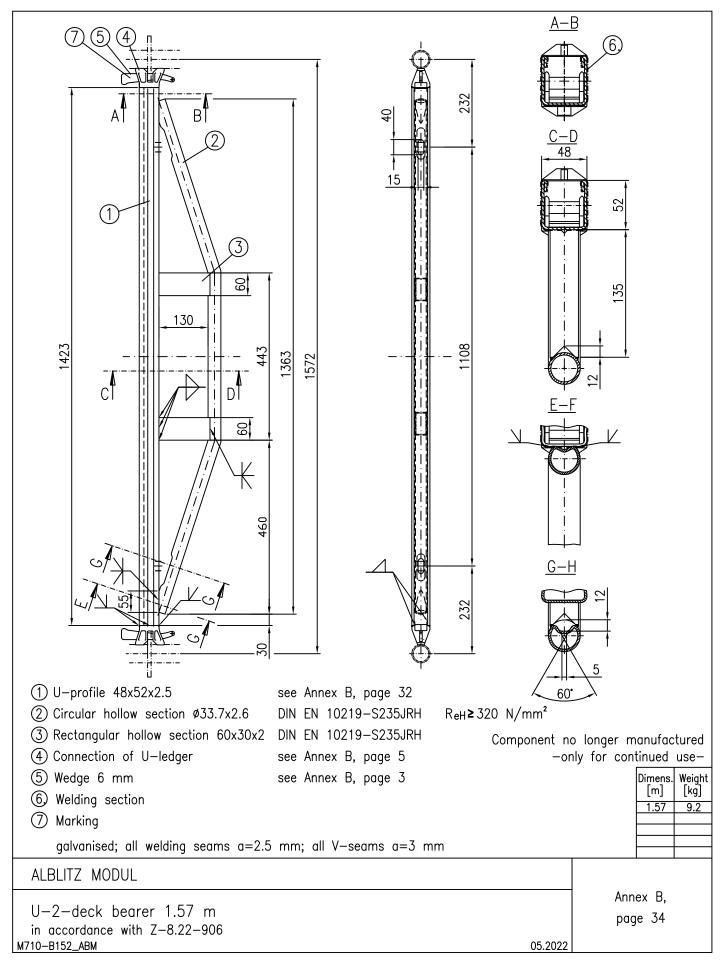


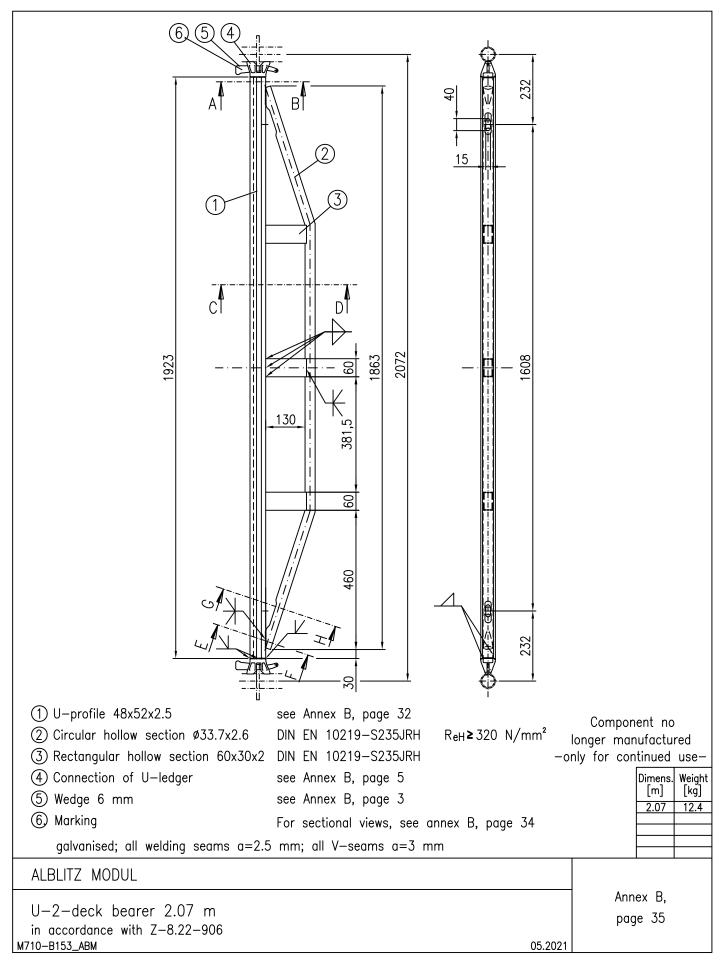


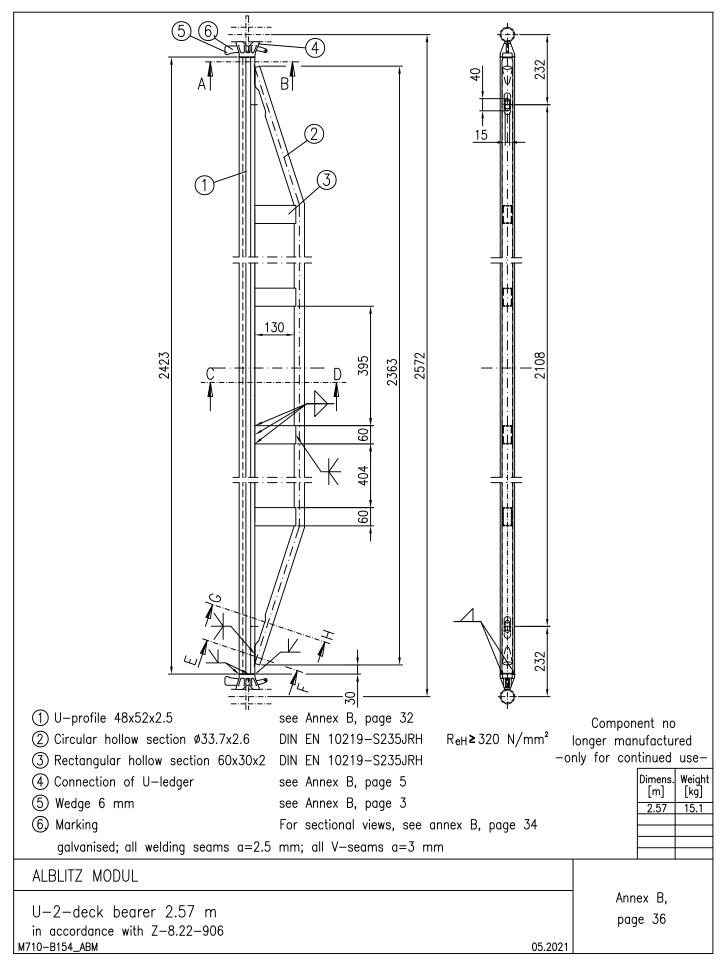
M710-B115_ABM

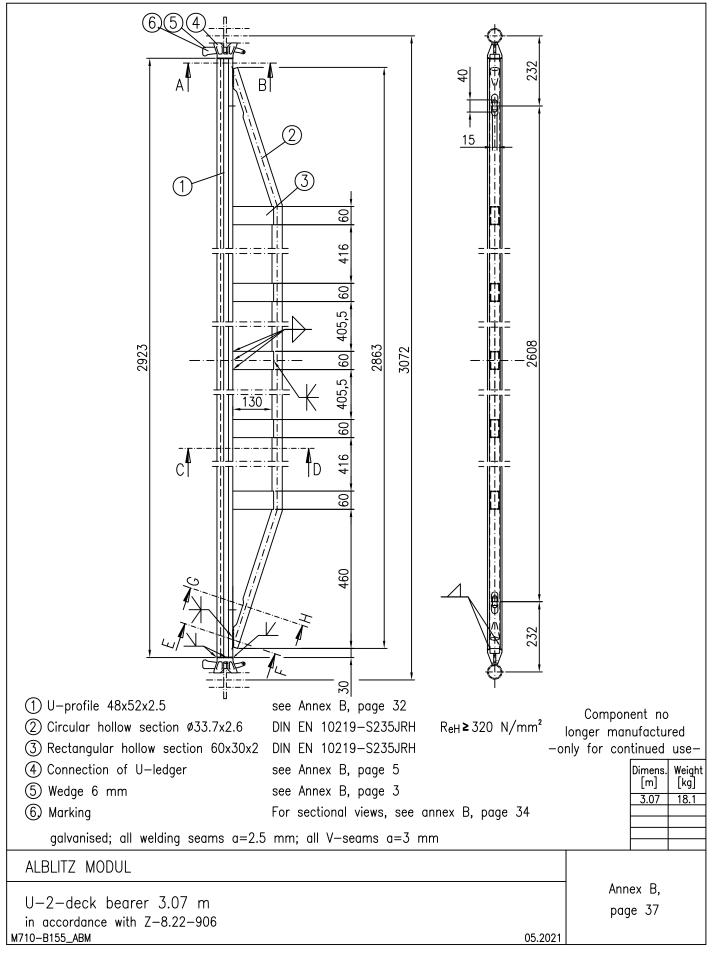
in accordance with Z-8.22-906





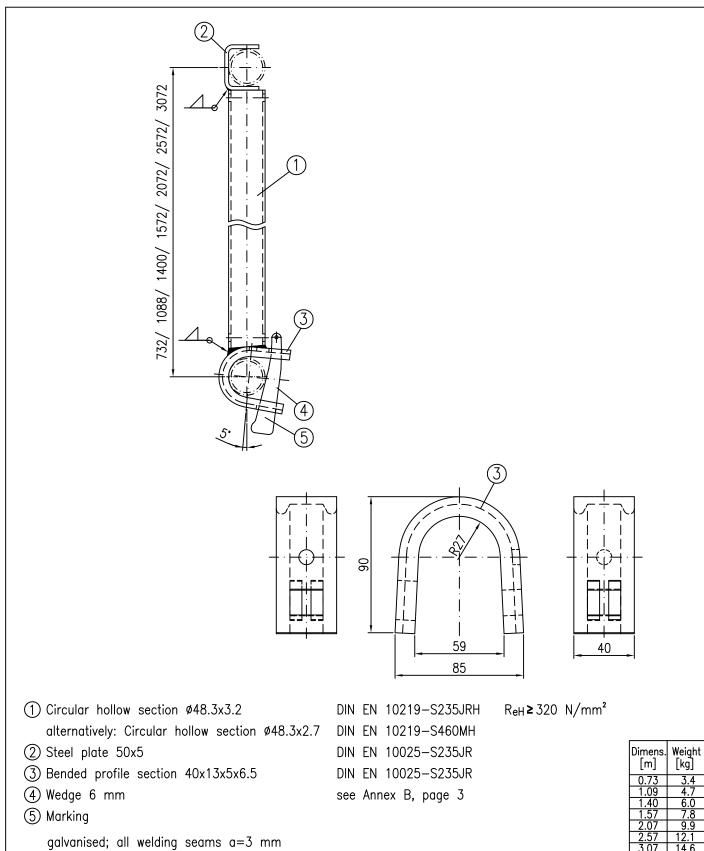






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| ALBLITZ MODUL Blank page | Annex B, page 38 |

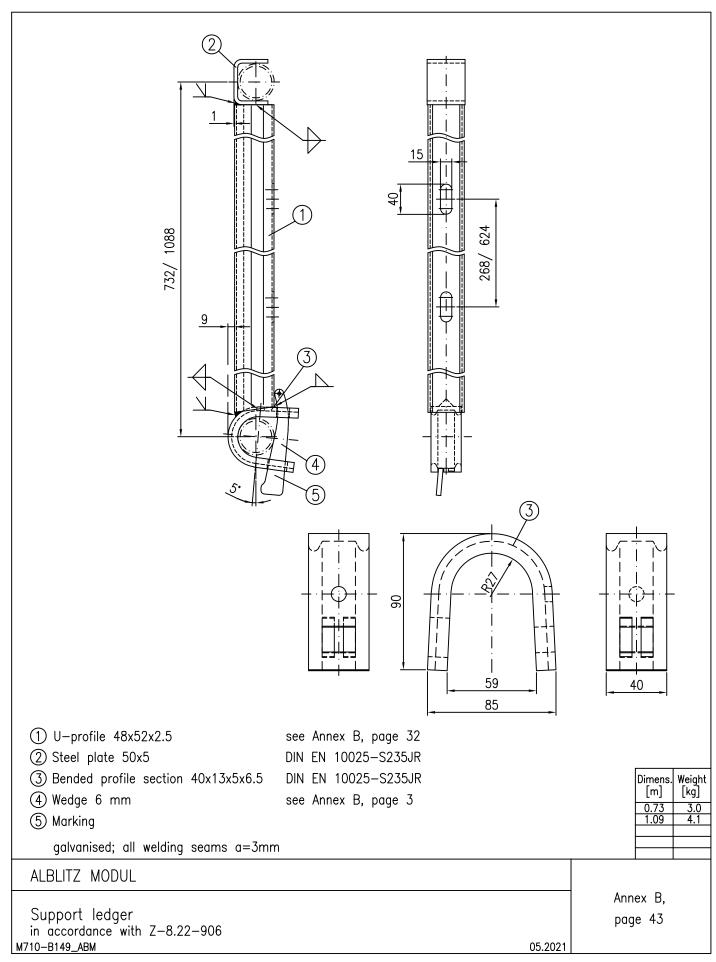
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|---------------------------|---------------------|
| ALBLITZ MODUL Blank page | Annex B, page 39 |
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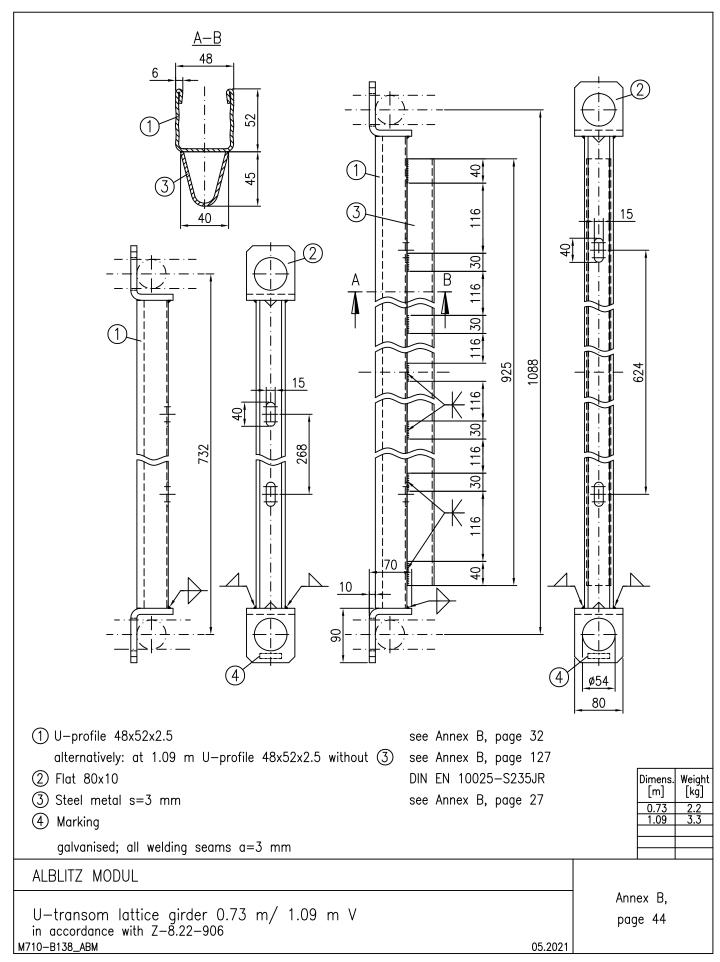


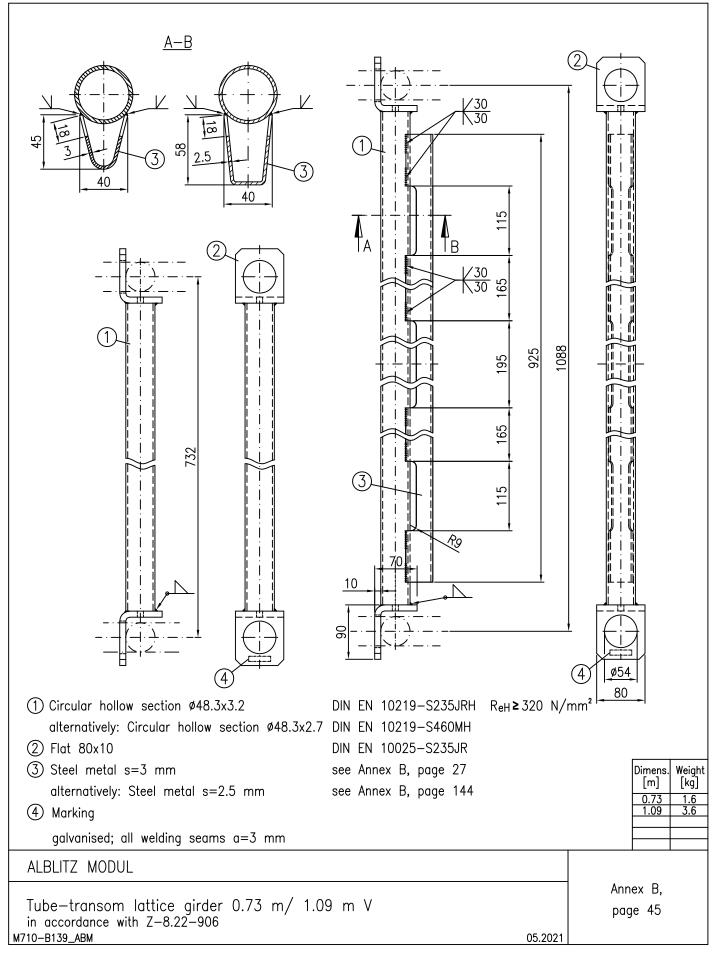
| | 0.07 | 11.0 |
|--|---------------------|------|
| ALBLITZ MODUL | | |
| Support ledger with tube fixture in accordance with Z-8.22-906 | Annex B, page 40 | |
| M710-B146_ABM 05.2021 | | |

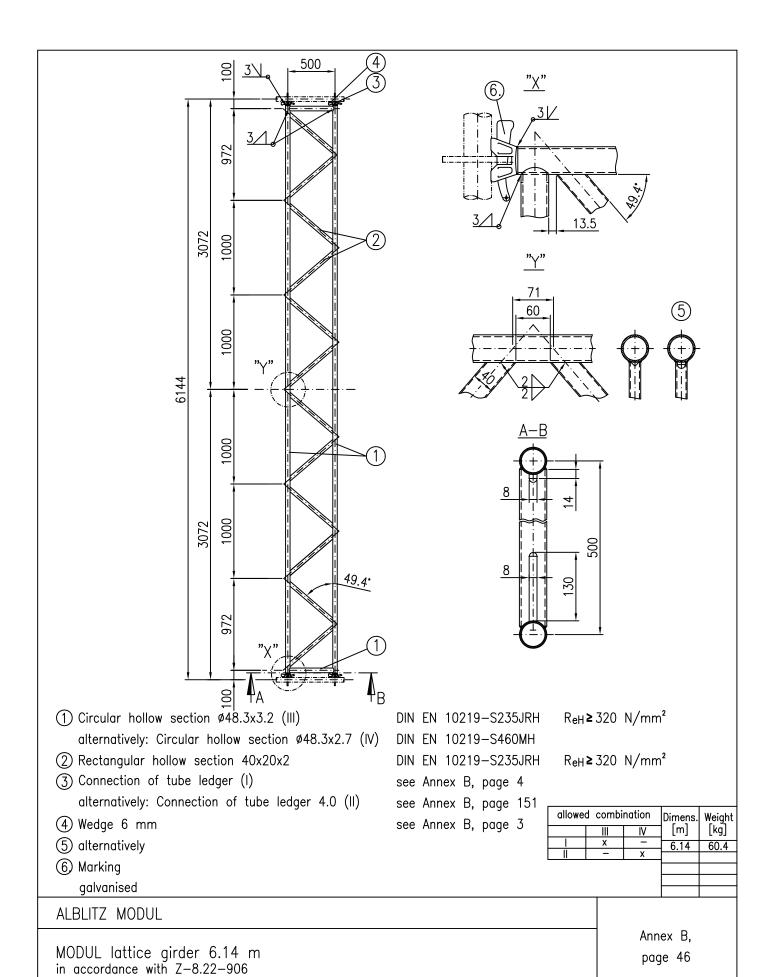
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| ALBLITZ MODUL Blank page | Annex B, page 41 |

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| ALBLITZ MODUL Blank page | Annex B, page 42 |

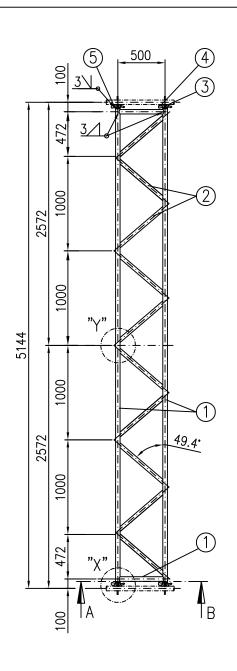


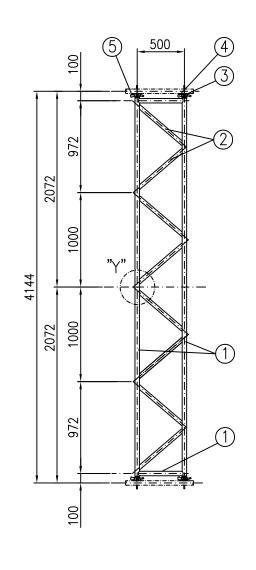






M710-B133_ABM





- (1) Circular hollow section Ø48.3x3.2 (III) alternatively: Circular hollow section Ø48.3x2.7 (IV) DIN EN 10219-S460MH
- (2) Rectangular hollow section 40x20x2
- (3) Connection of tube ledger (I) alternatively: Connection of tube ledger 4.0 (II)
- (4) Wedge 6 mm
- (5) Marking galvanised

DIN EN 10219-S235JRH R_{eH}≥320 N/mm²

DIN EN 10219-S235JRH R_{eH}≥320 N/mm²

see Annex B, page 4

see Annex B, page 151

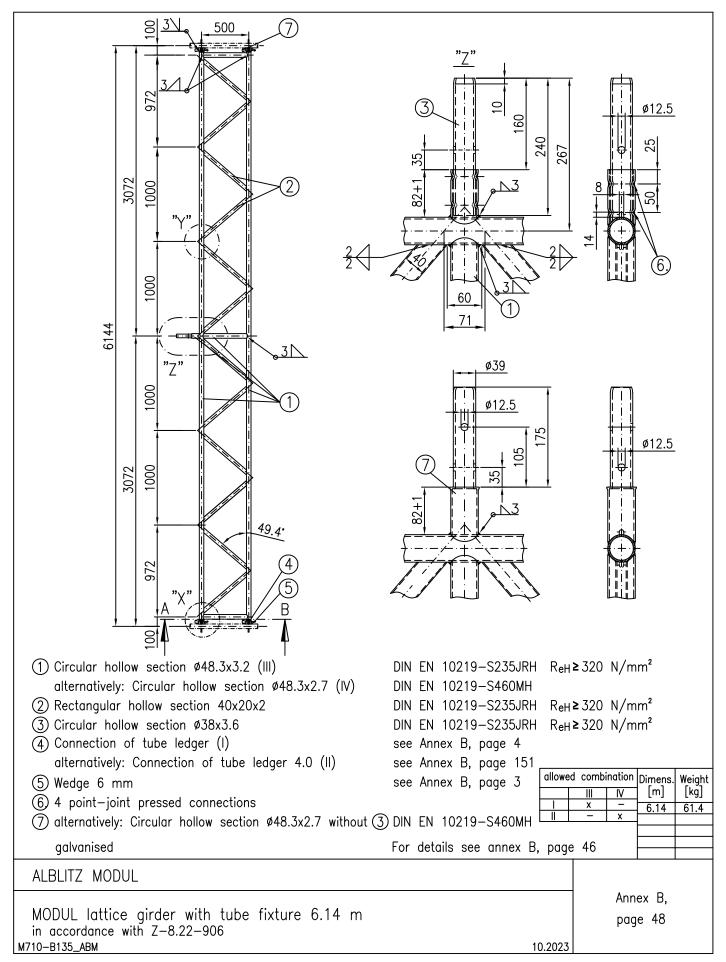
see Annex B, page 3

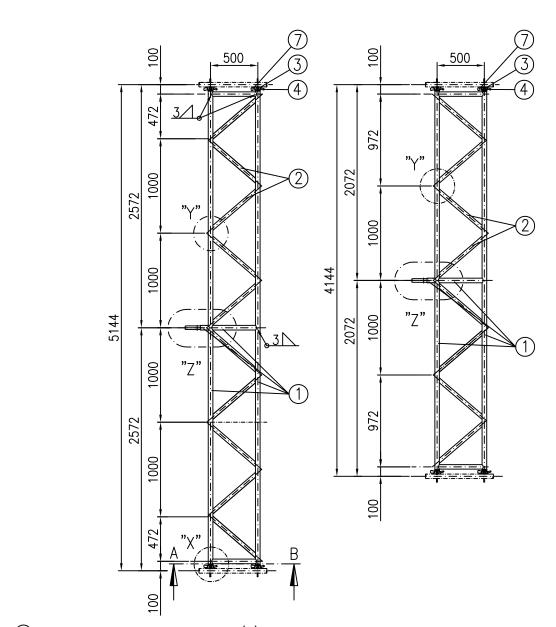
| allowed combination | | Dimens. | Weight | |
|---------------------|-----|---------|--------|------|
| | III | IV |] [m] | [kg] |
| | X | _ | 4.14 | 46.0 |
| | _ | Х | 5.14 | 50.0 |
| | | | | |

For details, please refer to annex B, page 46

ALBLITZ MODUL

MODUL lattice girder 4.14 m/ 5.14 m in accordance with Z-8.22-906 M710-B134_ABM





- (1) Circular hollow section Ø48.3x3.2 (III) alternatively: Circular hollow section Ø48.3x2.7 (IV) DIN EN 10219-S460MH
- (2) Rectangular hollow section 40x20x2
- (3) Circular hollow section Ø38x3.6
- (4) Connection of tube ledger (1) alternatively: Connection of tube ledger 4.0 (II)
- (5) Wedge 6 mm
- 6 4 point-joint pressed connections
- (7) Marking

galvanised

- DIN EN 10219-S235JRH R_{eH}≥320N/mm²
- DIN EN 10219-S2

DIN EN 10219-S2

see Annex B, page see Annex B, page

see Annex B, page 3

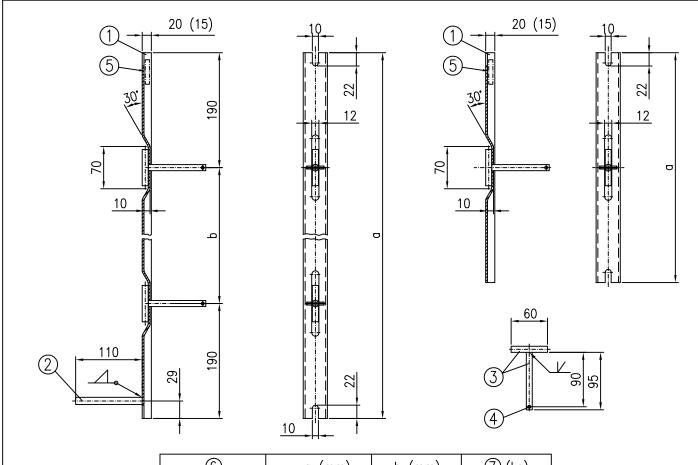
| R _{eH} ≥320N/mm² |
|---------------------------|
| R _{eH} ≥320N/mm² |
| |
| |
| |

| allowed combination | | Dimens. | Weight | |
|---------------------|-----|---------|--------|------|
| | III | IV | [[m] | [kg] |
| | X | _ | 4.14 | 47.0 |
| II | _ | Х | 5.14 | 51.0 |
| | | | | |

For details see annex B, pages 46 and 48

ALBLITZ MODUL

MODUL lattice girder with tube fixture 4.14 m/ 5.14 m in accordance with Z-8.22-906M710-B136_ABM



| 6 | a (mm) | b (mm) | ⑦ (kg) |
|------|--------|--------|--------|
| 390 | 306 | 1 | 0.7 |
| 450 | 366 | 1 | 0.8 |
| 732 | 648 | 268 | 1.4 |
| 1036 | 952 | 572 | 1.8 |
| 1088 | 1004 | 624 | 1.9 |
| 1286 | 1202 | 822 | 2.2 |
| 1400 | 1316 | 936 | 2.5 |
| 1572 | 1488 | 1108 | 2.9 |
| 2072 | 1988 | 1608 | 3.9 |
| 2572 | 2488 | 2108 | 4.8 |
| 3072 | 2988 | 2608 | 5.4 |

① U-profile 20 (15)x40x15x3

DIN EN 10025-S235JR

② Round Ø12 (alternatively for toeboard ALFIX)

DIN EN 10025-S235JR

(3) Round Ø10

DIN EN 10025-S235JR

4 Grooved cylindrical pin

DIN EN ISO 8740-5x30-steel-galvanised

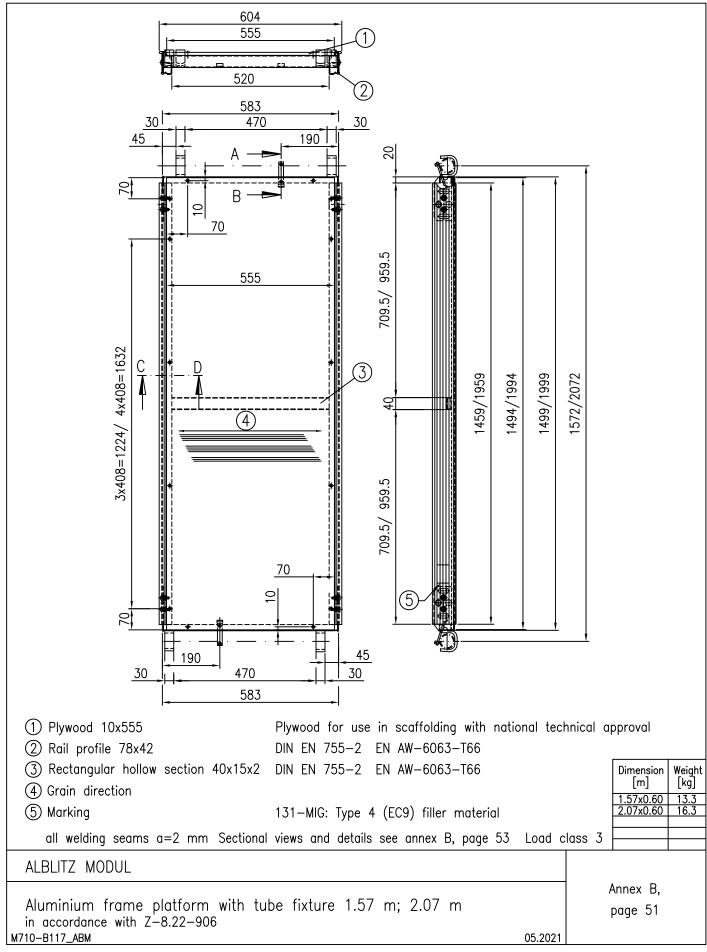
(5) Marking

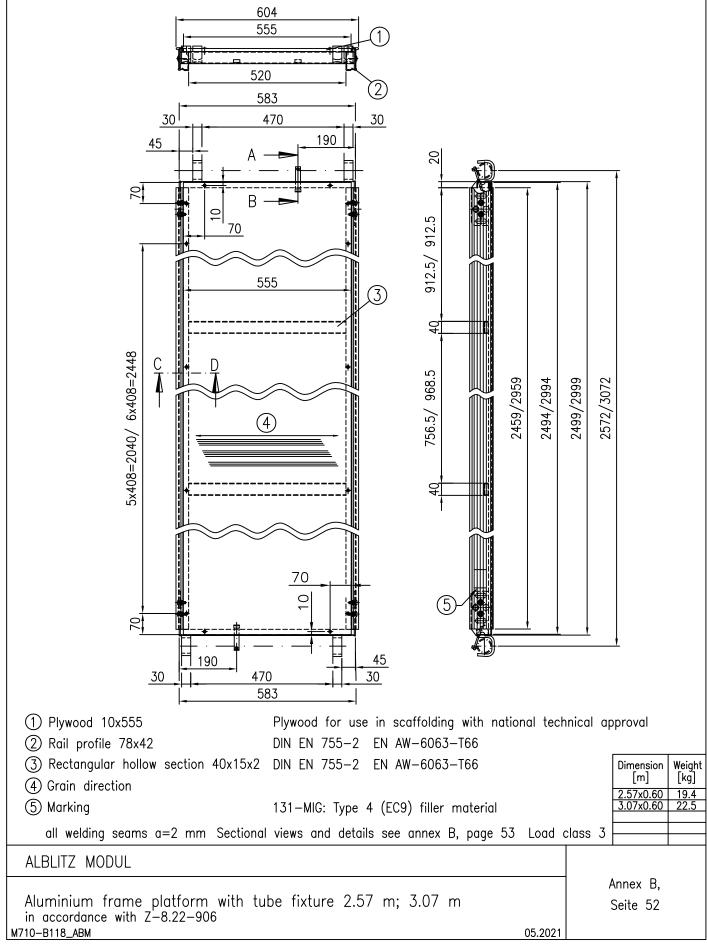
6 Length L (mm)

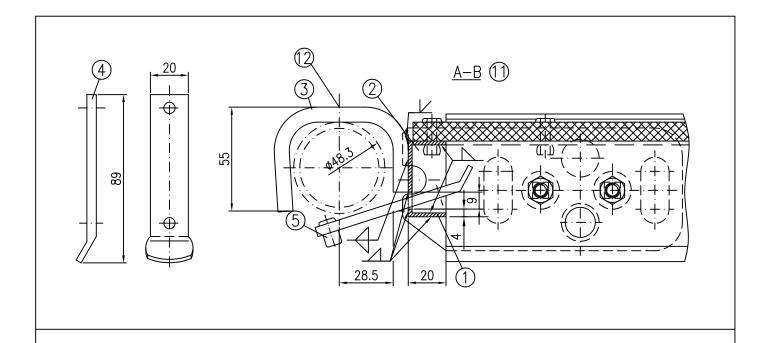
galvanised

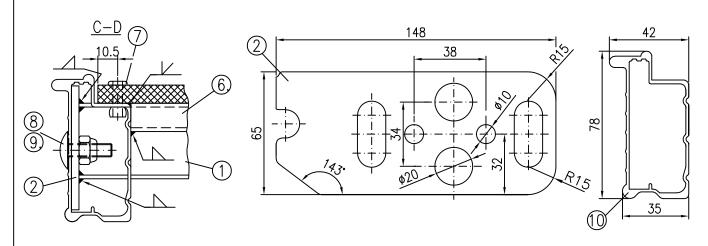
| ALBLITZ | MODUL |
|---------|-------|
| | |

 $\begin{array}{lll} \text{MODUL lift-off preventer} \\ \text{in accordance with Z-8.22-906} \end{array}$ M710-B130_ABM









- 1 U-profile 40x20x2
- ② Suspension lug steel metal 4x65x148
- 3 Steel plate 30x8
- 4 Lift-off prevention tube fixture flat 20x5
- (5) Blind rivet Ø4.8x16/ 5x16
- 6 Rectangular hollow section 40x15x2
- (7) Blind rivet \$5x20
- (8) Round-head screw
- (9) Hexagon nut, self-locking
- (10) Rail profile 78x42
- (1) Head piece, hot-dip galvanised
- (12) Marking

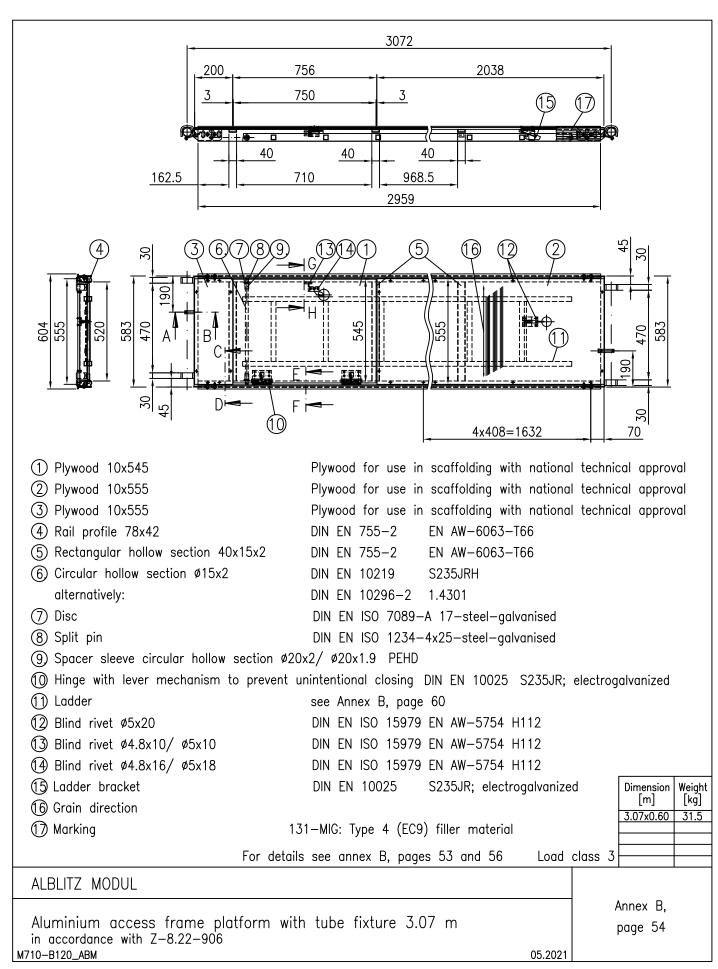
- DIN EN 10025-S235JR
- DIN EN 10025-S235JR
- DIN EN 10025-S355J2 alternatively: DIN EN 10149-S355MC

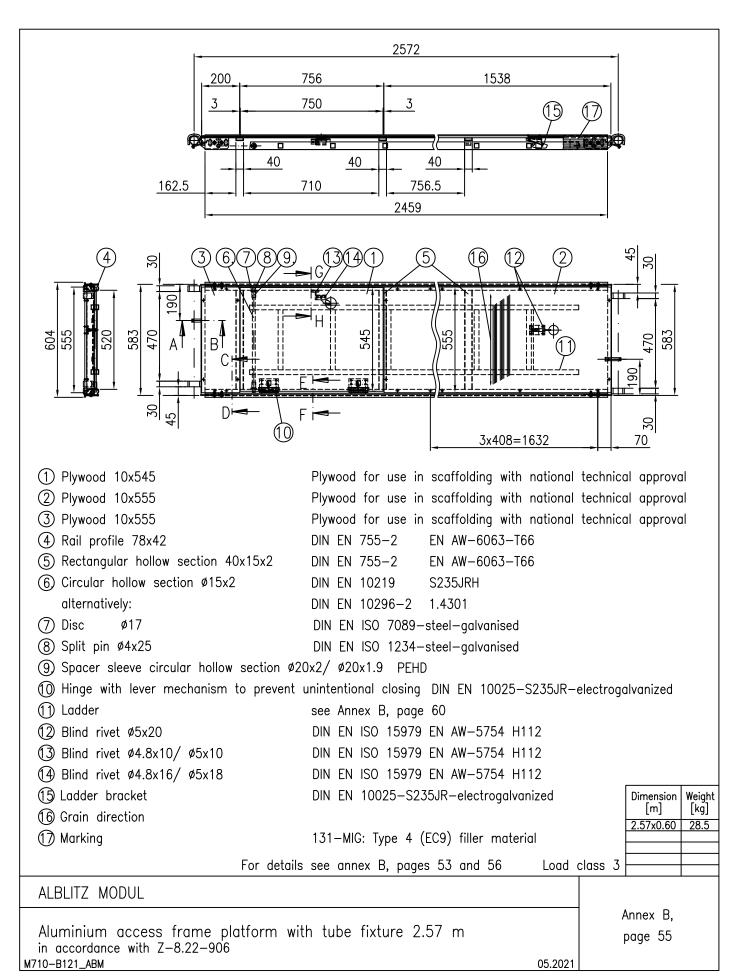
EN AW-6063-T66

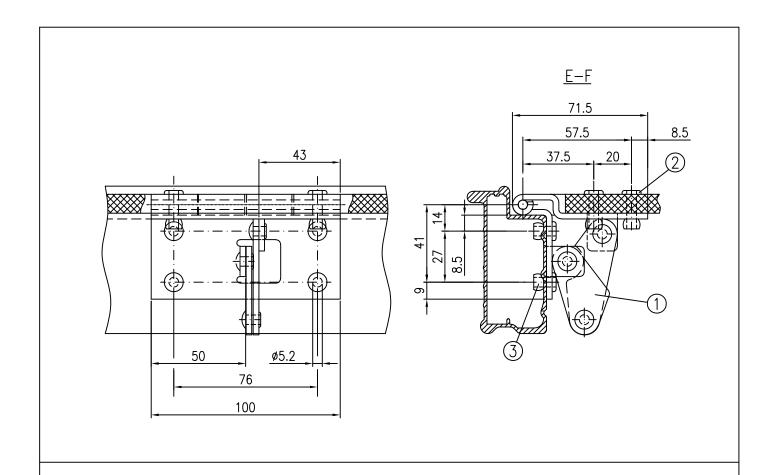
- DIN EN 10025-S235JR hot-dip galvanised
- DIN EN ISO 15979 EN AW-5754 H112
- DIN EN 755-2
- DIN EN ISO 15979 EN AW-5754 H112
- DIN 603-M8x20-8.8-galvanised
- DIN EN ISO 10511-M8-8-galvanised
- DIN EN 755-2 EN AW-6063-T66

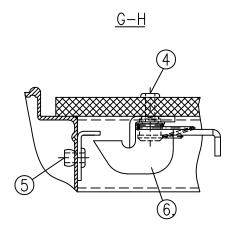
ALBLITZ MODUL

Detailed view of aluminium frame platform with tube fixture in accordance with Z-8.22-906 M710-B119_ABM









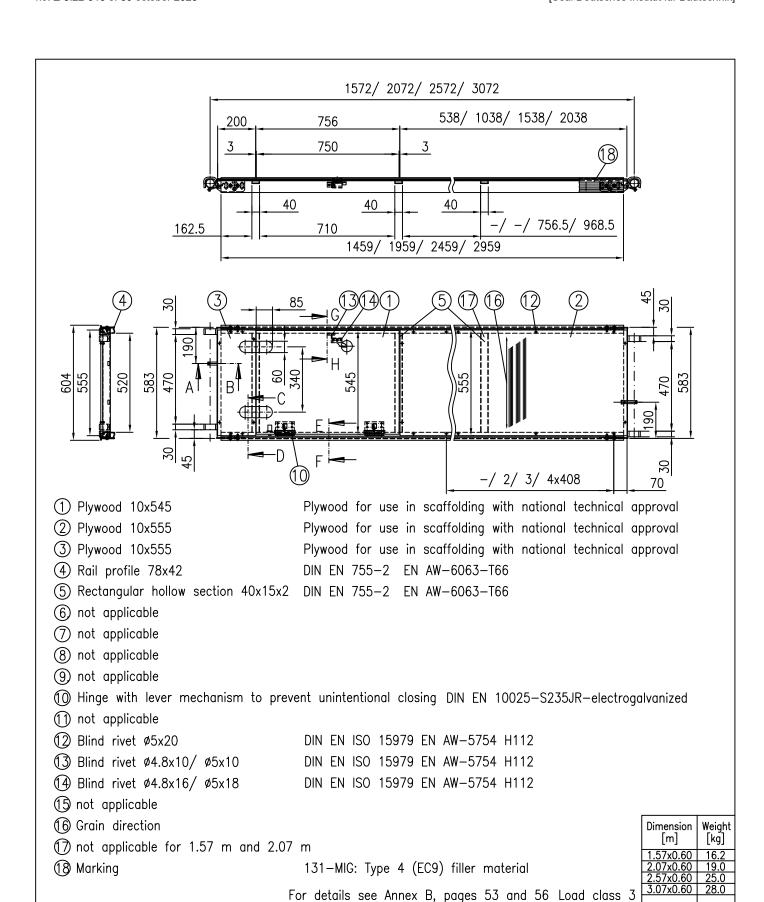
- 1 Hinge with lever mechanism to prevent unintentional closing DIN EN 10025—S235JR—electrogalvanized
- 2 Blind rivet ø5x20
- (3) Blind rivet Ø5x12
- 4 Blind rivet Ø4.8x16/Ø5x18
- (5) Blind rivet \(\psi 4.8 \times 10 / \phi 5 \times 10 \)
- (6) Ledger

DIN EN ISO 15979 EN AW-5754 H112

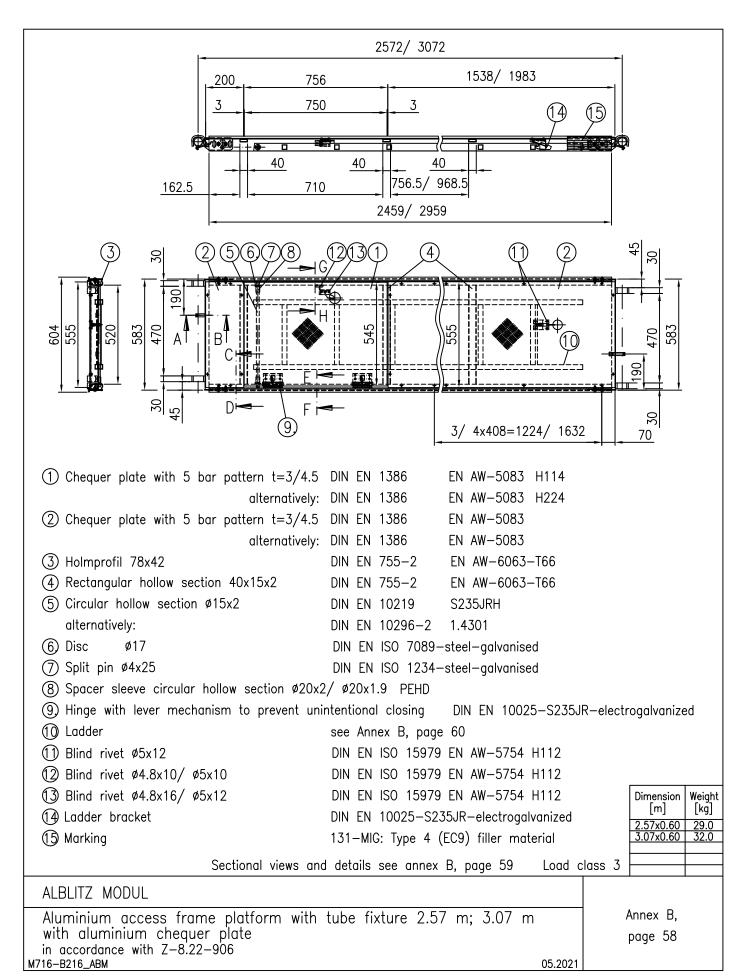
DIN EN 10025-S235JR-electrogalvanized

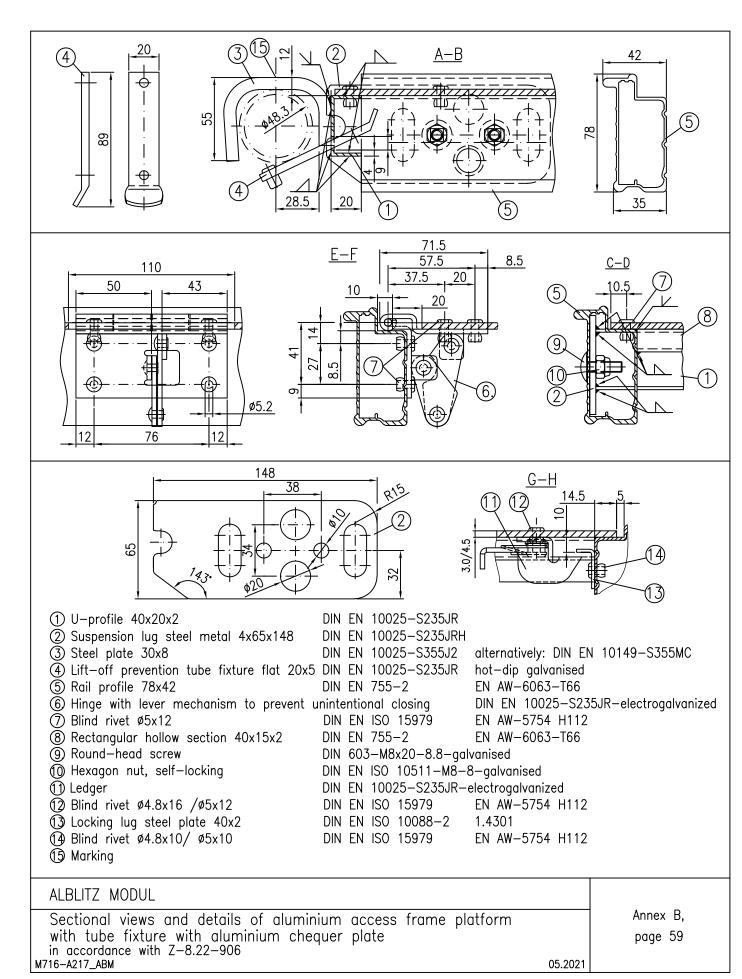
ALBLITZ MODUL

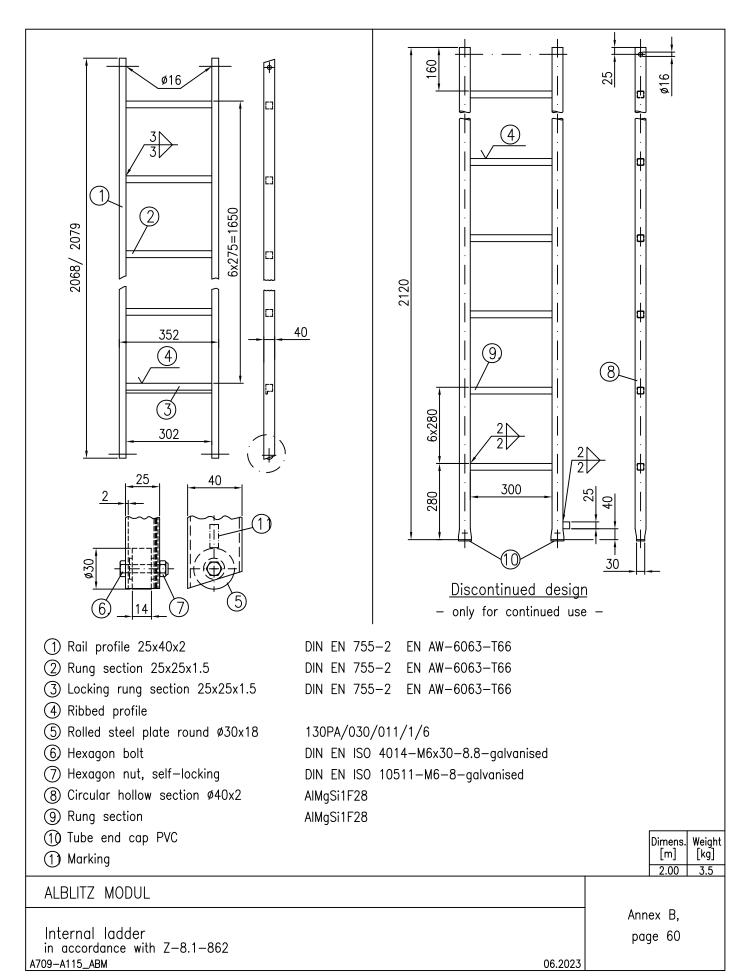
Detailed view of aluminium access frame platform with tube fixture in accordance with Z-8.22-906 M710-B122_ABM

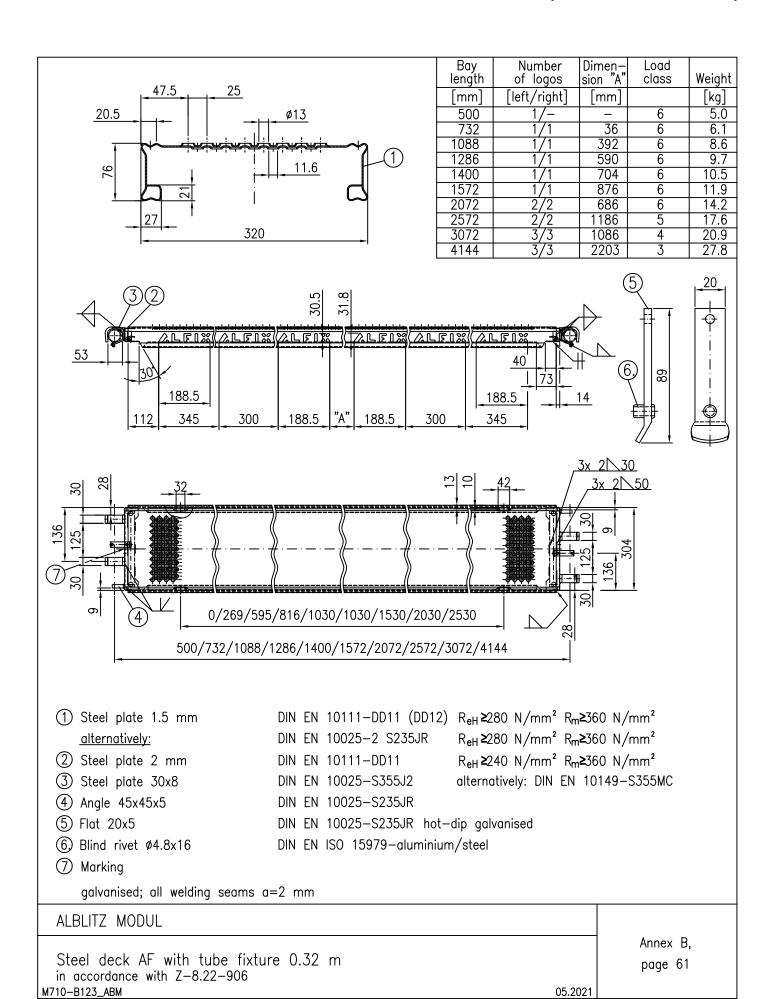


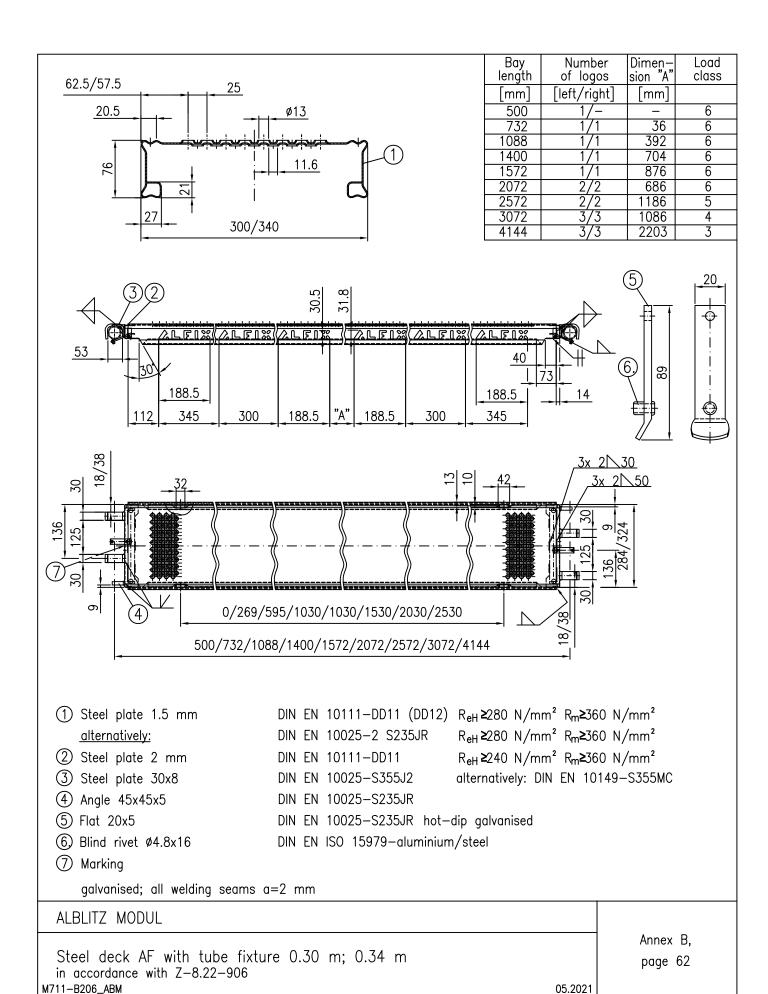
| • • • | |
|--|---------------------|
| ALBLITZ MODUL | |
| Aluminium access frame platform with tube fixture 1.57 m—3.07 m without ladder in accordance with Z—8.22—906 | Annex B, page 57 |
| M716—R215_ARM 05 2021 | |

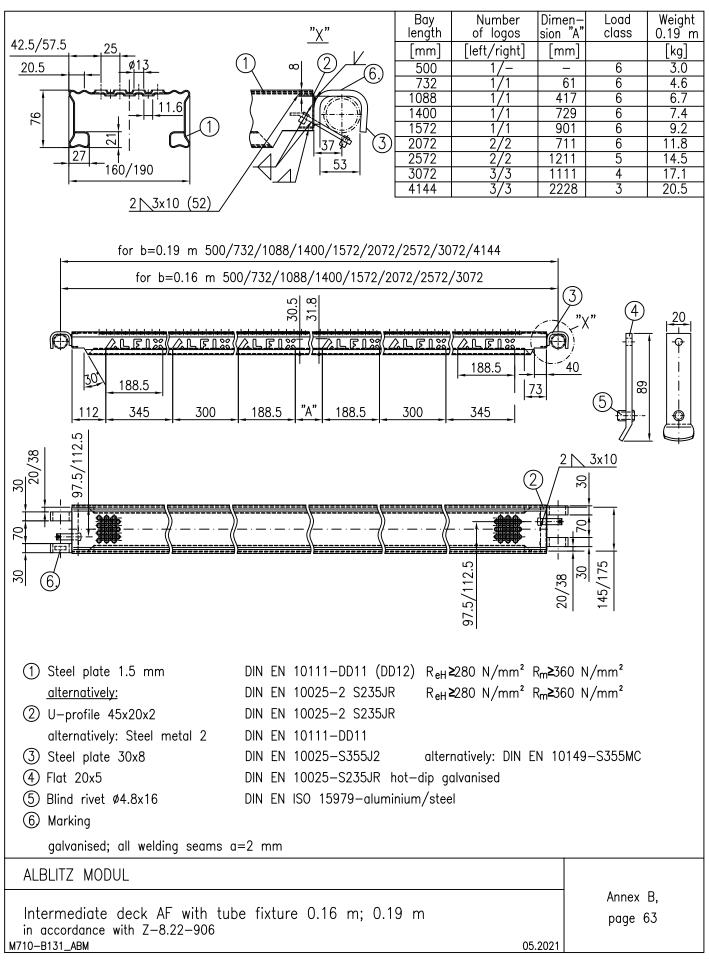


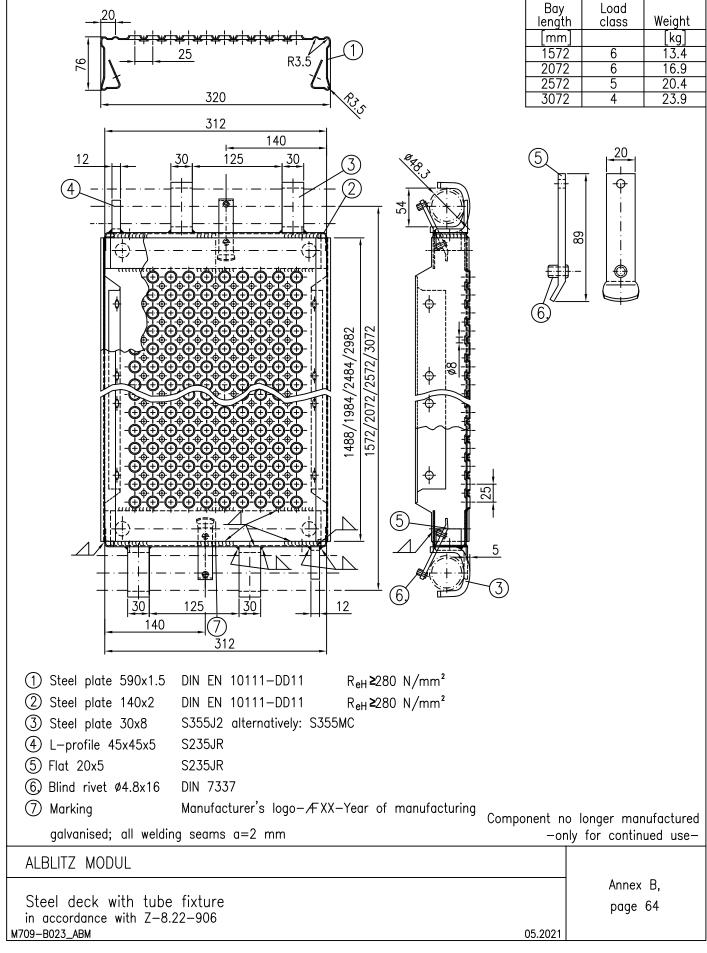


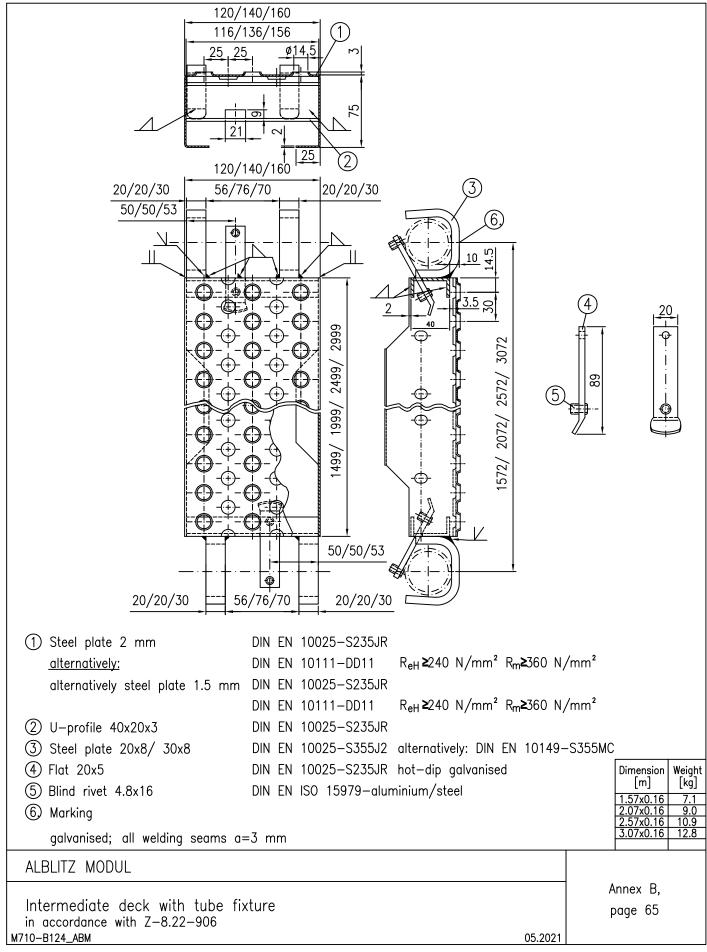


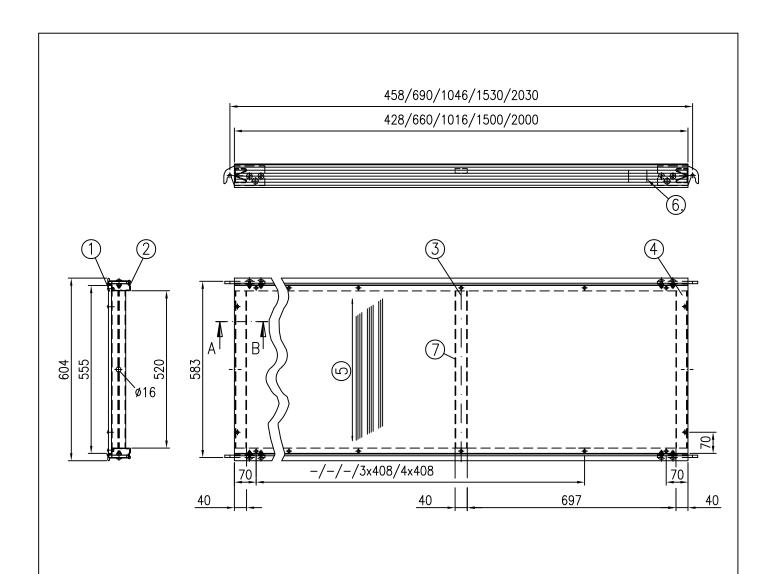












- 1) Plywood 10x555
- (2) Rail profile 78x42
- 3 Rectangular hollow section 40x15x2
- (4) Grip profile; bridging deck thickness 2 mm DIN EN 755-2 EN AW-6063-T66
- (5) Grain direction
- (6) Marking
- 7) not applicable for 0.50 m 1.09 m for 0.73 m and 1.09 m blind rivets in centre

131—MIG: Type 4 (EC9) filler material

For details see annex A, page 68

DIN EN 755-2 EN AW-6063-T66

DIN EN 755-2 EN AW-6063-T66

Load class 3

Plywood for use in scaffolding with national technical approval

-only for continued use
Dimens. Weight [kg]

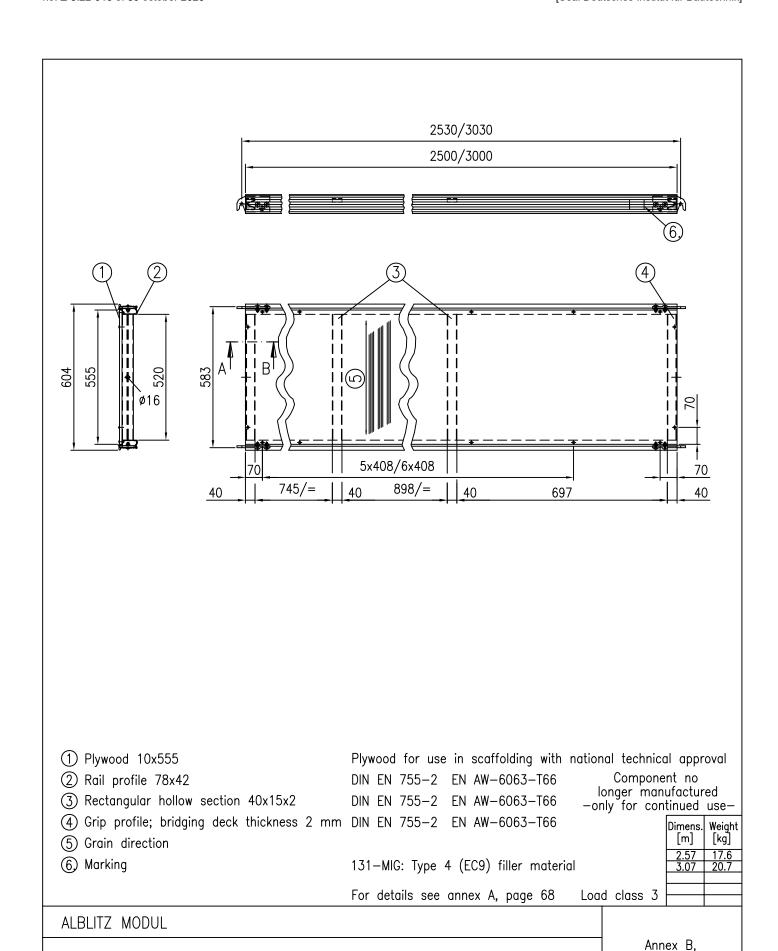
0.50 4.8

Component no

longer manufactured

| · · · | 2.07 | _ |
|---|---------------------|---|
| ALBLITZ MODUL | | |
| Aluminium frame platform with plywood 0.50 m — 2.07 m in accordance with Z-8.22-906 | Annex B, page 66 | |

A713-A230_ABM

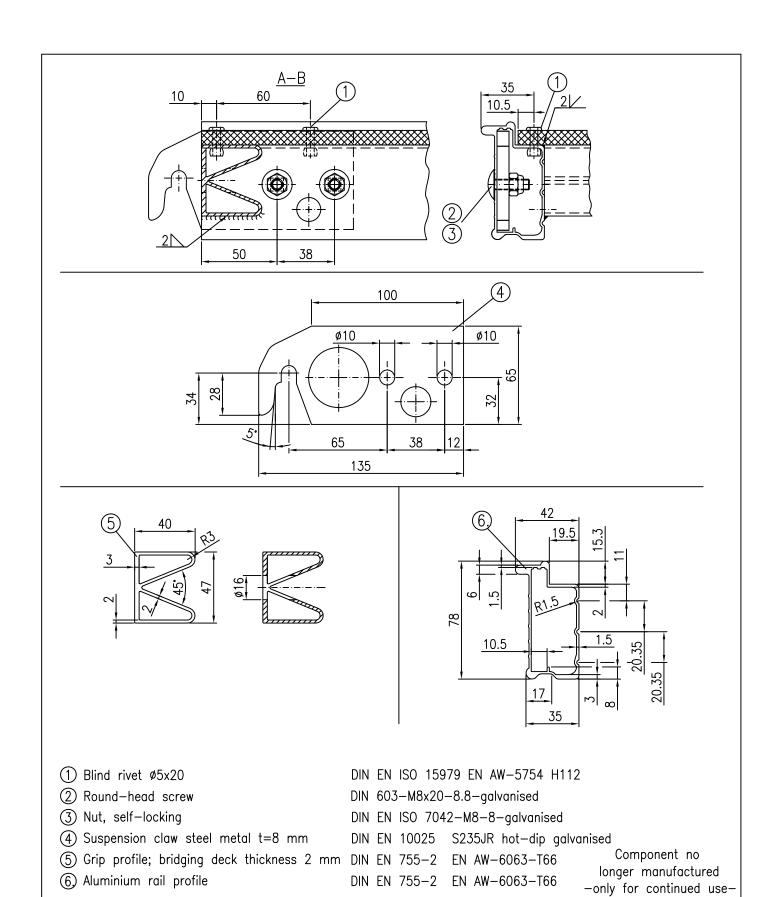


A713-A231_ABM

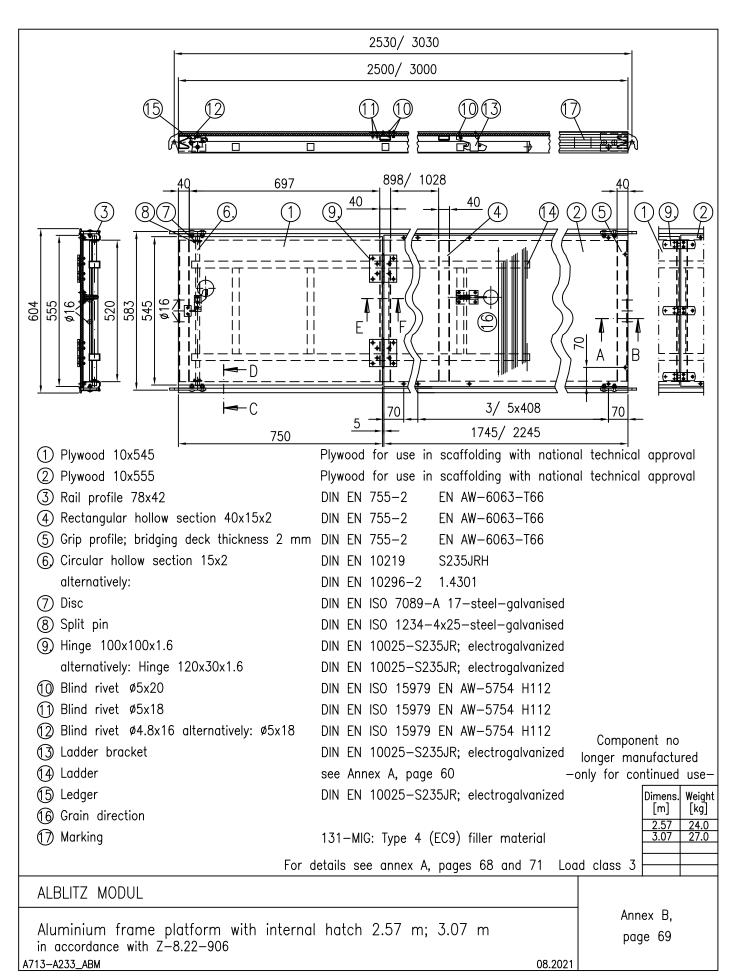
in accordance with Z-8.22-906

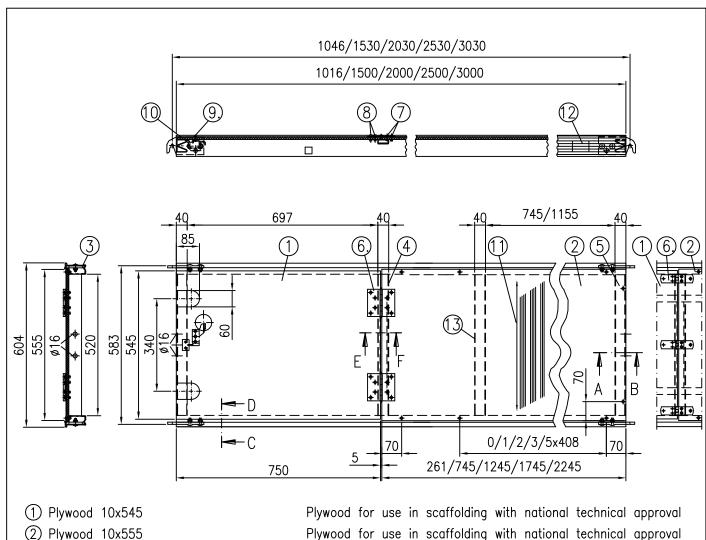
Aluminium frame platform with plywood 2.57 m; 3.07 m

page 67



| ALBLITZ MODUL | |
|---|---------------------|
| Detailed view of aluminium frame platform in accordance with Z-8.22-906 | Annex B, page 68 |
| A713-A232_ABM | 8.2021 |





- (2) Plywood 10x555
- (3) Rail profile 78x42
- (4) Rectangular hollow section 40x15x2
- (5) Grip profile; bridging deck thickness 2 mm DIN EN 755-2
- (6) Hinge 100x100x1.6 alternatively: Hinge 120x30x1.6
- (7) Blind rivet Ø5x20
- (8) Blind rivet Ø5x18
- (9) Blind rivet Ø4.8x16 alternatively: Ø5x18
- (10) Ledger
- (11) Grain direction
- (12) Marking
- (13) not applicable for 1.09 m 2.07 m

DIN EN 755-2 EN AW-6063-T66

DIN EN 755-2 EN AW-6063-T66

EN AW-6063-T66

DIN EN 10025-S235JR; electrogalvanized

DIN EN 10025-S235JR; electrogalvanized

DIN EN ISO 15979 EN AW-5754 H112

DIN EN ISO 15979 EN AW-5754 H112

DIN EN ISO 15979 EN AW-5754 H112

Component no DIN EN 10025-S235JR; electrogalvanized longer manufactured

-only for continued use-

| 131-MIG: | Type | 4 | (EC9) | filler | material |
|----------|------|---|-------|--------|----------|
|----------|------|---|-------|--------|----------|

For details see annex A, pages 68 and 71 Load class 3 $\frac{2.57}{3.07}$ $\frac{22.0}{25.5}$

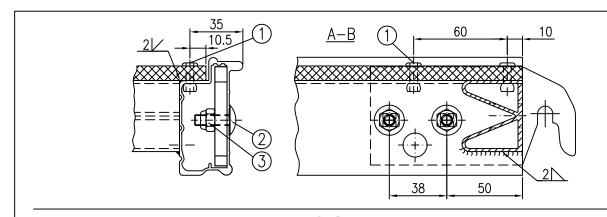
| Dimens. [m] | Weight [kg] | | | |
|----------------|----------------|--|--|--|
| 1.09 | 10.5 | | | |
| 1.57 | 13.0 | | | |
| 2.07 | 17.0 | | | |
| 2.57 | 22.0 | | | |
| 7.07 | OF E | | | |

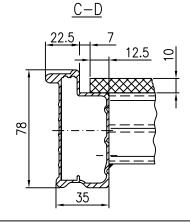
ALBLITZ MODUL

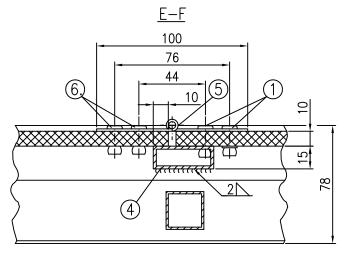
Aluminium frame platform with internal hatch 1.09 m - 3.07 m without ladder in accordance with Z-8.22-906

Annex B. page 70

A713-A236_ABM







- 1) Blind rivet Ø5x20
- (2) Round-head screw
- 3 Nut, self-locking
- 4 Rectangular hollow section 40x15x2 DIN EN 755-2
- (5) Hinge 100x100x1.6 alternatively: Hinge 120x30x1.6
- (6) Blind rivet Ø5x18

- DIN EN ISO 15979 EN AW-5754 H112
- DIN 603-M8x20-8.8-galvanised
- DIN EN ISO 7042-M8-8-galvanised
- DIN EN 755-2 EN AW-6063-T66
- DIN EN 10025-S235JR; electrogalvanized
- DIN EN 10025-S235JR; electrogalvanized
- DIN EN ISO 15979 EN AW-5754 H112

Component no longer manufactured —only for continued use—

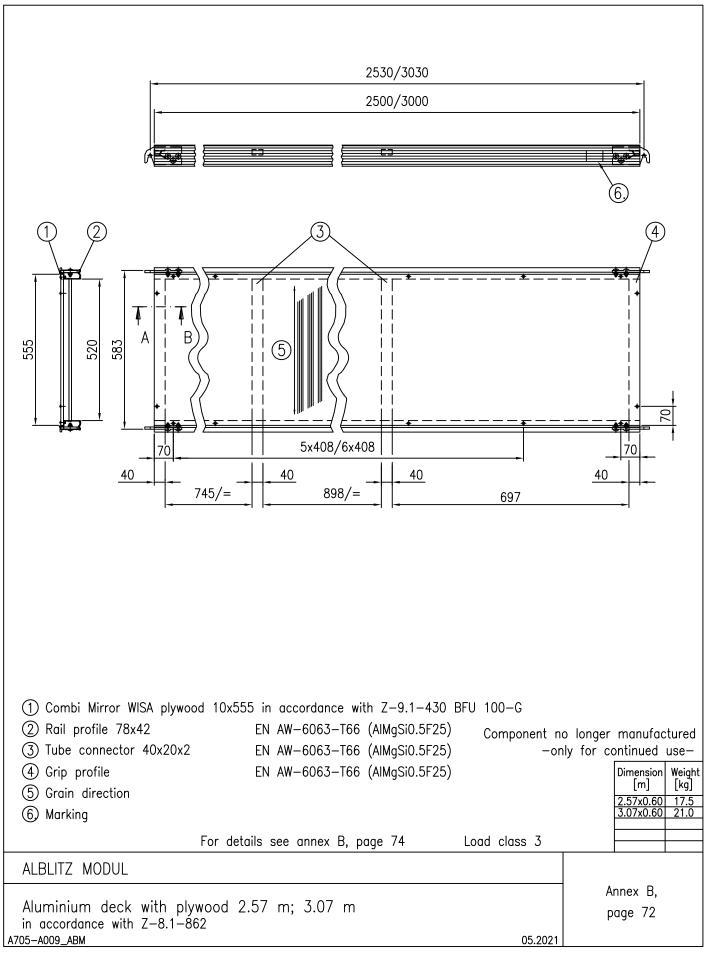
Annex B,

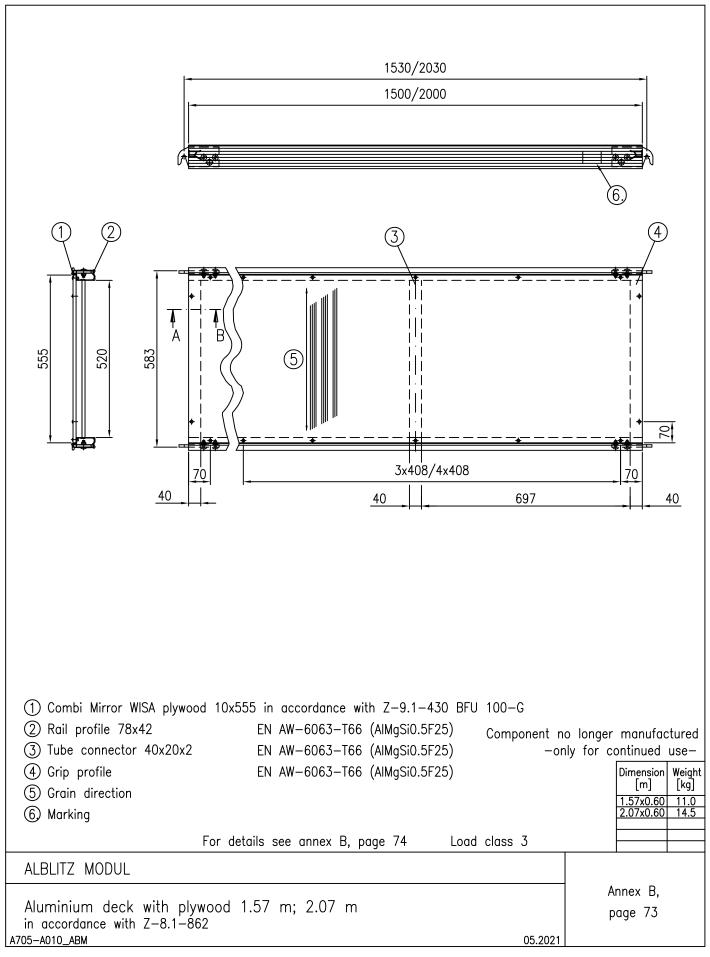
ALBLITZ MODUL

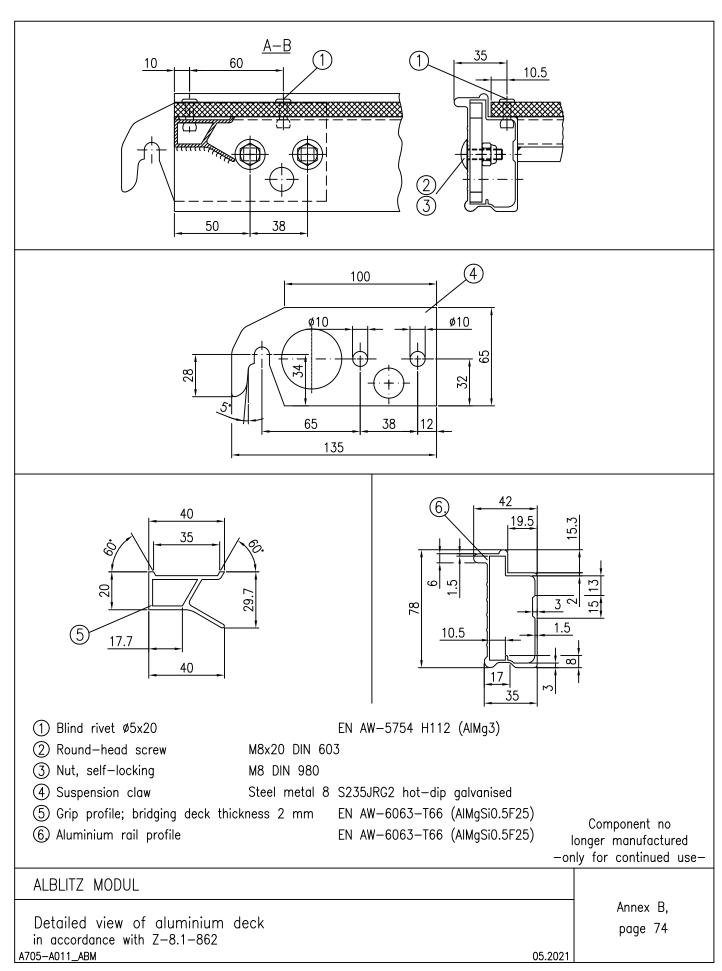
Sectional views of aluminium frame platform with internal hatch in accordance with Z-8.22-906
A713-A235_ABM

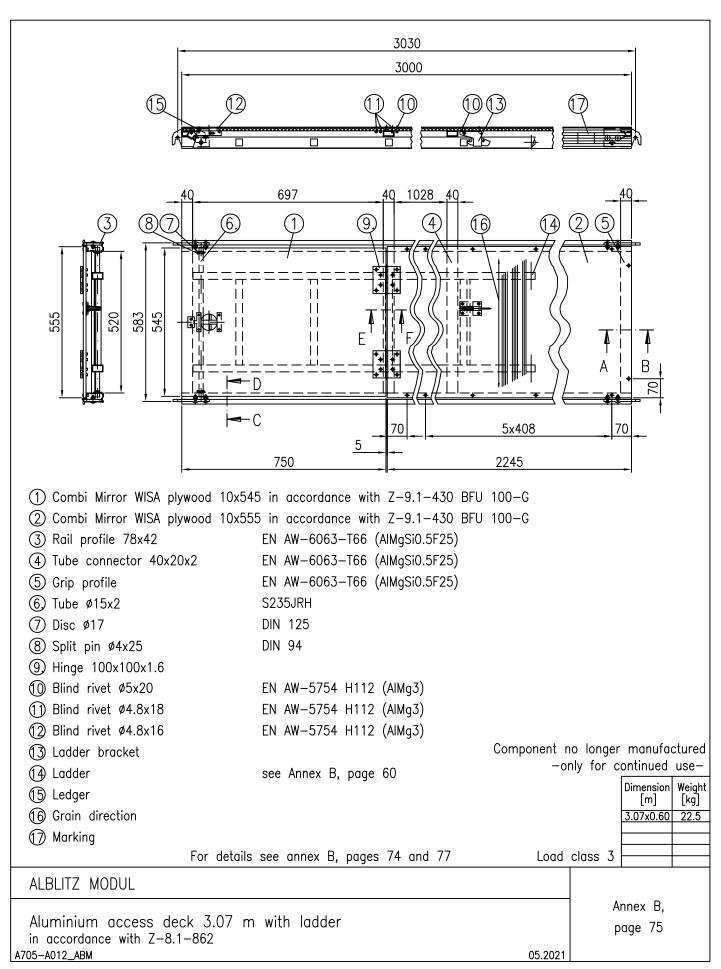
page 71

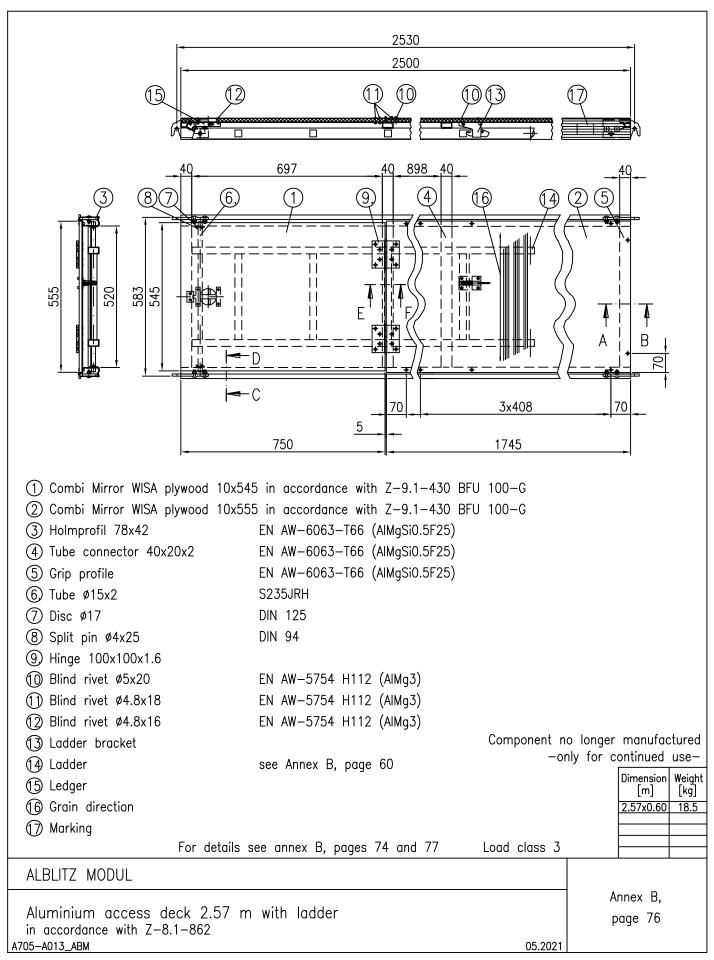
8.2021

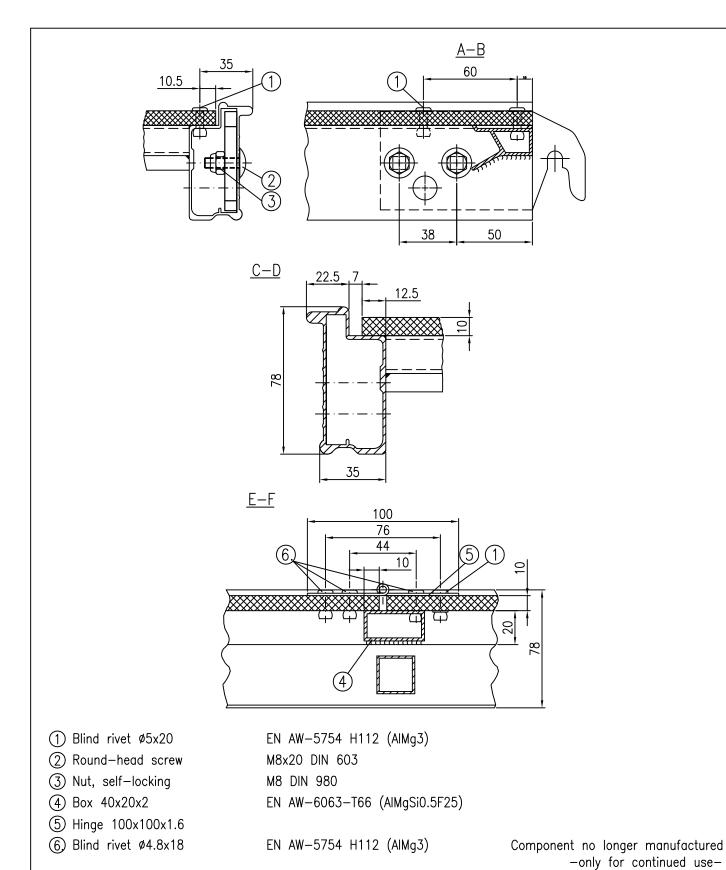




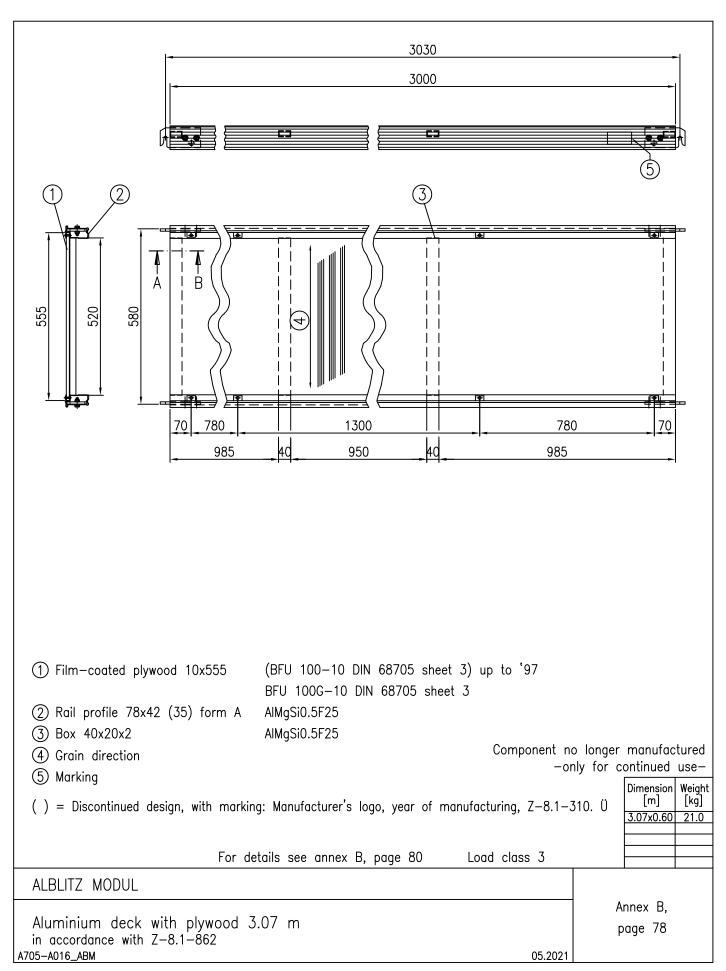


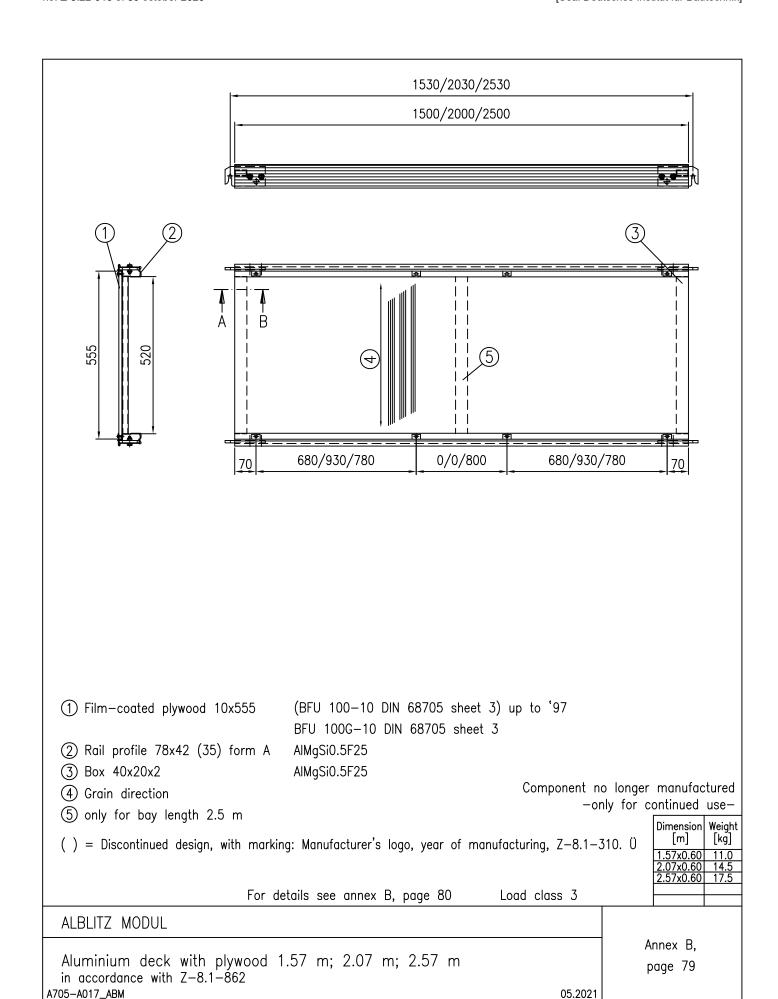


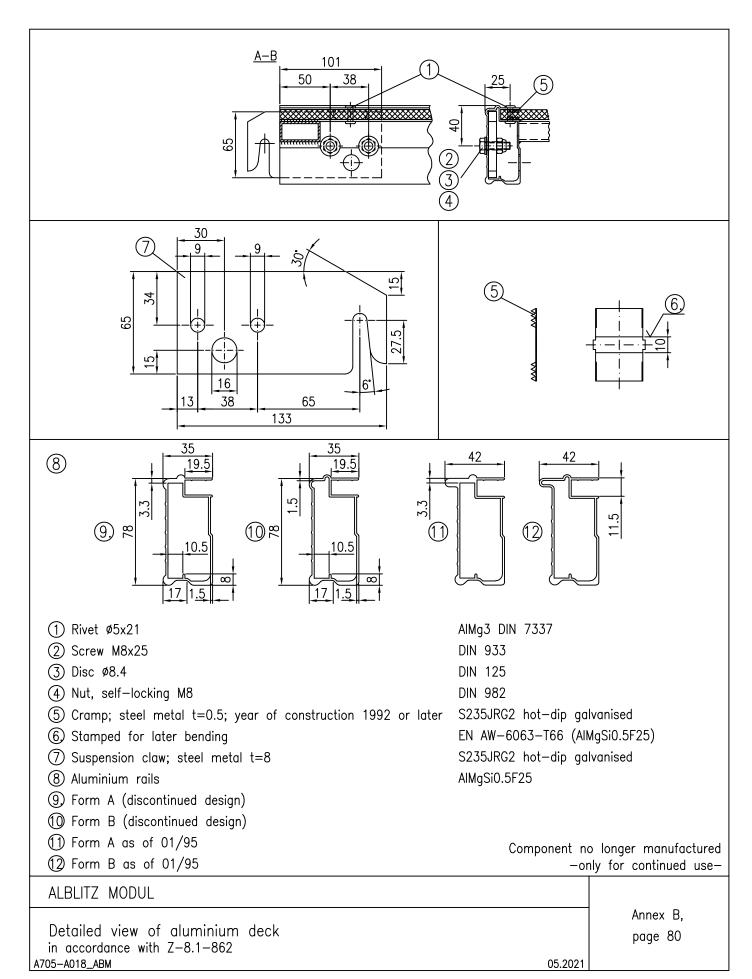


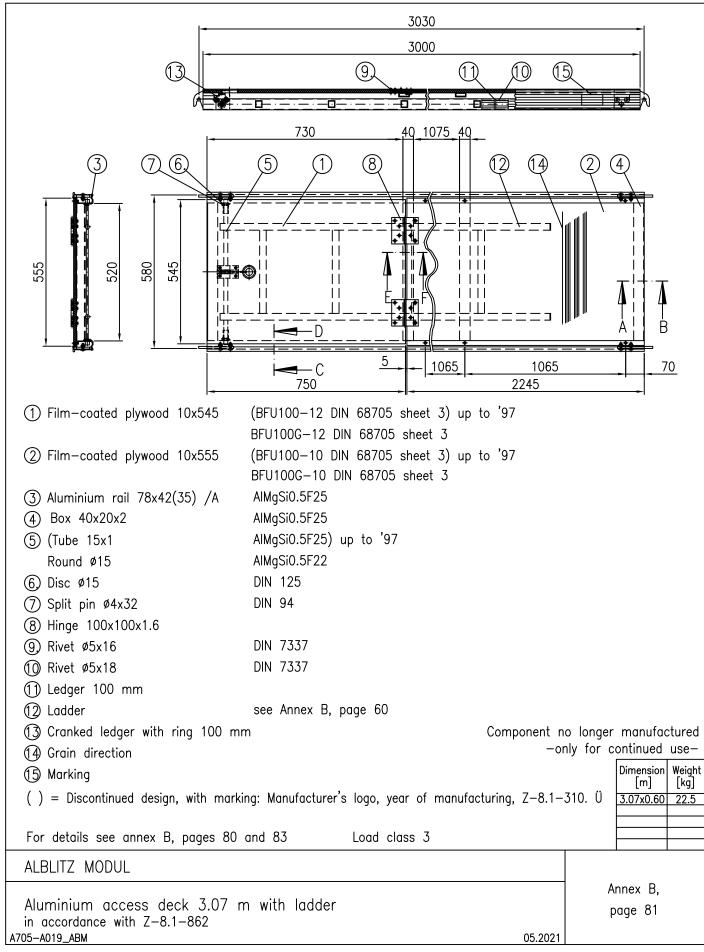


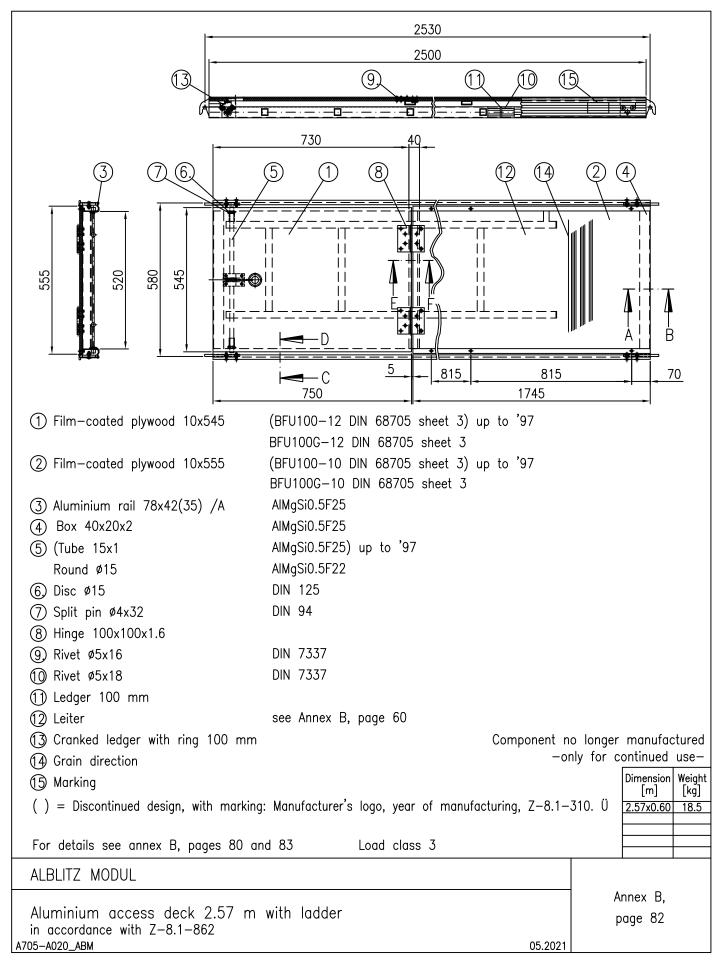
| ALBLITZ MODUL | |
|---|---------------------|
| Sectional views of aluminium access deck in accordance with Z-8.1-862 | Annex B, page 77 |
| A705-A014_ABM 05.202 | |

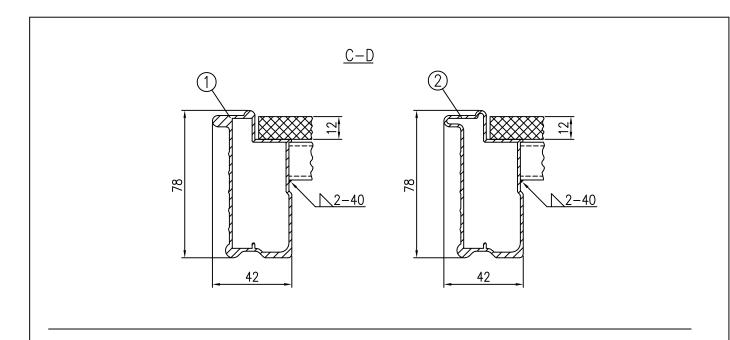


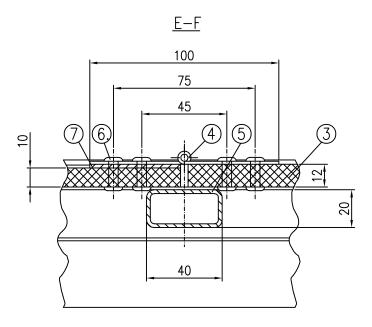








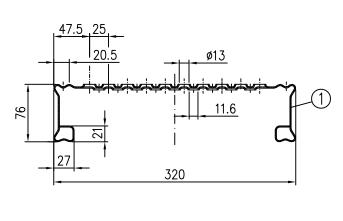




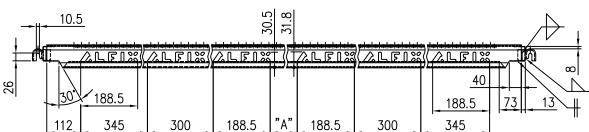
- 1) Form A
- ② Form B
- 3 Hatch
- 4 Hinge 100x100x1.6
- (5) Box 40x20x2 AlMgSi0.5F25 (6) Aluminium blind rivet Ø5x16 DIN 7340
- 7 Thickness compensation

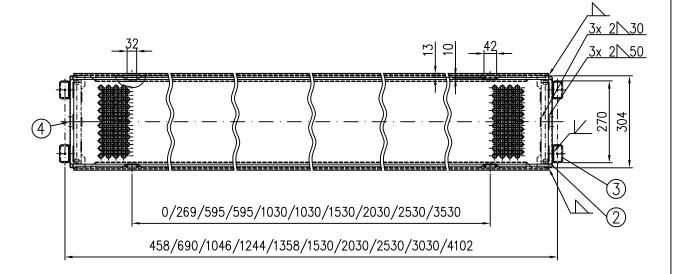
Component no longer manufactured —only for continued use—

| ALBLITZ MODUL | | |
|---|---------|---------------------|
| Sectional views of aluminium access deck in accordance with Z-8.1-862 | | Annex B, page 83 |
| A705-A021_ABM | 05.2021 | |



| Bay length | Number of logos | Dimen- sion"A" | Load class | Weight |
|-----------------|--------------------|-------------------|---------------|--------|
| [mm] | [left/right] | [mm] | | [kg] |
| 500 | 1/- | _ | 6 | 4.1 |
| 732 | 1/1 | 36 | 6 | 5.6 |
| 1088 | 1/1 | 392 | 6 | 8.1 |
| 1286 | 1/1 | 590 | 6 | 9.5 |
| 1400 | 1/1 | 704 | 6 | 10.2 |
| 1572 | 1/1 | 876 | 6 | 11.4 |
| 2072 | 2/2 | 686 | 6 | 13.7 |
| 2572 | 2/2 | 1186 | 5 | 17.1 |
| 3072 | 3/3 | 1086 | 4 | 20.5 |
| 4144 | 3/3 | 2203 | 3 | 32.1 |





(1) Steel plate 1.5 mm DIN EN 10111-DD11 (DD12) DIN EN 10025-2 S235JR

R_{eH} ≥280 N/mm² R_m≥360 N/mm² R_{eH} ≥280 N/mm² R_m≥360 N/mm²

<u>alternatively:</u> 2 Steel plate 1.5 mm DIN EN 10111-DD11

R_{eH} ≥240 N/mm² R_m≥360 N/mm²

3 Steel plate 4 mm

DIN EN 10111-DD13

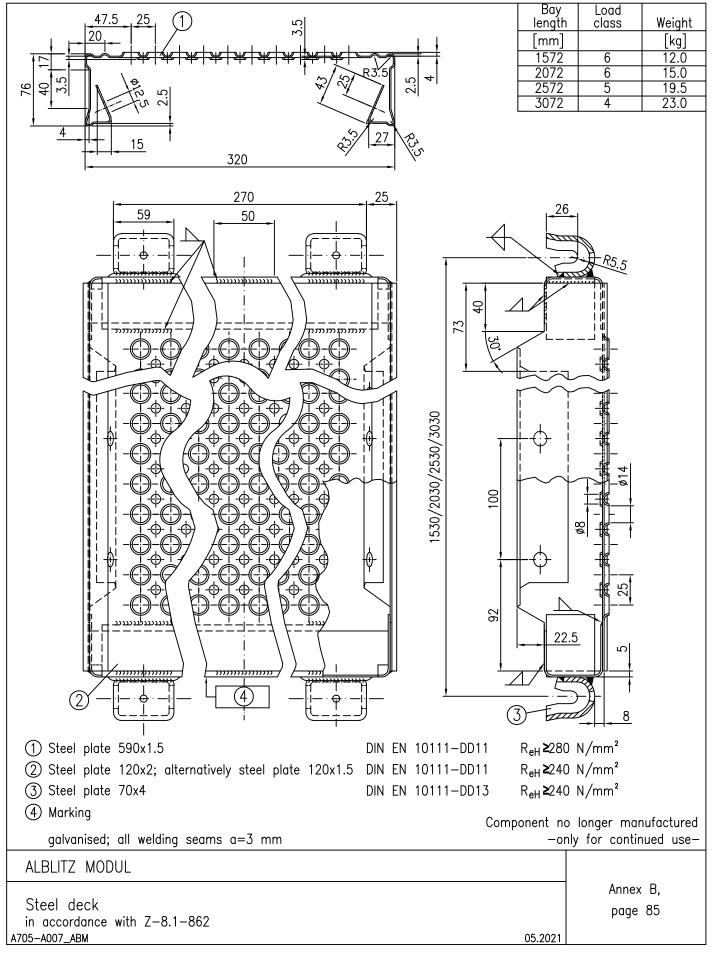
R_{eH} ≥240 N/mm² R_m≥360 N/mm²

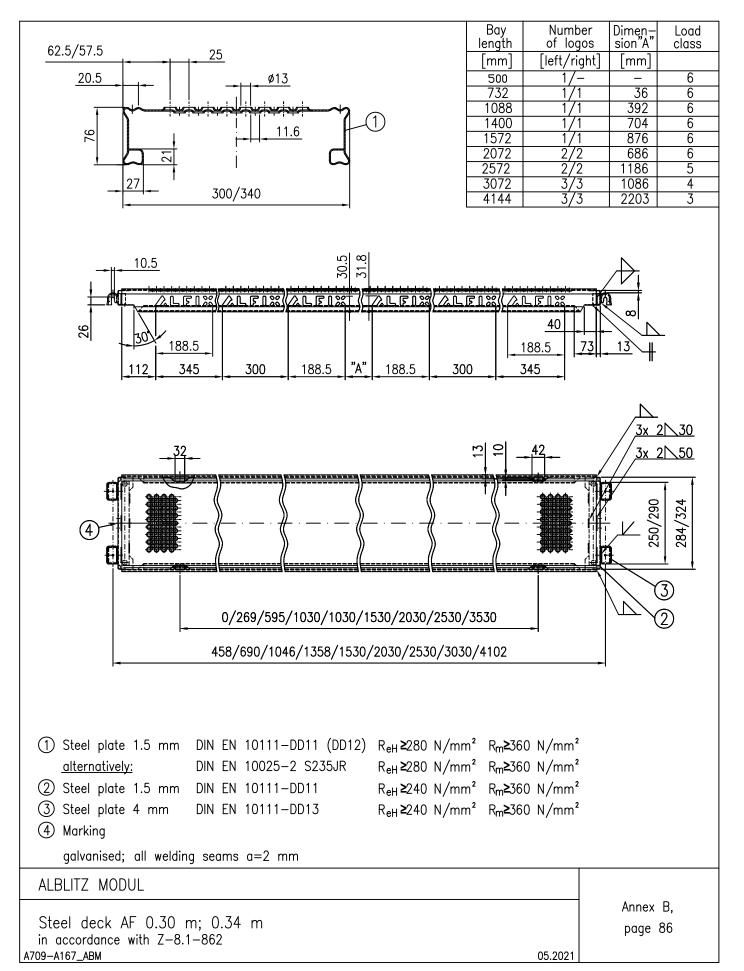
(4) Marking

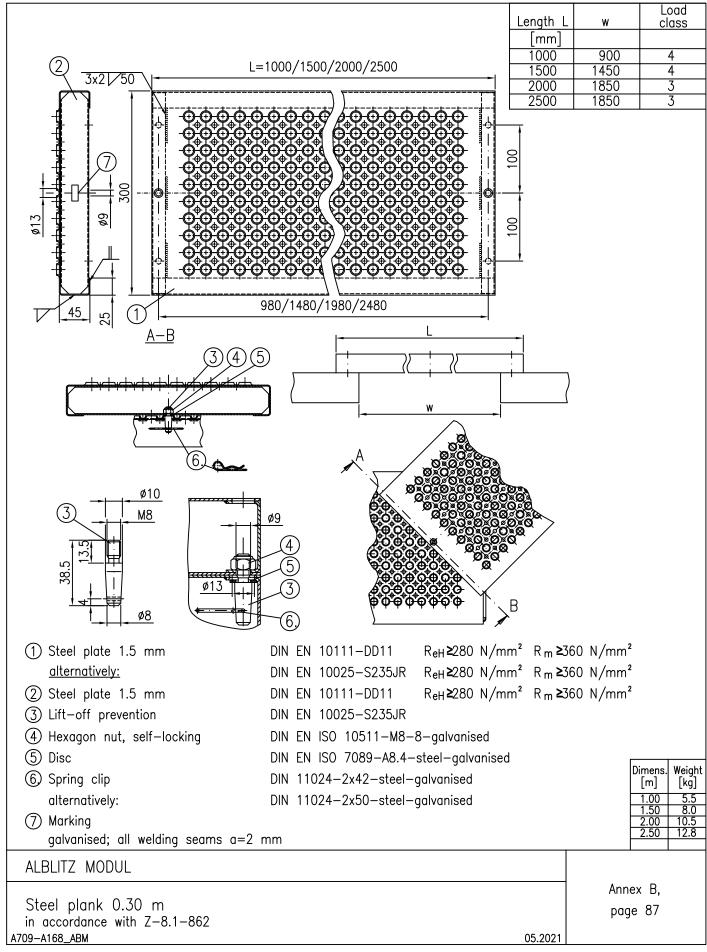
galvanised; all welding seams a=2 mm

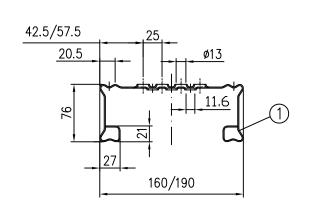
ALBLITZ MODUL

Steel deck AF 0.32 m in accordance with Z-8.1-862 A709-A107_ABM



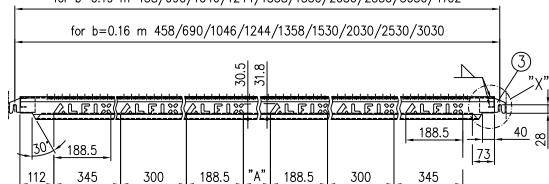


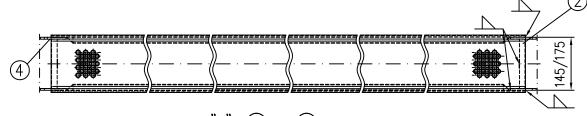


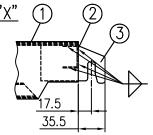


| Bay Iength | Number of logos | Dimen- sion"A" | Load class | Weight 0.19 m |
|---------------|--------------------|-------------------|---------------|------------------|
| [mm] | [left/right] | [mm] | | [kg] |
| 500 | 1/- | _ | 6 | 3.1 |
| 732 | 1/- | _ | 6 | 4.3 |
| 1088 | 1/1 | 417 | 6 | 6.1 |
| 1286 | 1/1 | 615 | 6 | 7.1 |
| 1400 | 1/1 | 729 | 6 | 7.7 |
| 1572 | 1/1 | 901 | 6 | 8.6 |
| 2072 | 2/2 | 711 | 6 | 11.2 |
| 2572 | 2/2 | 1211 | 5 | 13.9 |
| 3072 | 3/3 | 1111 | 4 | 16.5 |
| 4144 | 3/3 | 2228 | 3 | 22.0 |

for b=0.19 m 458/690/1046/1244/1358/1530/2030/2530/3030/4102







1 Steel plate 1.5mm alternatively:

DIN EN 10025-2 S235JR

DIN EN 10111-DD11 (DD12) R_{eH} ≥280 N/mm² R_m≥360 N/mm² R_{eH} ≥280 N/mm² R_m≥360 N/mm²

② U-profile 30x20x1.5 alternatively: U-profile 25x25x1.5 DIN EN 10162

DIN EN 10025-2 S235JR

S235JR

③ Flat 50x6

DIN EN 10025-2 S235JR

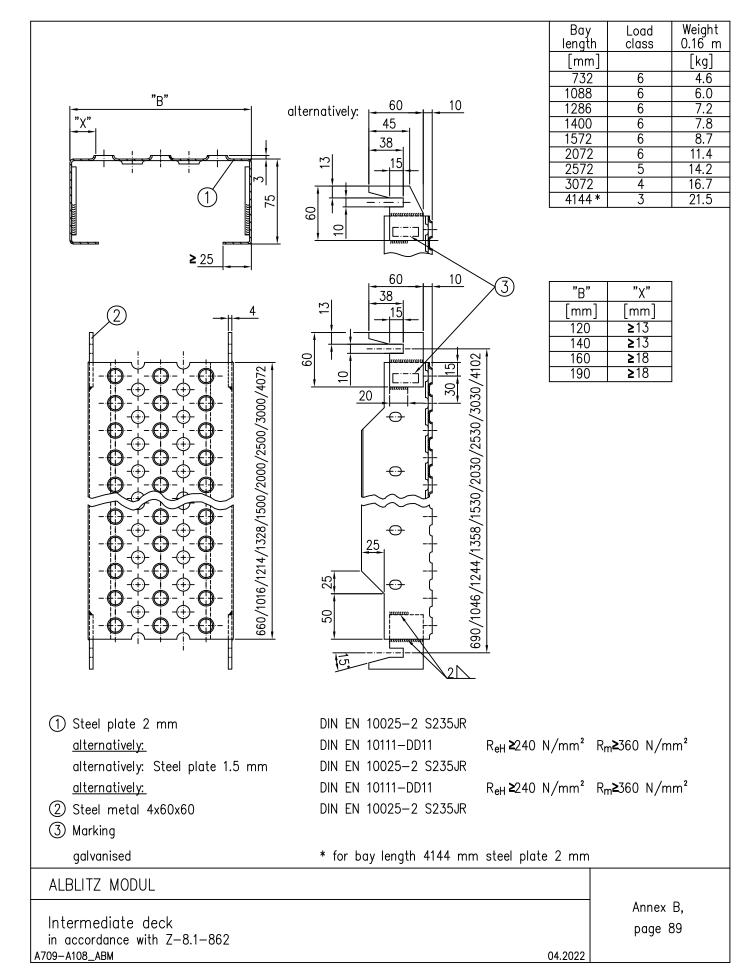
(4) Marking

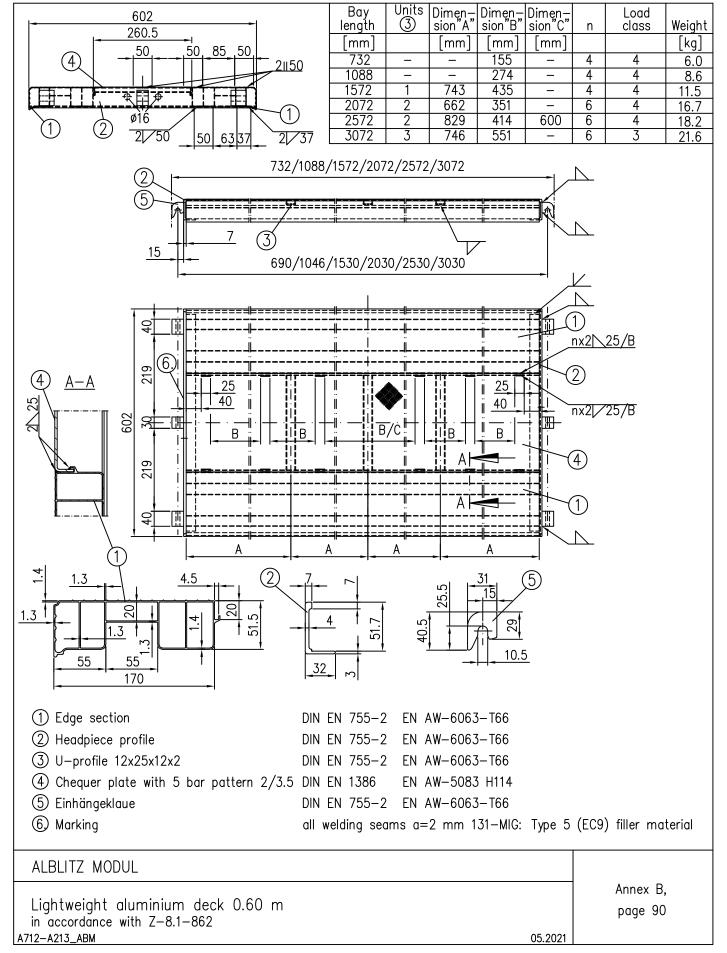
galvanised; all welding seams a=2 mm

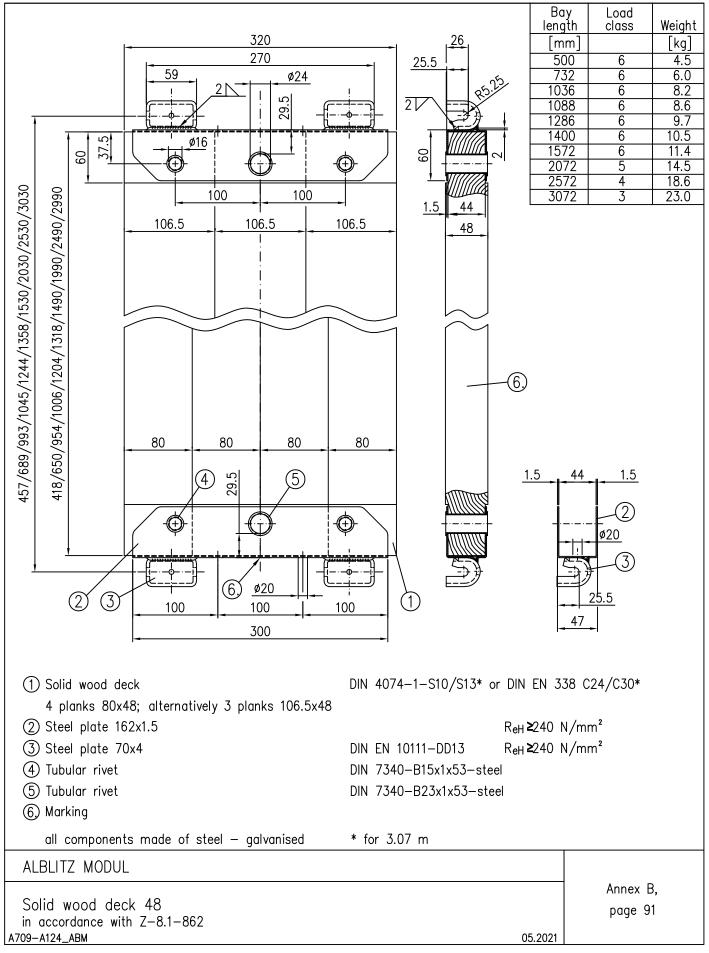
ALBLITZ MODUL

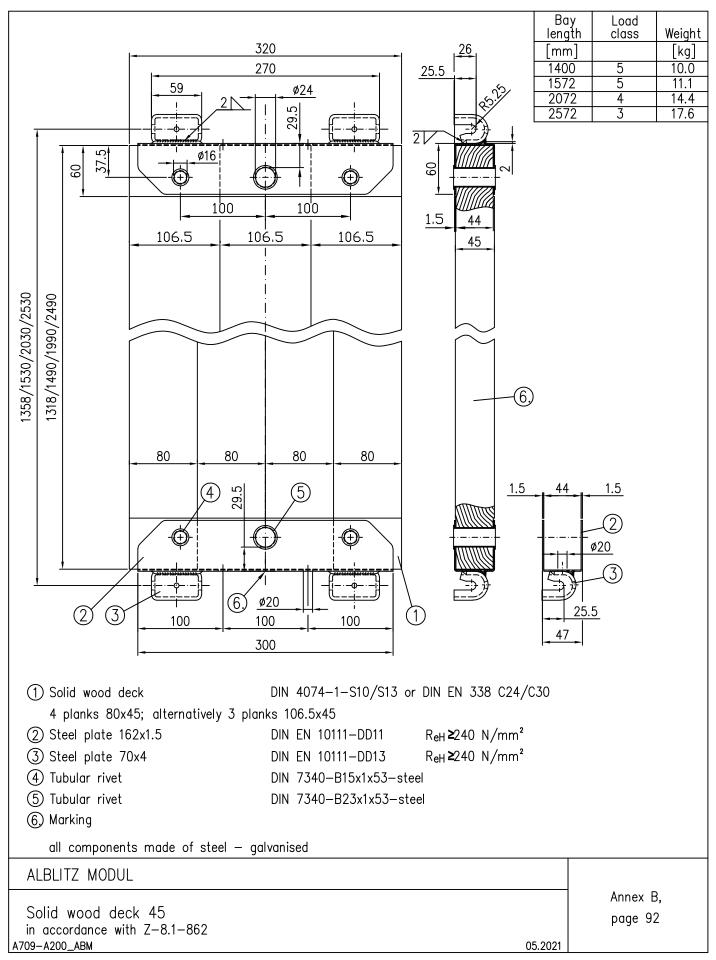
Intermediate deck AF 0.16 m; 0.19 m

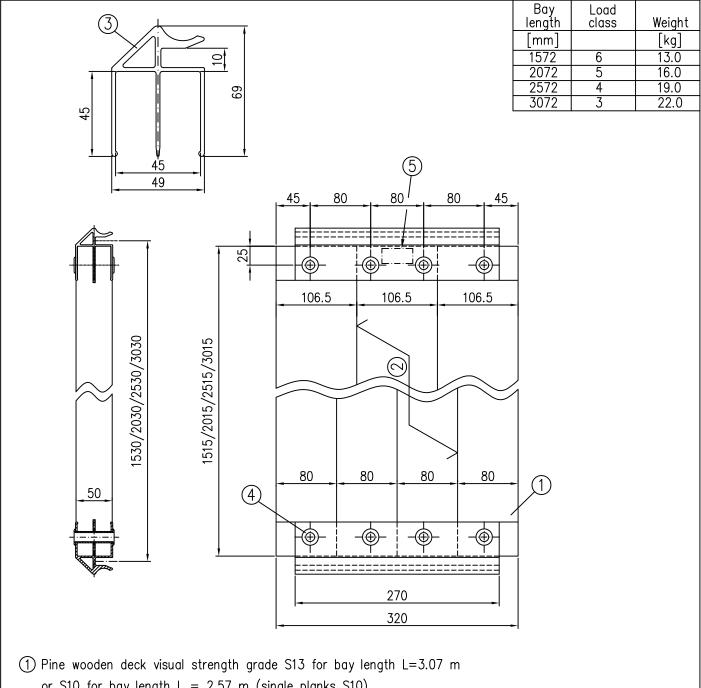
in accordance with Z-8.1-862A709-A181_ABM









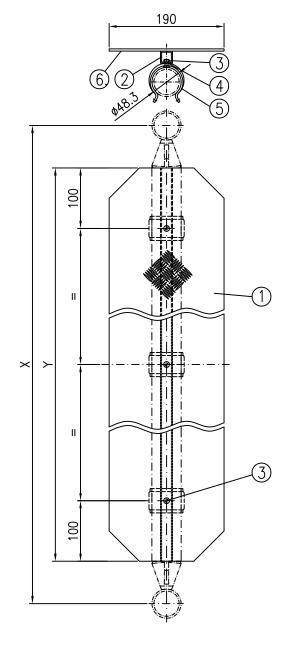


- or S10 for bay length L = 2.57 m (single planks S10) alternatively: - 4 planks made from wooden decks 80x50 - 3 planks made from wooden decks 106.5x50
- ② glued as structural component
- 3 Suspension section EN AW-6082-T5 (AIMgSi1F28)
- (4) Tubular rivet DIN 7340-B15x1x53-steel-galvanised
- (5) Marking

Component no longer manufactured

-only for continued use-

| ALBLITZ MODUL | | |
|--|---------|---------------------|
| Wooden deck in accordance with Z-8.1-862 | | Annex B, page 93 |
| A705-A124_ABM | 05.2021 | |



| Χ | X | Y | Weight |
|------|------|------|--------|
| [m] | [mm] | [mm] | [kg] |
| 0.73 | 732 | 591 | 1.2 |
| 1.09 | 1088 | 947 | 2.0 |
| 1.29 | 1286 | 1145 | 2.2 |
| 1.40 | 1400 | 1259 | 2.4 |
| 1.57 | 1572 | 1431 | 3.0 |
| 2.07 | 2072 | 1931 | 4.0 |
| 2.57 | 2572 | 2431 | 5.1 |
| 3.07 | 3072 | 2931 | 6.1 |
| 4.14 | 4144 | 4003 | 7.3 |
| | | | |

- ① Chequer plate with 5 bar pattern 2.5/3.3x190alternatively: Chequer plate with 5 bar pattern 2.5/4.0x190 DIN EN 1386 EN AW-5754 H111/ H114
- (2) Rectangular hollow section 20x20x2
- (3) Blind rivet
- (4) Disc alternatively:
- (5) Tube clamp, hot-dip galvanised
- (6) Marking

DIN EN 1386 EN AW-5083 H224

DIN EN 755-2 EN AW-6060-T66

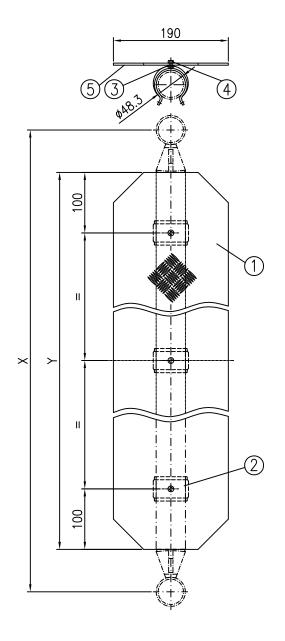
DIN EN ISO 15979-A5x12 EN AW-5754 H112

DIN EN ISO 7089-5.3-steel-galvanised

DIN EN ISO 7094-5.5-steel-galvanised

ALBLITZ MODUL

MODUL gap cover in accordance with Z-8.22-906 M710-B170_ABM



| Χ | Υ | Weight |
|------|---|--|
| [mm] | [mm] | [kg] |
| 732 | 591 | 1.0 |
| 1088 | 947 | 1.6 |
| 1286 | 1145 | 1.9 |
| 1400 | 1259 | 2.1 |
| 1572 | 1431 | 2.4 |
| 2072 | 1931 | 3.3 |
| 2572 | 2431 | 4.1 |
| 3072 | 2931 | 5.0 |
| 4144 | 4003 | 6.8 |
| | [mm] 732 1088 1286 1400 1572 2072 2572 3072 | [mm] [mm] 732 591 1088 947 1286 1145 1400 1259 1572 1431 2072 1931 2572 2431 3072 2931 |

- ① Chequer plate with 5 bar pattern 2.5/3.3x190alternatively: Chequer plate with 5 bar pattern 2.5/4.0x190 DIN EN 1386 EN AW-5754 H111/ H114
- 2 Tube clamp, hot-dip galvanised
- (3) Disc alternatively:
- (4) Blind rivet
- (5) Marking

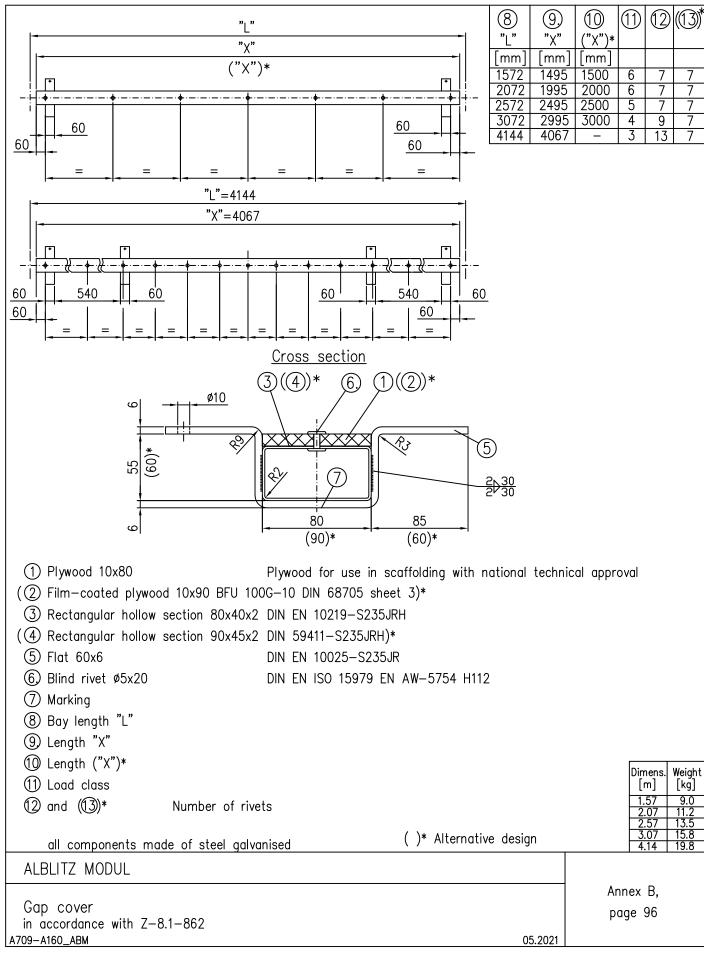
DIN EN 1386 EN AW-5083 H224

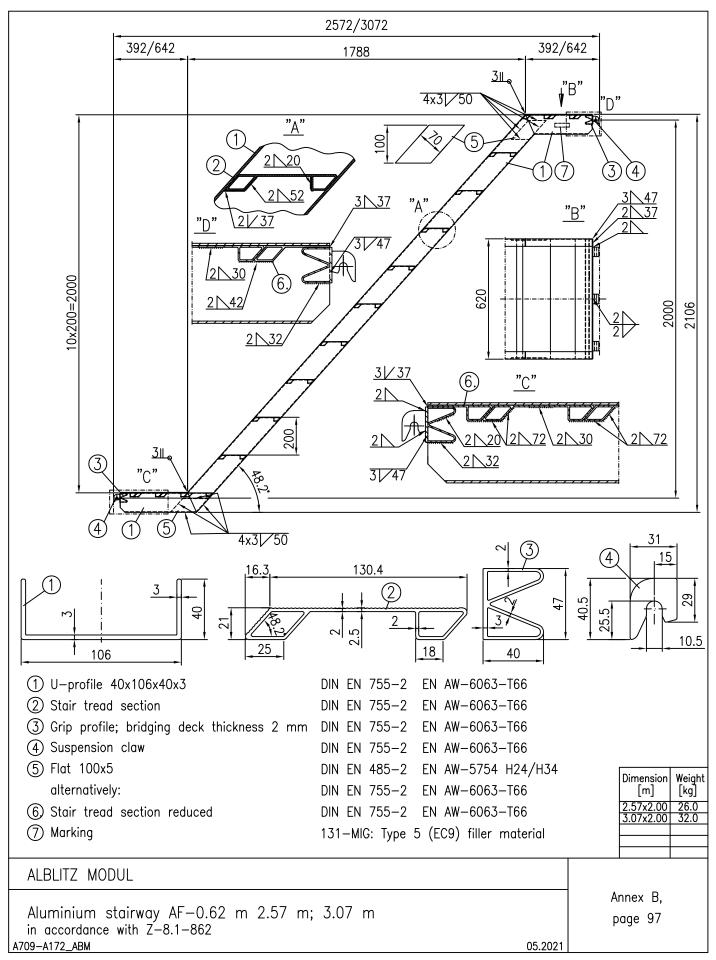
DIN EN ISO 7089-5.3-steel-galvanised DIN EN ISO 7094-5.5-steel-galvanised

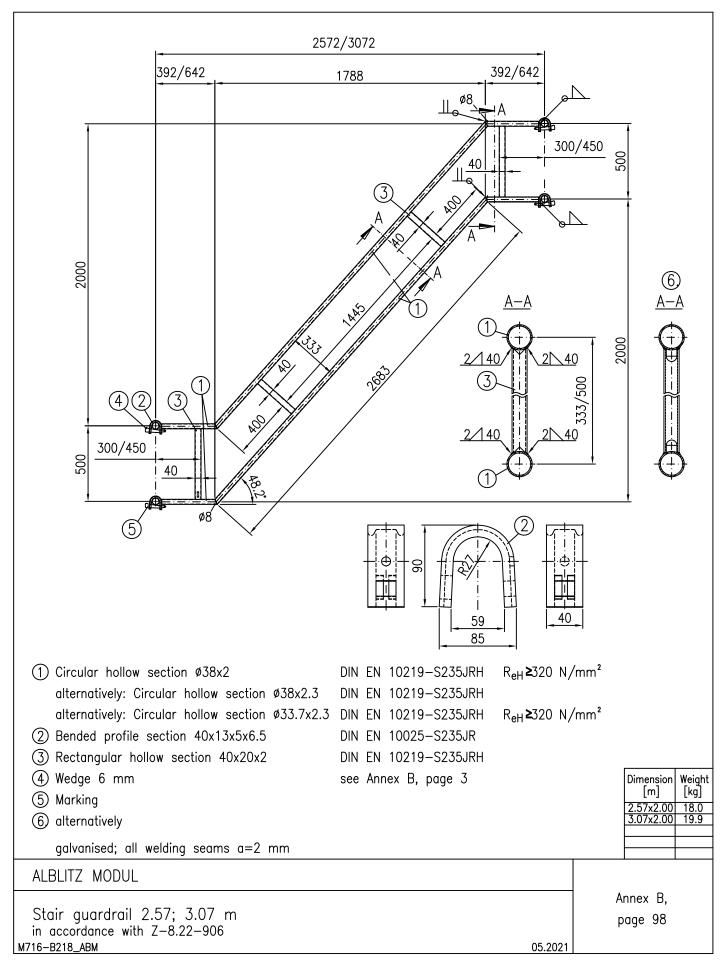
DIN EN ISO 15979-A5x12 EN AW-5754 H112

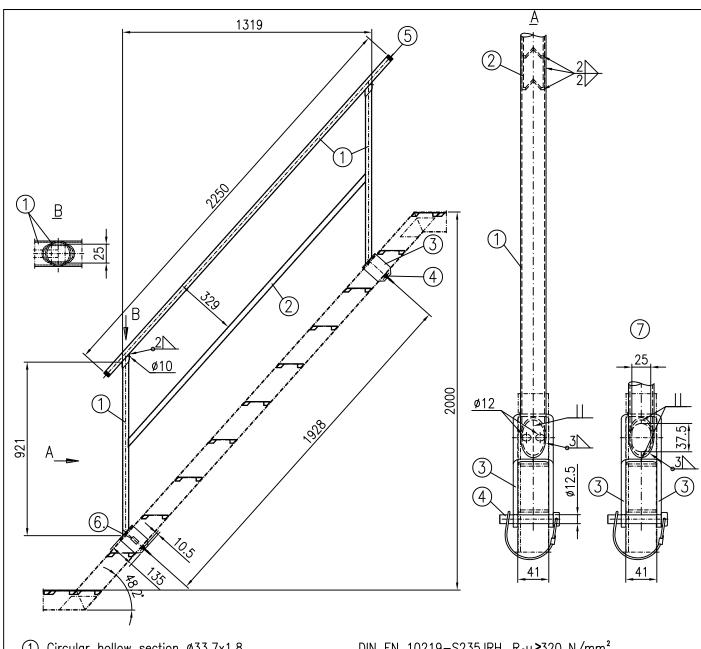
| ALBLITZ | MODUL |
|---------|-------|
| | |

MODUL gap cover with tube fixture in accordance with Z-8.22-906 M710-B132_ABM









- (1) Circular hollow section Ø33.7x1.8 alternatively: Circular hollow section Ø33.7x2.0
- 2 Rectangular hollow section 30x30x2
- (3) Flat 100x6
- 4 Tube linchpin RK 112 12x70 with snap-on lock Pin
 - Ring
- 5 Tube end cap GL 34 S-Poly.
- (6) Marking
- (7) alternatively galvanised

- DIN EN 10219-S235JRH R_{eH}≥320 N/mm²
- DIN EN 10219-S235JRH
- DIN EN 10219-S235JRH
- DIN EN 10025-S235JR

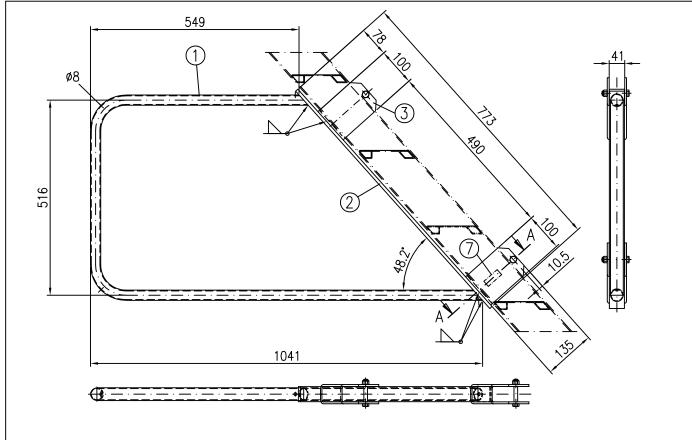
DIN EN 10025-S355J2

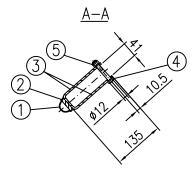
DIN 17223 B spring steel wire

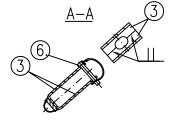
| Dimens. [m] | Weight [kg] |
|----------------|-------------|
| 2.00 | 13.3 |
| | |
| | |
| | |

ALBLITZ MODUL

Inner guardrail for aluminium stairway 2.00 m in accordance with Z-8.1-862 A709-A174_ABM







- (1) Circular hollow section Ø33.7x1.8 alternatively: Circular hollow section Ø33.7x2.0 DIN EN 10219-S235JRH
- (2) Flat 40x8
- (3) Flat 100x6
- 4 Hexagon bolt
- (5) Hexagon nut, self-locking
- 6 alternatively: Tube linchpin RK 112 12x70 with snap-on lock

Pin Ring

(7) Marking

galvanised; all welding seams a=3 mm

- R_{eH}≥320 N/mm² DIN EN 10219-S235JRH

DIN EN 10025-S235JR

DIN EN 10025-S235JR

DIN EN ISO 4014-M10x65-8.8-galvanised

DIN EN ISO 10511-M10-8-galvanised

DIN EN 10025-S355J2

DIN 17223 B spring steel wire

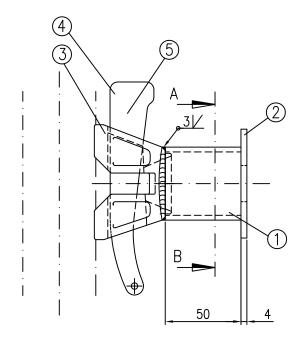
| Dimension [m] | Weight [kg] |
|------------------|----------------|
| 1.00 x 0.50 | 8.8 |
| | |
| | |
| | |

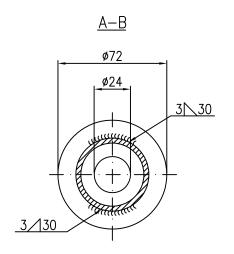
Annex B,

page 100

Stair stringer fall protection 1.00 x 0.50 m in accordance with Z-8.1-862

A709-A175_ABM





(1) Circular hollow section Ø48.3x3.2

alternatively: Circular hollow section Ø48.3x2.7 DIN EN 10219-S460MH

2) Steel metal 4 t=4 mm alternatively: Disc

3 Connection of tube ledger alternatively: Connection of tube ledger 4.0

4) Wedge 6 mm

(5) Marking galvanised

R_eH≥320 N/mm² DIN EN 10219-S235JRH

DIN EN 10025-S235JR

DIN EN ISO 7093-1-26x70x4-steel

see Annex B, page 4

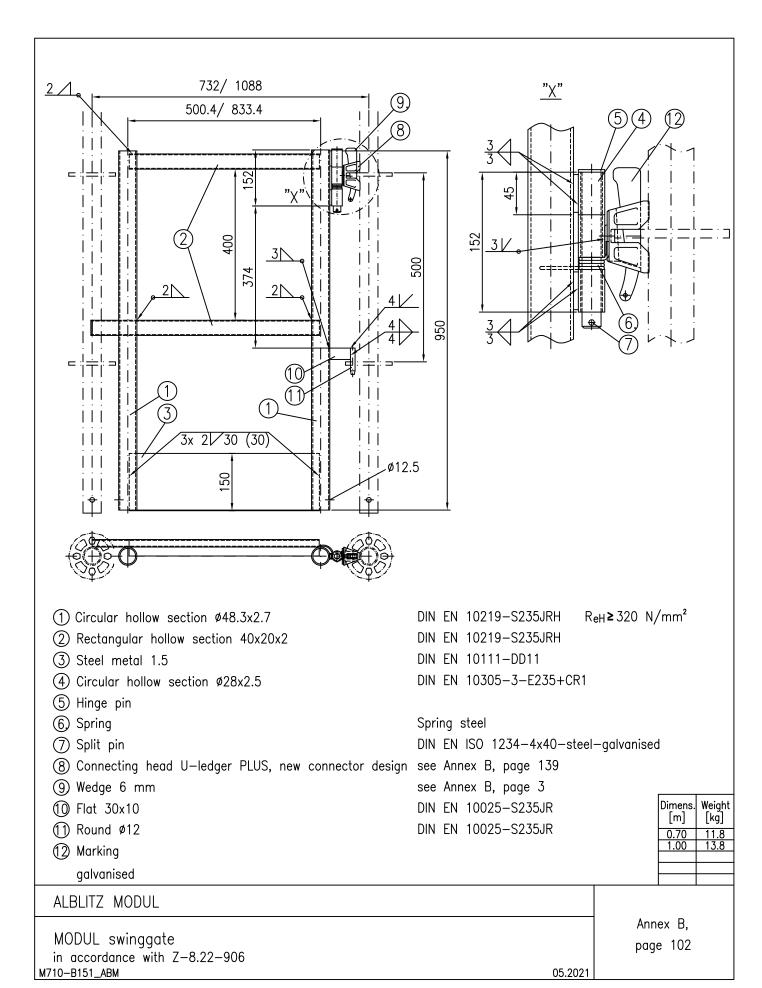
see Annex B, page 151

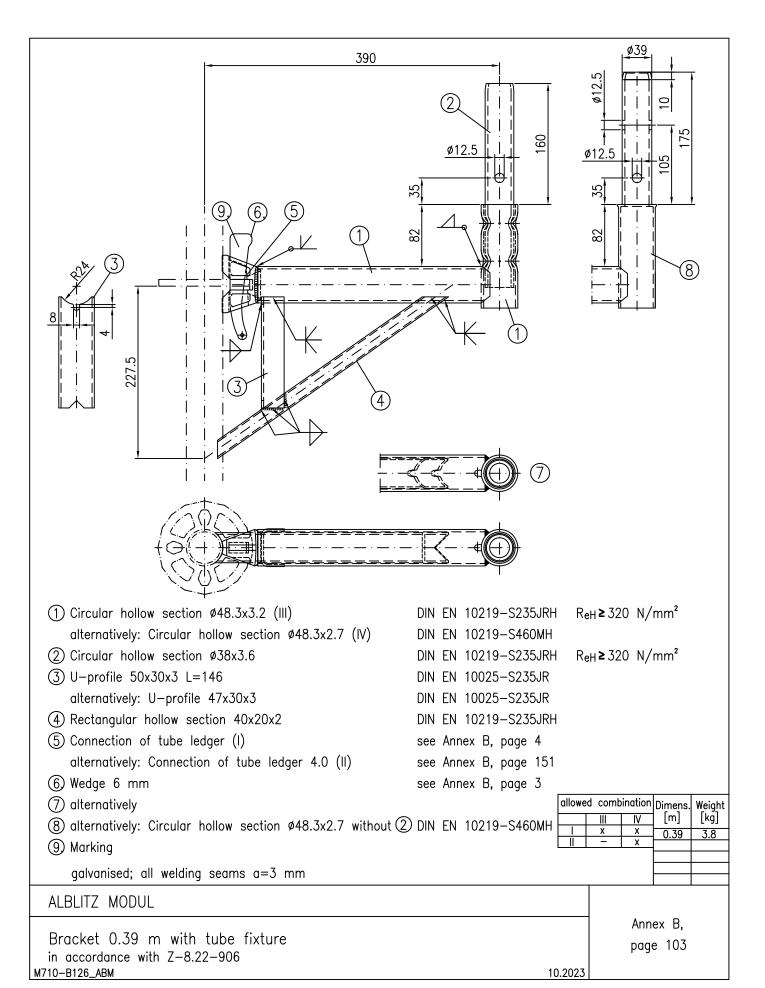
see Annex B, page 3

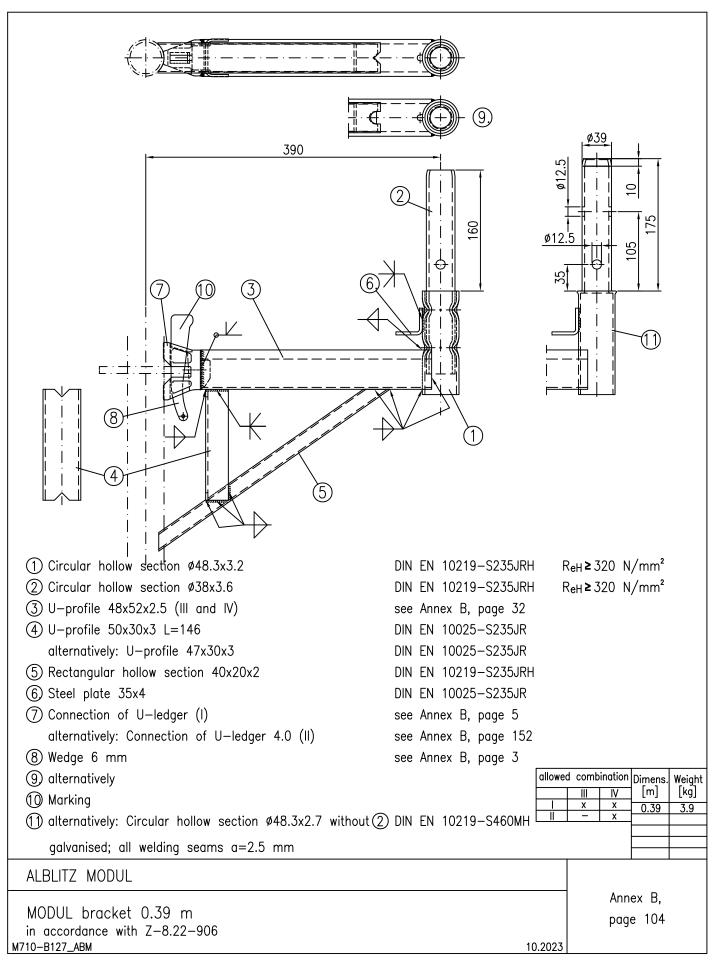
| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| - | 0.8 |
| | |
| | |
| | |

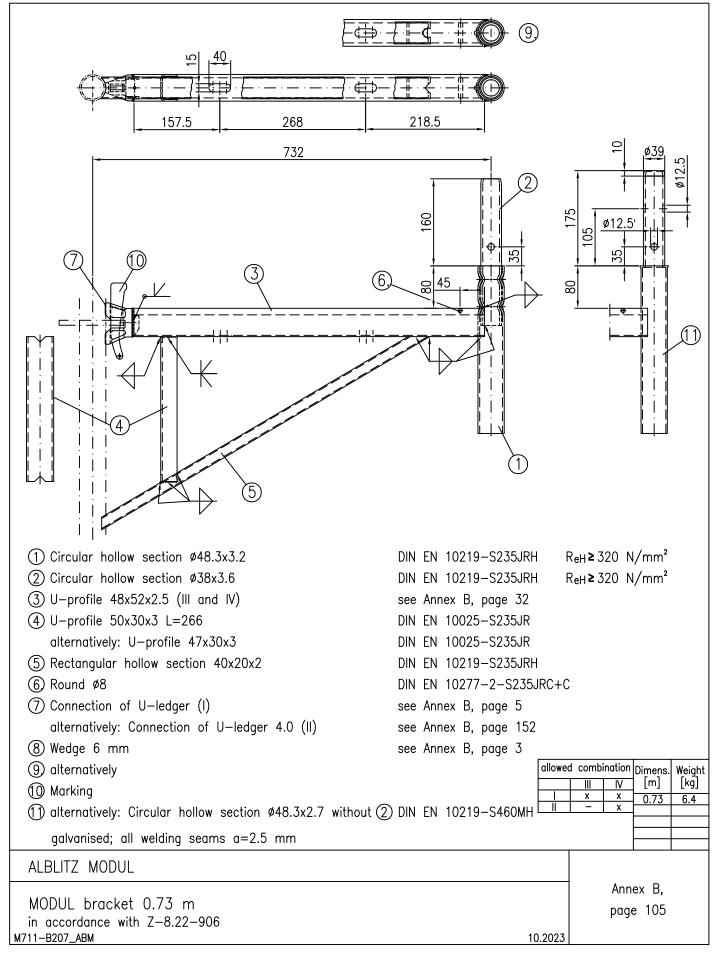
ALBLITZ MODUL

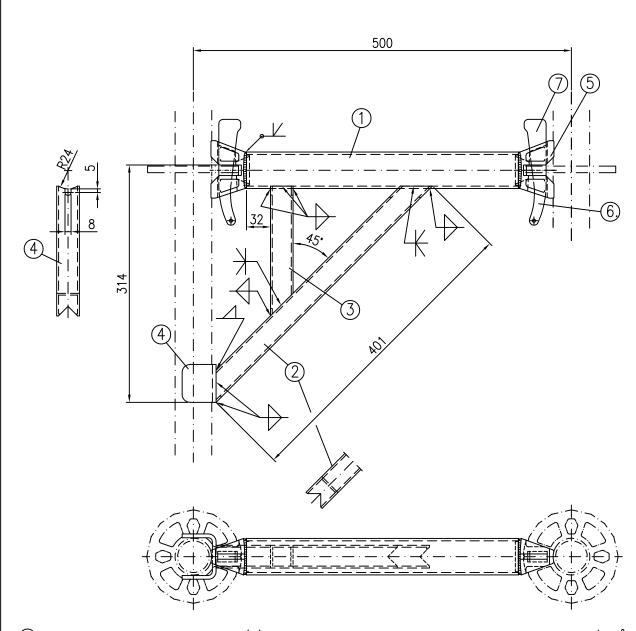
MODUL stair guardrail holder in accordance with Z-8.22-906 M711-B209_ABM











- 1 Circular hollow section Ø48.3x3.2 (III) alternatively: Circular hollow section Ø48.3x2.7 (IV) DIN EN 10219-S460MH
- (2) Rectangular hollow section 30x30x2.5
- 3 Rectangular hollow section 30x30x2.5
- 4) Steel plate 50x5
- (5) Connection of tube ledger (I) alternatively: Connection of tube ledger 4.0 (II)
- 6) Wedge 6 mm
- (7) Marking

galvanised; all welding seams a=3 mm

DIN EN 10219-S235JRH

DIN EN 10219-S235JRH

DIN EN 10219-S235JRH

DIN EN 10025-S235JR

see Annex B, page 4

see Annex B, page 151

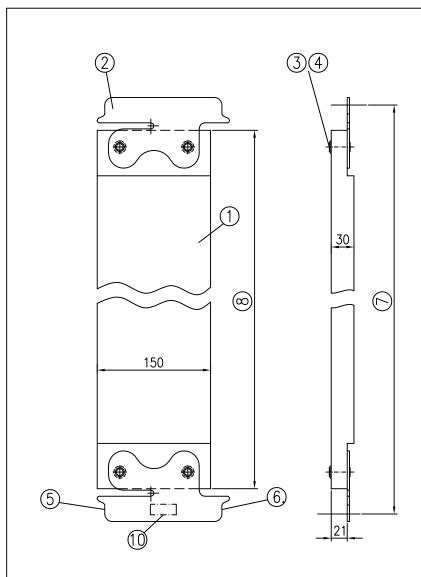
see Annex B, page 3

| R _{eH} ≥ 320 | N/mm ² |
|-----------------------|-------------------|
|-----------------------|-------------------|

| allowed combination | | | Dimens. | Weight |
|---------------------|-----|----|---------|--------|
| | III | IV | [m] | [kg] |
| | Х | Х | 0.50 | 3.6 |
| ll l | - | Х | 0.00 | |
| | | | | |

ALBLITZ MODUL

Bracket with tube fixture 0.50 m in accordance with Z-8.22-906 M711-B204_ABM



| 8 | 9 |
|------|--|
| [mm] | [kg] |
| 323 | 0.7 |
| 383 | 1.0 |
| 433 | 1.2 |
| 665 | 1.6 |
| 1021 | 2.3 |
| 1219 | 2.7 |
| 1333 | 3.0 |
| 1505 | 3.3 |
| 2005 | 4.2 |
| 2505 | 5.3 |
| 3005 | 6.3 |
| | 323 383 433 665 1021 1219 1333 1505 2005 2505 |

- 1) Pine wooden deck visual strength grade S10
- ② Slit strip 175x2 alternatively:
- 3 Tubular rivet
- (4) Disc
- (5) Bearing surface connection of tube ledger
- (6) Bearing surface connection of U-ledger
- 7 Bay length
- 8 Length L
- (9) Weight
- (1) Marking

DIN 4074-1

DIN EN 10111-DD11

DIN EN 10346-DX51D+Z275

DIN 7340-A8x0.75x28-steel-electrogalvanized

DIN EN ISO 7089-A8.4-steel-galvanised

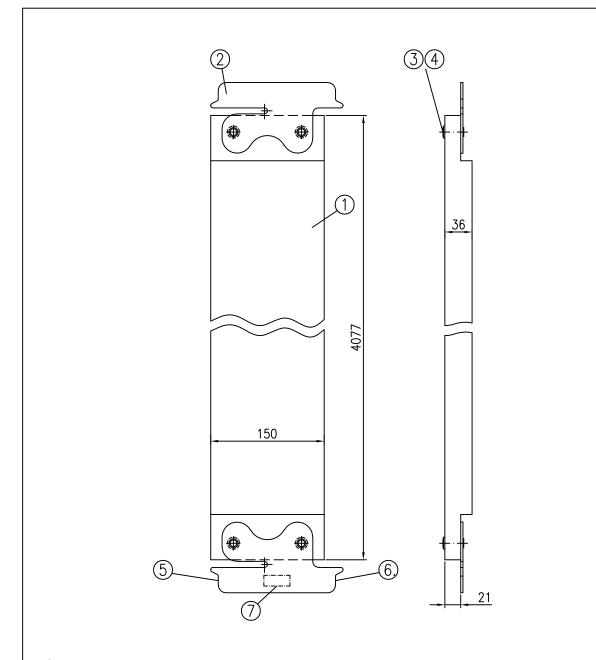
| ALBLITZ | MODUL |
|---------|-------|
| | |

MODUL toeboard

in accordance with Z-8.22-906 M710-B125_ABM

Annex B, page 107

09.2023



- 1) Pine wooden deck visual strength grade S10
- ② Slit strip 175x2 alternatively:
- (3) Tubular rivet
- (4) Disc
- (5) Bearing surface connection of tube ledger
- 6 Bearing surface connection of U-ledger
- Marking

DIN 4074-1

DIN EN 10111-DD11

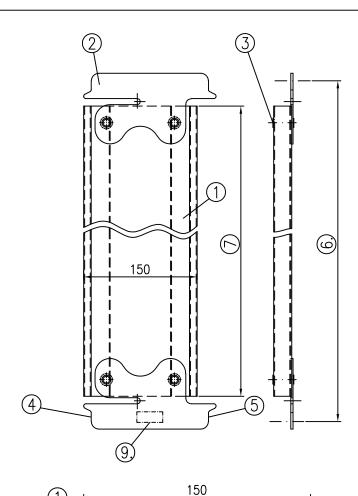
DIN EN 10346-DX51D+Z275

DIN 7340-A8x0.75x34-steel-electrogalvanized

DIN EN ISO 7089-A8.4-steel-galvanised

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 4.14 | 8.1 |

| | | 4.14 8.1 |
|---|---------|----------------------|
| ALBLITZ MODUL | | |
| MODUL toeboard 4.14 m in accordance with Z-8.22-906 | | Annex B, page 108 |
| M710-B166_ABM | 09.2023 | |



| 6 | 7 | 8 |
|------|------|------|
| [mm] | [mm] | [kg] |
| 390 | 323 | 0.6 |
| 732 | 665 | 1.2 |
| 1088 | 1021 | 1.7 |
| 1286 | 1219 | 1.9 |
| 1400 | 1333 | 2.1 |
| 1572 | 1505 | 2.3 |
| 2072 | 2005 | 3.0 |
| 2572 | 2505 | 3.6 |
| 3072 | 3005 | 4.3 |

| | | <u> </u> | |
|----------|---|----------|--|
| 24 | | | |
| <u> </u> | · | | |

- (1) Sectional view aluminium toeboard; s=1.25 mm DIN EN 755-2 EN AW-6063-T66
- 2) Slit strip 175x2 alternatively:
- (3) Tubular rivet alternatively:
- 4 Bearing surface connection of tube ledger
- (5) Bearing surface connection of U-ledger
- 6 Bay length
- 7 Length L
- (8) Weight
- (9) Marking

all components made of steel - galvanised

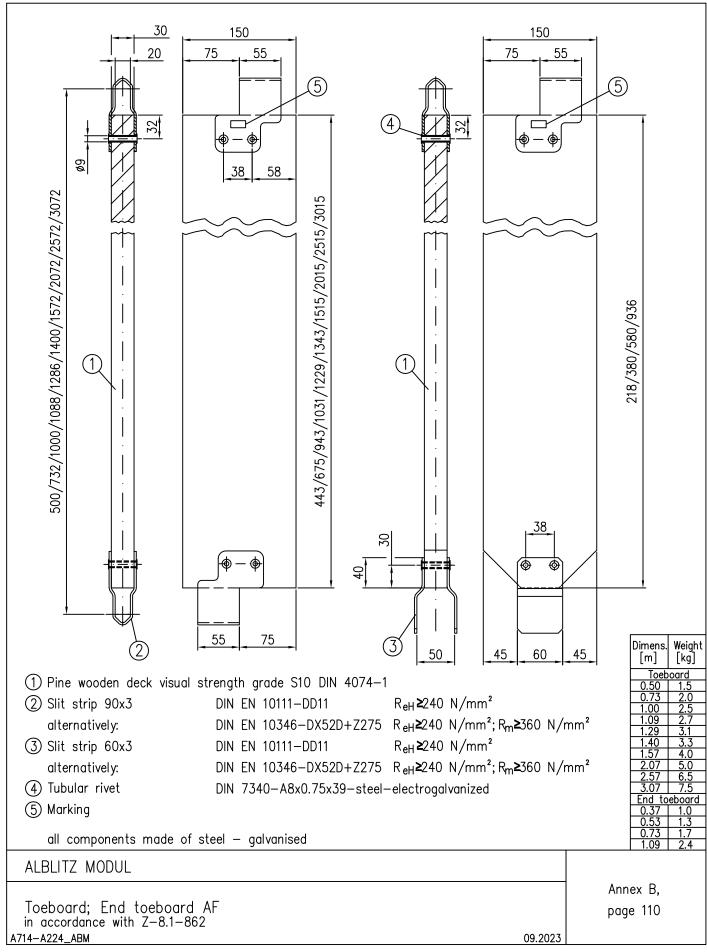
DIN EN 10111-DD11 hot-dip galvanised

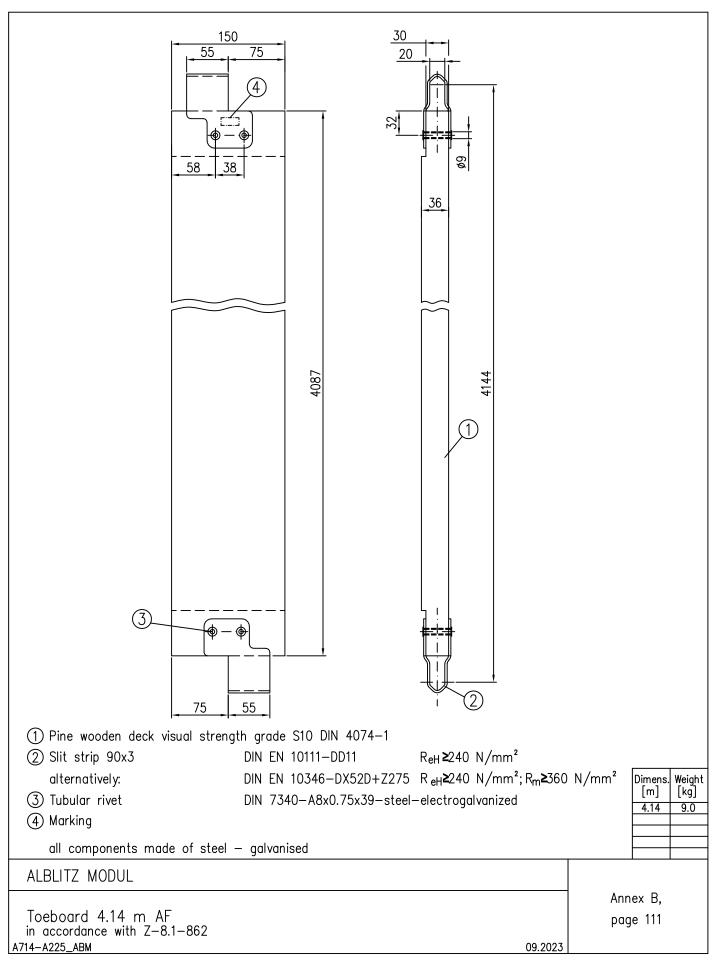
DIN EN 10346-DX51D+Z275

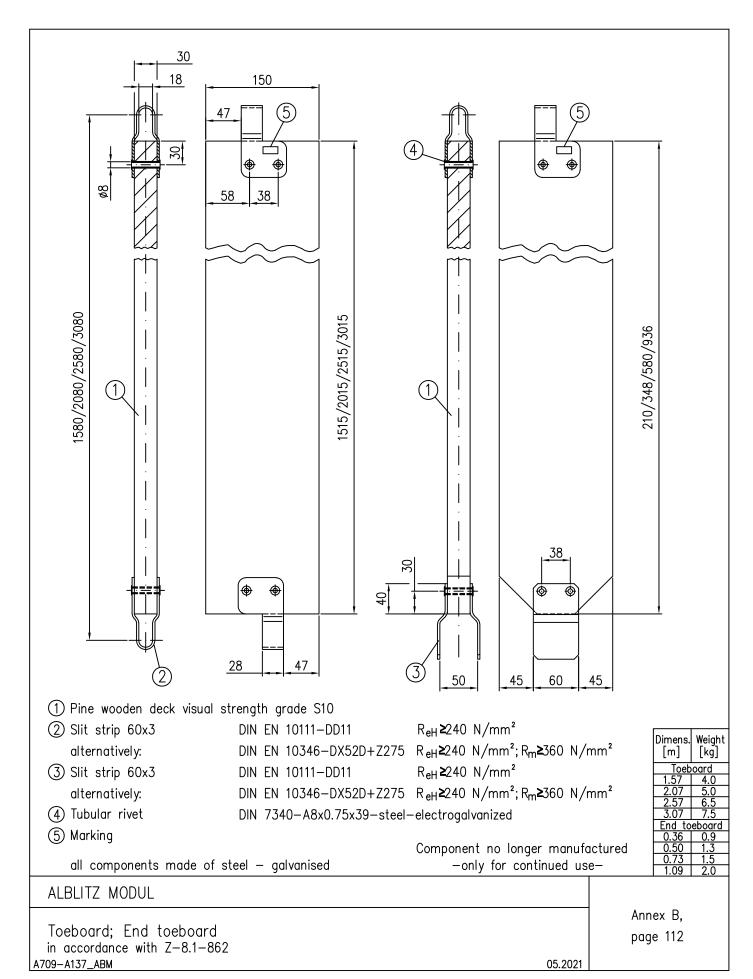
DIN 7340-A8x0.75x29-steel-galvanised

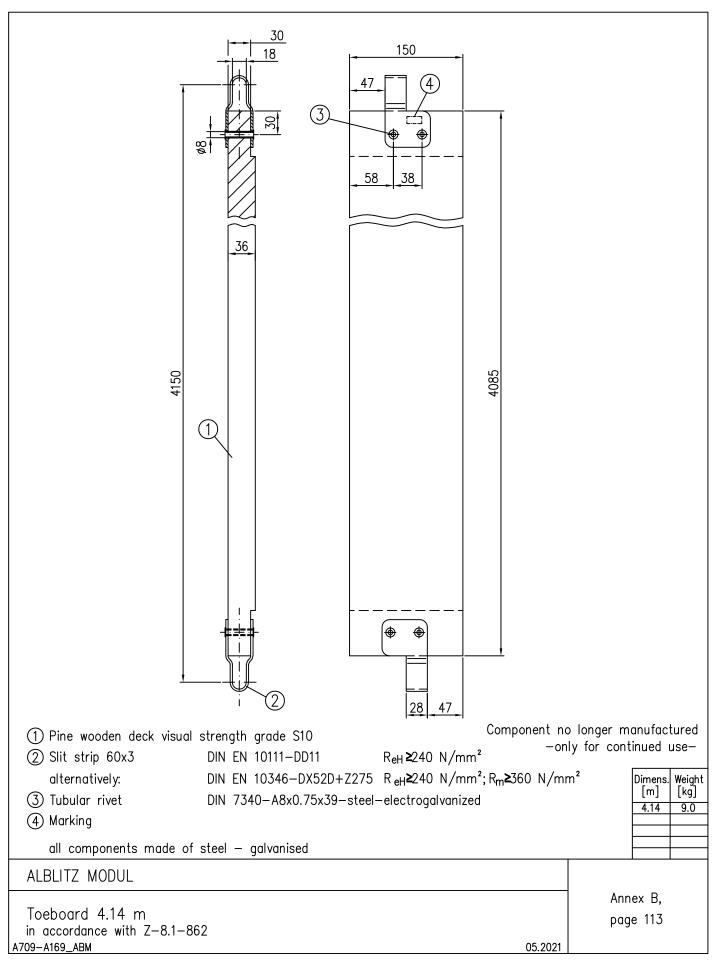
DIN 7340-A8x1x28-steel-galvanised

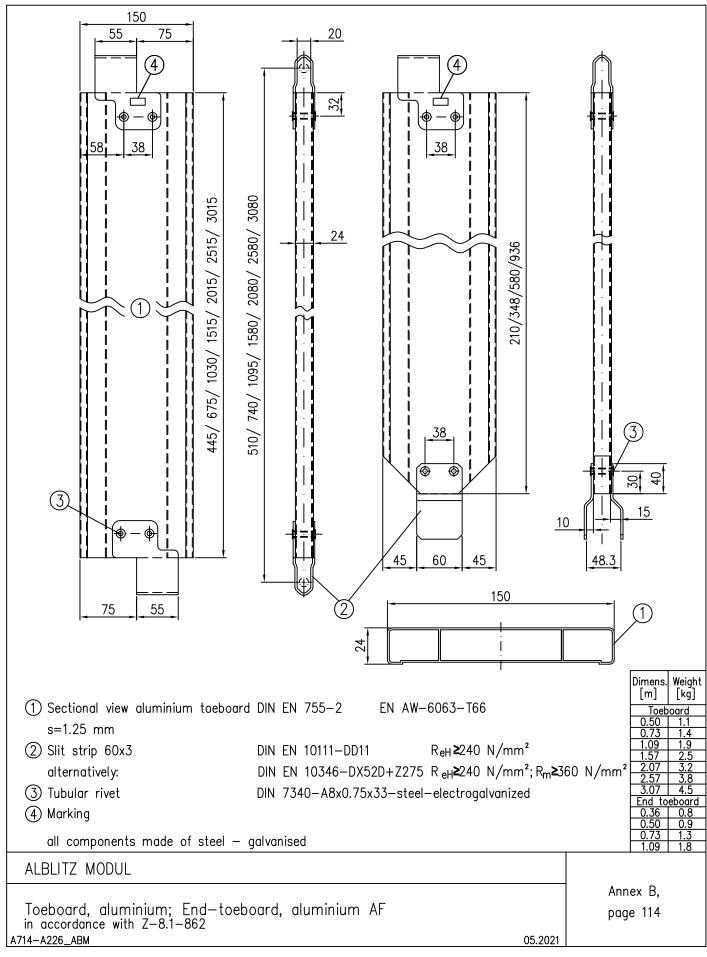
MODUL toeboard, aluminium in accordance with Z-8.22-906 M710-B171_ABM

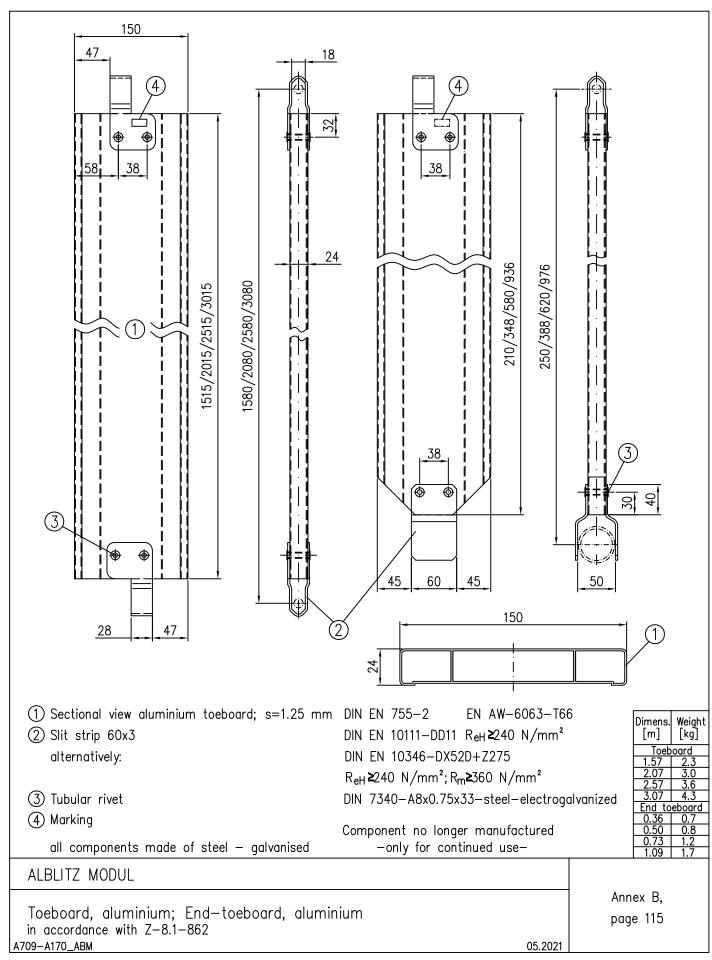


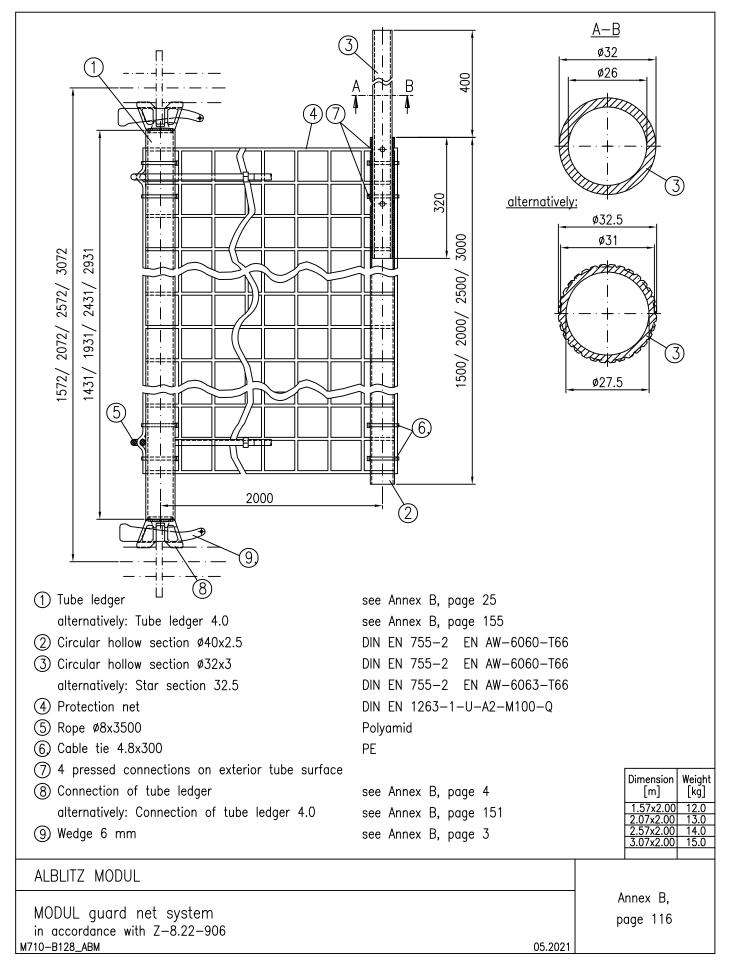


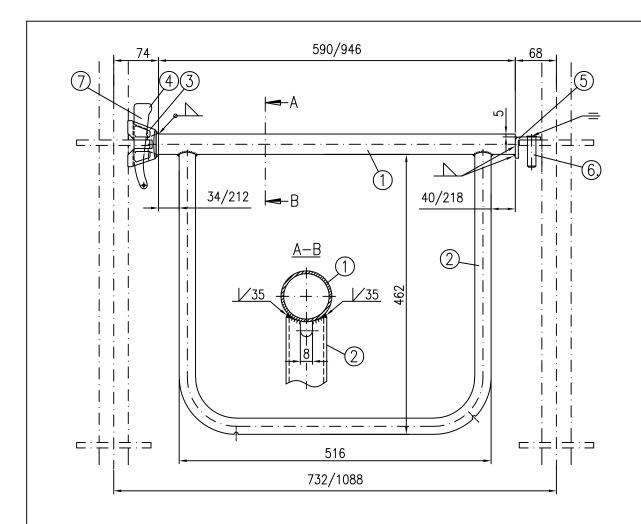














- (1) Circular hollow section Ø33.7x1.8 alternatively: Circular hollow section Ø33.7x2.0 DIN EN 10219-S235JRH
- 2 Circular hollow section Ø26.9x2
- (3) Connection of tube ledger alternatively: Connection of tube ledger 4.0
- 4) Wedge 6 mm
- (5) Steel plate 50x5
- 6) Round Ø12
- Marking

DIN EN 10219-S235JRH R_{eH}≥320 N/mm²

DIN EN 10219-S235JRH

see Annex B, page 4

see Annex B, page 151

see Annex B, page 3

DIN EN 10025-S235JR

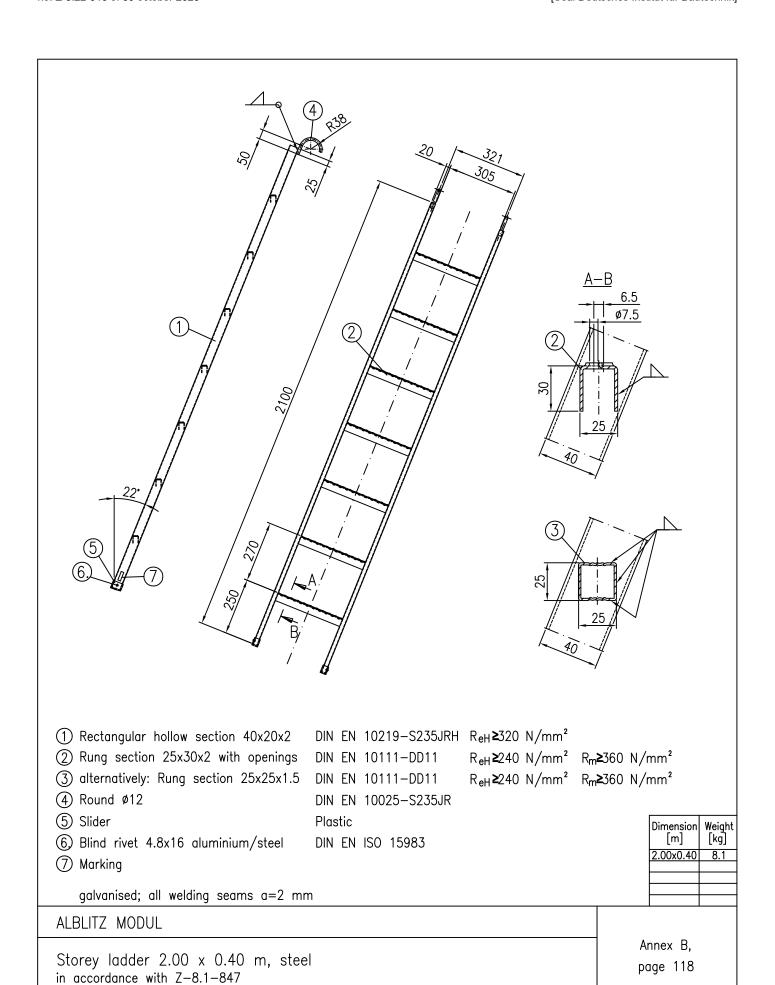
DIN EN 10025-S235JR

| Dimens. | Weight [kg] |
|---------|-------------|
| 0.73 | 3.5 |
| 1.09 | 4.2 |
| | |
| | |

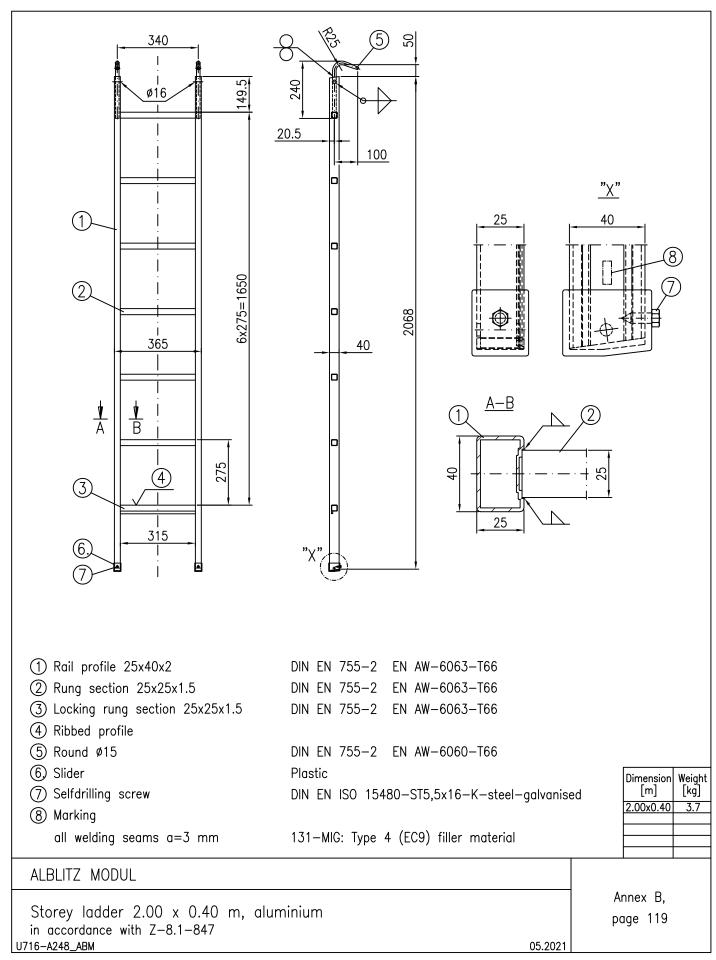
galvanised; all welding seams a=2.5 mm

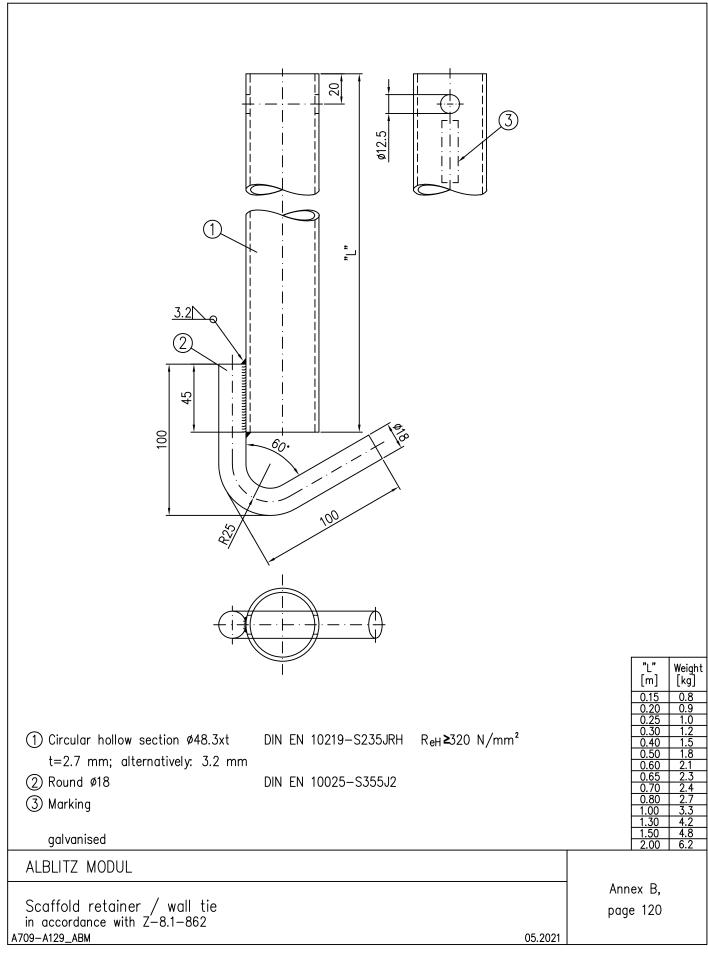
| ALBLITZ MO | JL | ΙL | ΙL |
|------------|----|----|----|
|------------|----|----|----|

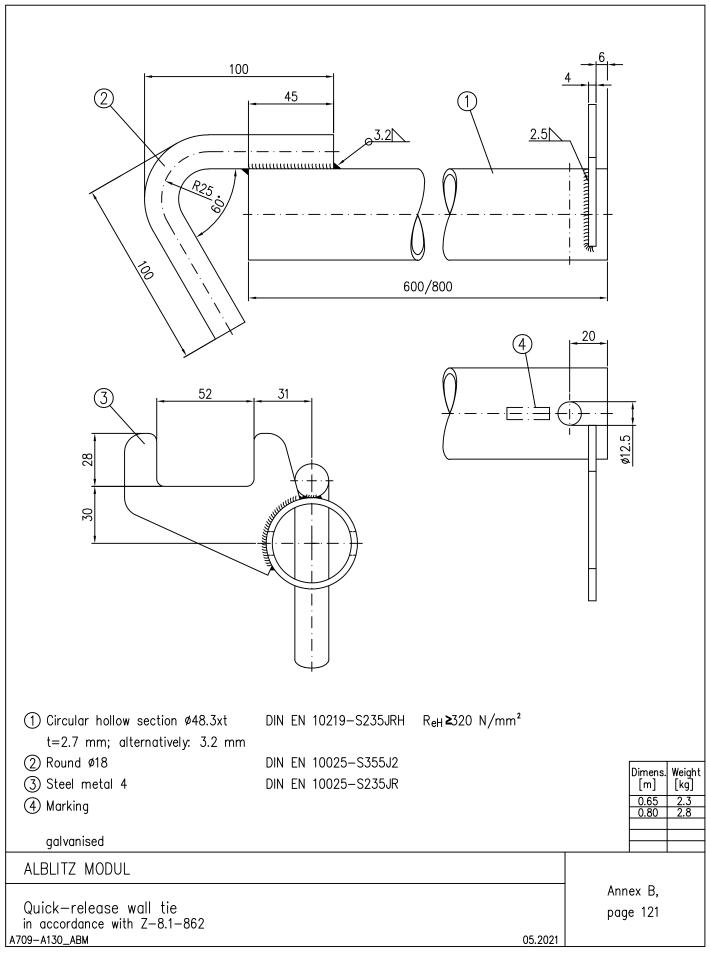
MODUL double end guardrail in accordance with Z-8.22-906 M711-B208_ABM

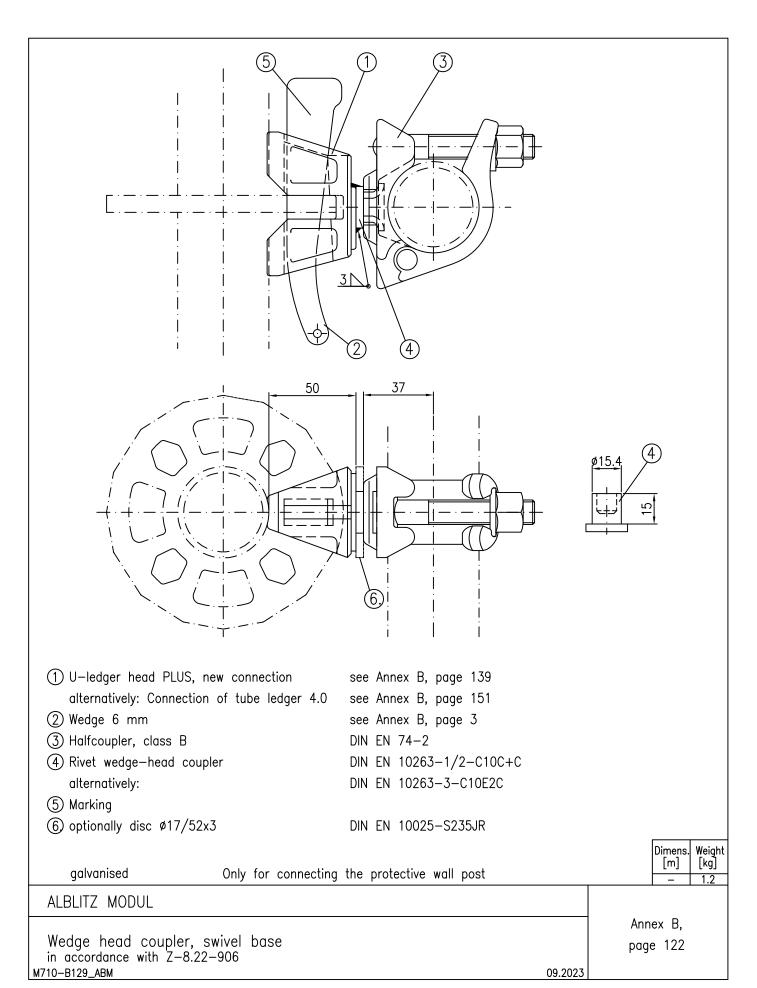


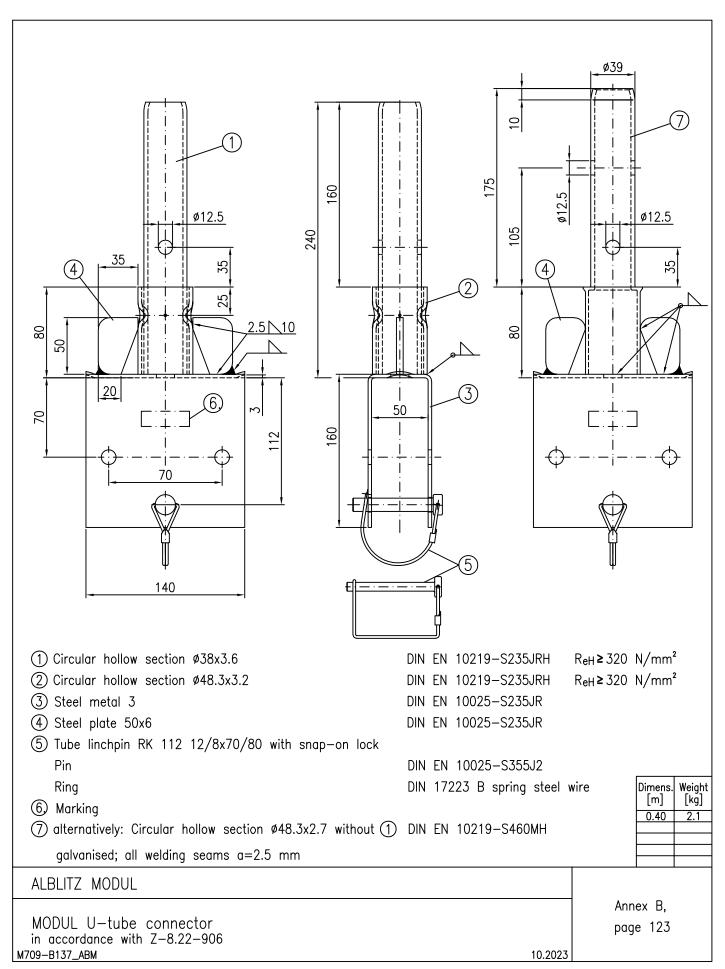
U716-A247_ABM

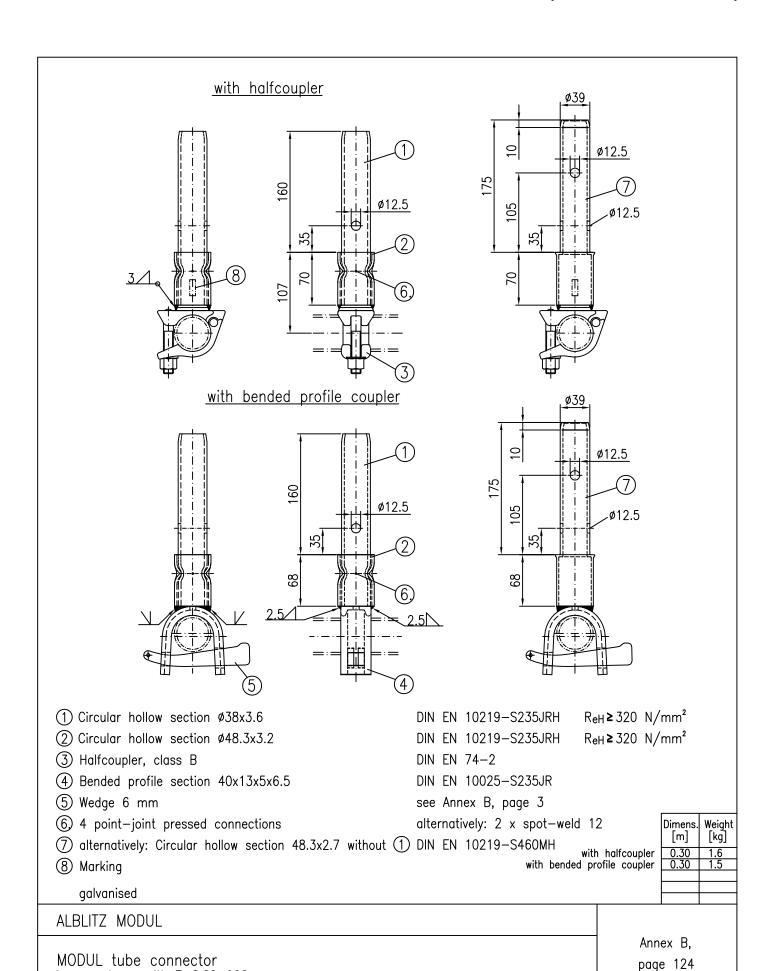








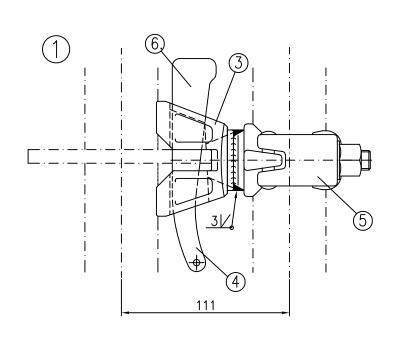


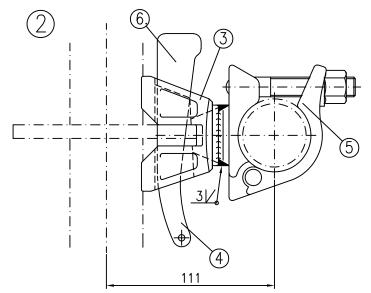


| 71 | ın | 1 | വ | 7 | 22 |
|----|----|-----|--------------|---|----|
| | | - 1 | \ <i>i /</i> | | |

M709-B140_ABM

in accordance with Z-8.22-906





- 1 Wedge-head coupler, rigid, parallel
- ② Wedge-head coupler, rigid, right angle
- 3 Connection of tube ledger alternatively: Connection of tube ledger 4.0 see Annex B, page 151
- 4 Wedge 6 mm
- (5) Halfcoupler, class B
- (6) Marking

galvanised

see Annex B, page 4

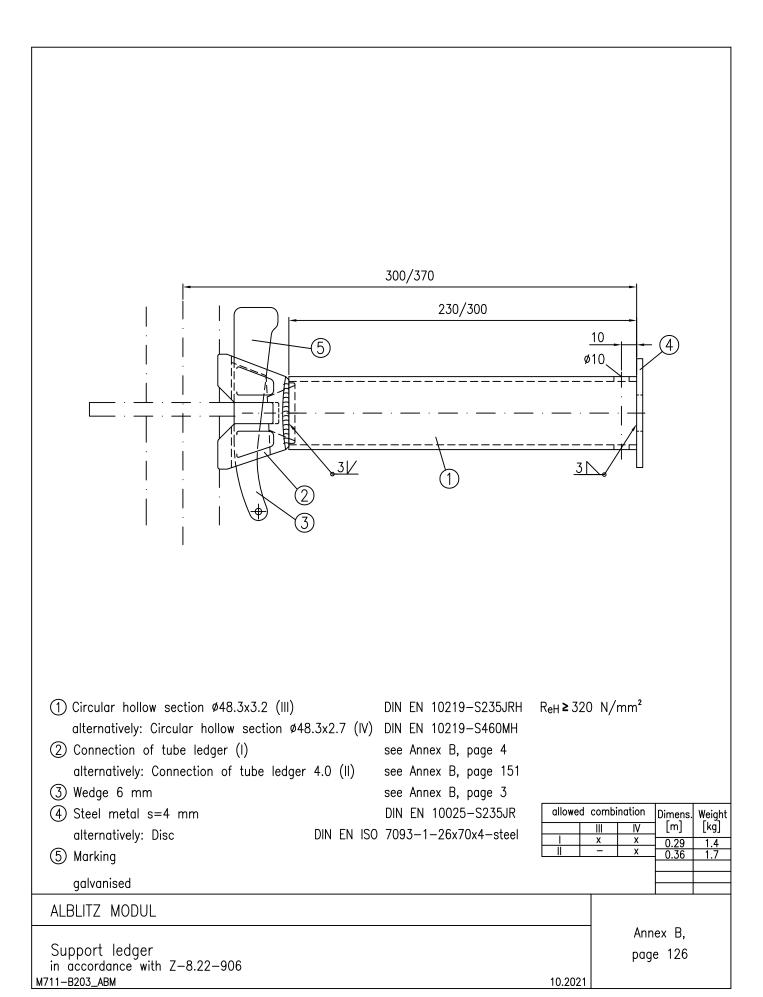
see Annex B, page 3

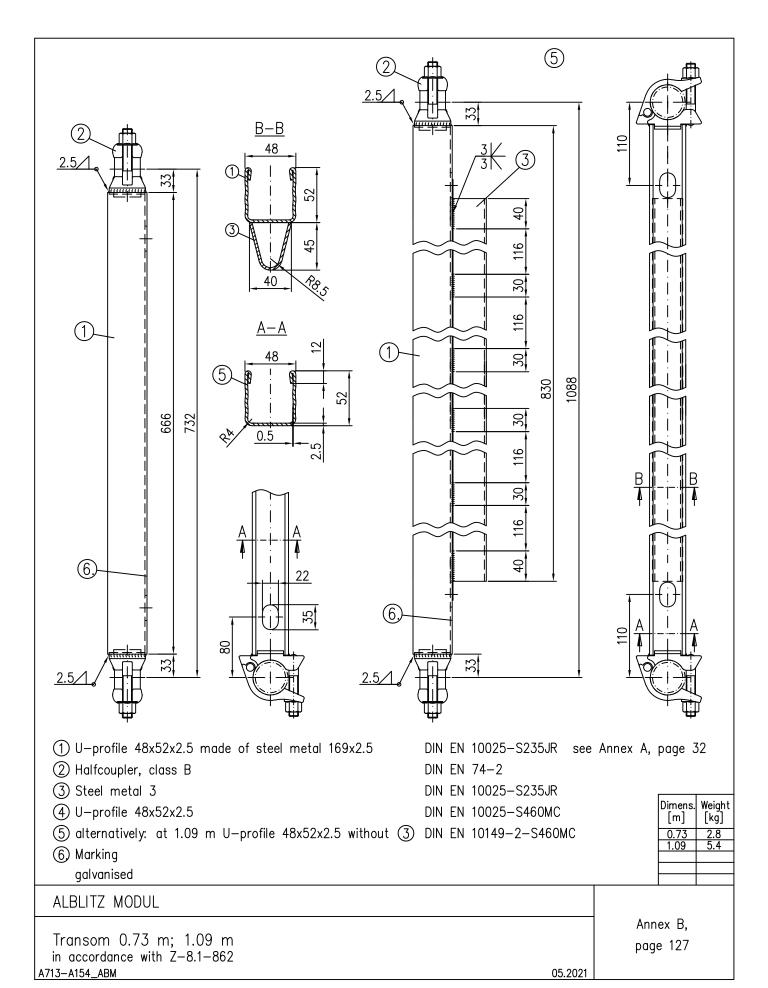
DIN EN 74-2

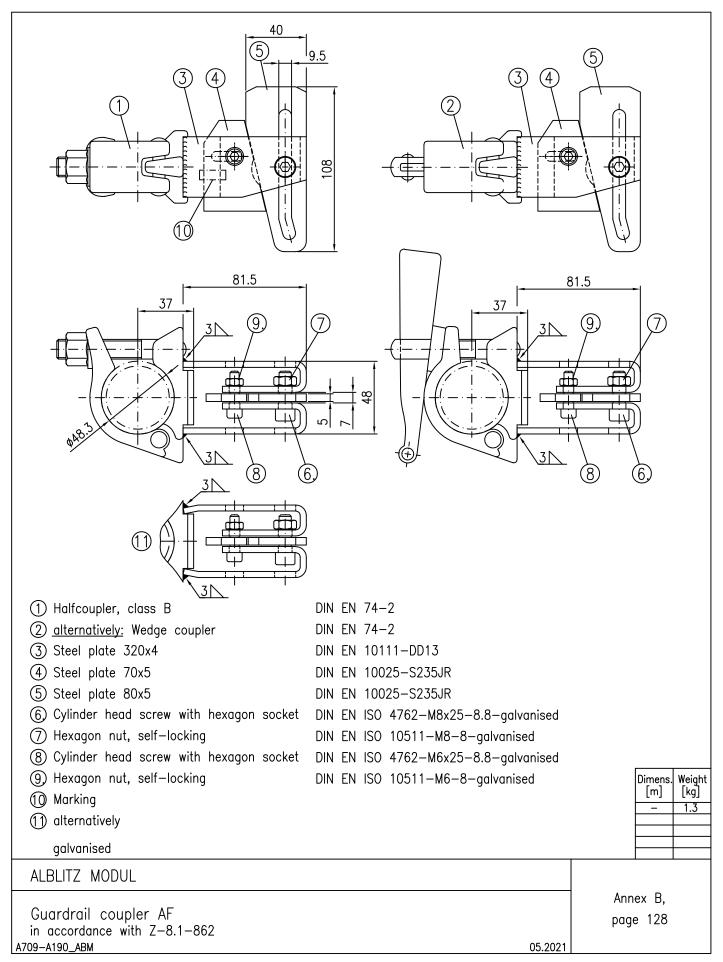
| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| _ | 1.0 |
| | |
| | |

ALBLITZ MODUL

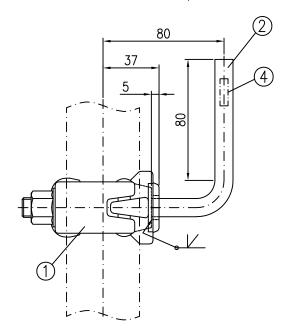
Wedge-head coupler, rigid in accordance with Z-8.22-906 M710-B150_ABM



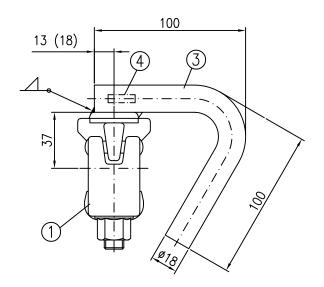




Toeboard coupler



Halfcoupler with hook



1 Halfcoupler, class B

DIN EN 74-2

② Round Ø12

DIN EN 10025-S235JR

(3) Round Ø18

DIN EN 10025-S355J2

(4) Marking

galvanised; all welding seams a=3 mm

() = Discontinued design

| Designation | Weight [kg] |
|-----------------------|----------------|
| Toeboard coupler | 0.6 |
| Halfcoupler with hook | 0.9 |
| | |
| | |

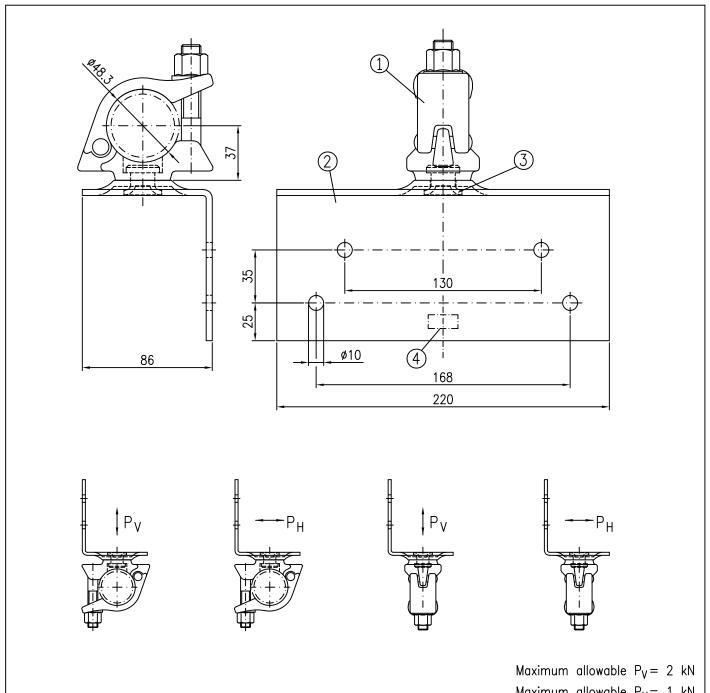
Annex B,

page 129

ALBLITZ MODUL

Toeboard coupler; Halfcoupler with hook in accordance with Z-8.1-862

A709-A191_ABM



Maximum allowable $P_H = 1 kN$

- 1 Halfcoupler, class B
- 2 Steel metal 4
- 3 Rivet squared timber coupler Ø16 DIN EN 10263-1/2-C10C+C alternatively:
- (4) Marking

galvanised

DIN EN 74-2

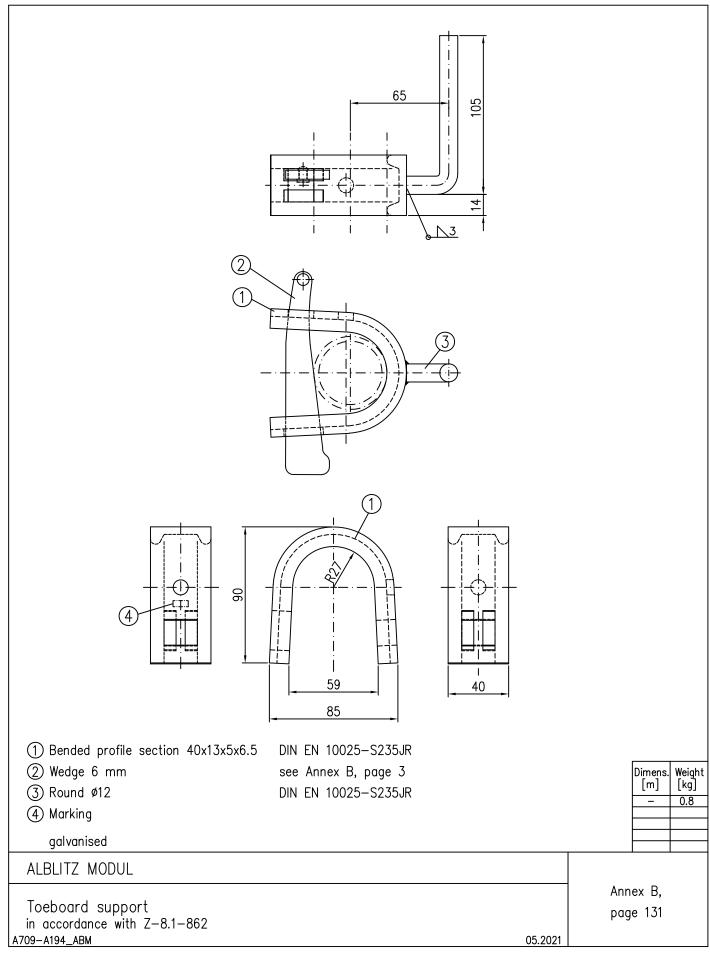
DIN EN 10025-S235JR

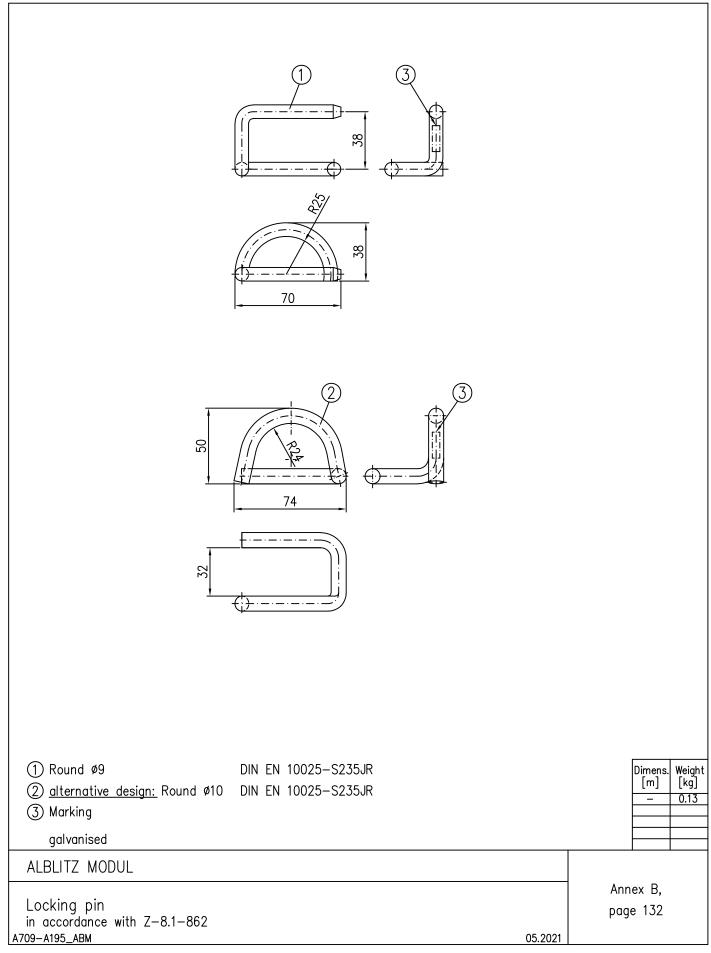
DIN EN 10263-3-C10E2C

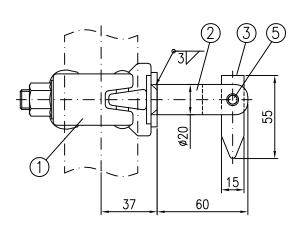
| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| - | 1.8 |
| | |
| | |
| | |
| | |

ALBLITZ MODUL

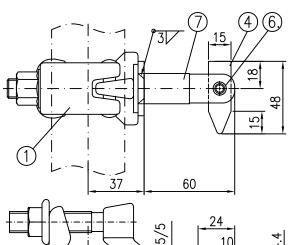
Squared timber coupler in accordance with Z-8.1-862 A709-A192_ABM

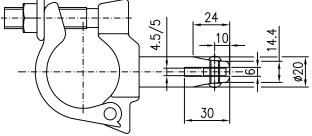






alternatively





- 1) Halfcoupler, class B
- 2 Tilting pin \$20x60 alternatively: changed design 7 DIN EN 10025—S235JR
- (3)(4)Locking lug; s=4 mm; hot-dip galvanised alternatively: s=4.5 mm/5 mm
- (5) Clamping sleeve
- 6 Blind rivet 6x18 aluminium/steel

DIN EN 74-2

DIN EN 10025-S235JR

DIN EN ISO 8752-6x18-steel-galvanised

ISO 15983

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| _ | 0.6 |
| | |
| | |
| | |

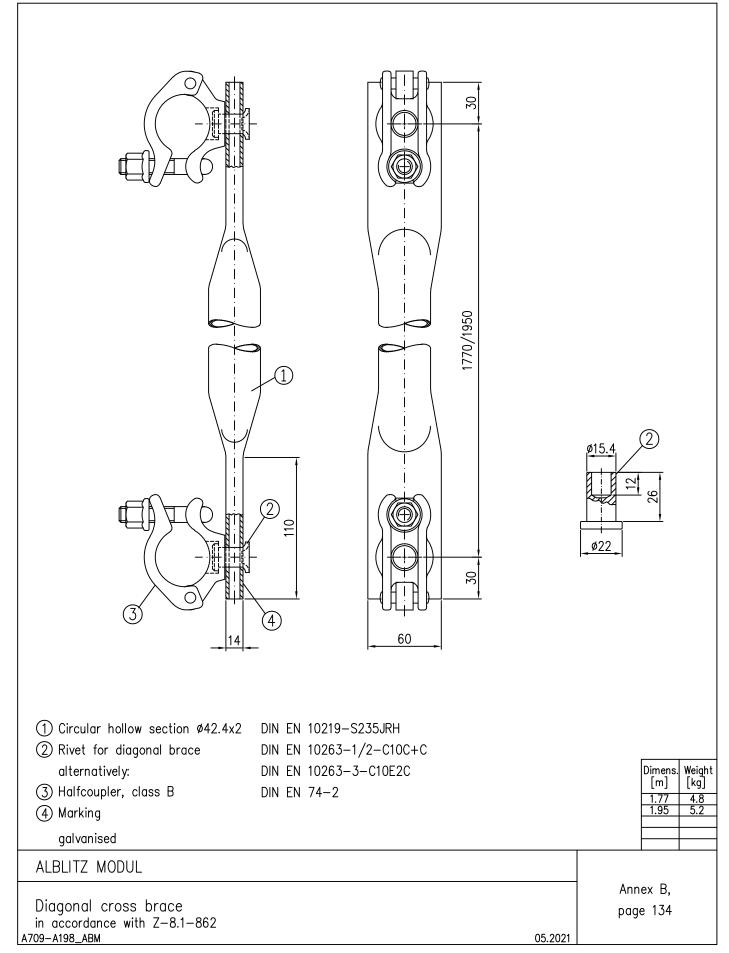
galvanised

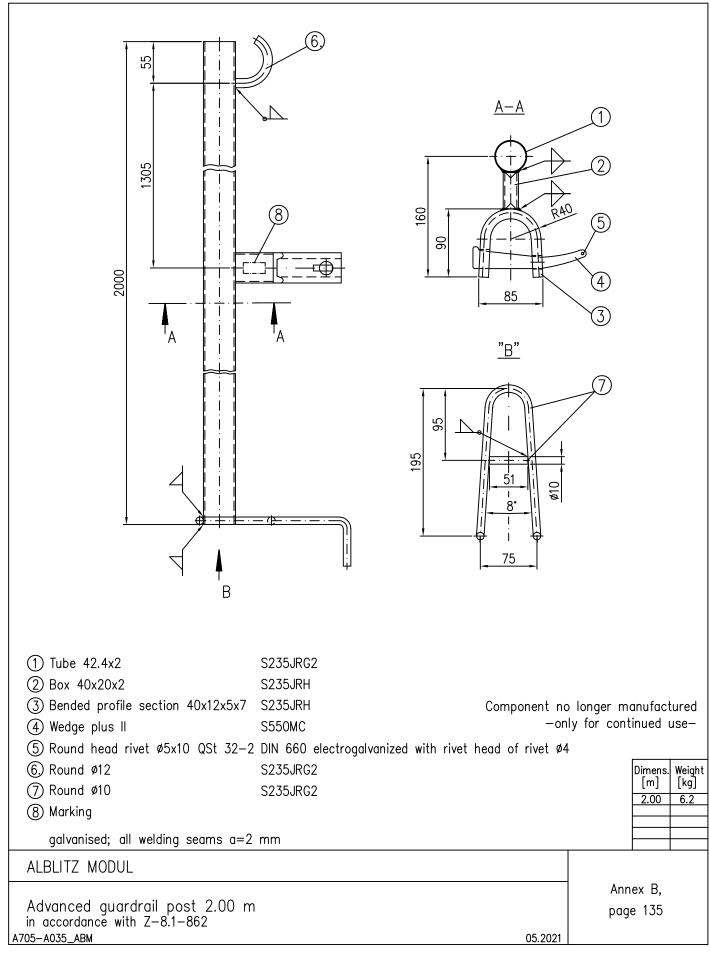
Only for accommodating side protection components

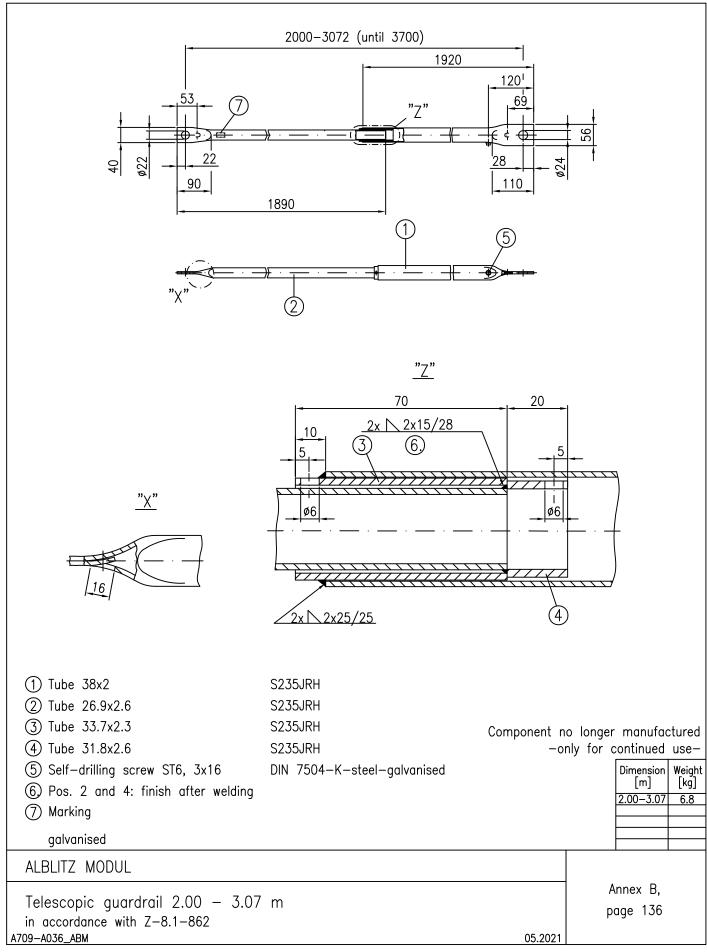
| ALBL | _ITZ | MODUL | |
|------|------|-------|--|
| | | | |

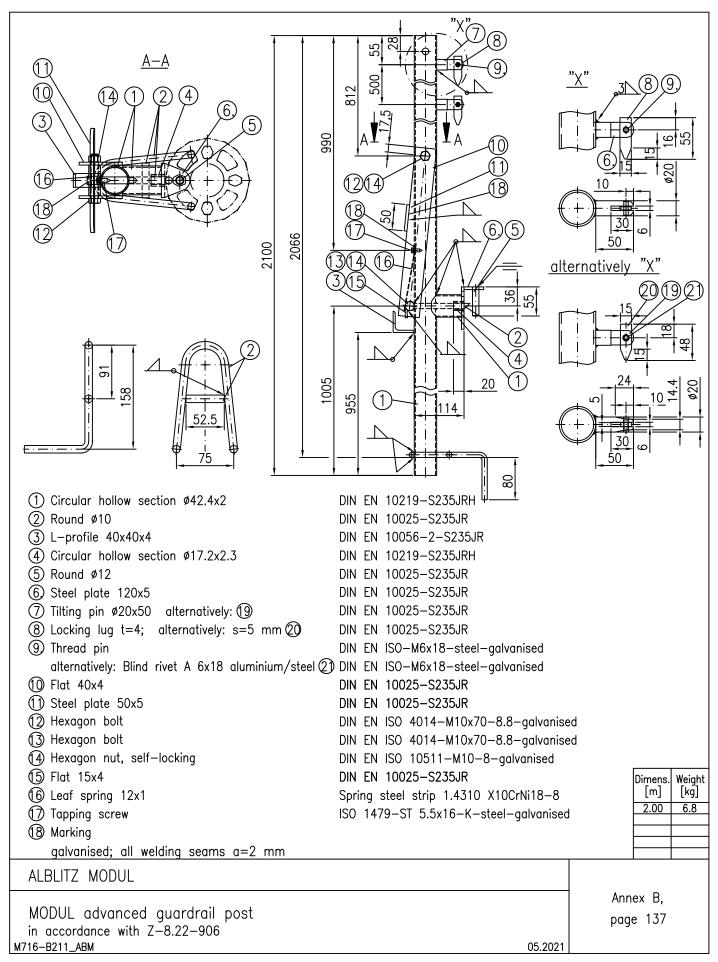
Putlog coupler

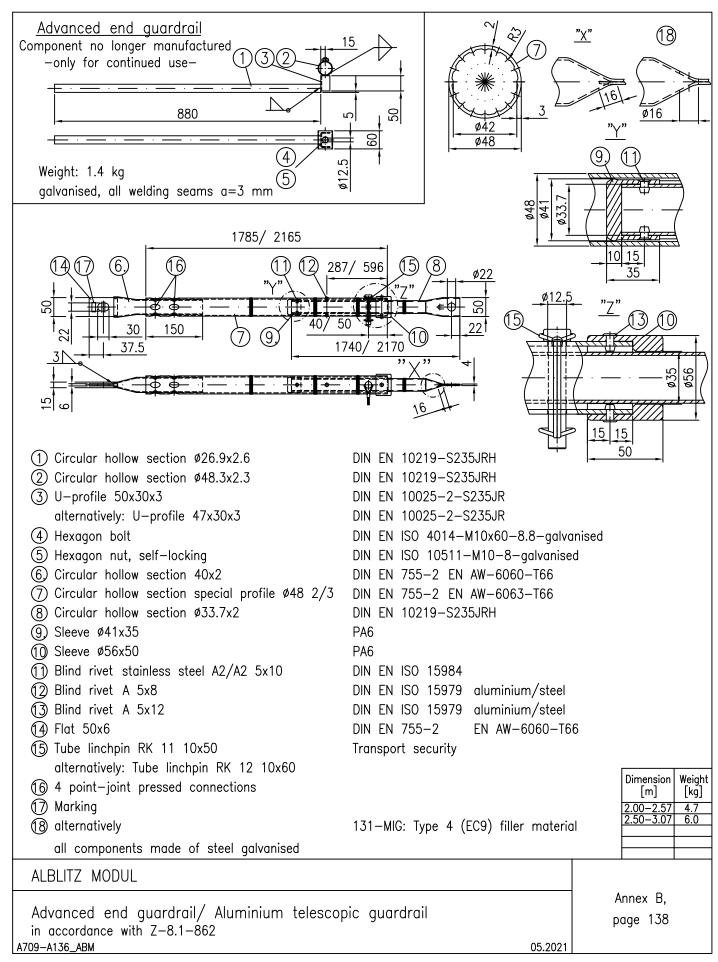
in accordance with Z-8.1-862 A709-A196_ABM

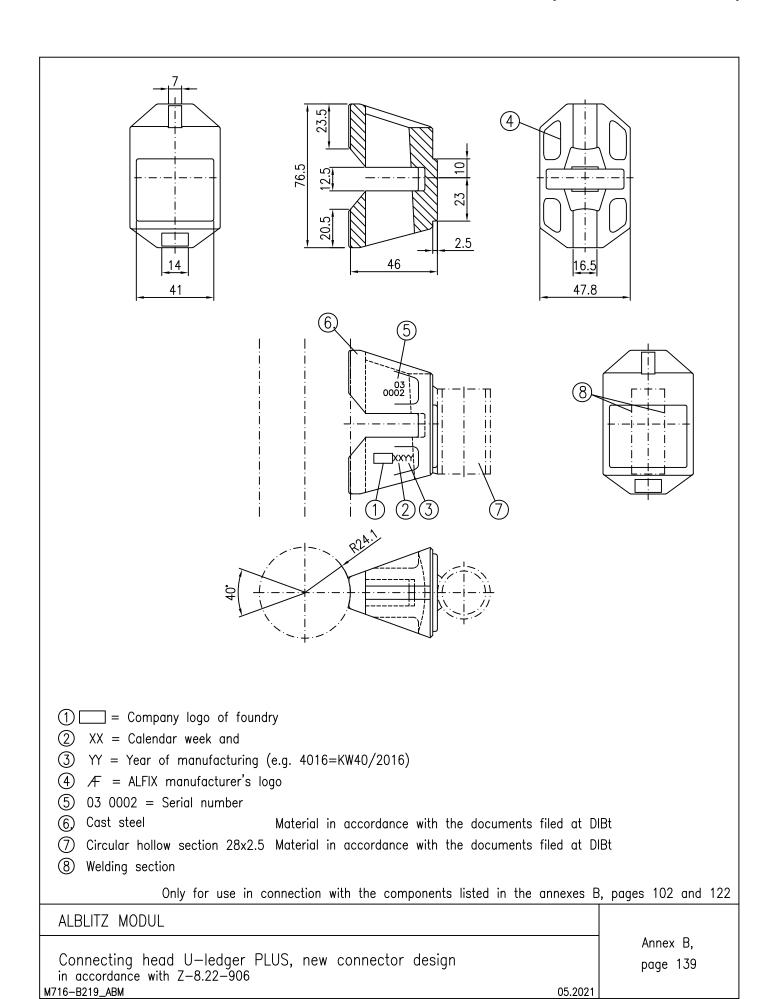


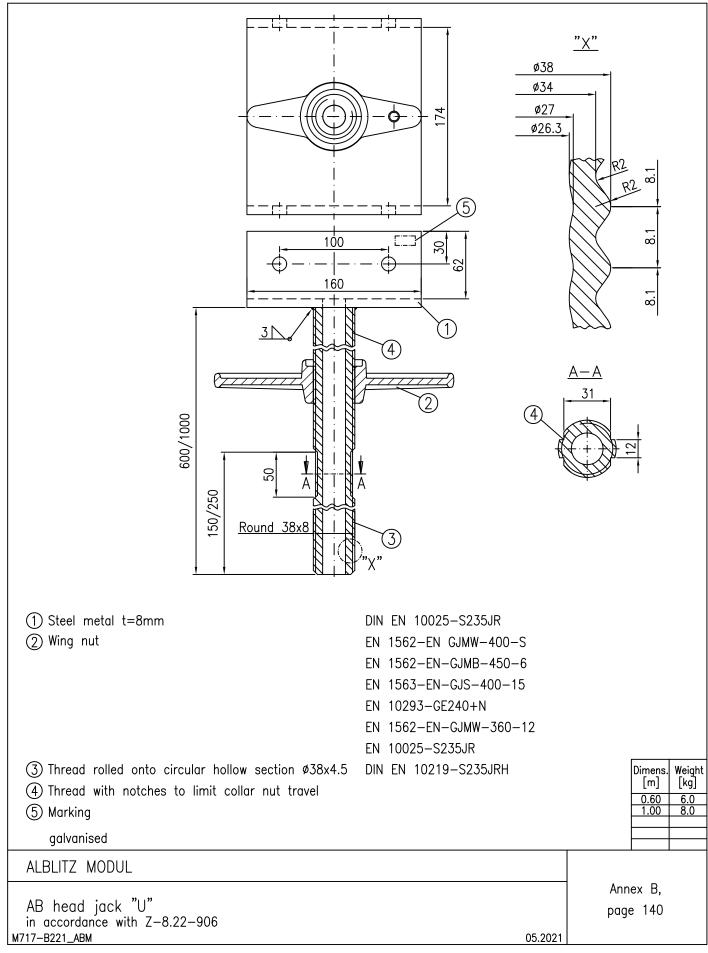


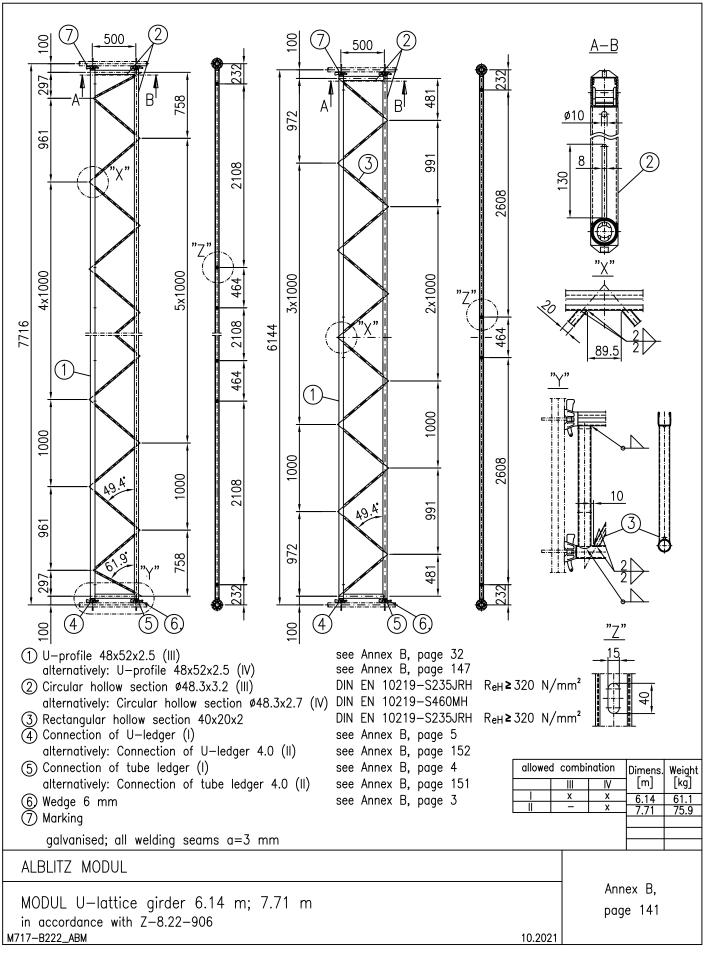


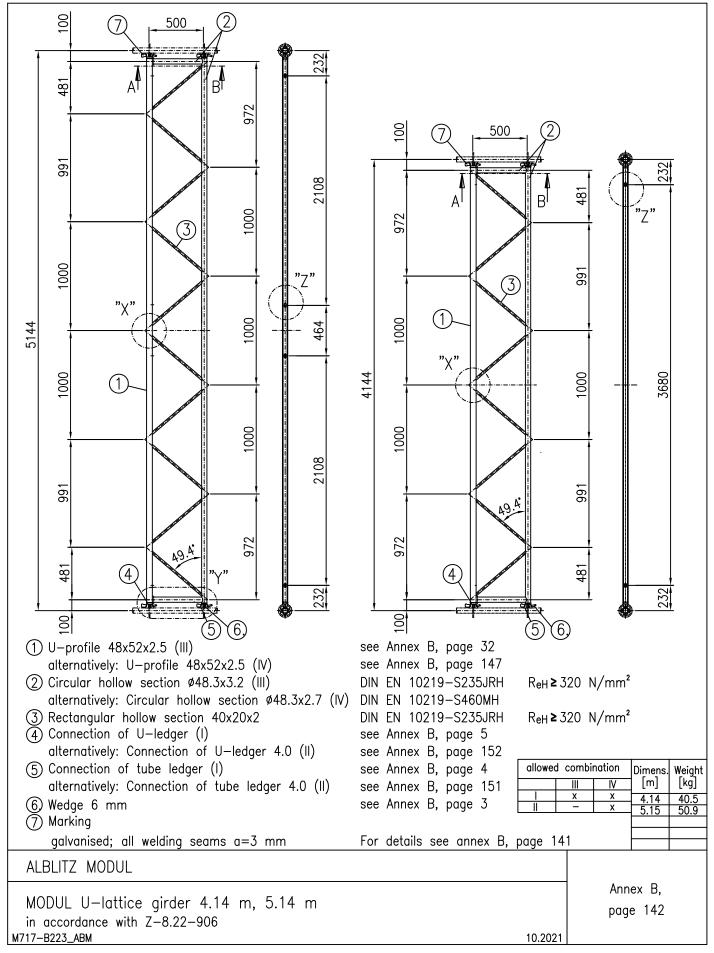


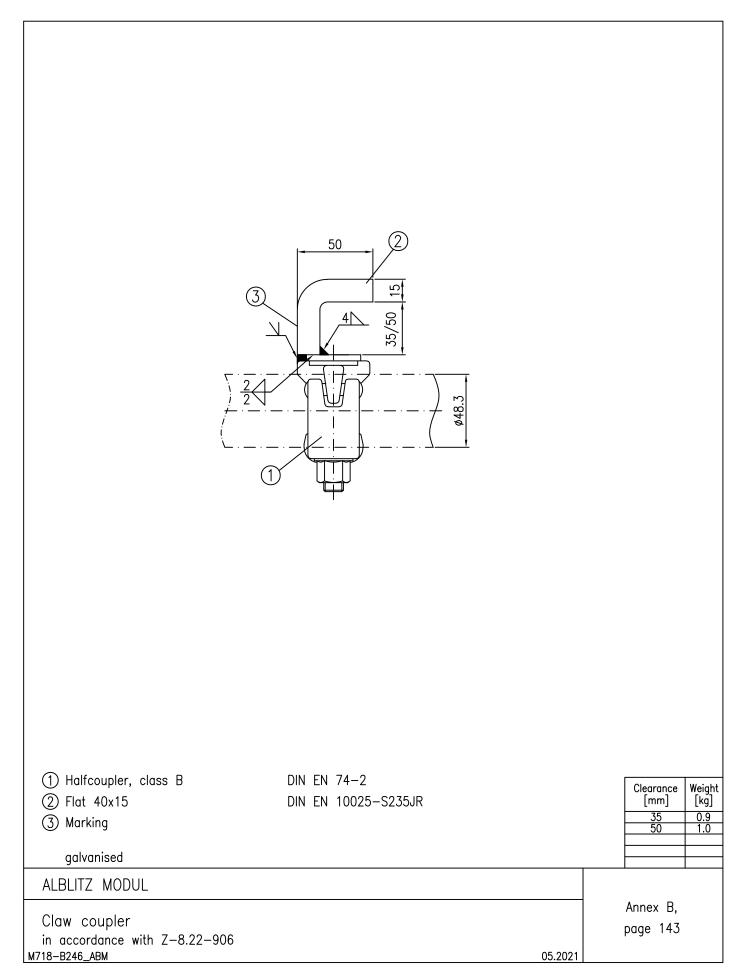


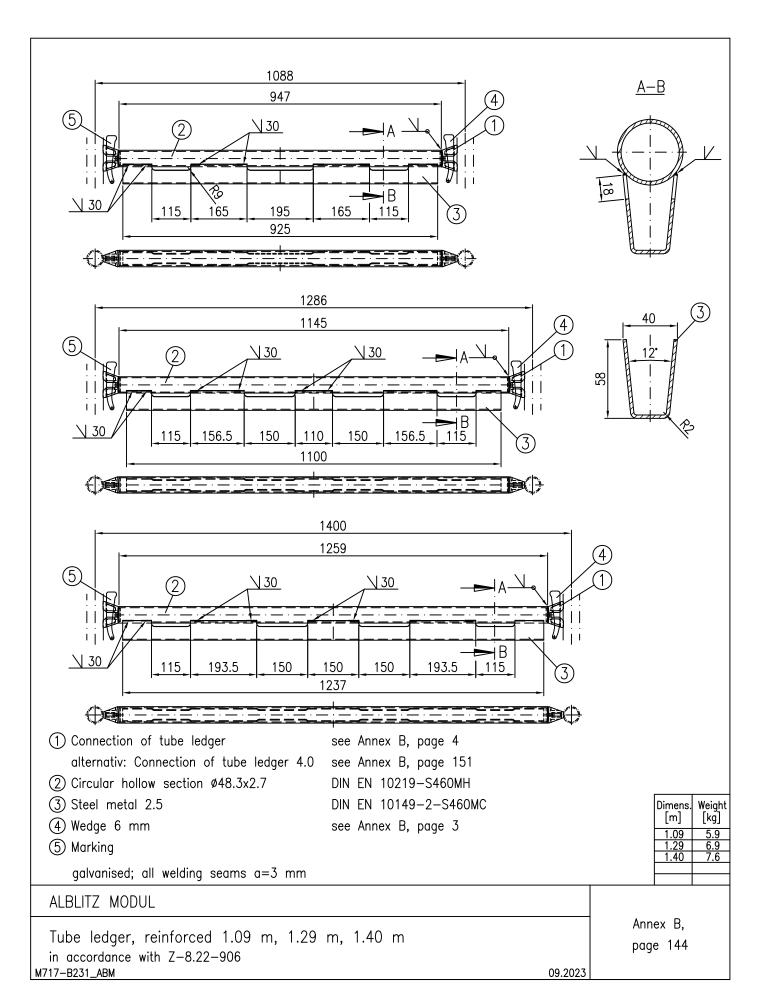


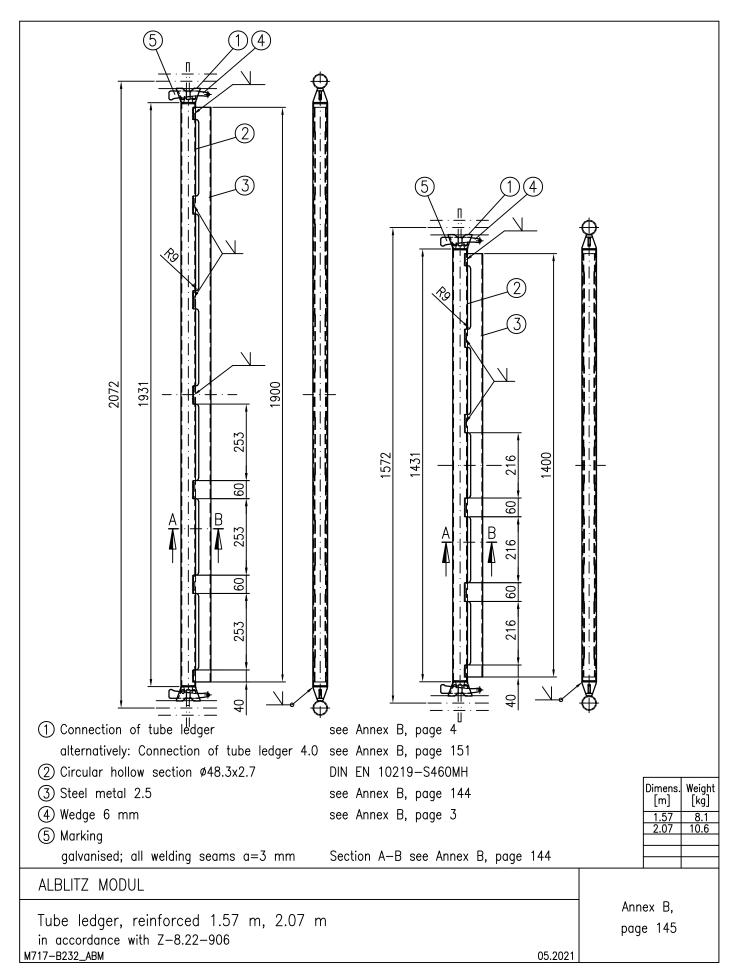


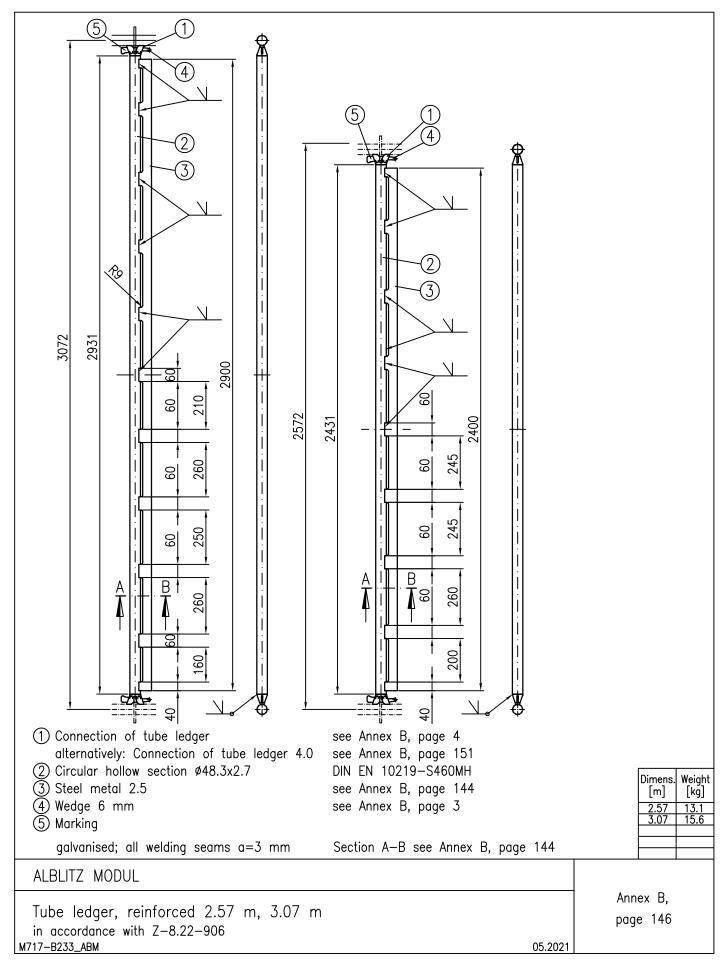


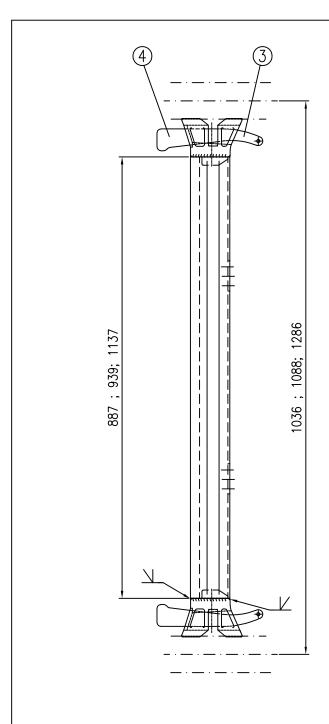


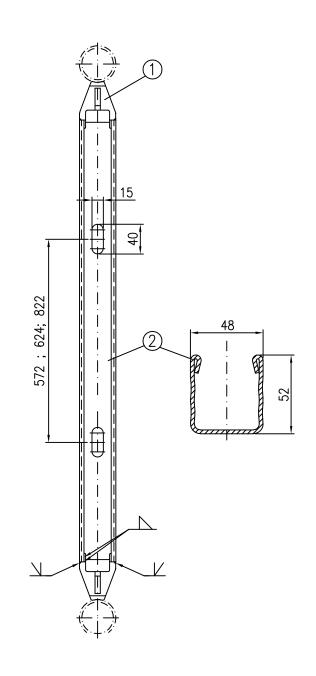












- 1 Connection of U-ledger alternatively: Connection of U-ledger 4.0
- ② U-profile 48x52x2.5
- 3 Wedge 6 mm
- (4) Marking

galvanised; all welding seams a=3 mm

see Annex B, page 5 see Annex B, page 152

DIN EN 10149-2-S460MC see Annex B, page 3

| Dimens. [m] | Weight [kg] |
|----------------|------------------|
| 1.04 | 4.2 |
| 1.09 | 4.4 |
| 1.29 | 5.1 |
| | |
| | |

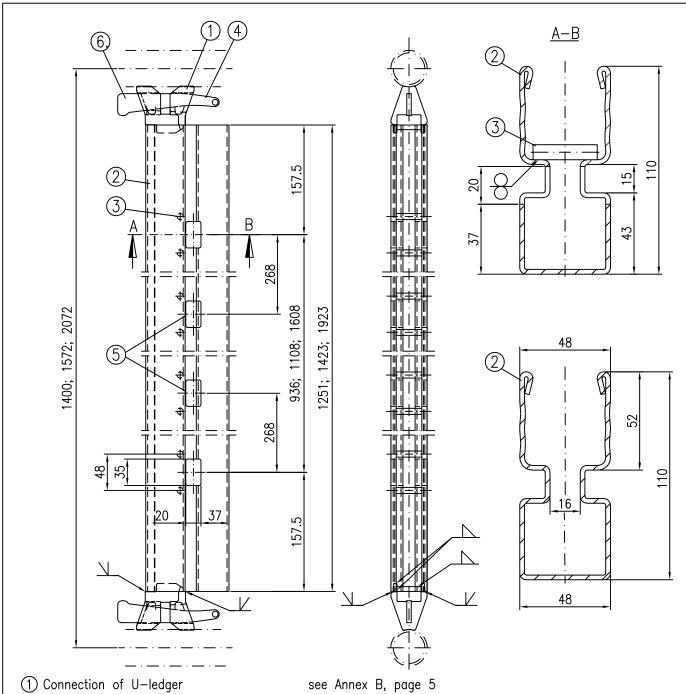
Annex B,

page 147

| ALE | 3LIT2 | 7 M | ODUL |
|-----|-------|-----|------|
| | | | |

U-ledger 1.04 m; 1.09 m; 1.29 m in accordance with Z-8.22-906

M717-B236_ABM



- (1) Connection of U-ledger alternatively: Connection of U-ledger 4.0
- 2 Transom profile with integrated joist
- (3) Round Ø8
- 4 Wedge 6 mm
- (5) only for 1.40 m
- (6) Marking

galvanised; all welding seams a=3 mm

see Annex B, page 5 see Annex B, page 152 DIN EN 10149-2-S460MC

DIN EN 10025-S235JRC+C

see Annex B, page 3

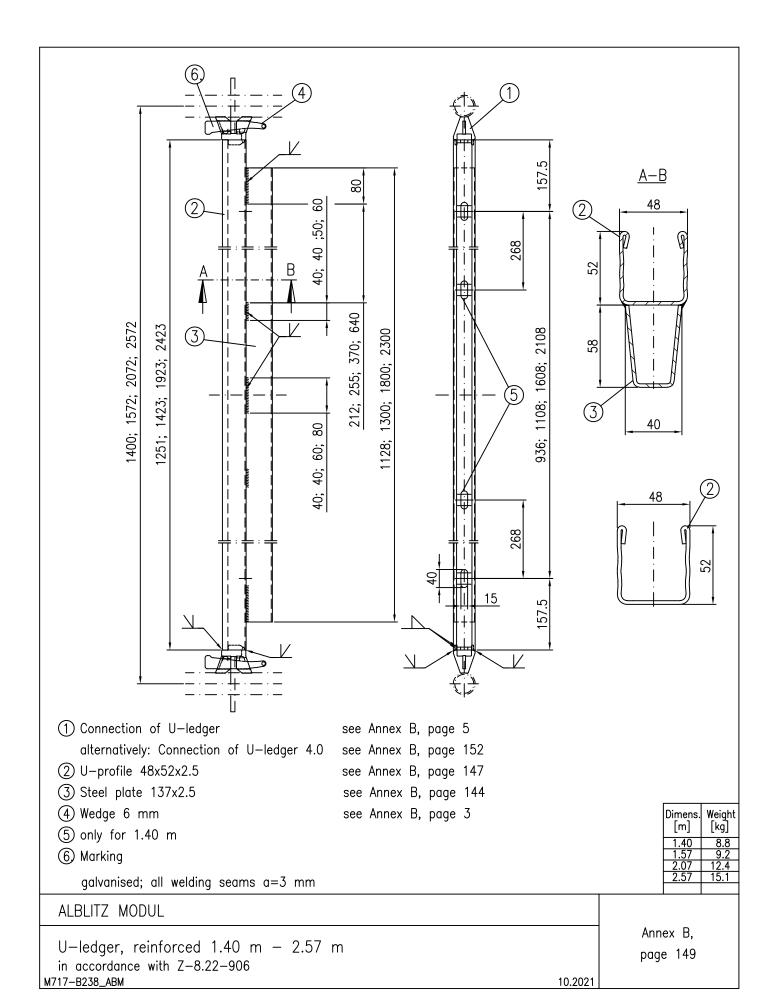
| | Dimens. [m] | Weight [kg] |
|---|----------------|----------------|
| [| 1.40 | 8.8 |
| | 1.57 | 9.9 |
| [| 2.07 | 13.0 |
| | | |
| Ī | | |

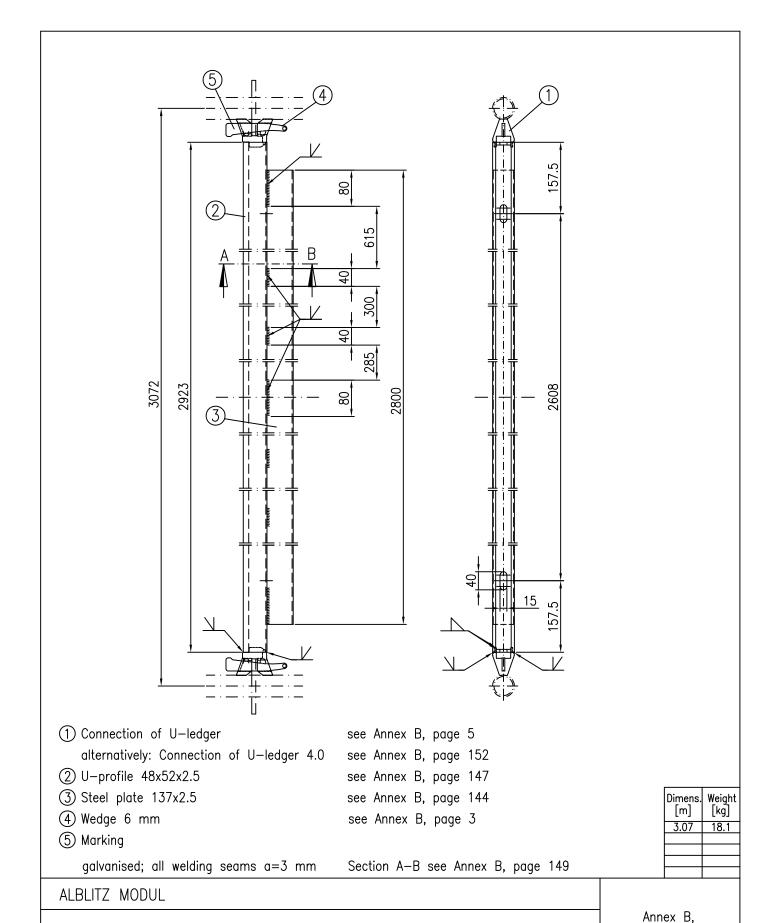
ALBLITZ MODUL

U-ledger with integrated joist 1.40 m - 2.07 m in accordance with Z-8.22-906
M717-B237_ABM

Annex B, page 148

.



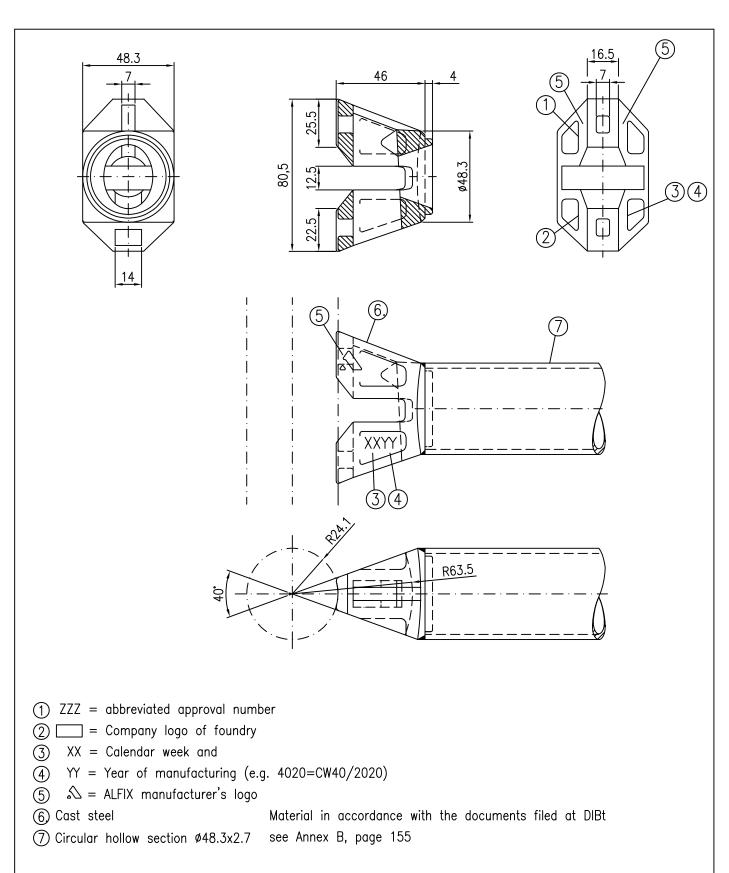


M717-B239_ABM

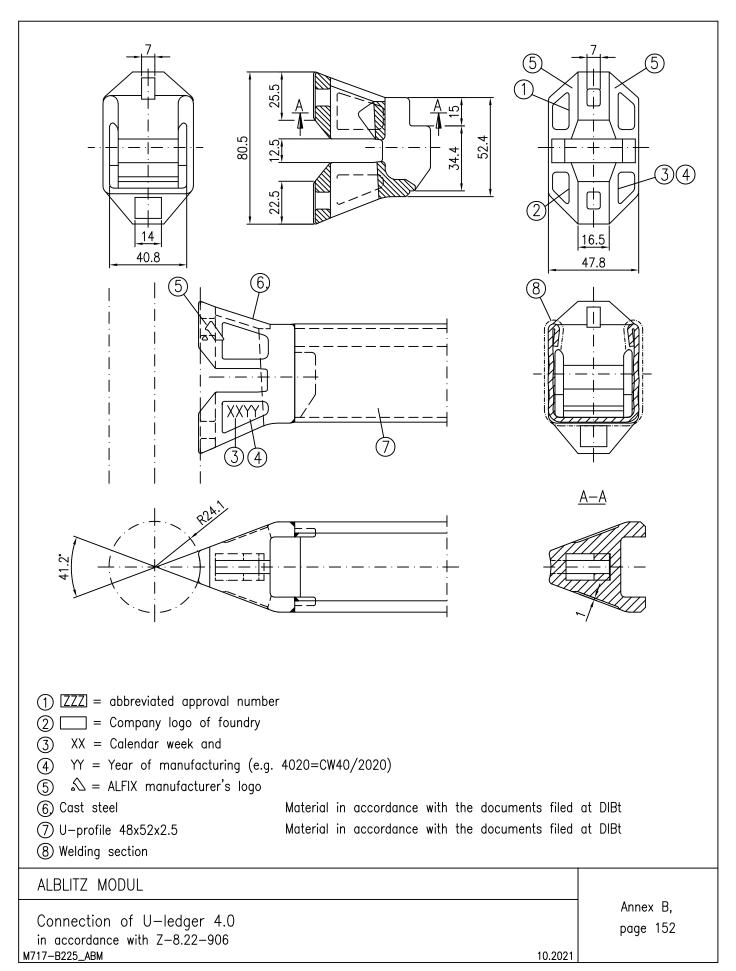
U-ledger, reinforced 3.07 m

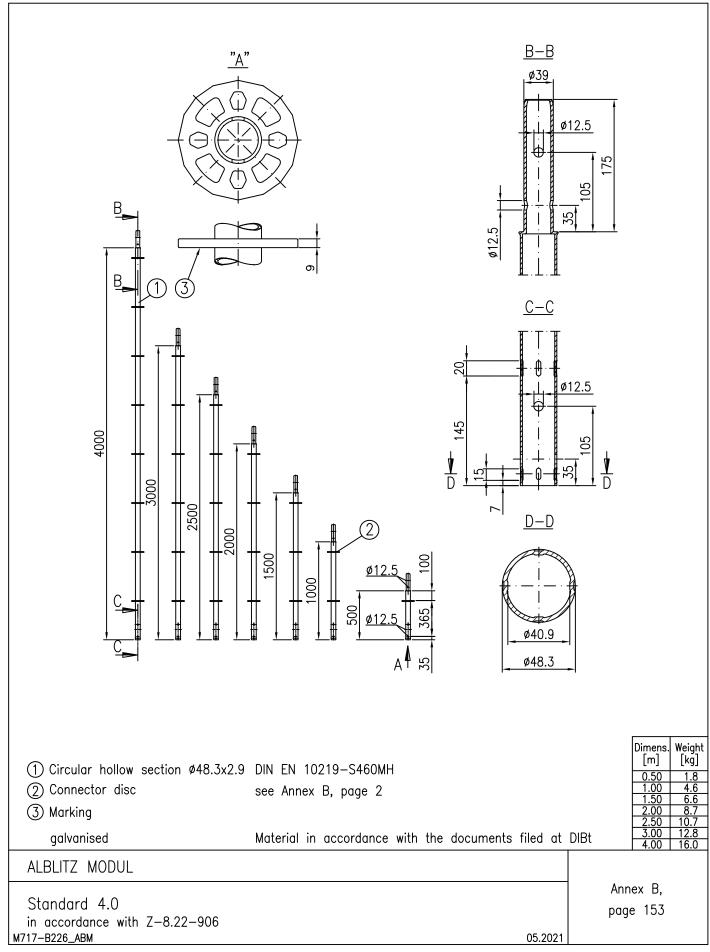
in accordance with Z-8.22-906

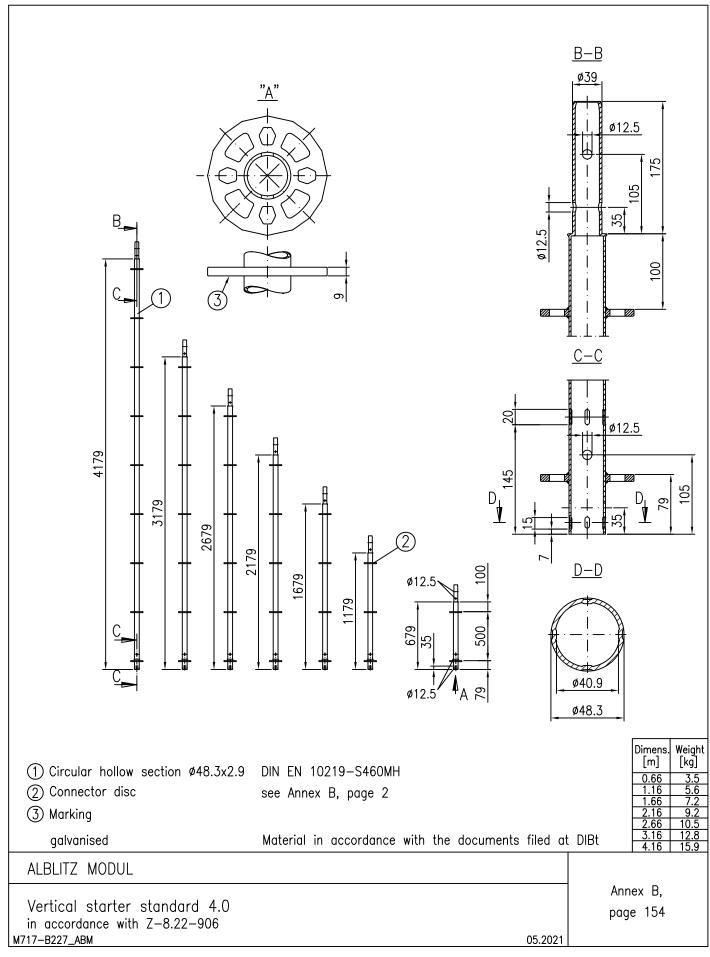
page 150

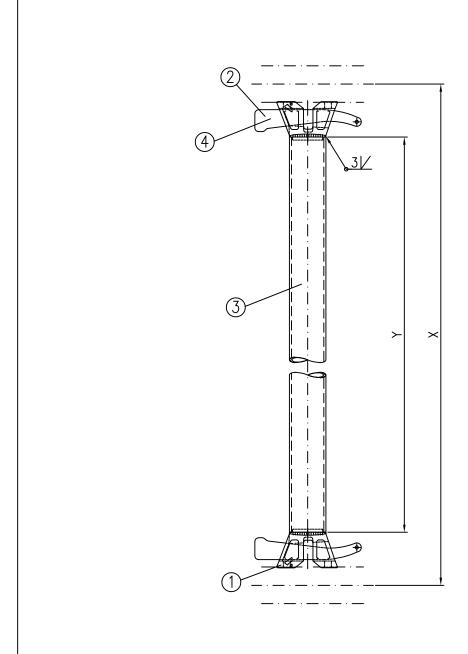


| ALBLITZ MODUL | | |
|---|--------|----------------------|
| Connection of tube ledger 4.0 in accordance with Z-8.22-906 | | Annex B, page 151 |
| M717-B224_ABM C | 9.2021 | |









| | | 0.36 | 356 | 215 | 1.4 |
|---------------------------------------|-------------------------|------|------|------|-----|
| | | 0.37 | 366 | 225 | 1.6 |
| | | 0.39 | 390 | 249 | 1.7 |
| | | 0.45 | 450 | 309 | 1.9 |
| (1) Connection of tube ledger 4.0 | see Annex B, page 151 | 0.73 | 732 | 591 | 2.7 |
| (1) Confidential of tube leager 4.0 | see Allilex D, page 131 | 1.04 | 1036 | 895 | 3.6 |
| ② Wedge 6 mm | see Annex B, page 3 | 1.09 | 1088 | 947 | 3.7 |
| | occ Allinex B, page o | 1.29 | 1286 | 1145 | 4.4 |
| ③ Circular hollow section Ø48.3x2.7 | DIN EN 10219-S460MH | 1.40 | 1400 | 1259 | 4.7 |
| _ | 5111 E11 10210 01001111 | 1.57 | 1572 | 1431 | 5.2 |
| (4) Marking | | 2.07 | 2072 | 1931 | 6.7 |
| · · · · · · · · · · · · · · · · · · · | | 2.57 | 2572 | 2431 | 8.1 |
| | | 7.07 | 7070 | 0074 | ~ - |

galvanised ALBLITZ MODUL

Tube ledger 4.0

in accordance with Z-8.22-906 M717-B229_ABM

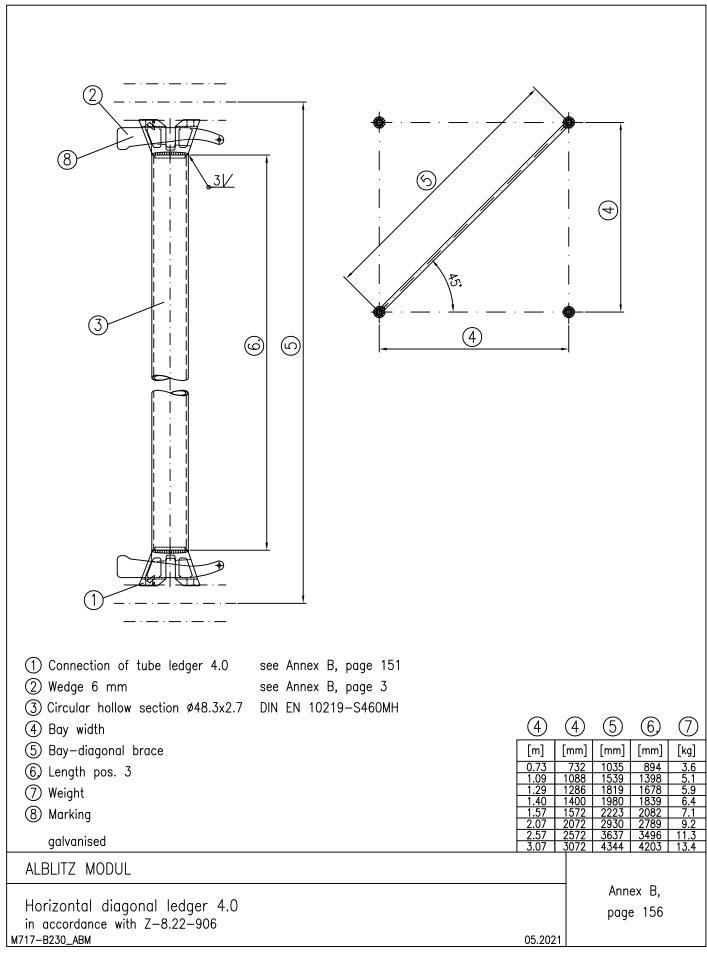
Annex B, page 155

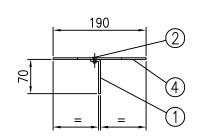
"X" [m]

"X" [mm]

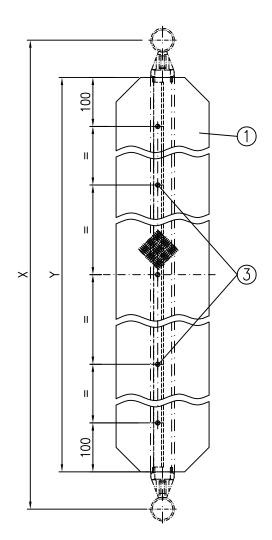
[mm]

Weight [kg]





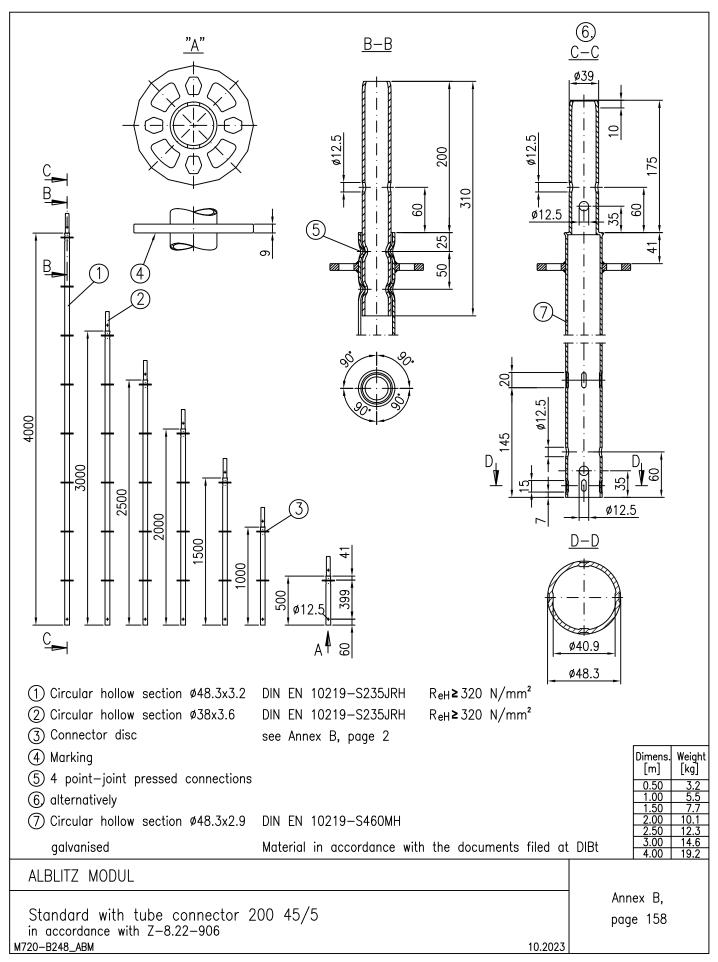
| | X [m] | X [mm] | Y [mm] | Weight [kg] |
|---|----------|-----------|-----------|----------------|
| Ī | 0.73 | 732 | 591 | 1.2 |
| | 1.09 | 1088 | 947 | 2.0 |
| | 1.29 | 1286 | 1145 | 2.4 |
| | 1.40 | 1400 | 1259 | 2.6 |
| | 1.57 | 1572 | 1431 | 3.0 |
| | 2.07 | 2072 | 1931 | 4.0 |
| | 2.57 | 2572 | 2431 | 5.0 |
| | 3.07 | 3072 | 2931 | 6.0 |

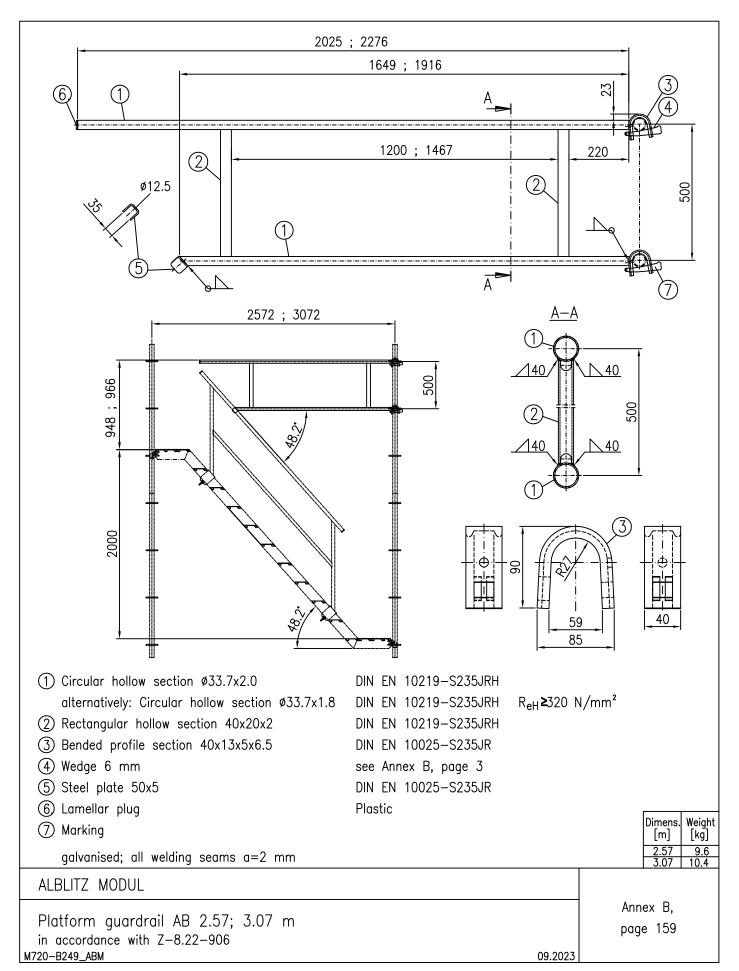


- 1 Chequer plate with 5 bar pattern 2.5/3.3x190 alternatively: Chequer plate with 5 bar pattern 2.5/4.0x190 DIN EN 1386 EN AW-5754 H111/ H114
- (2) Blind rivet
- ③ only for 2.57 m and 3.07 m
- 4 Marking

DIN EN 1386 EN AW-5083 H224 DIN EN ISO 15979-A5x12 EN AW-5754 H112

| ALBLITZ MODUL | | |
|---|---------|----------------------|
| MODUL gap cover, T-shaped in accordance with Z-8.22-906 | | Annex B, page 157 |
| M719-B247_ABM | 05.2021 | |





Product marking code key

XX Ü 906/932 AF XX

XX = Supplier number

 \ddot{U} = Mark of conformity \ddot{U}

906/932 = abbreviated approval number

AF = ALFIX manufacturer's logo

XX = Year of manufacturing

| Year | XX |
|------|------|
| 2015 | 15 |
| 2016 | 16 |
| 2017 | 17 |
| 2018 | 18 |
| 2019 | 19 |
| 2020 | 20 |
| etc. | etc. |
| | |

For further marking codes, please refer to annex B, pages 2, 4-7, 151, 152

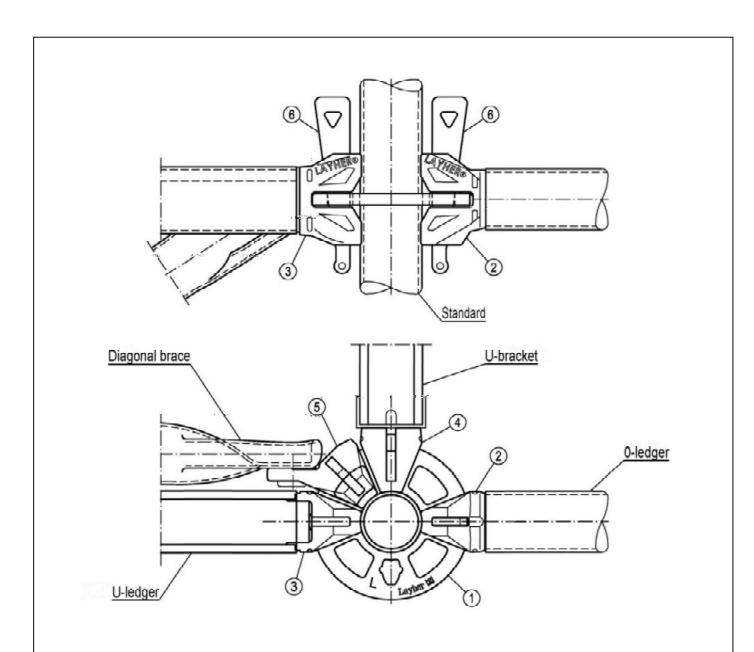
| ALBLITZ MODUL | |
|---------------|--|
|---------------|--|

Product marking code key in accordance with Z-8.22-906 M716-B220_ABM

Annex B, page 160

| Page 1

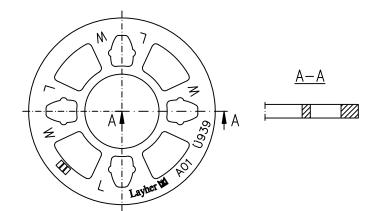
05.2021



| | | "Lightweight design" | "K2000+ design" |
|-------------------------------------|------------------------------|----------------------|-----------------|
| ① Connector disc | (in accordance with Annex B, | page 165) | page 171) |
| ② Connector head for O—ledger | (in accordance with Annex B | page 166) | page 172) |
| 3 Connector head for U—ledger | (in accordance with Annex B, | page 167) | page 173) |
| 4 Connector head for U—bracket | (in accordance with Annex B, | page 168) | page 174) |
| ⑤ Connector head for diagonal brace | (in accordance with Annex B, | page 169) | page 175) |
| 6 Wedge | (in accordance with Annex B, | page 170) | page 176) |

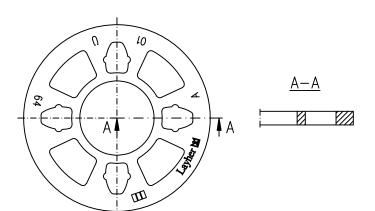
Corrosion protection: hot-dip galvanization in accordance with EN ISO 1461

| ALBLITZ MODUL | | |
|---|---------|----------------------|
| Overview nodes "lightweight design" and "K2000+ design" in accordance with Z-8.22-949 | | Annex B, page 161 |
| ABM721-B001 | 05.2021 | |



"Lightweight design"

<u>Connector disc die−cut Ø 124</u> in accordance with Annex B, page 165



"K2000+ design"

<u>Connector disc die—cut Ø 124</u>
in accordance with Annex B, page 171

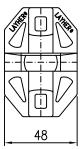
| ALBLITZ MODUL |
|---------------|
|---------------|

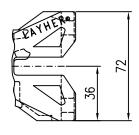
Overview connector discs "Lightweight design" and "K2000+ design" in accordance with Z-8.22-949

ABM721-B002

Annex B, page 162

page 10

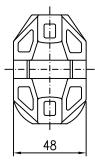


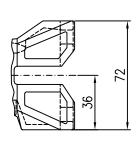


<u>Ledger connector head:</u>

"Lightweight design"

with recesses on the sides and ring-shaped end faces, 72 mm high in accordance with Annex B, pages 166, 167, 168





<u>Ledger connector head:</u>

"K2000+ design"

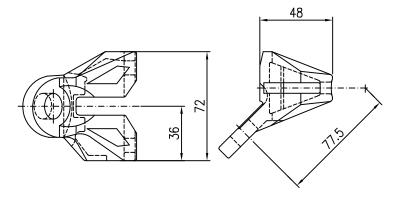
with recesses on the sides and ring-shaped end faces, 72 mm high in accordance with Annex B, pages 172, 173, 174

| Δ | LB | 1 | Г7 | М | Λ | Π | П | 1 |
|--------|----|---|-----|-----|---|----|---|---|
| \neg | ᄓ | | 1 🔼 | IVI | v | יט | U | ᆫ |

Overview connector heads for ledgers "Lightweight design" and "K2000+ design" in accordance with Z-8.22-949

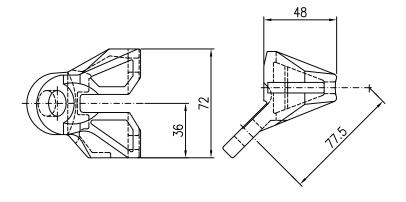
ABM721-B003

Annex B,



"Lightweight design"

only in connection with diagonal brace from tube Ø48.3 mm head 72 mm in height, in accordance with Annex B, page 169



"K2000+ design"

only in connection with diagonal brace from tube Ø48.3 mm head 72 mm in height, in accordance with Annex B, page 175

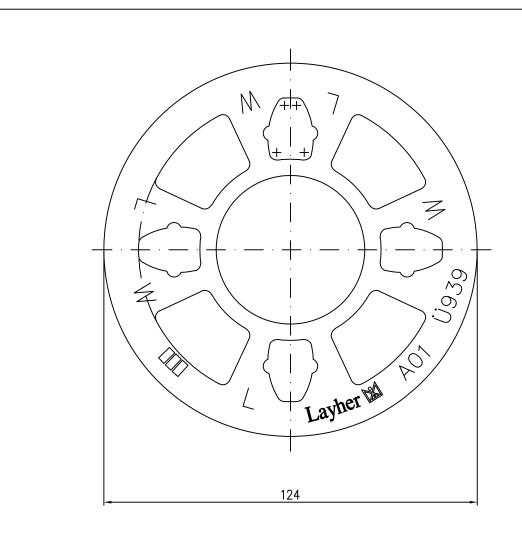
| | ΑL | _BLI | TΖ | MODUL |
|--|----|------|----|-------|
|--|----|------|----|-------|

Overview connector heads for diagonal braces "Lightweight design" and "K2000+ design" in accordance with Z-8.22-949

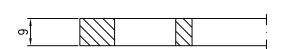
ABM721-B004

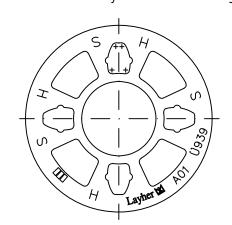
05.2021

Annex B, page 164

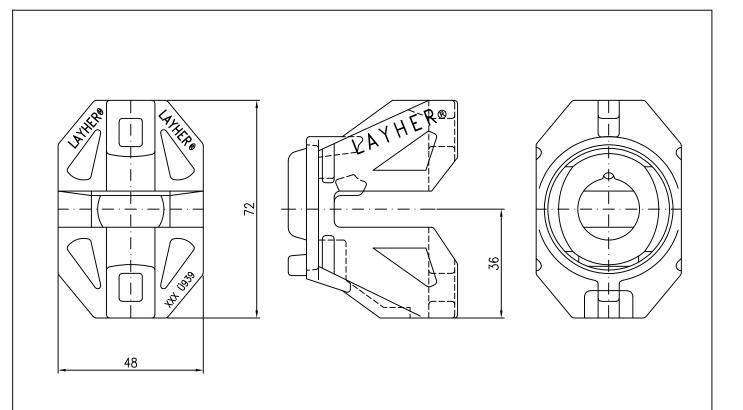


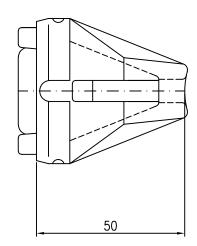
Alternatively with HS embossing



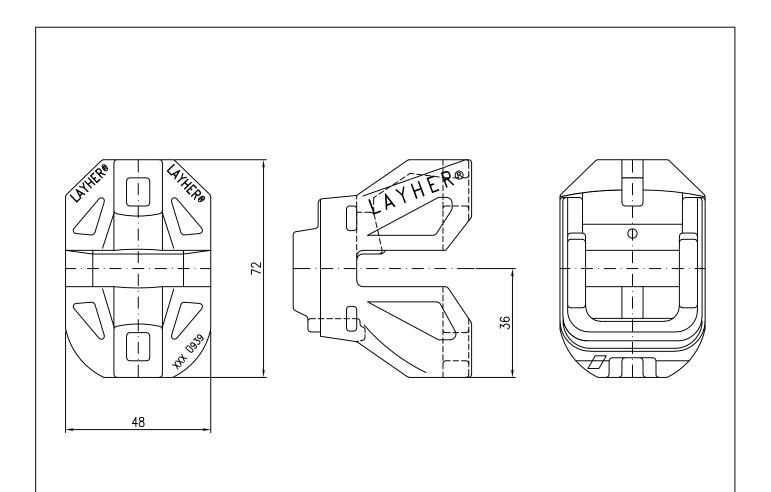


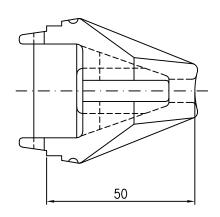
| ALBLITZ MODUL | | |
|--|----------------------|--|
| Connector disc die—cut Ø 124 mm "Lightweight design" in accordance with Z-8.22-939 | Annex B, page 165 | |
| ABM721-B005 | 02.2023 | |



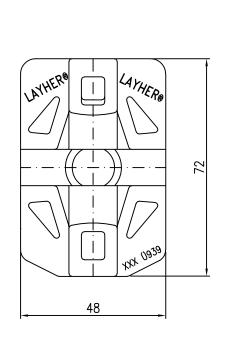


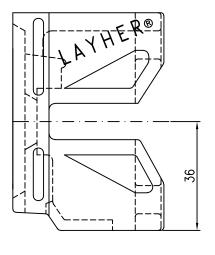
| ALBLITZ MODUL | |
|--|----------------------|
| Connector head for O—ledger "Lightweight design" in accordance with Z—8.22—939 | Annex B, page 166 |
| ABM721-B006 | 05.2021 |

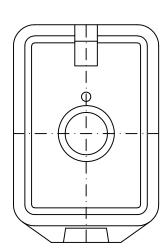


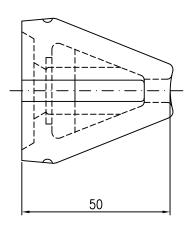


| ALBLITZ MODUL | |
|--|----------------------|
| Connector head for U—ledger "Lightweight design" in accordance with Z—8.22—939 | Annex B, page 167 |
| ABM721-B007 05.202 | 1 |

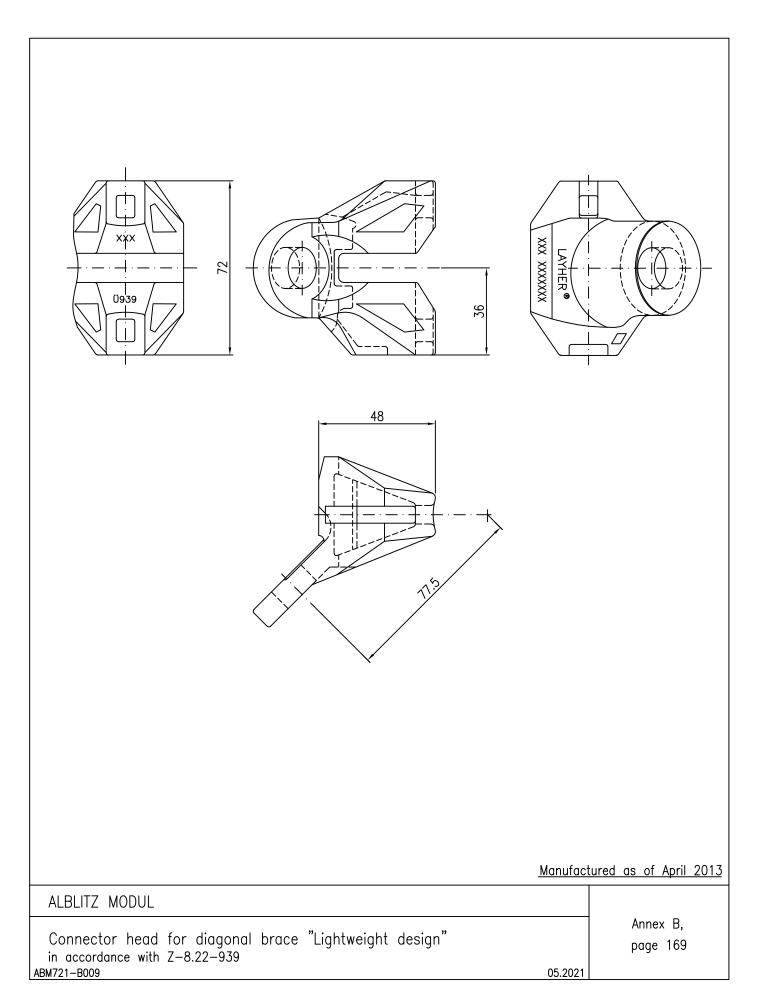


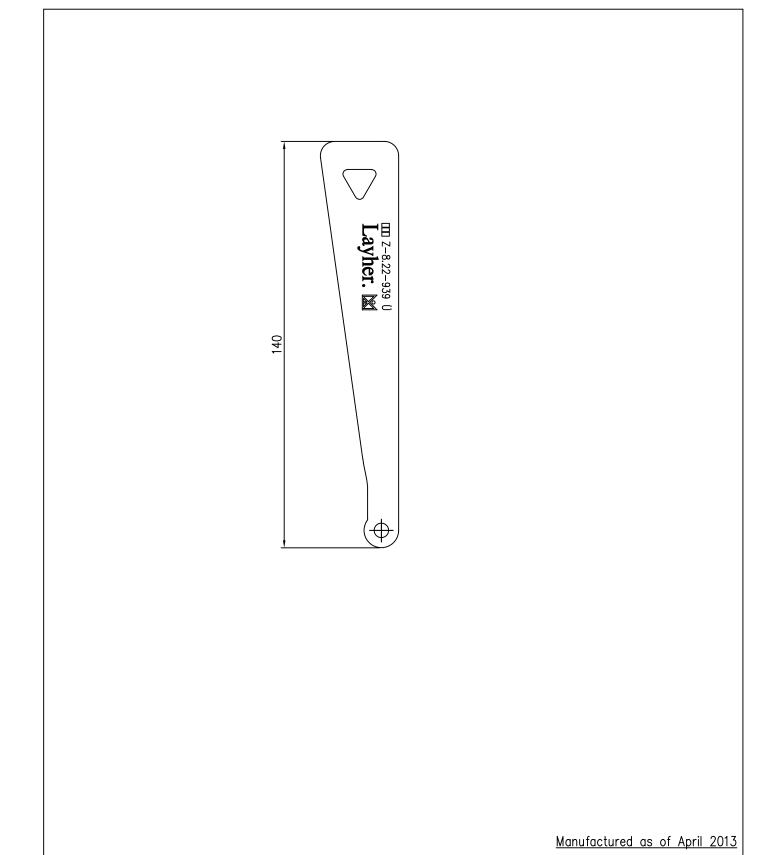




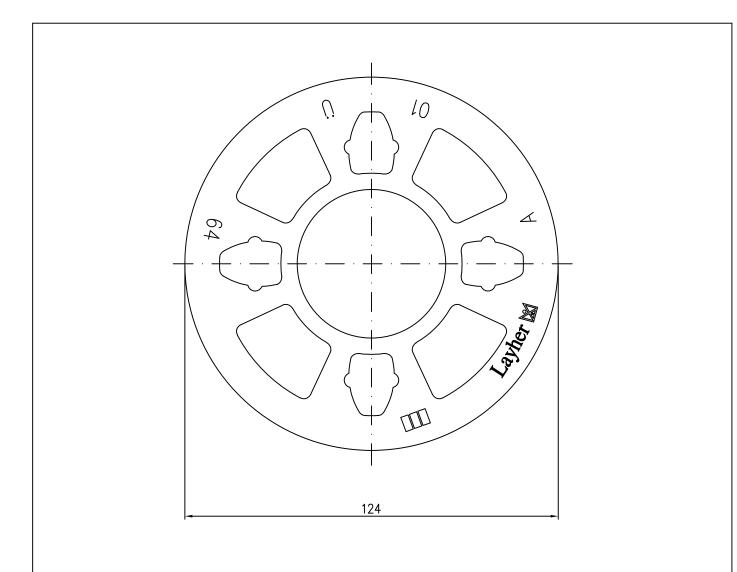


| ALBLITZ MODUL | | |
|---|---------|----------------------|
| Connector head for U—bracket "Lightweight design" in accordance with Z—8.22—939 | | Annex B, page 168 |
| ABM721-B008 | 05.2021 | |





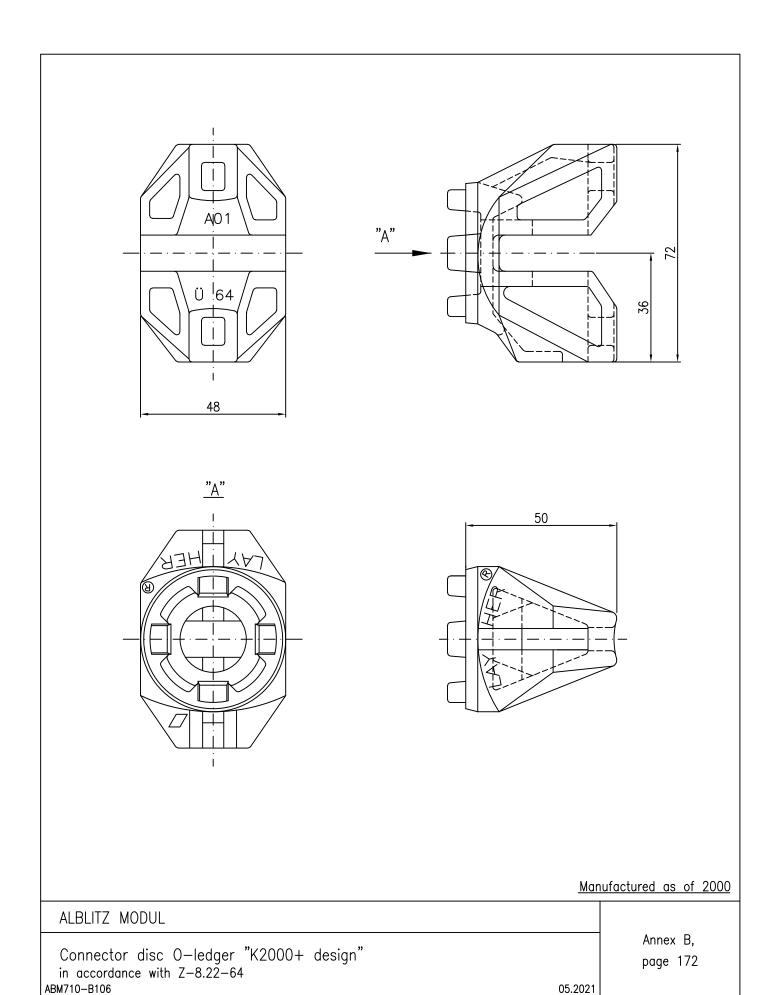
| ALBLITZ MODUL | | |
|--|---------|----------------------|
| Wedge "Lightweight design" in accordance with Z—8.22—939 | | Annex B, page 170 |
| ABM721-B010 | 05.2021 | |

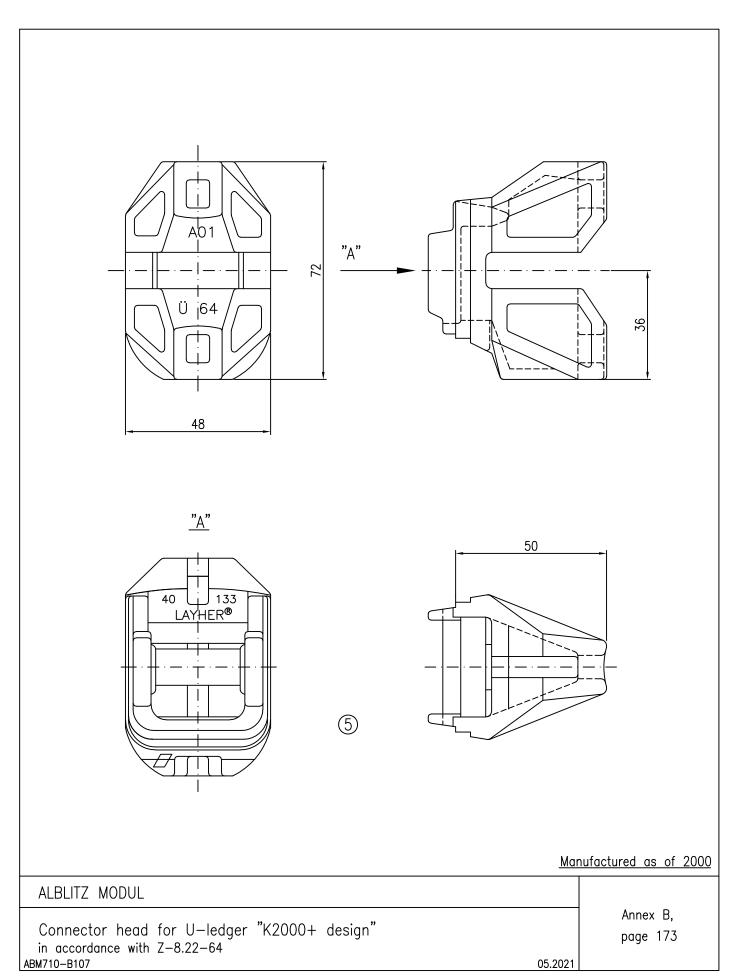


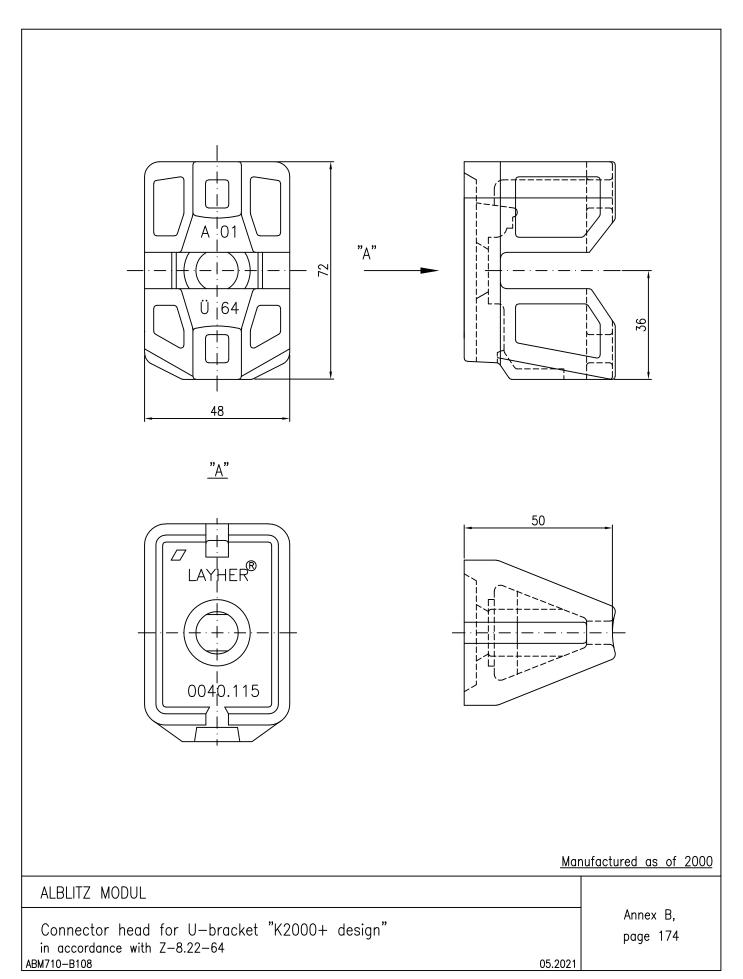


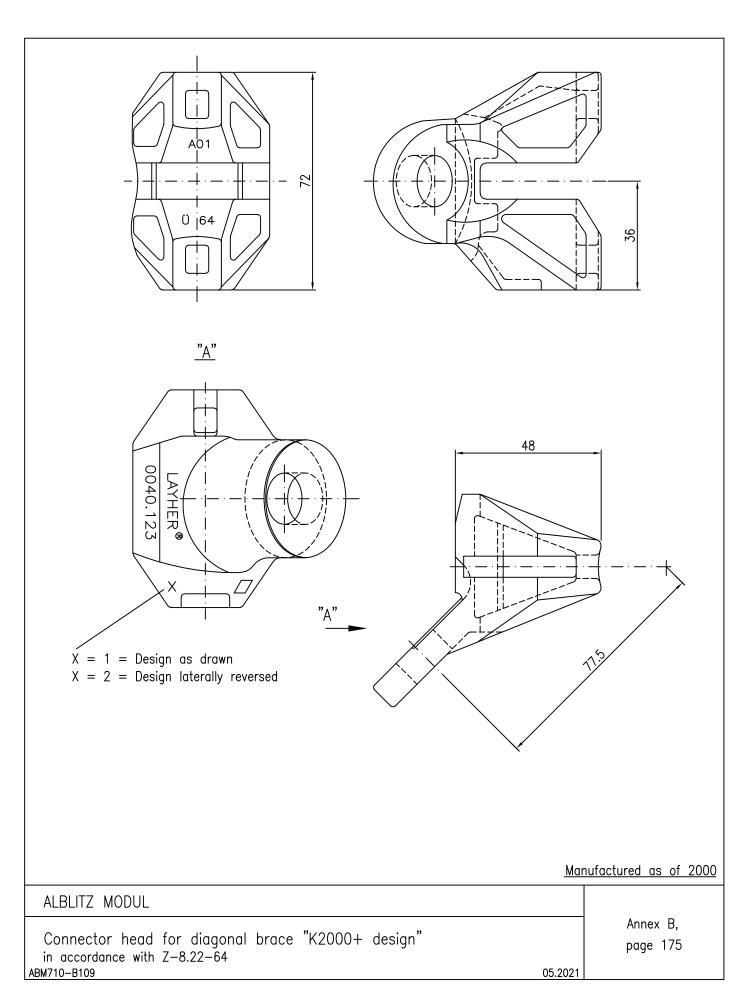
Manufactured as of 2000

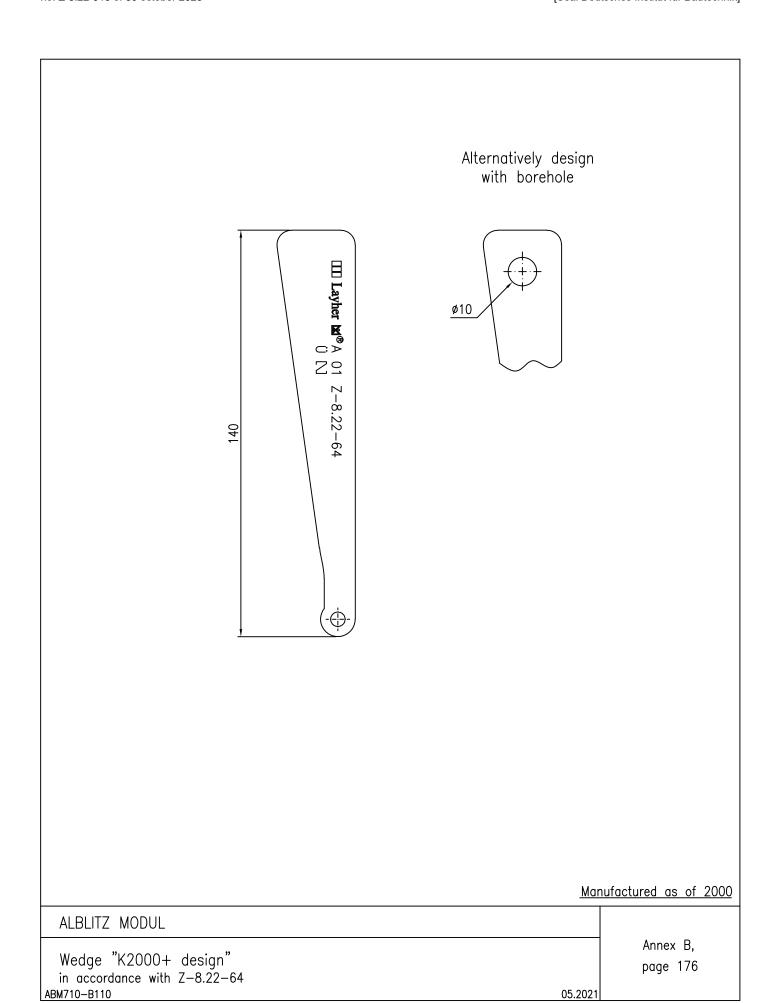
| ALBLITZ MODUL | |
|---|----------------------|
| Connector disc die—cut Ø 124 "K2000+ design" in accordance with Z-8.22-64 | Annex B, page 171 |
| ABM710-B105 05.2 | 021 |

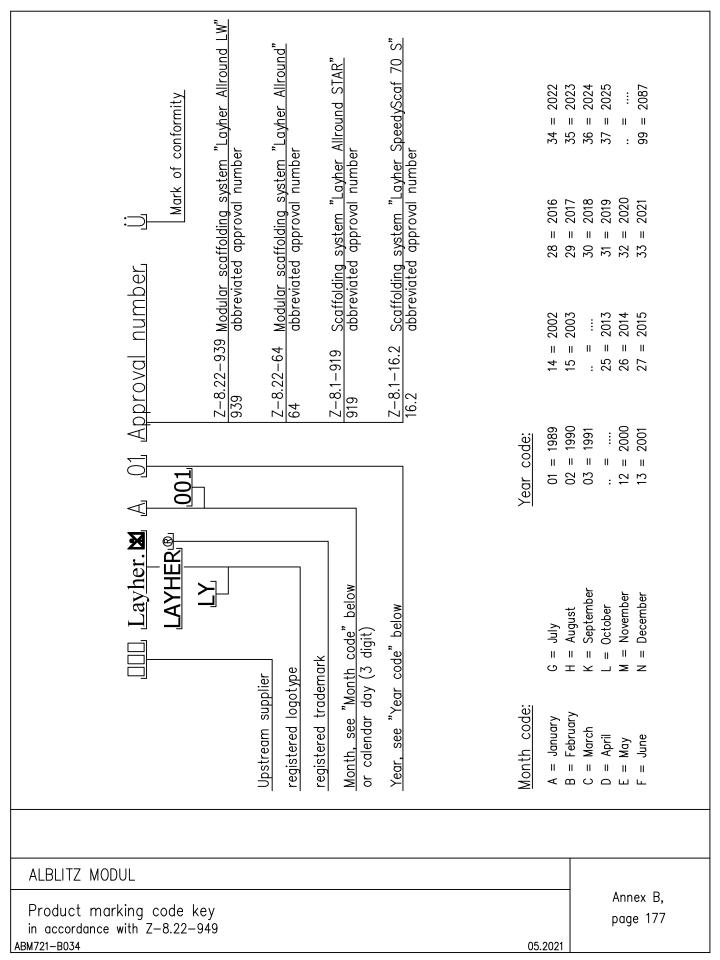


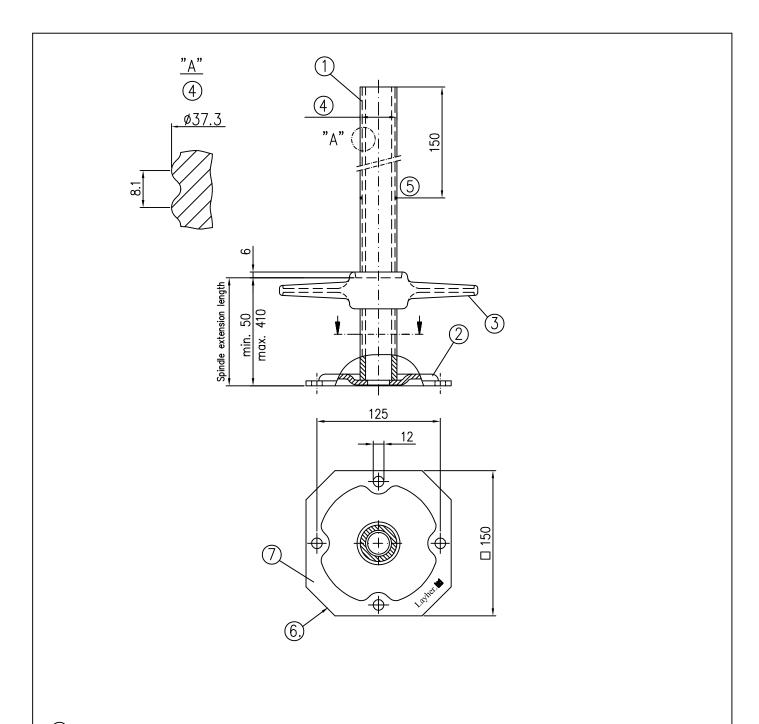












- 1) Tube
- 2 Base plate
- 3 Spindle nut
- 4 Special thread Ø38 x 8.1
- (5) Thread with notches to limit collar nut travel
- 6. Base plate according to EN 74-3
- (7) Marking

| Weight [kg] |
|----------------|
| 3.6 |

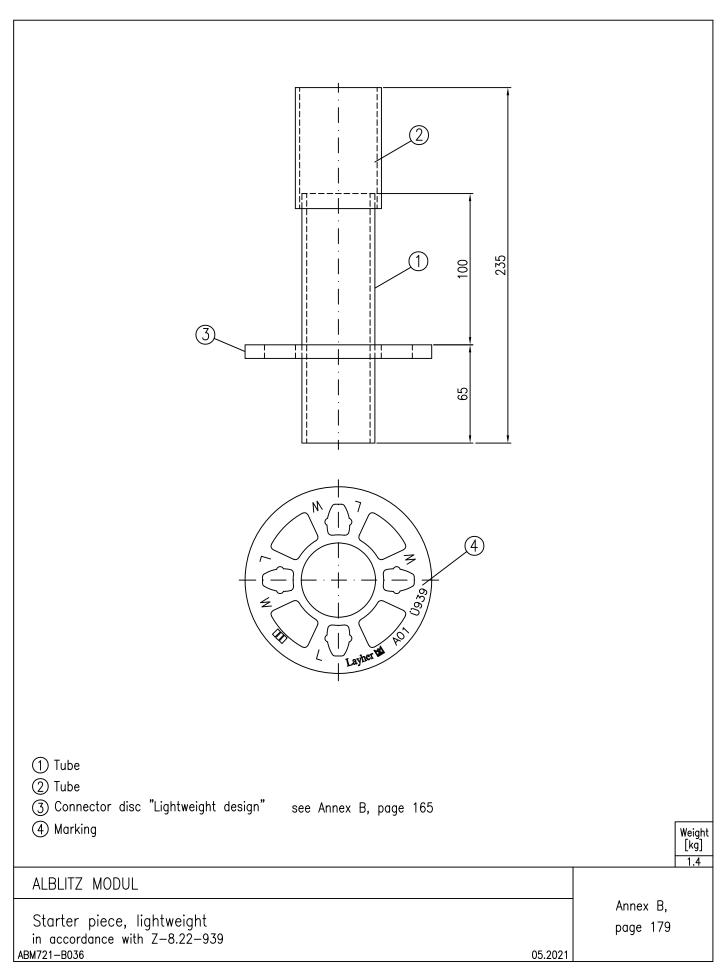
Base jack 60

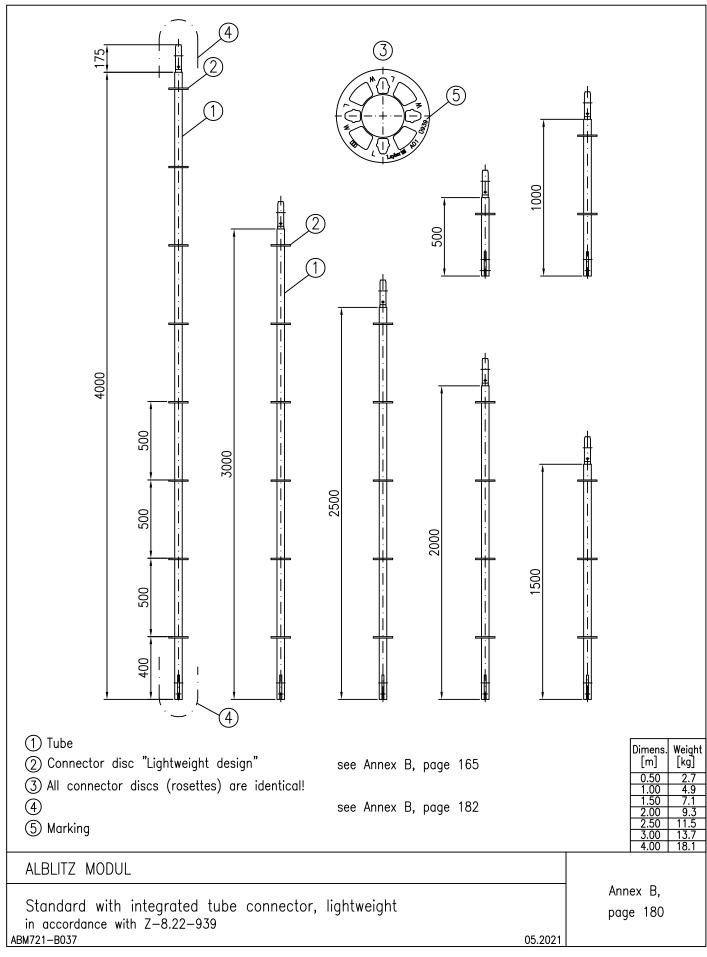
in accordance with Z-8.1-16.2

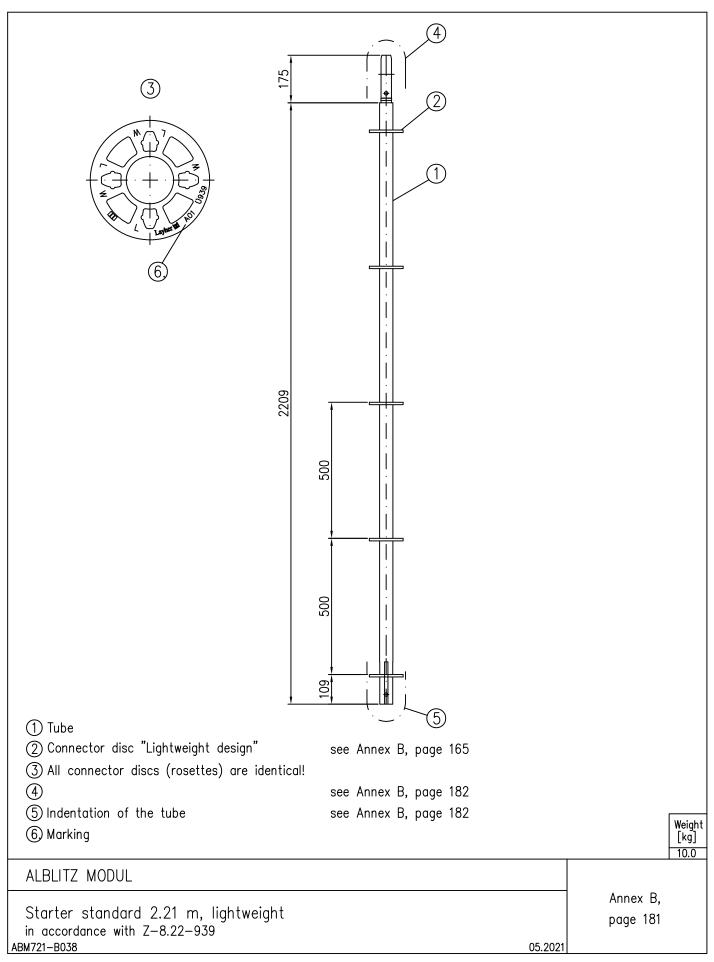
ABS710-A002_ABM

Annex B, page 178

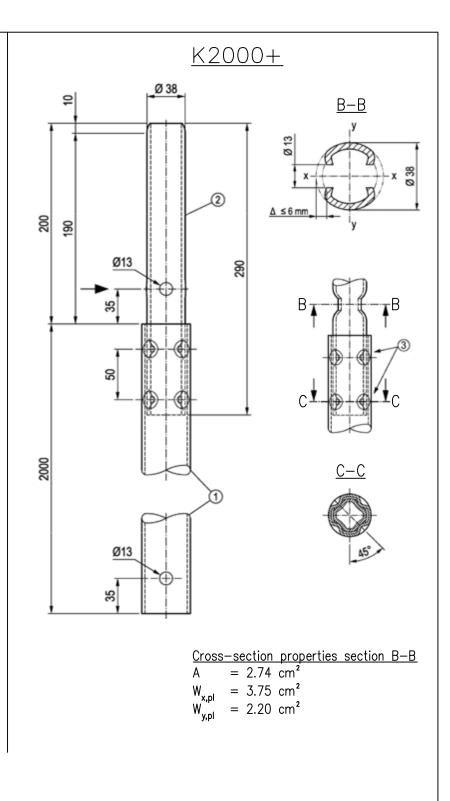
5.2021







Lightweight ø38 ø39 ø40.9 ø48.3

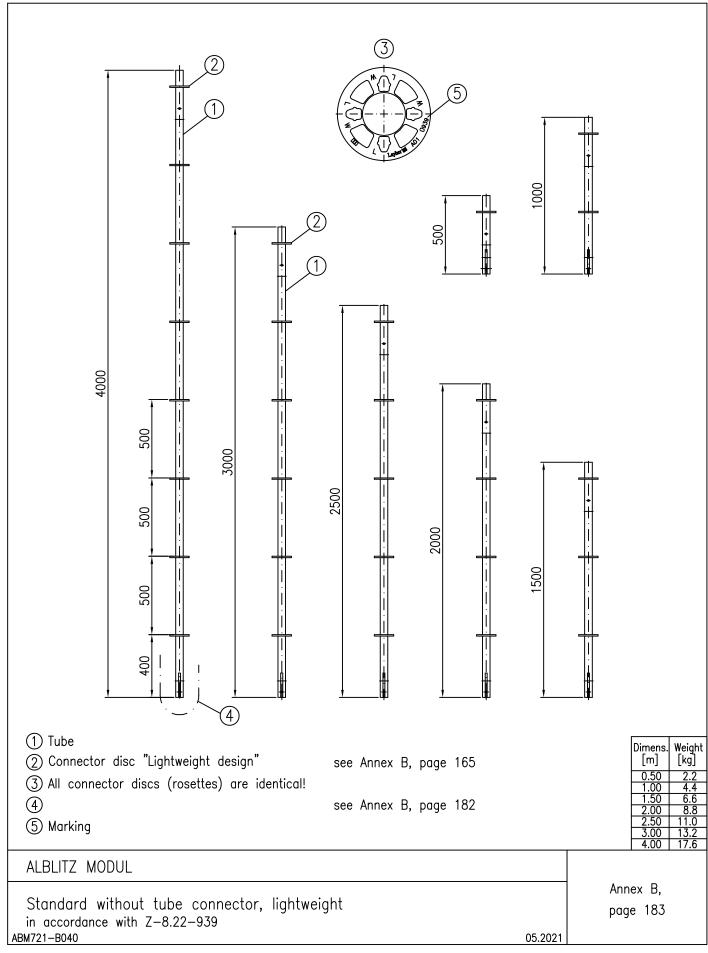


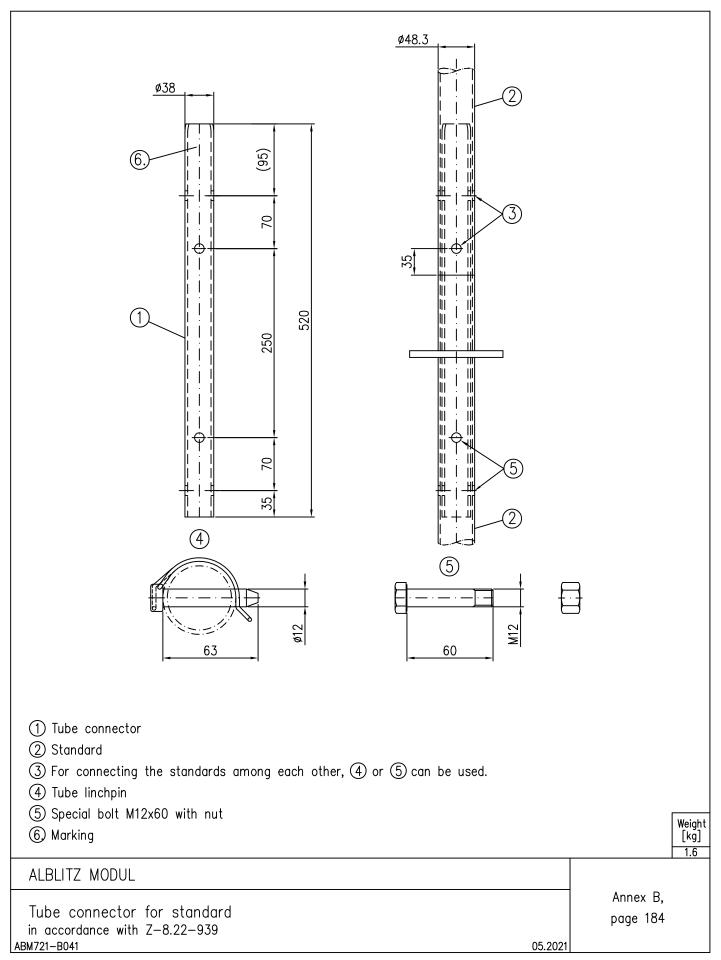
- 1 Tube
- 2 Tube connector
- 3 Tube connector pressed-in

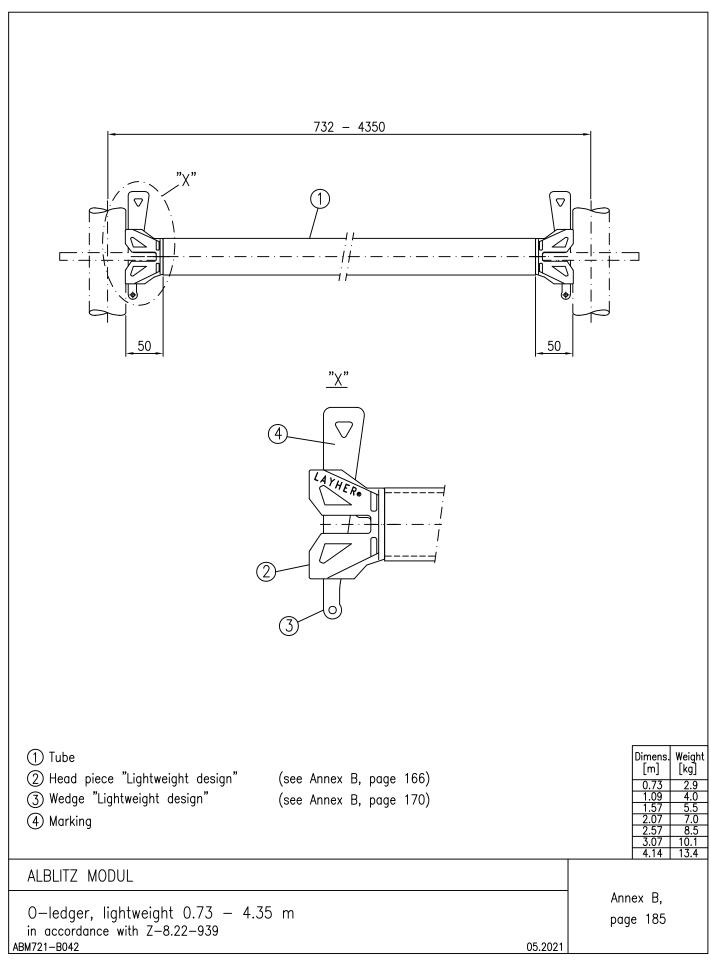
ALBLITZ MODUL

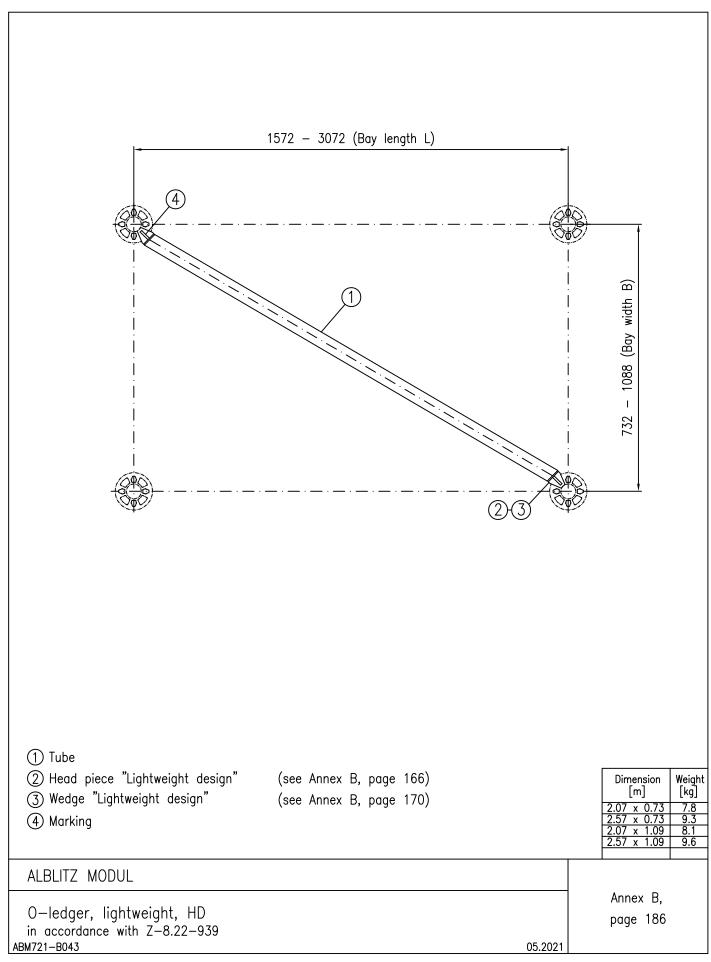
Detailed view: Standard with integrated tube connector, lightweight / Standard with in accordance with Z-8.22-939/Z-8.22-64 pressed-in tube connector "K200+ design" ABM721-B039

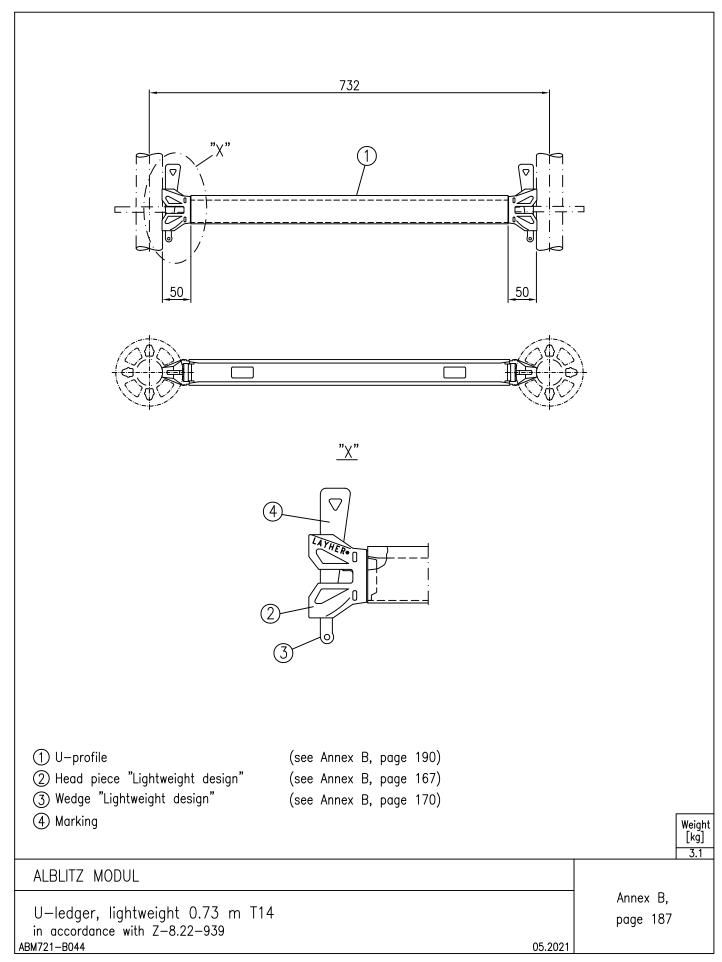
Annex B, page 182

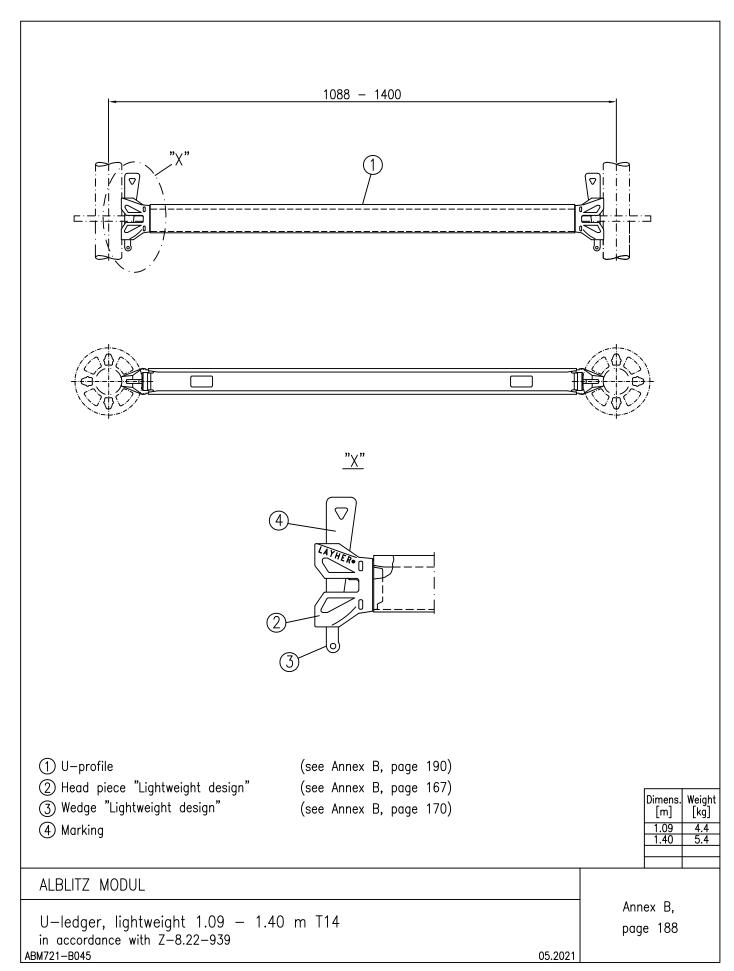


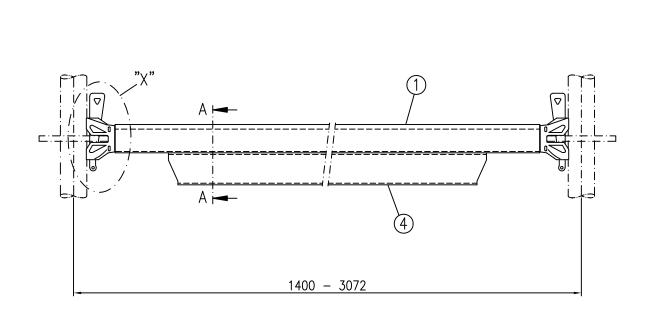


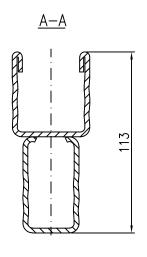


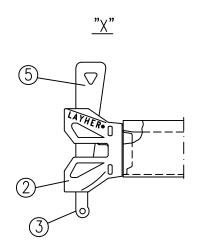












- 1 U-profile
- ② Head piece "Lightweight design"
- Wedge "Lightweight design"
- 4 U-profile
- (5) Marking

| (| see | Annex | В. | page | 190 |
|---|-----|-------|----|------|-----|
| | | | | | |

(see Annex B, page 167)

(see Annex B, page 170)

| Dimens. [m] | Weight [kg] |
|----------------|------------------|
| 1.40 | 8.9 |
| 1.57 | 9.4 |
| 2.07 | 12.7 |
| 2.57 | 15.7 |
| 3.07 | 19.0 |
| | |
| | |

ALBLITZ MODUL

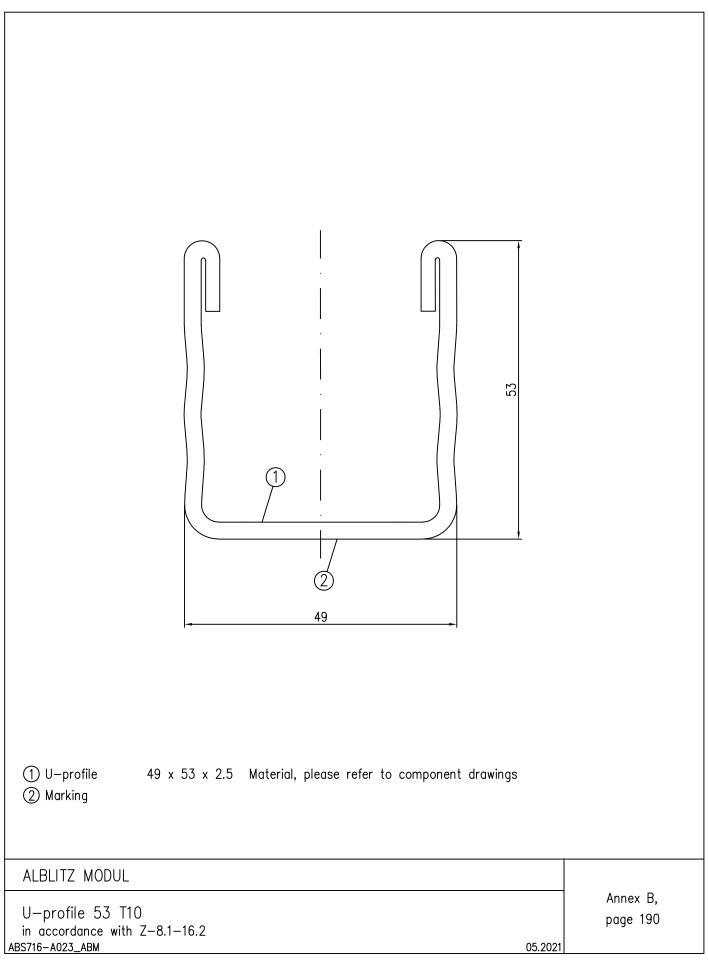
U-ledger, lightweight 1.40 - 3.07 m T14, reinforced in accordance with Z-8.22-939

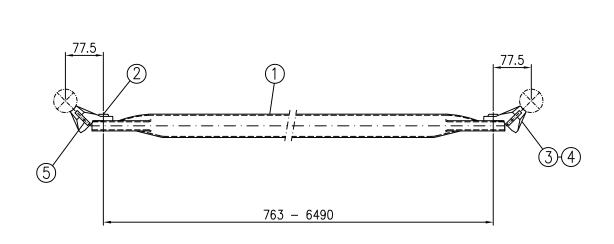
ABM721-B046

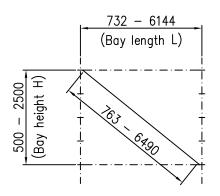
Annex B, page 189

05.2021

Z101025.23







- (1) Tube
- 2 Cylinder head rivet
- 3 Head piece "Lightweight design"

(see Annex B, page 169)

4 Wedge "Lightweight design"

(see Annex B, page 170)

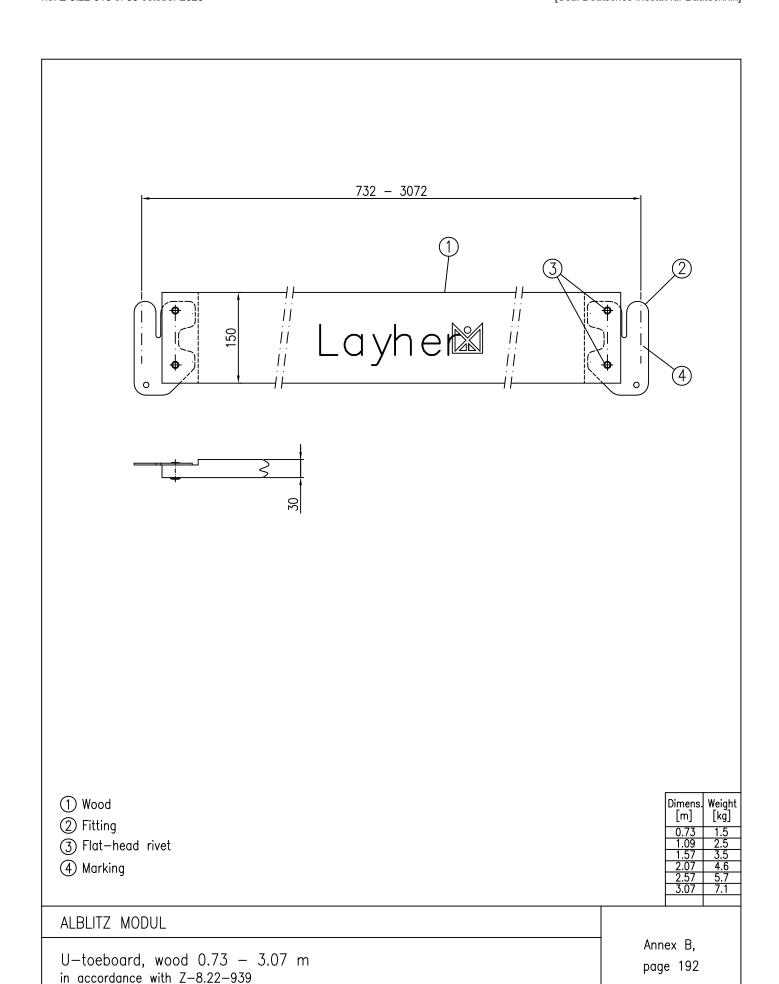
(5) Marking

| Dimension [m] | Weight [kg] |
|------------------|----------------|
| 2.07 x 2.00 | 8.9 |
| 2.57 x 2.00 | 9.5 |
| 2.07 x 1.50 | 8.2 |
| 2.57 x 1.50 | 9.5 |
| | |

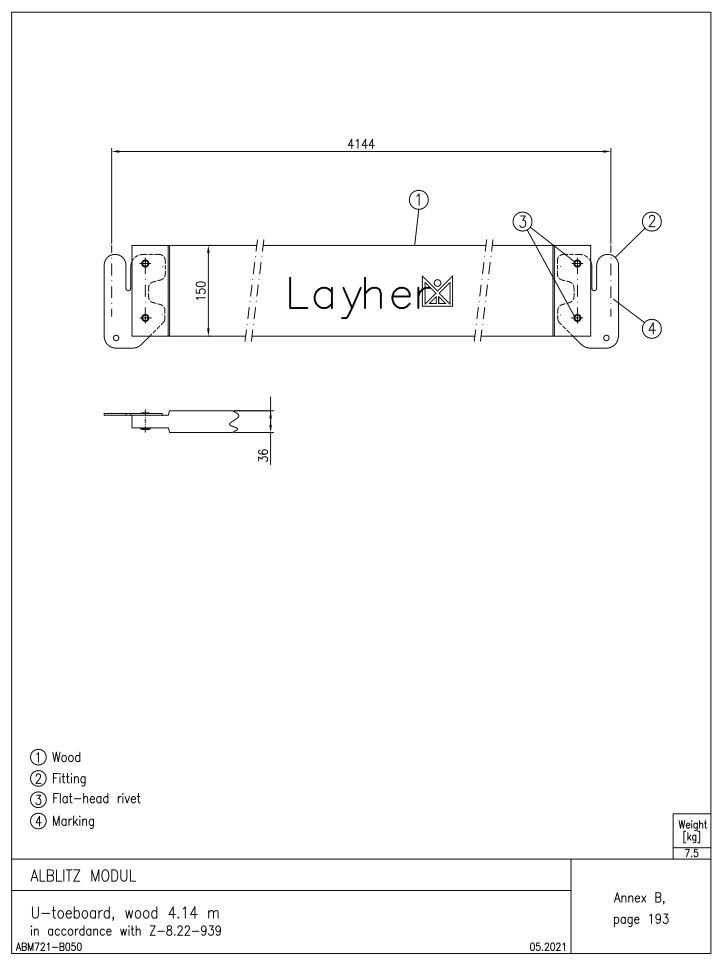
| ALB | LITZ | MO | DUL |
|-----|------|----|-----|
| | | | |

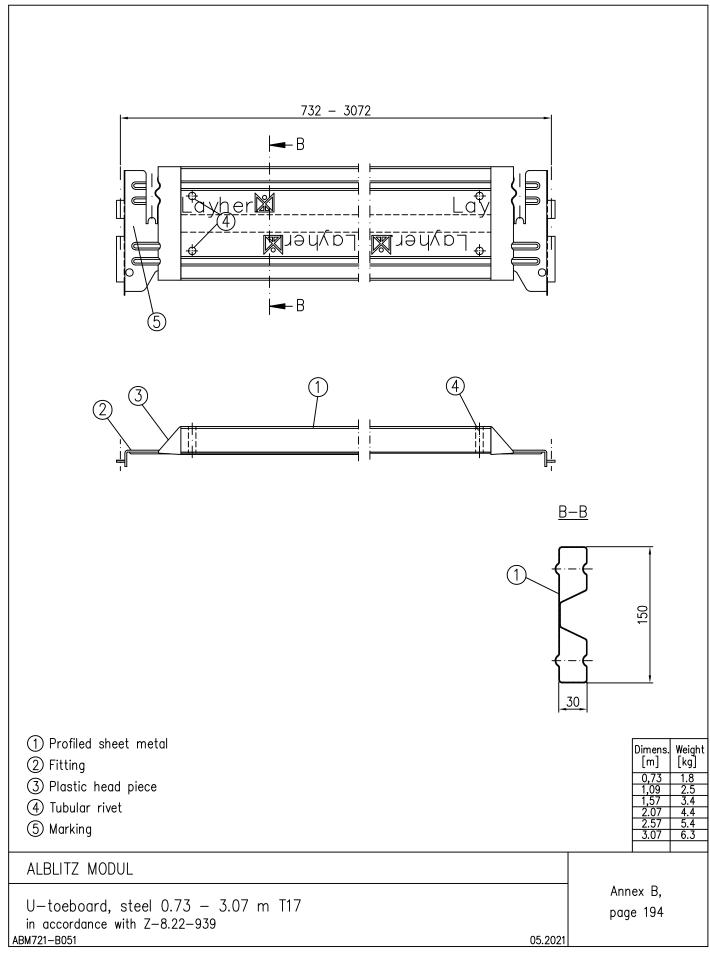
Diagonal brace "Lightweight design" in accordance with Z-8.22-939 ABM721-B048

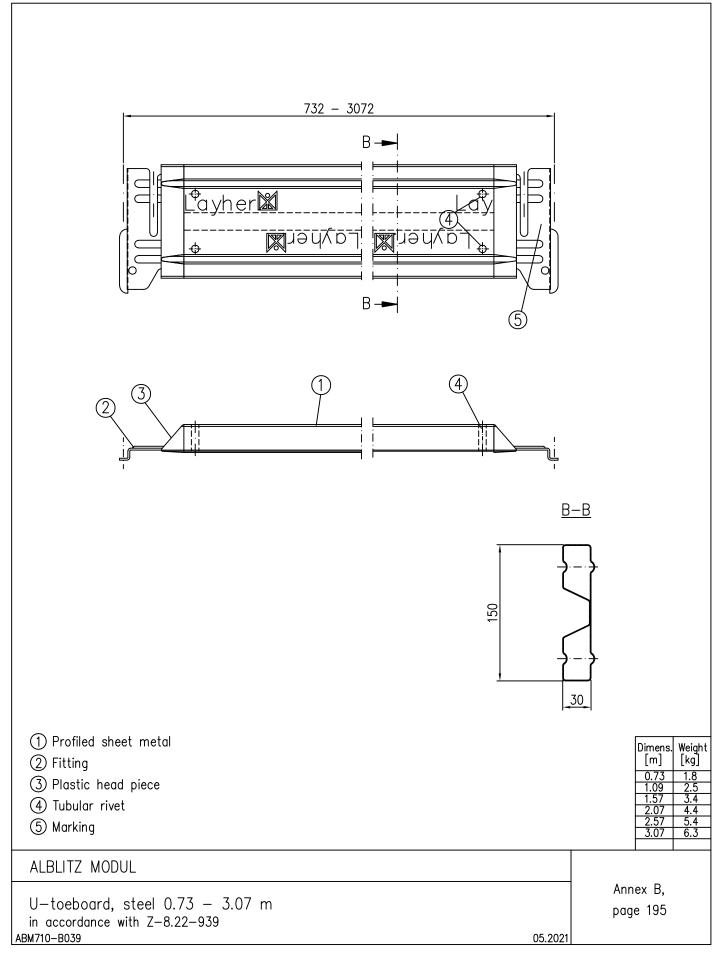
Annex B, page 191

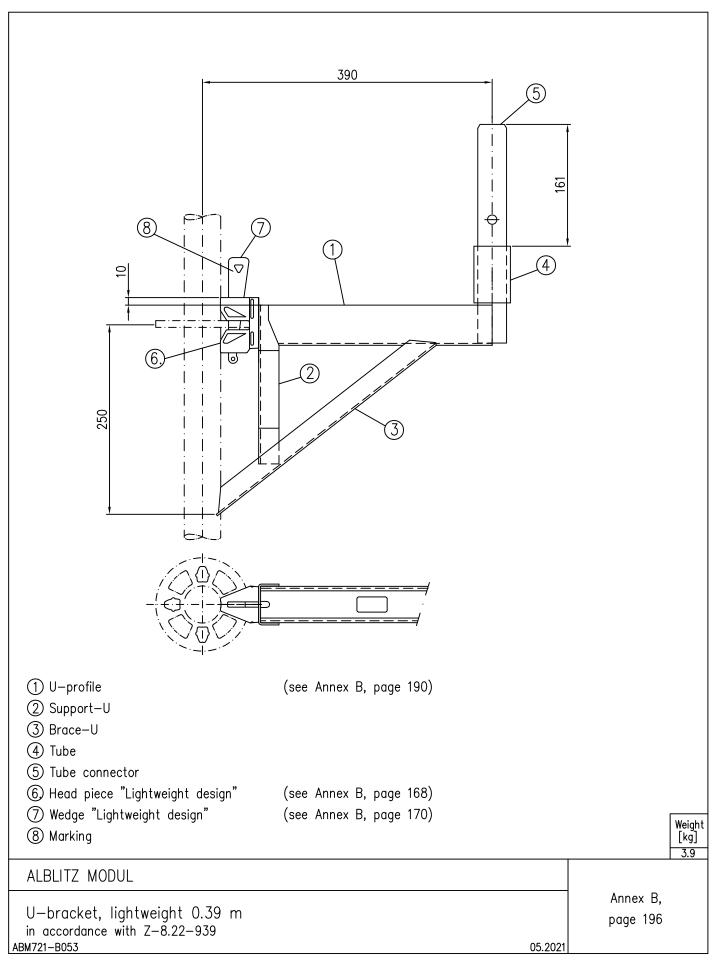


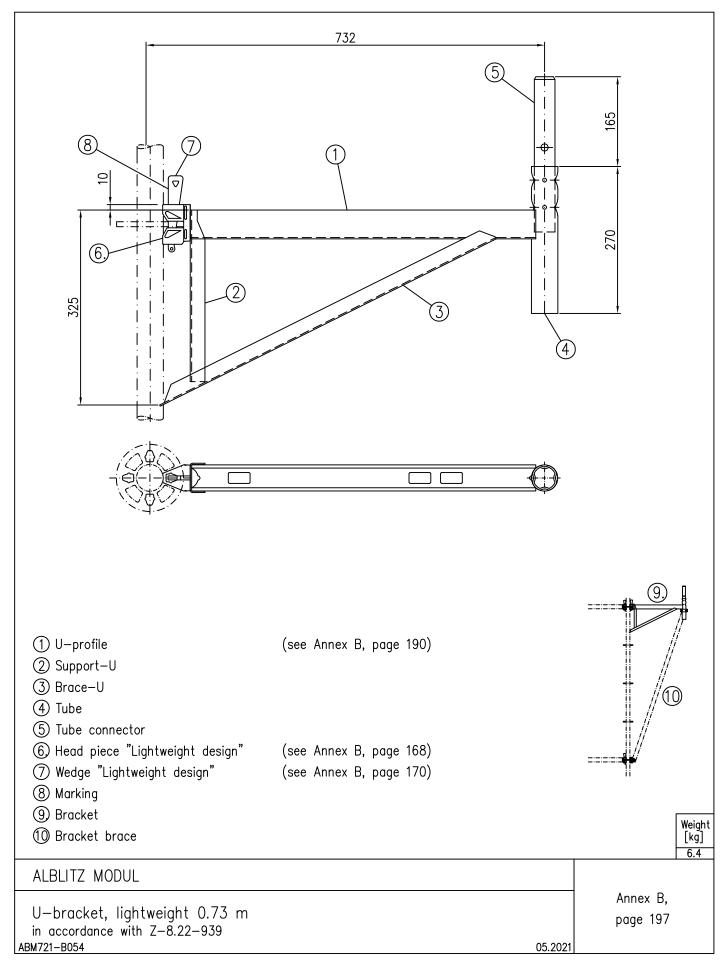
ABM710-B038

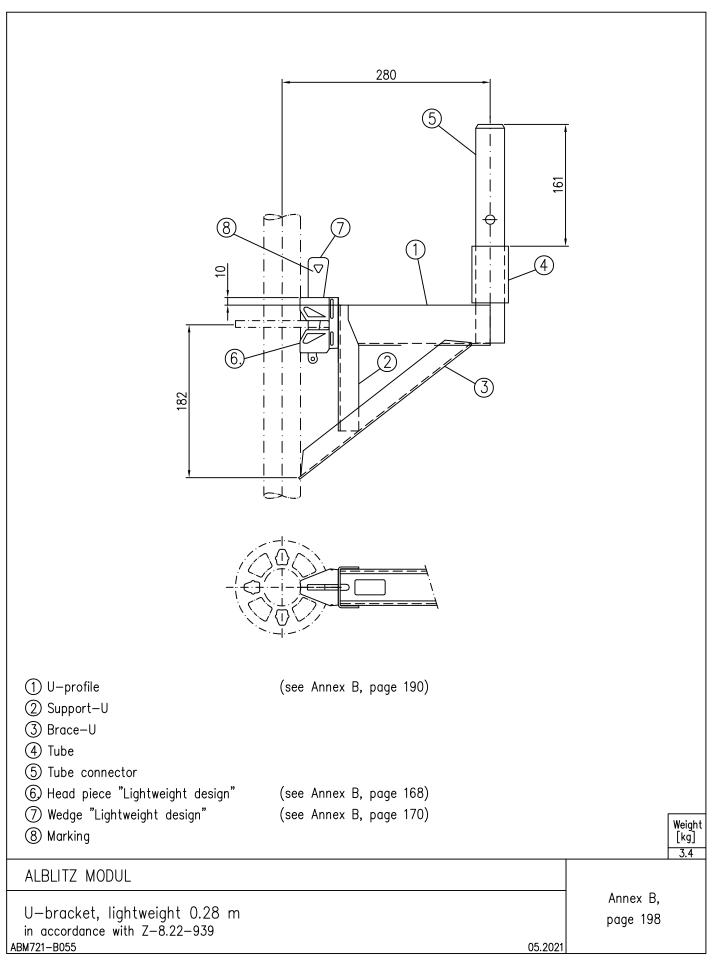


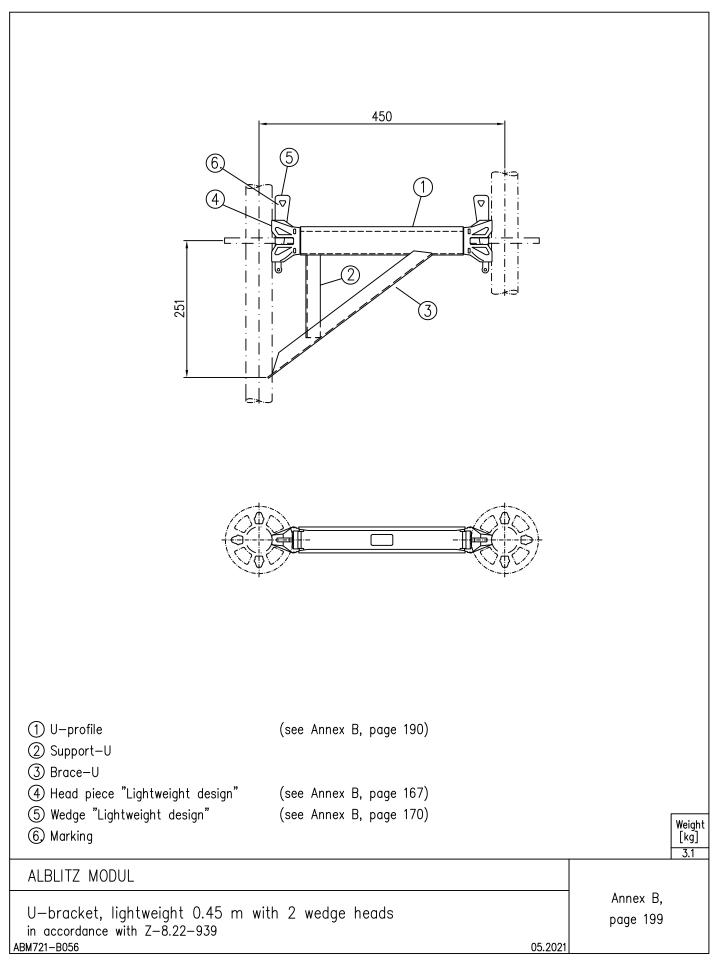


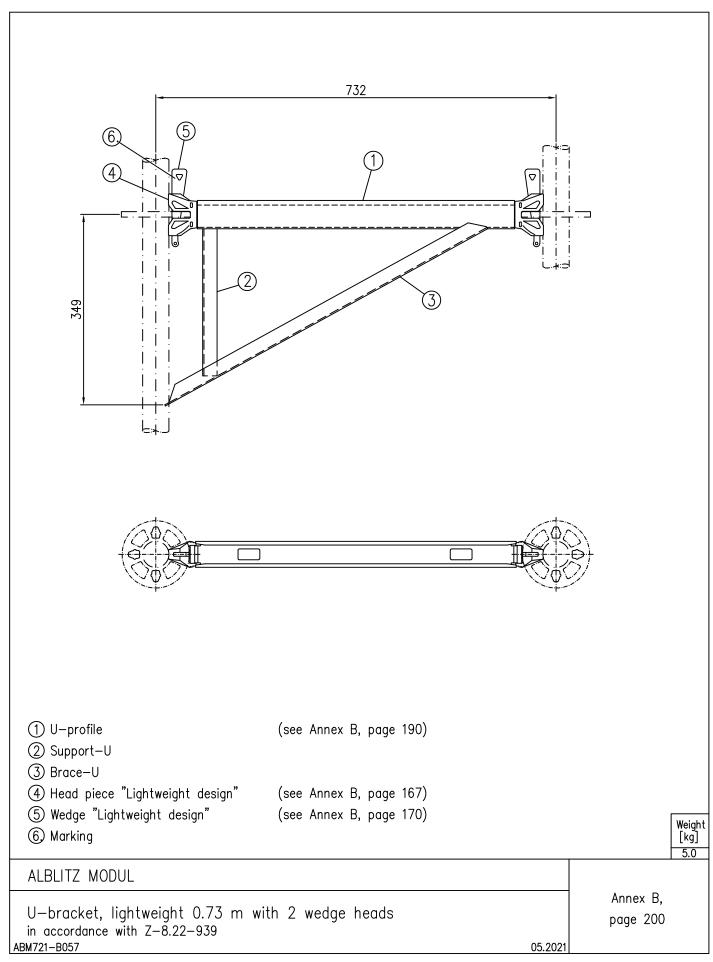


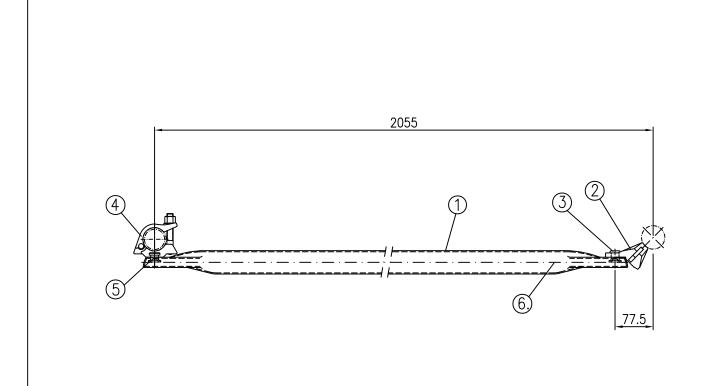












1) Tube

(2) Head piece + Wedge "Lightweight design"

(3) Cylinder head rivet

4 Halfcoupler with screw top

(5) Cylinder head rivet

6 Marking

(see Annex B, pages 169 + 170)

swivel, riveted

in accordance with approval Z-8.331-882

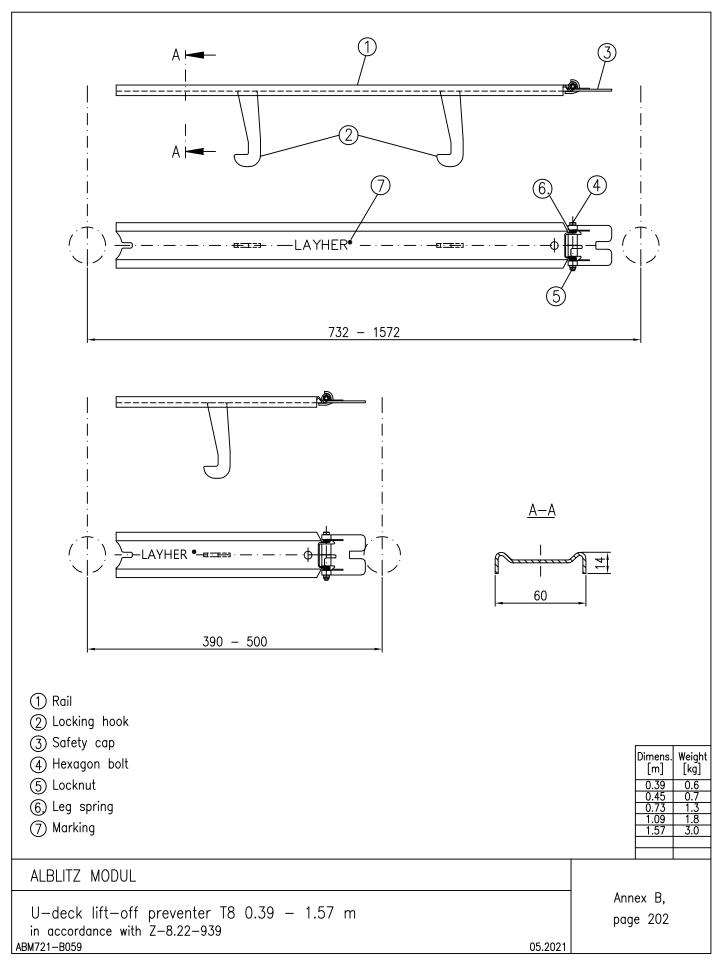
swivel, riveted

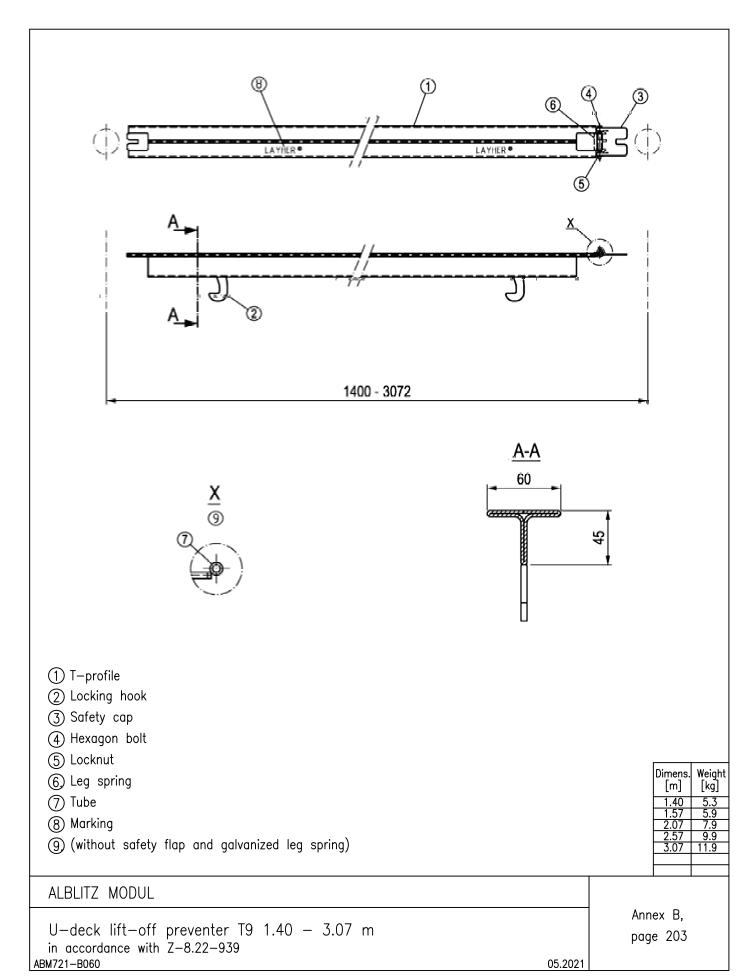
Weight [kg] 8.8

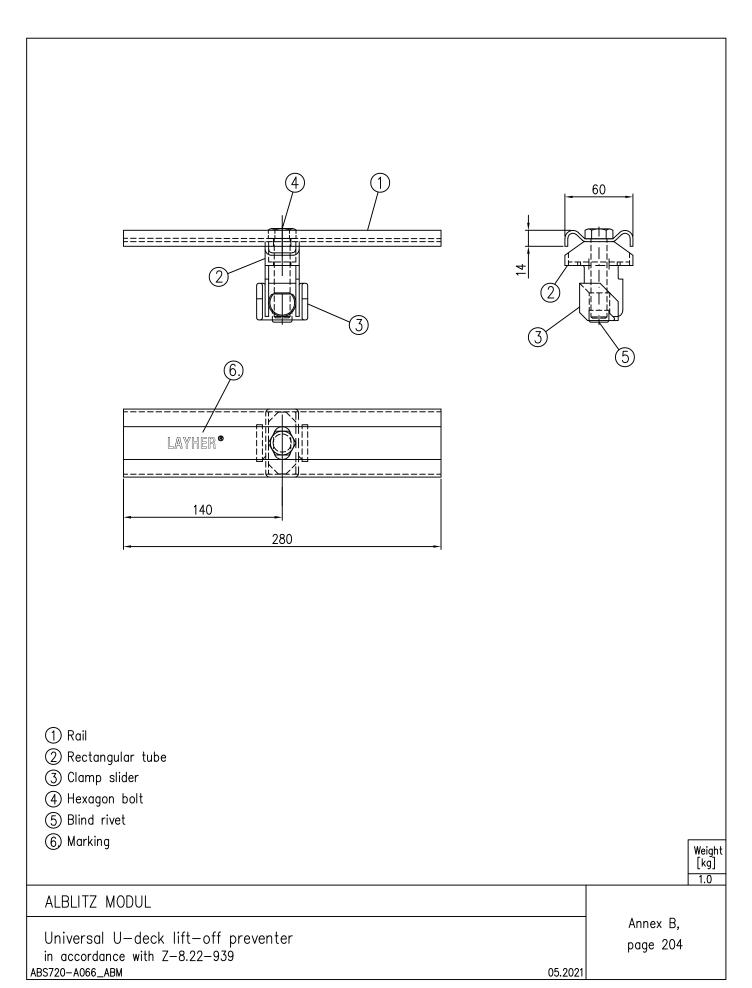
ALBLITZ MODUL

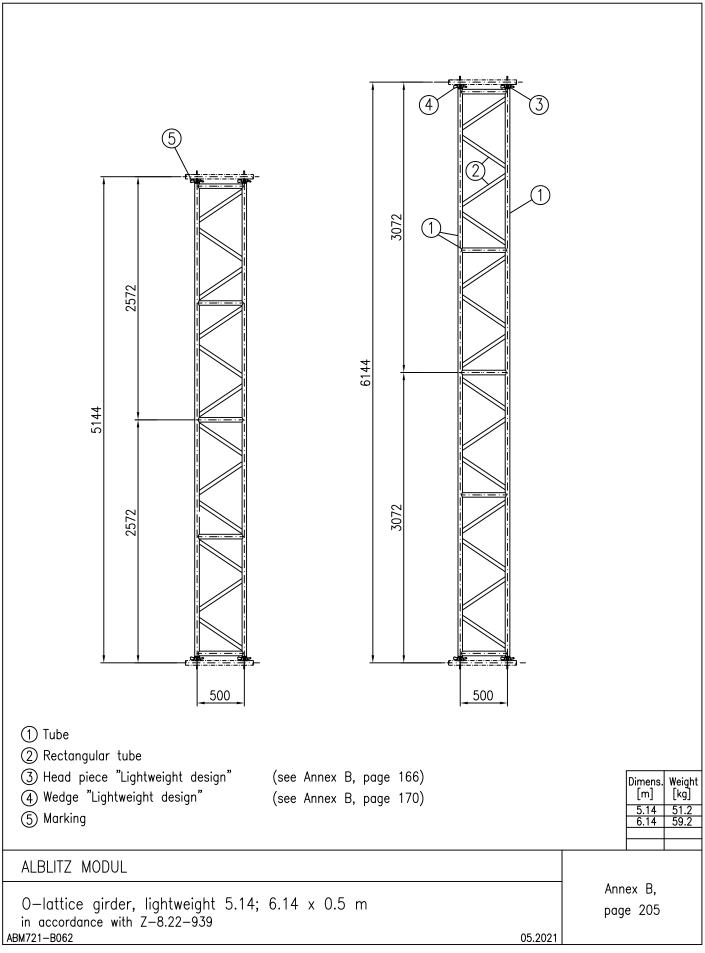
Bracket brace 2.05 m "Lightweight design" in accordance with Z-8.22-939
ABM721-B058

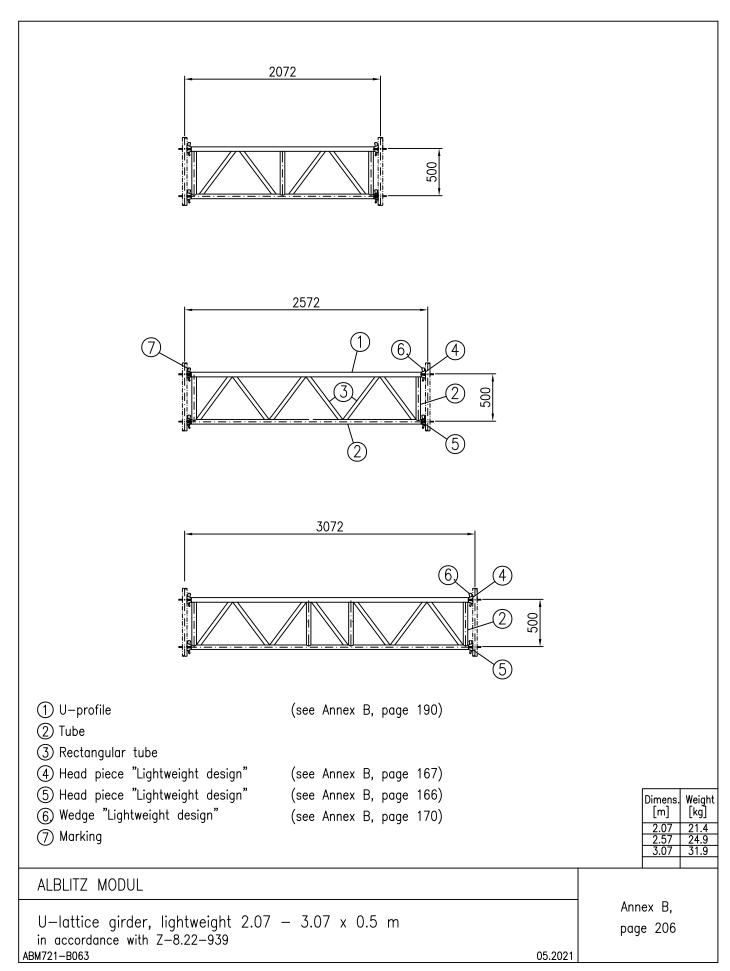
Annex B, page 201

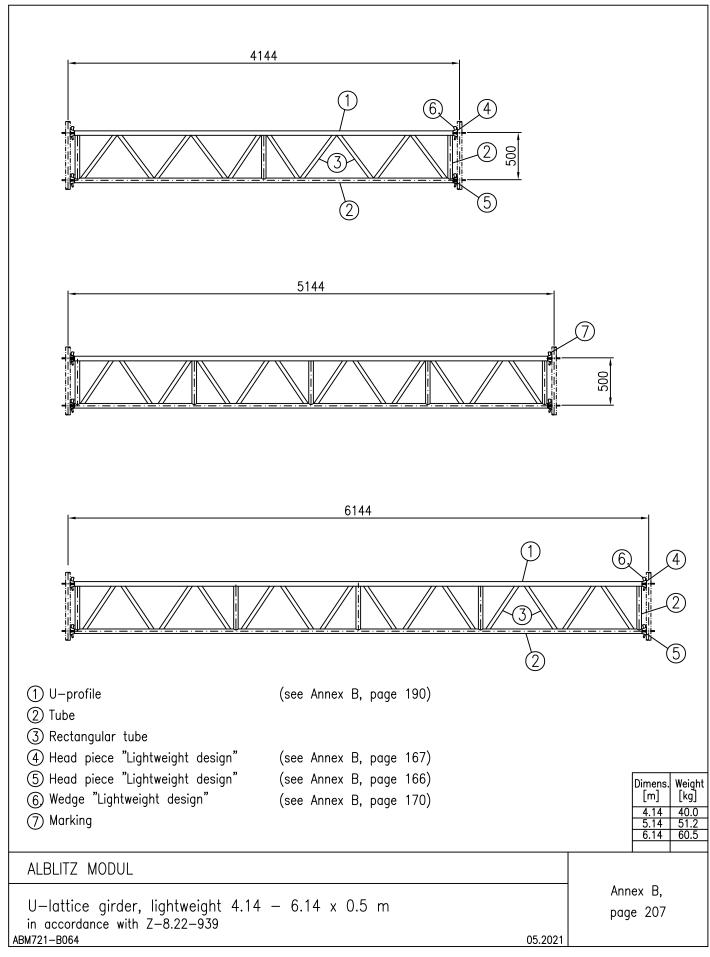


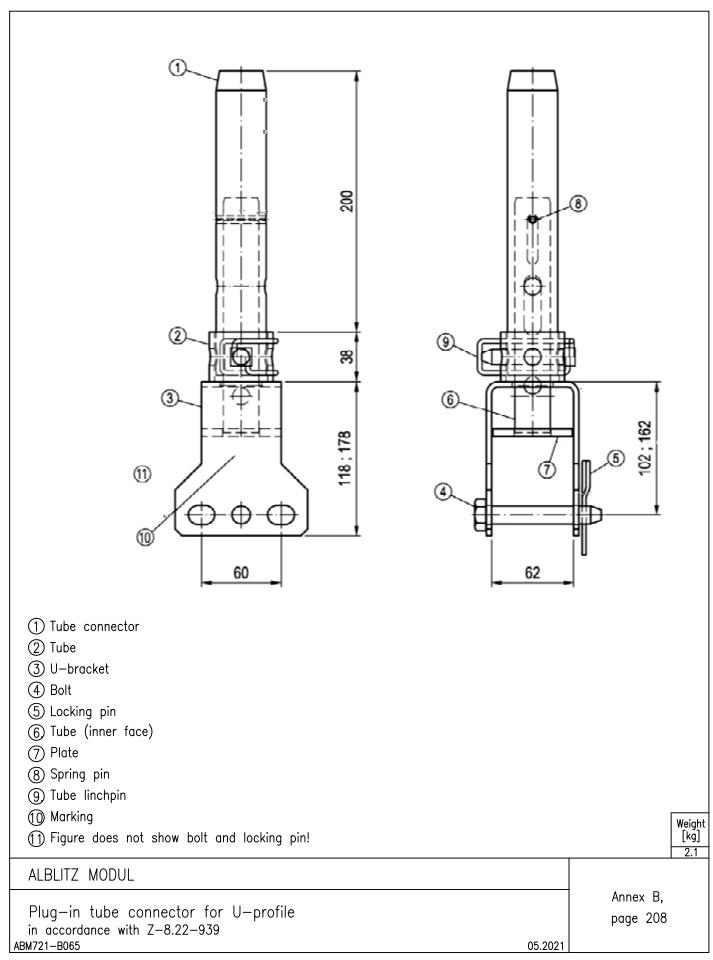


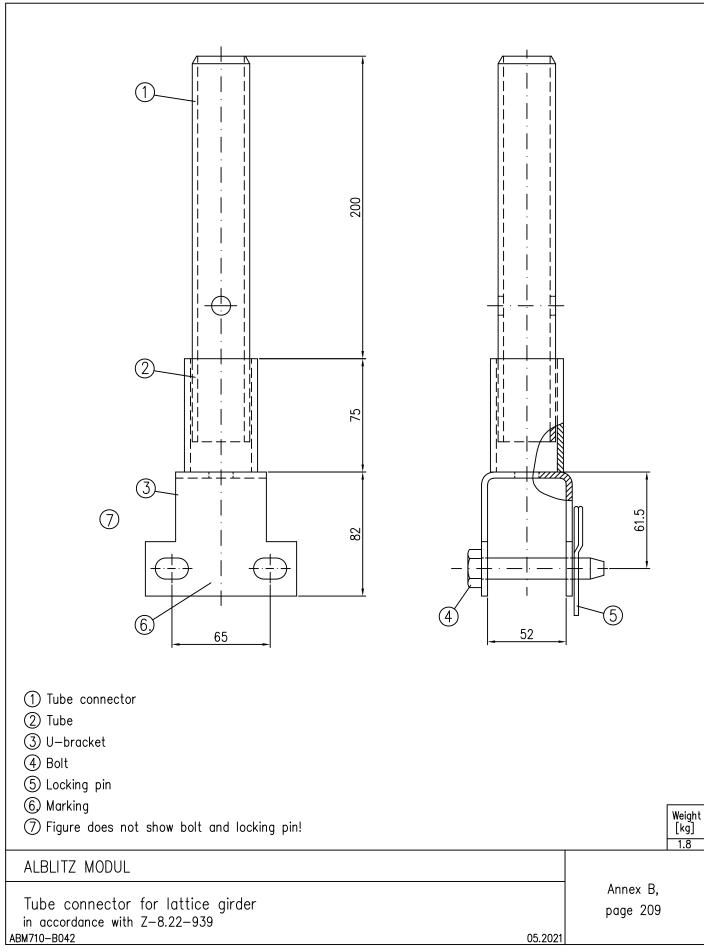


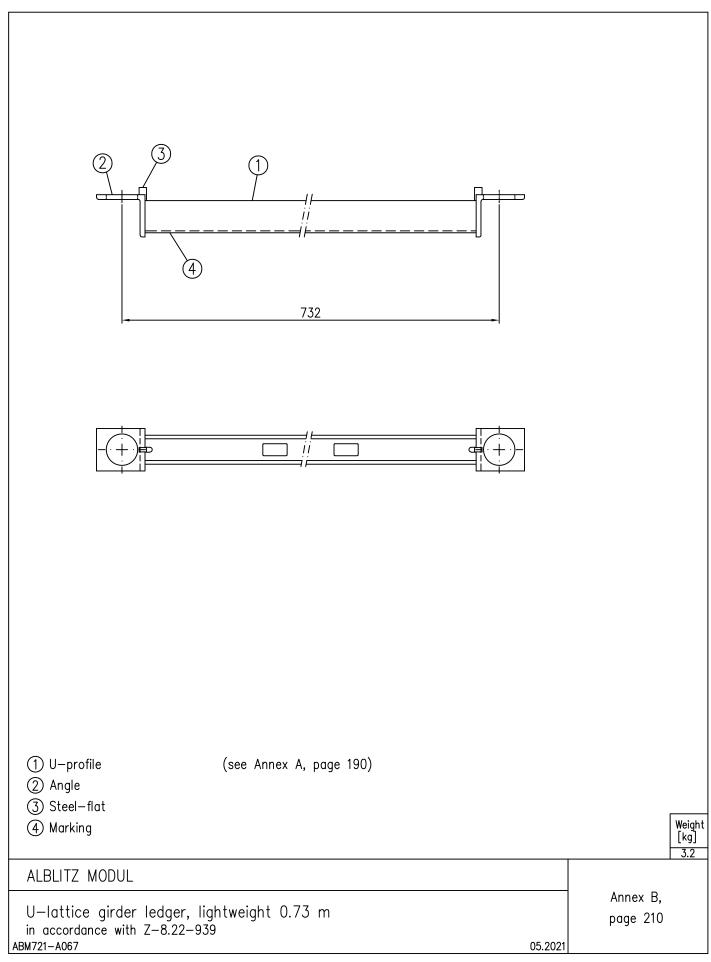


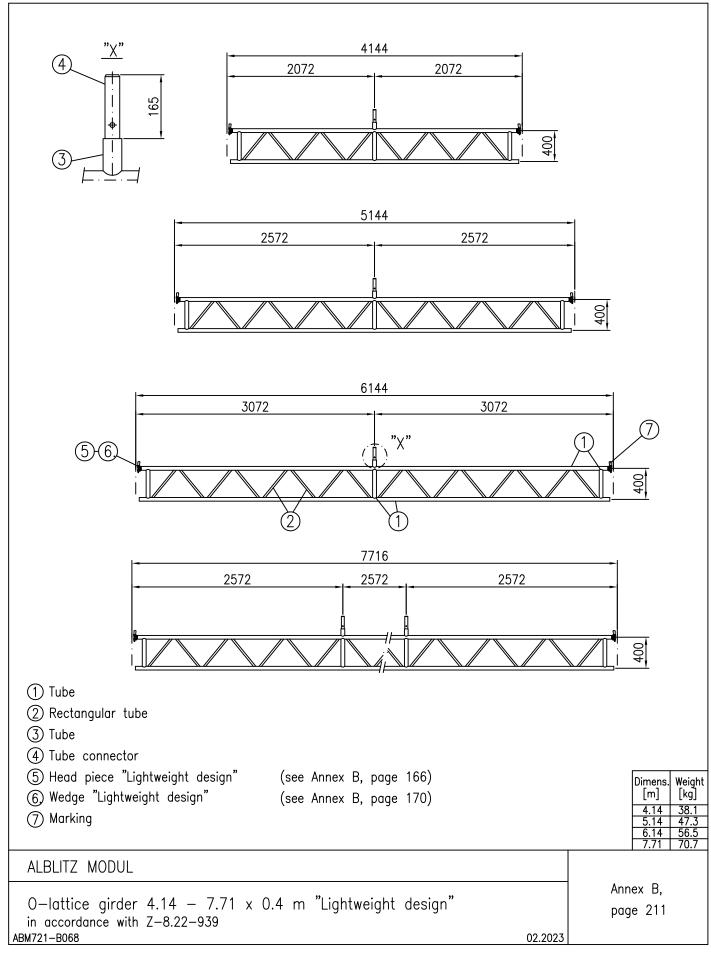


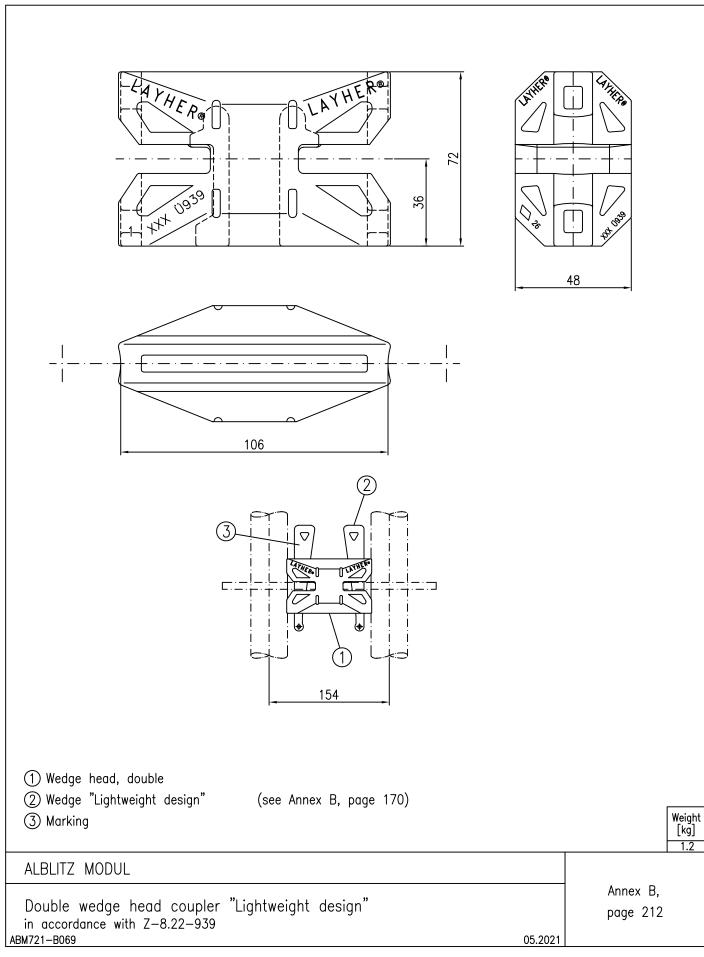


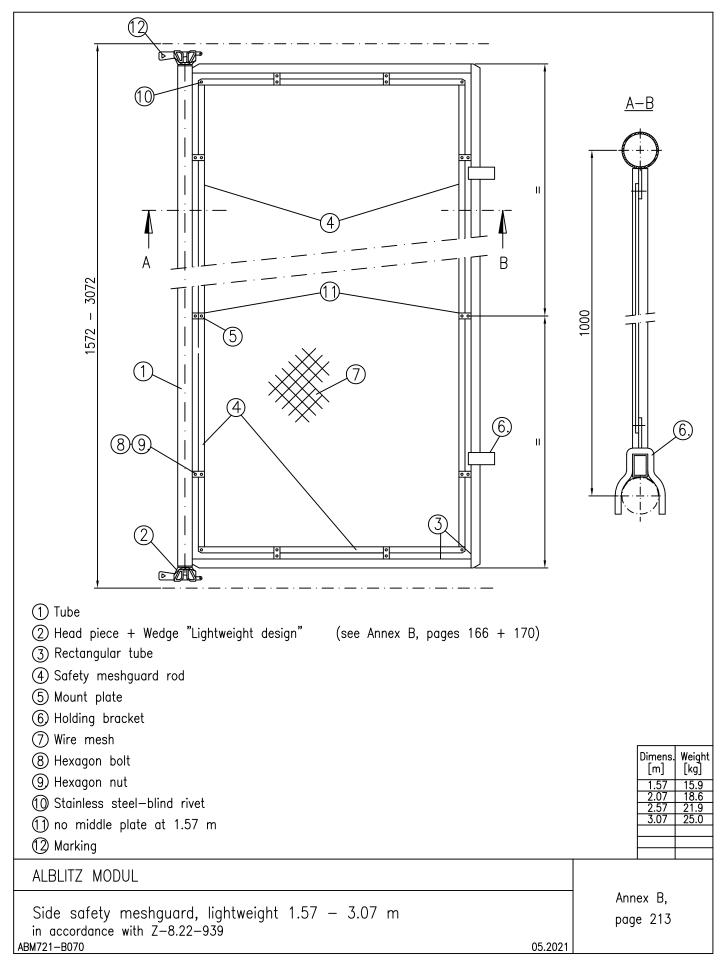


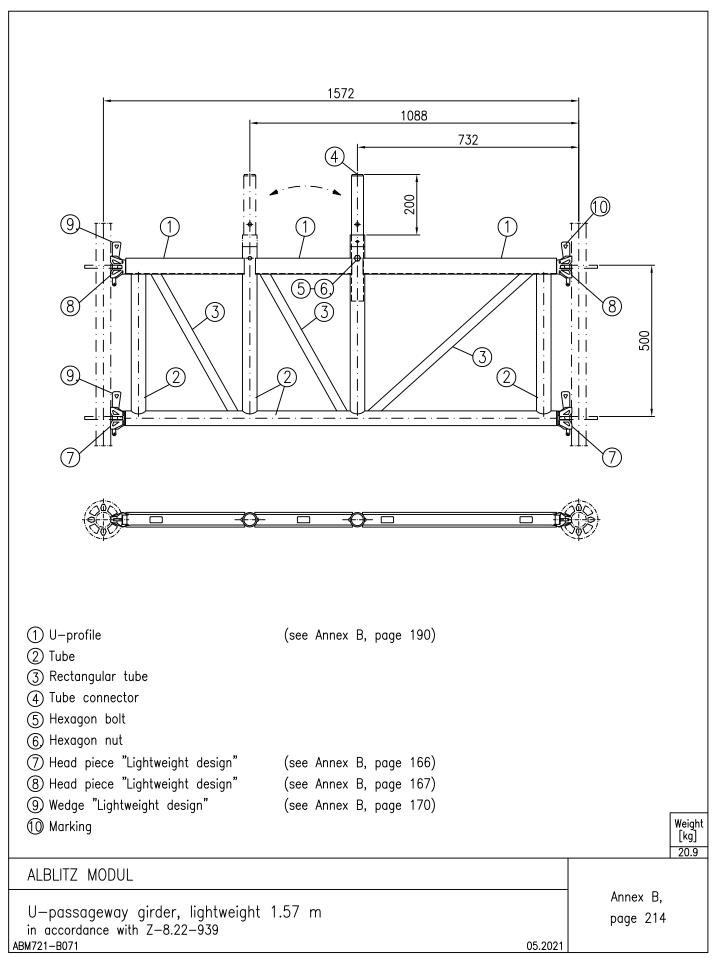


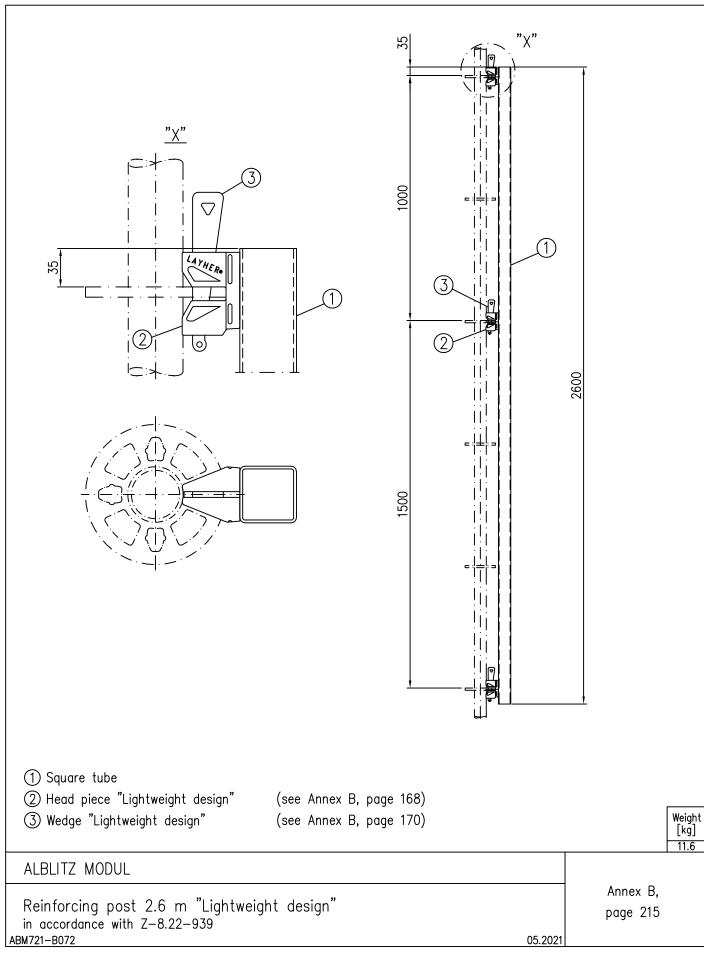


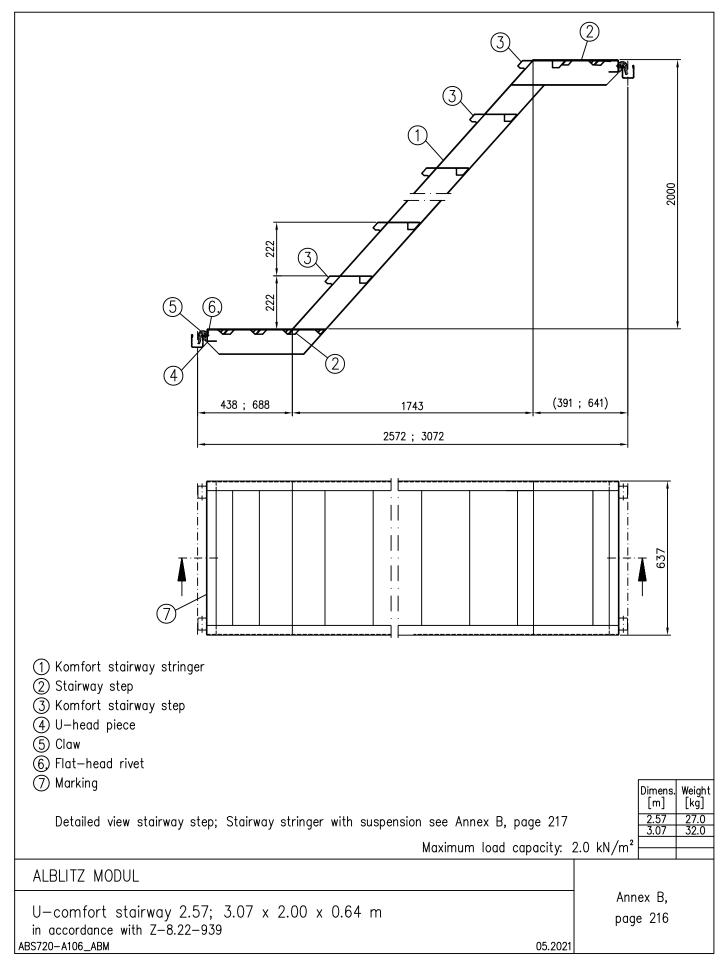


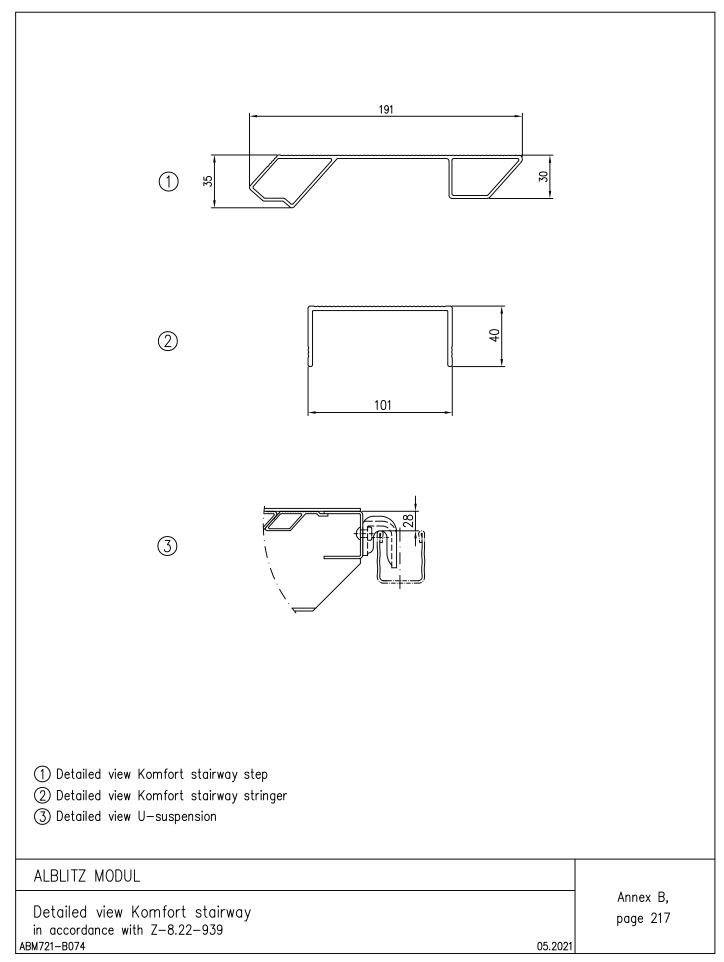


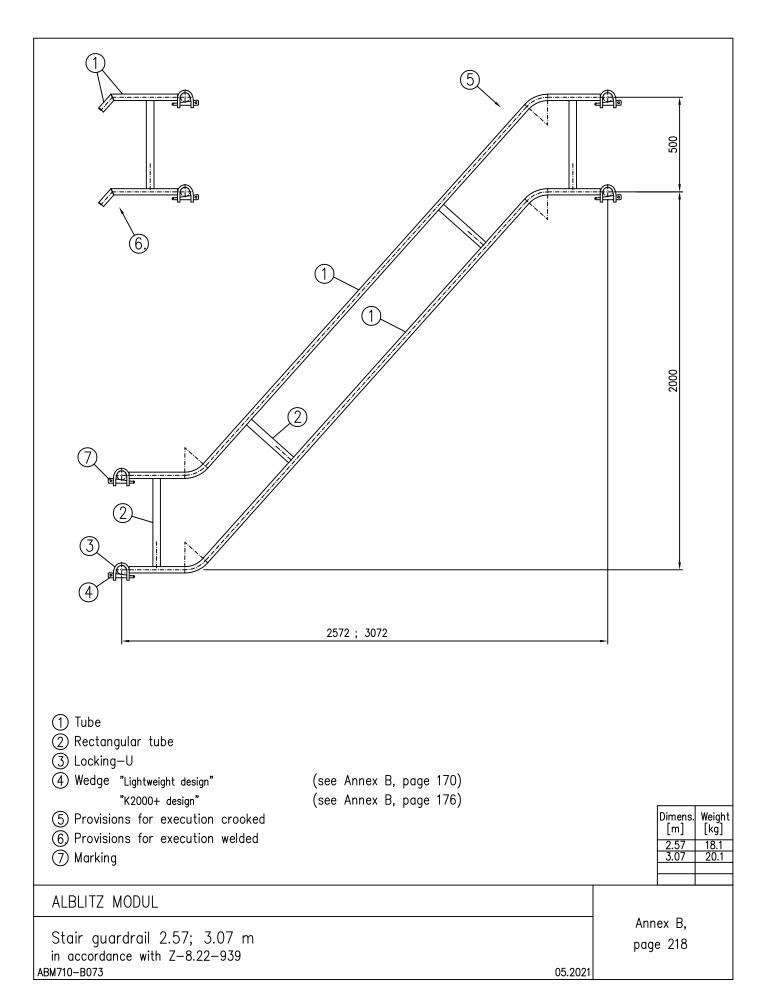


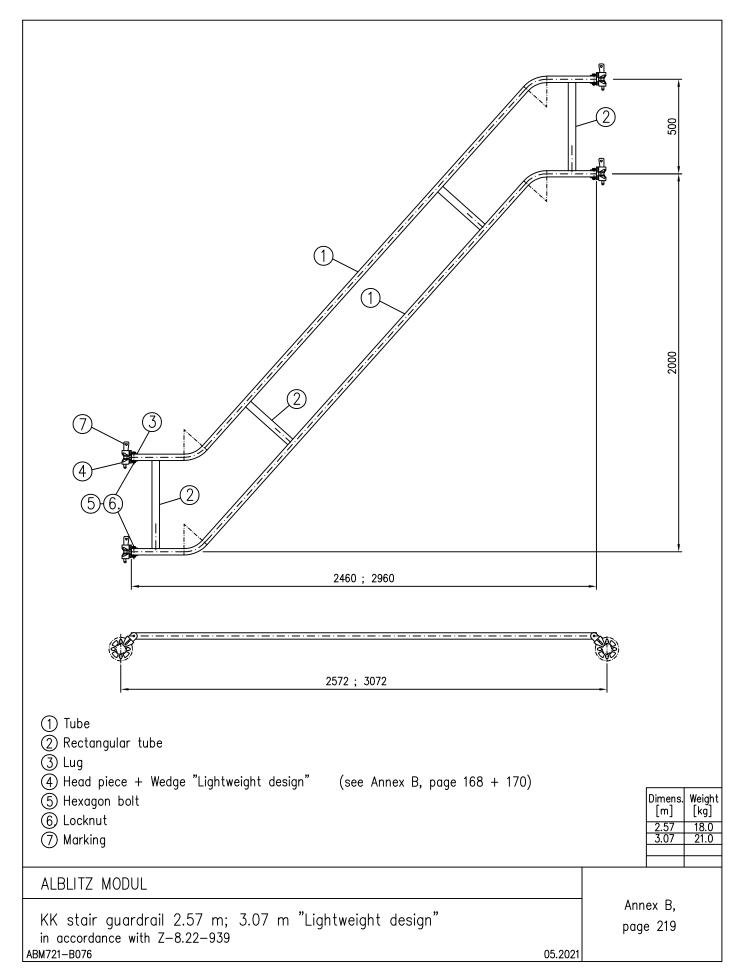


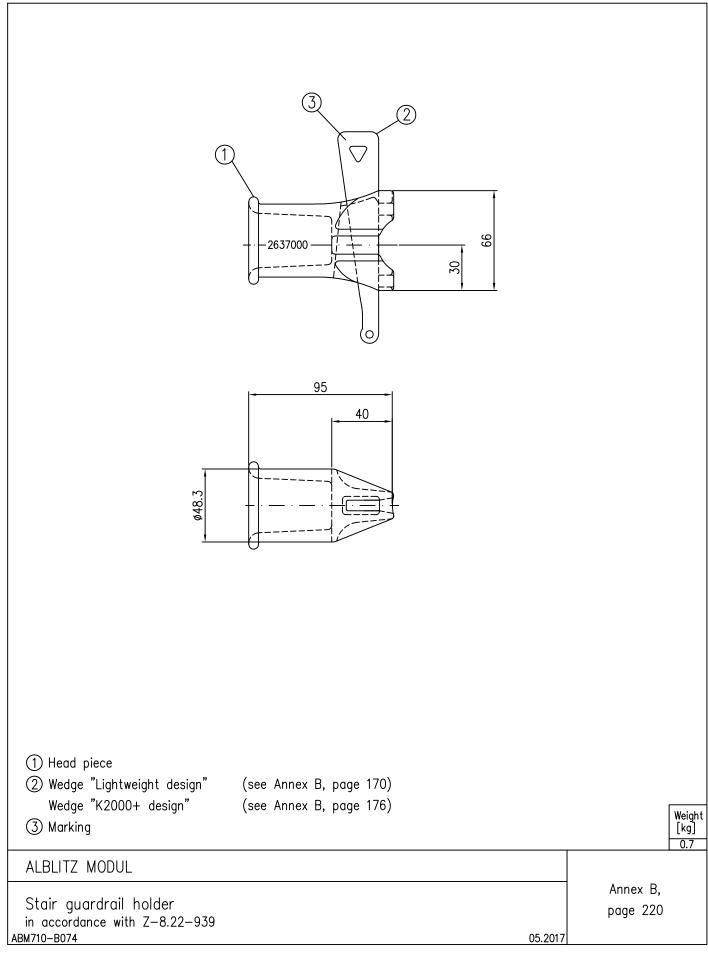


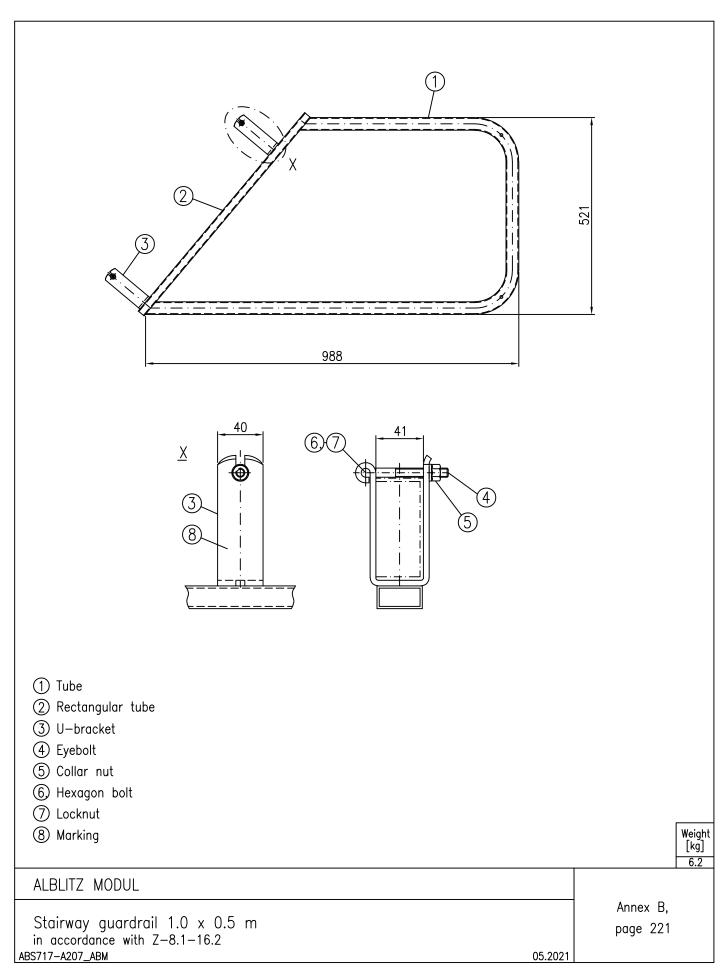


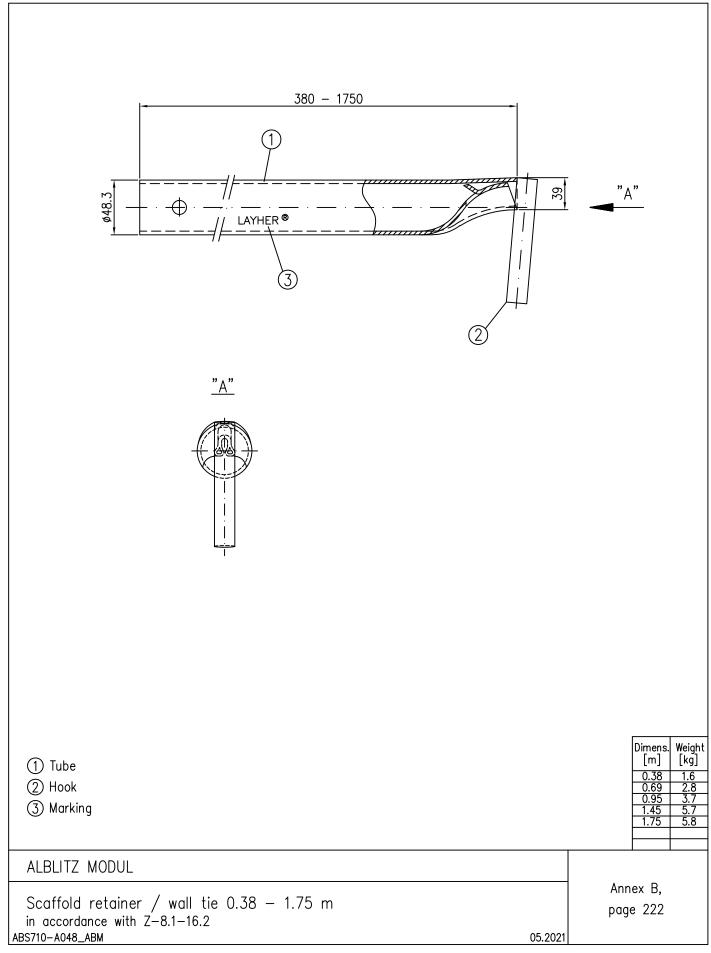


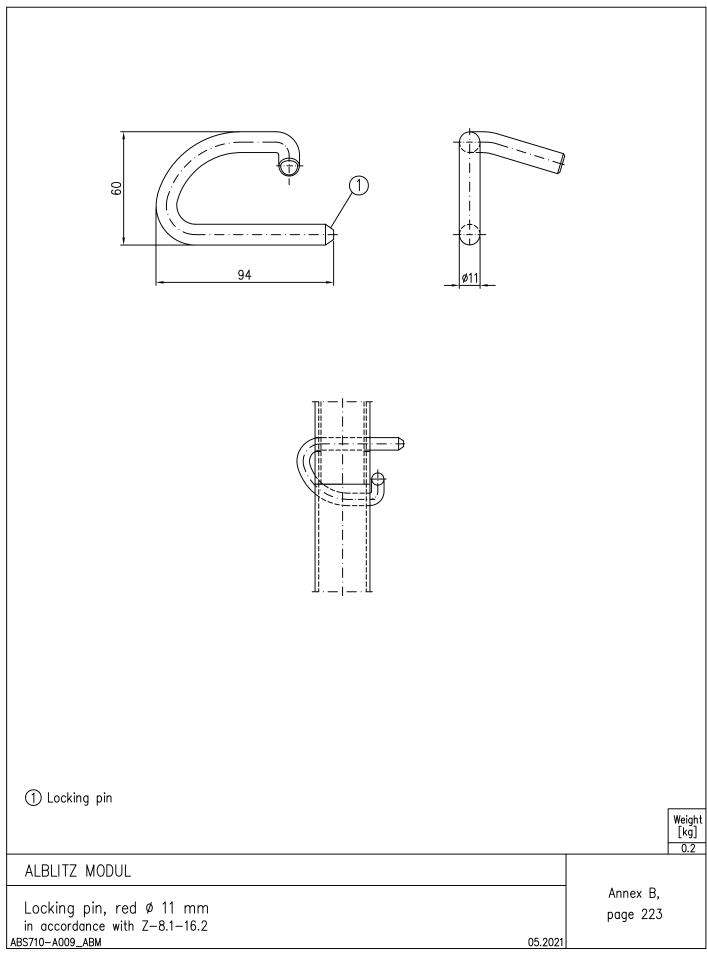


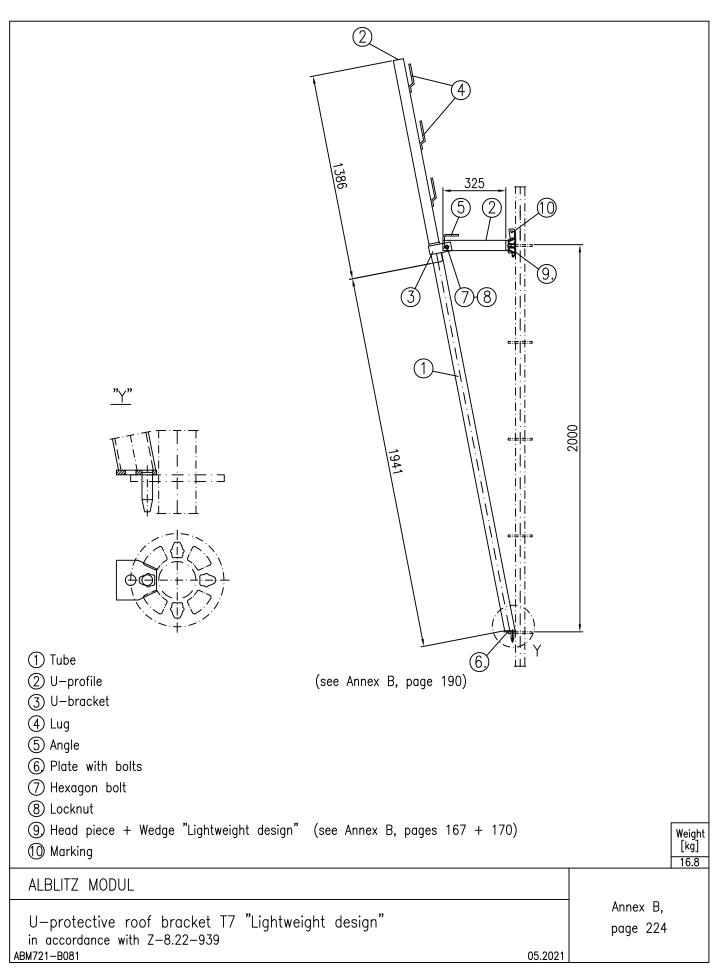


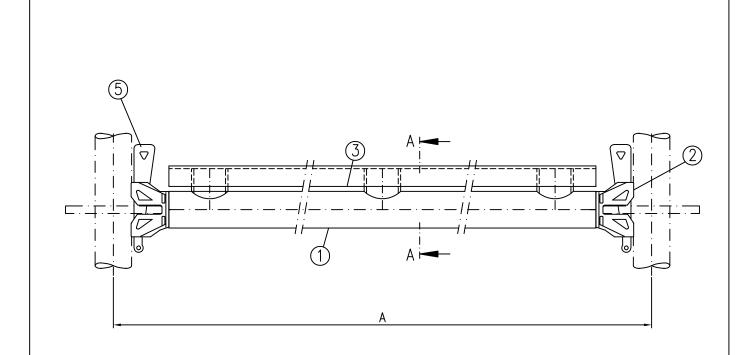






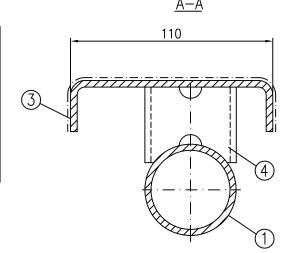






| Dimension A | | Maximum allowable p *) |
|-------------|------------|------------------------|
| [mm] | load class | [kN/m²] |
| 732 | | |
| 1088 | | |
| 1286 | | |
| 1400 | 6 | 10.0 |
| 1572 | ١ ٠ | 10.0 |
| 2072 | | |
| 2572 | | |
| 3072 | | |

*) acting on the entire sheet width



- (1) Tube
- ② Head piece + Wedge "Lightweight design" (see Annex B, pages 166 + 170)
- 3 Bulb plate
- 4 Spacer tube
- (5) Marking

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 0.73 | 5.2 |
| 1.09 | 7.6 |
| 1.29 | 8.9 |
| 1.40 | 9.7 |
| 1.57 | 10.8 |
| 2.07 | 14.2 |
| 2.57 | 17.6 |

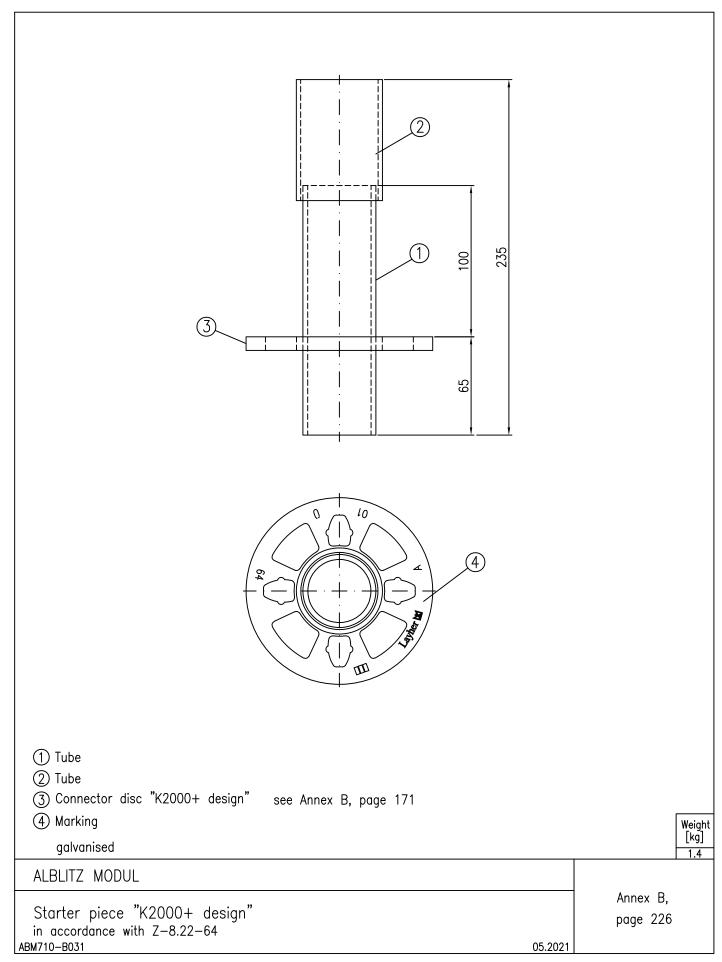
ALBLITZ MODUL

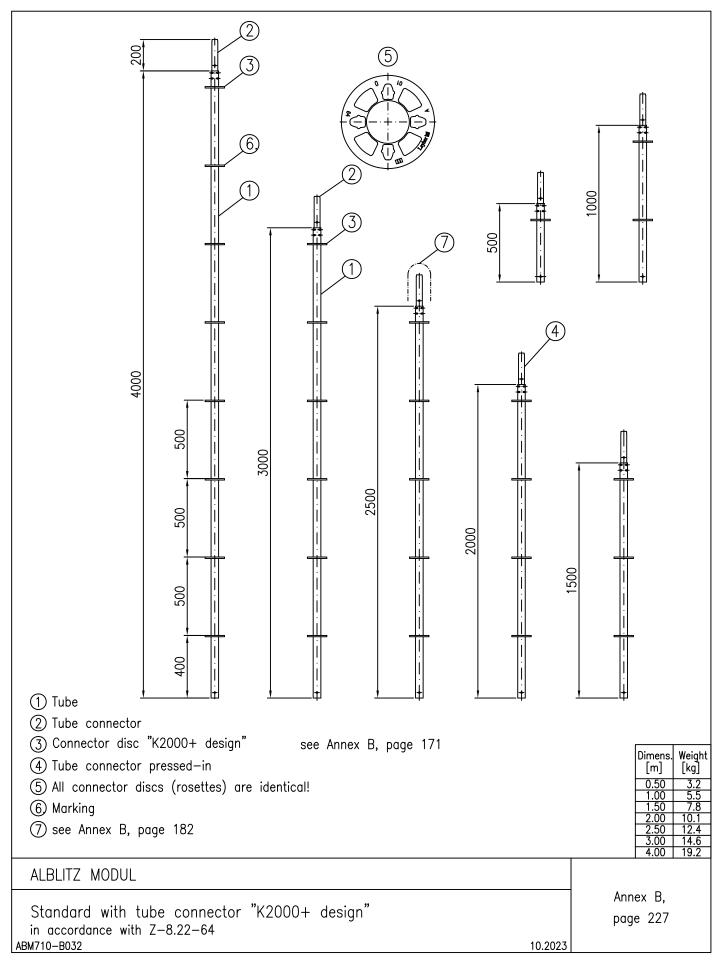
U-ledger with gap cover, lightweight 0.73 - 3.07 m in accordance with Z-8.1-939

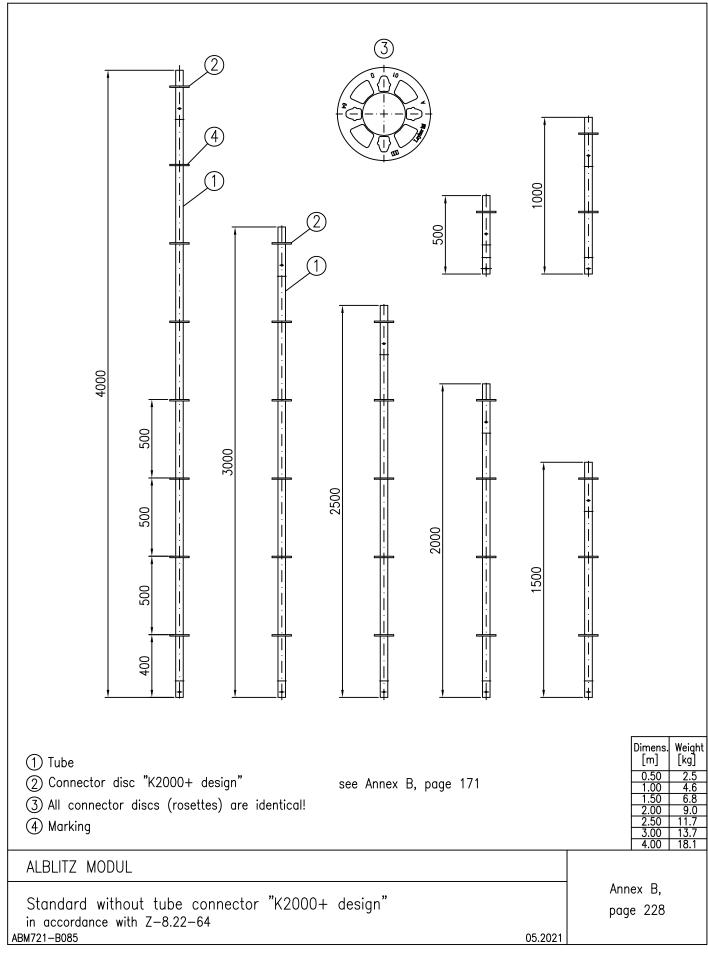
ABM721-B082

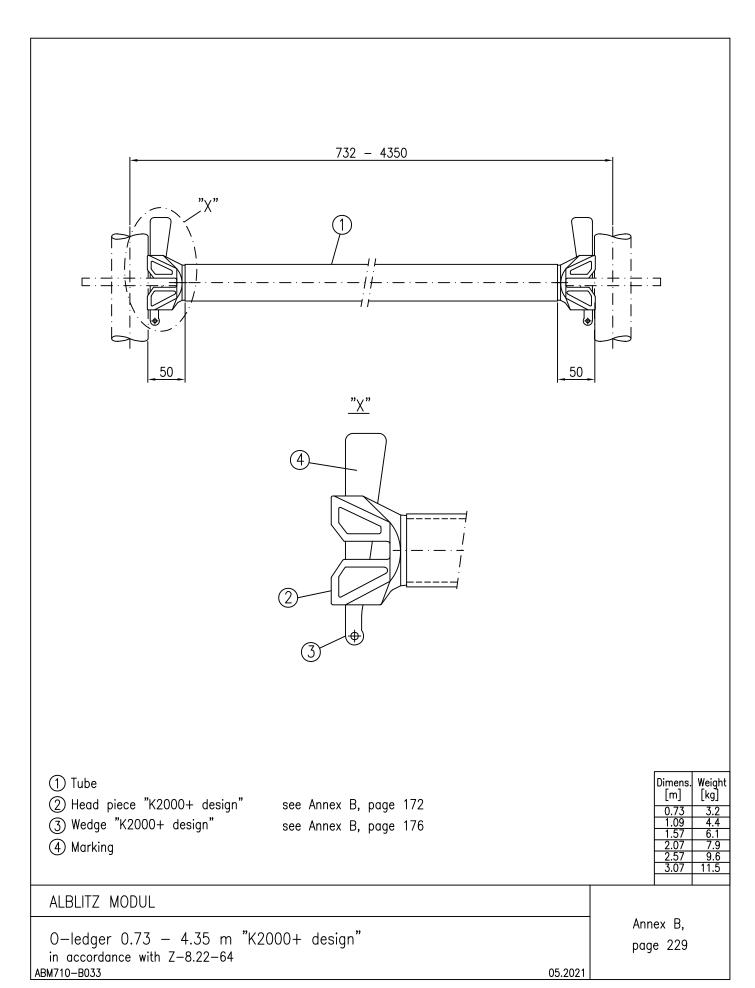
Annex B, page 225

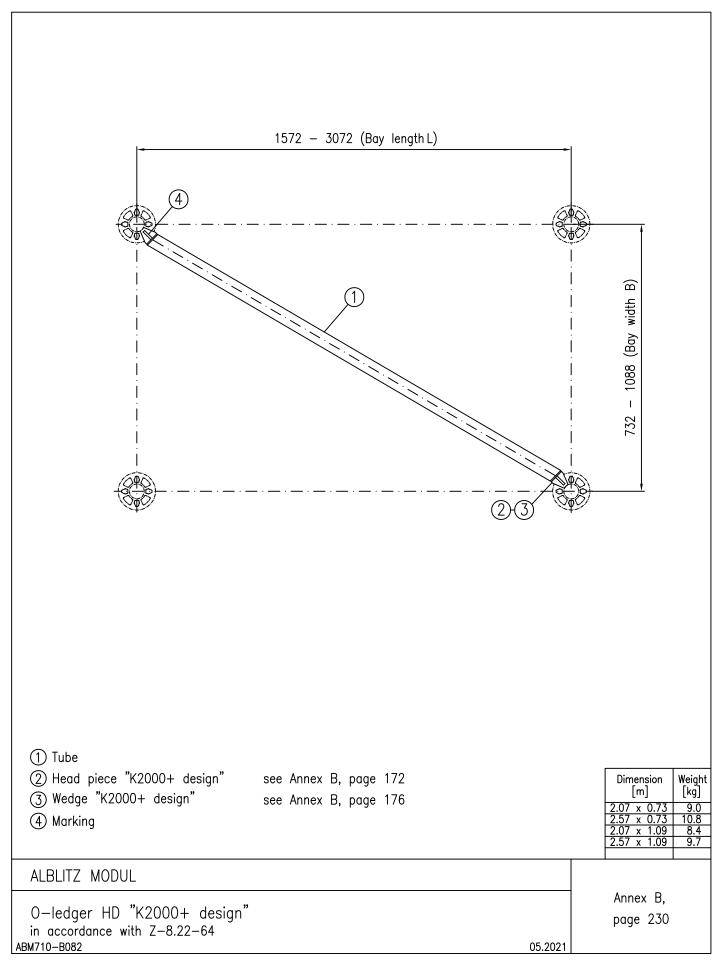
05.2021

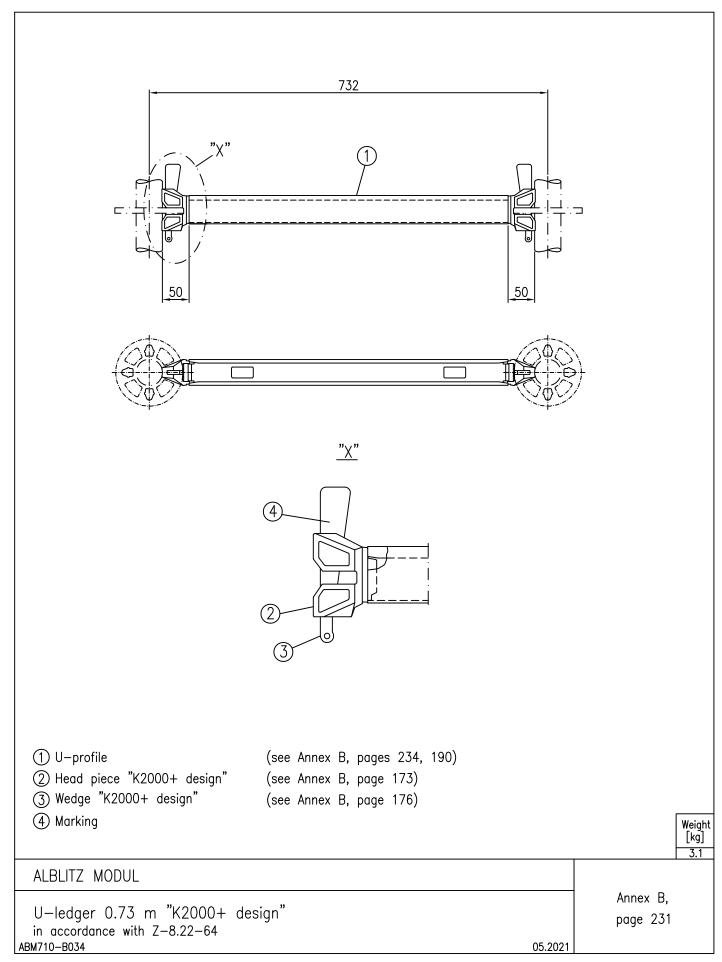


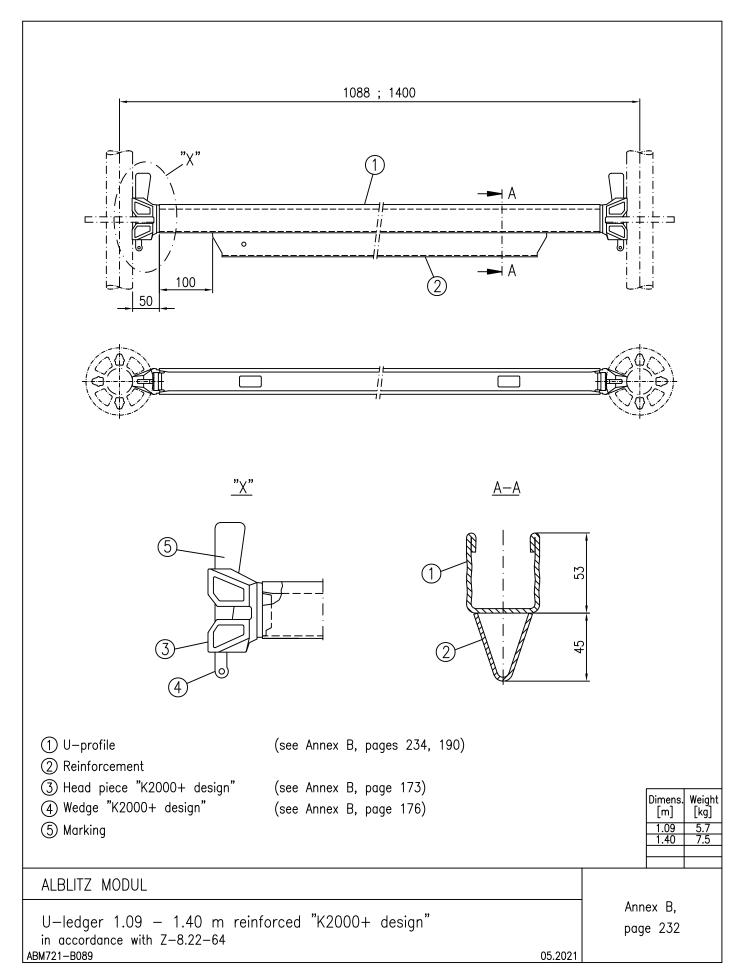


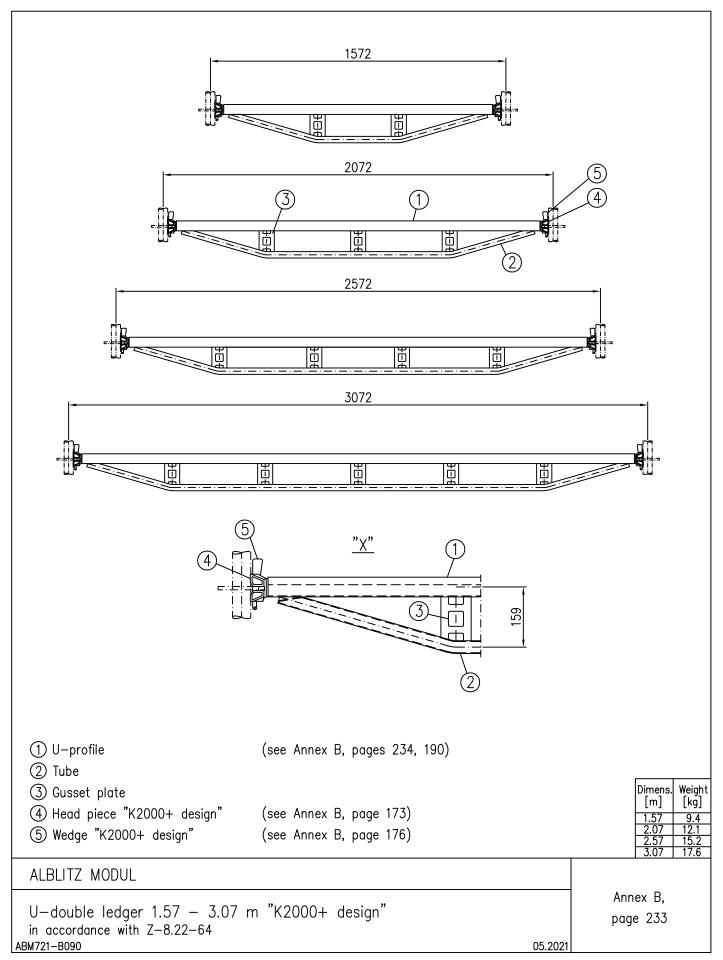


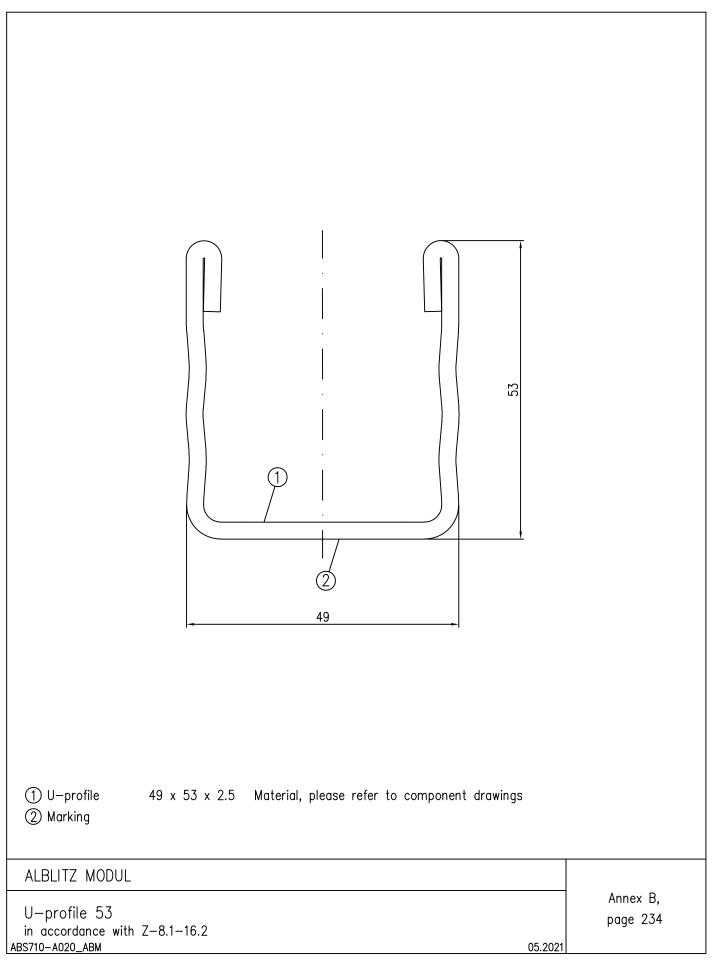


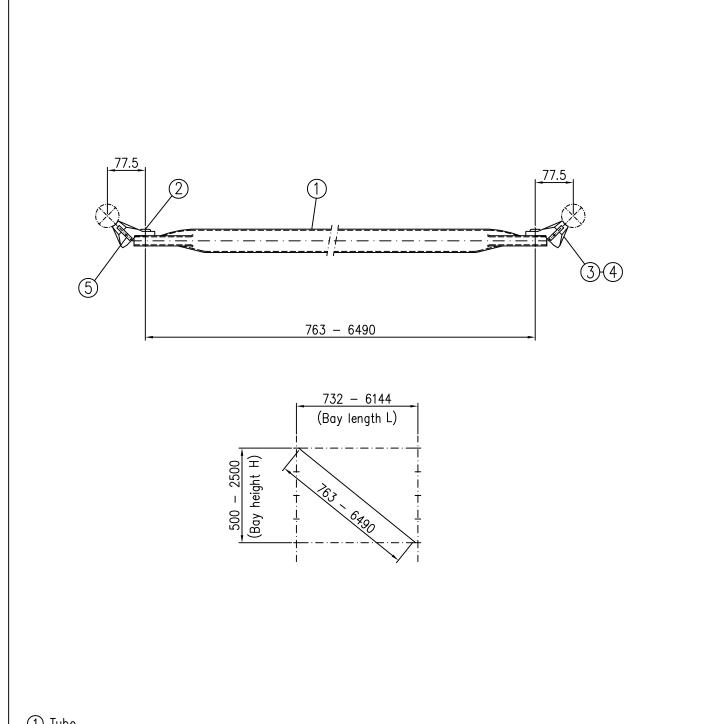












| (1) Iu | be |
|--------|----|
|--------|----|

2 Cylinder head rivet

3 Head piece "K2000+ design" see Annex B, page 175

4 Wedge "K2000+ design"

see Annex B, page 175 see Annex B, page 176

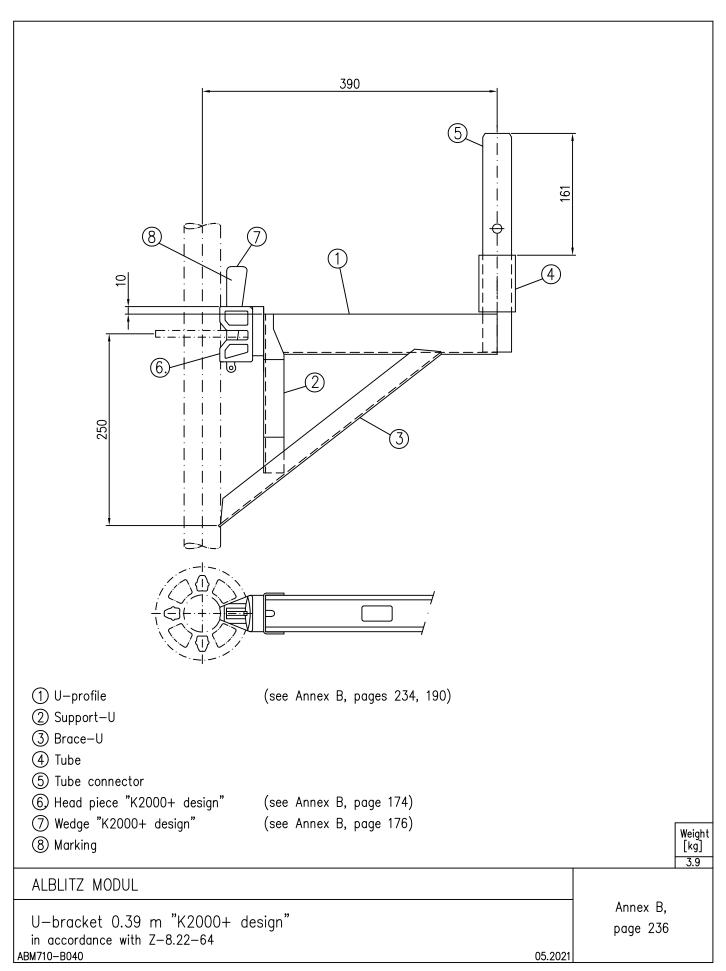
Marking

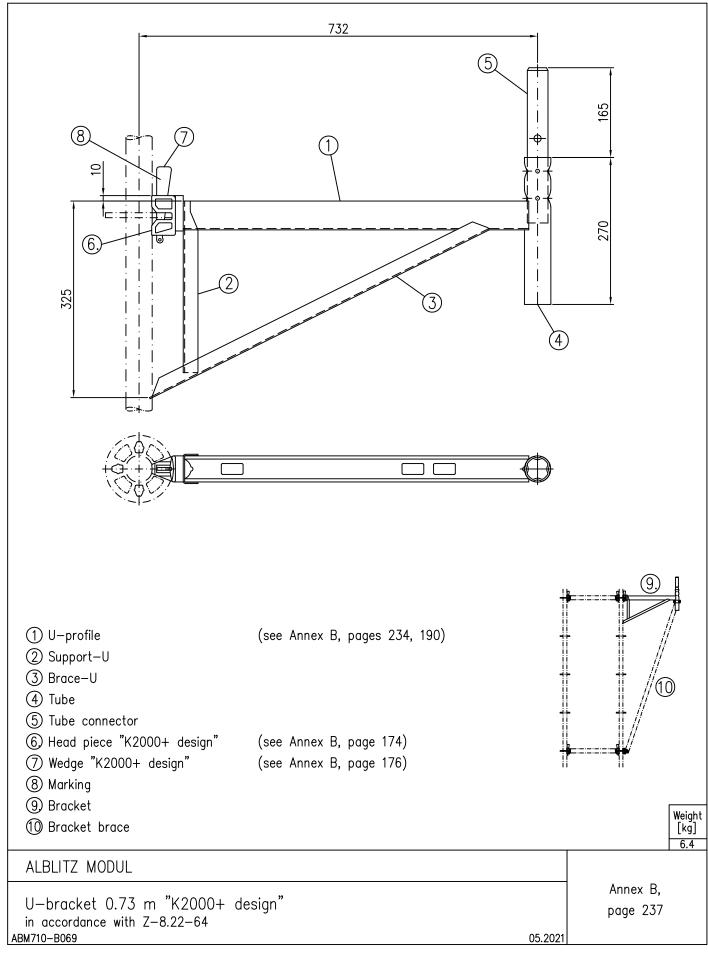
| Dimension [m] | Weight [kg] |
|------------------|----------------|
| 2.07 x 2.00 | 8.9 |
| 2.57 x 2.00 | 9.5 |
| 2.07 x 1.50 | 8.2 |
| 2.57 x 1.50 | 9.5 |
| | |

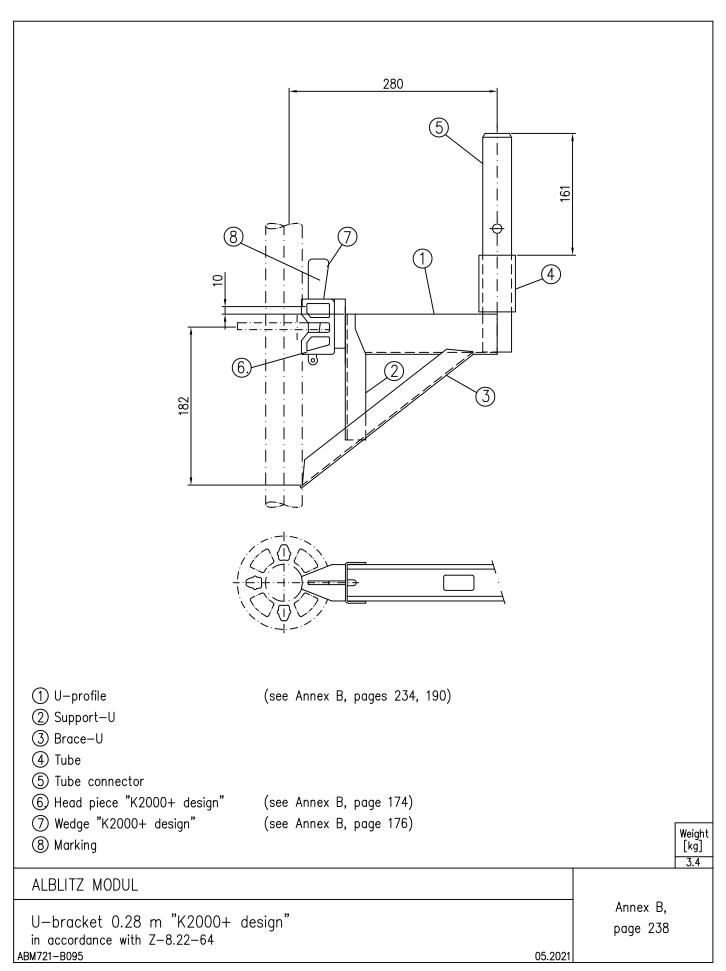
| Al B | IT7 | MODUL |
|------|-----|-------|
| | | |

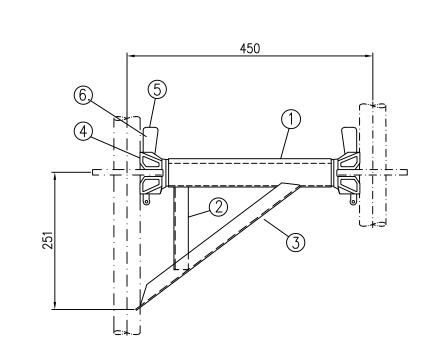
Diagonal brace "K2000+ design" in accordance with Z-8.22-64
ABM710-B036

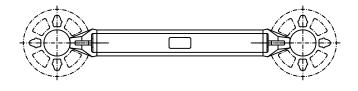
Annex B, page 235











- ① U-profile (see Annex B, pages 234, 190)
- ② Support-U
- 3 Brace-U
- 4 Head piece "K2000+ design"
- (5) Wedge "K2000+ design"

6 Marking

(see Annex B, page 173)

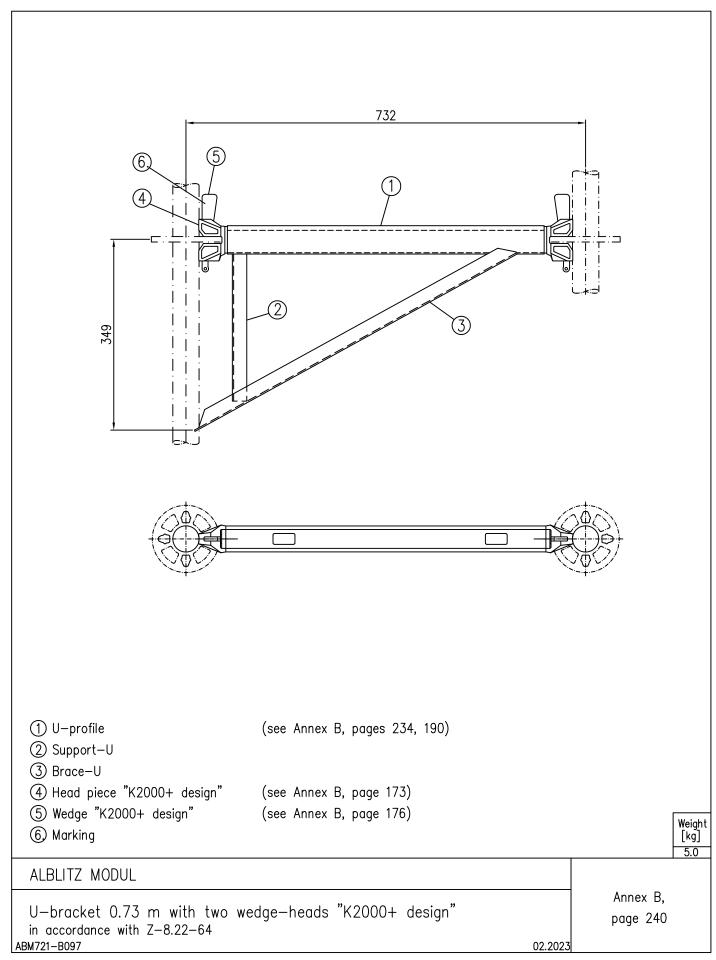
(see Annex B, page 176)

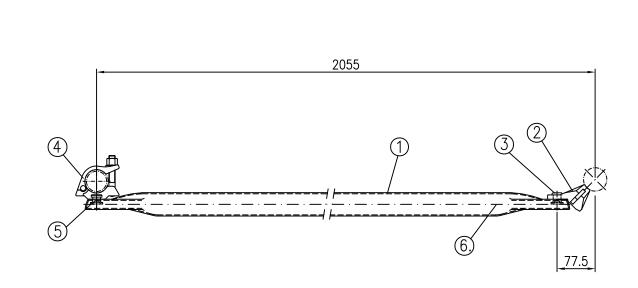
ALBLITZ MODUL

Annex B, U-bracket 0.45 m with two wedge-heads "K2000+ design" page 239 in accordance with Z-8.22-64

ABM721-B096

Weight [kg]





1 Tube

(2) Head piece + Wedge "K2000+ design"

(3) Cylinder head rivet

4 Halfcoupler with screw top

(5) Cylinder head rivet

(6) Marking

(see Annex B, pages 175 + 176)

swivel, riveted

in accordance with approval Z-8.331-882

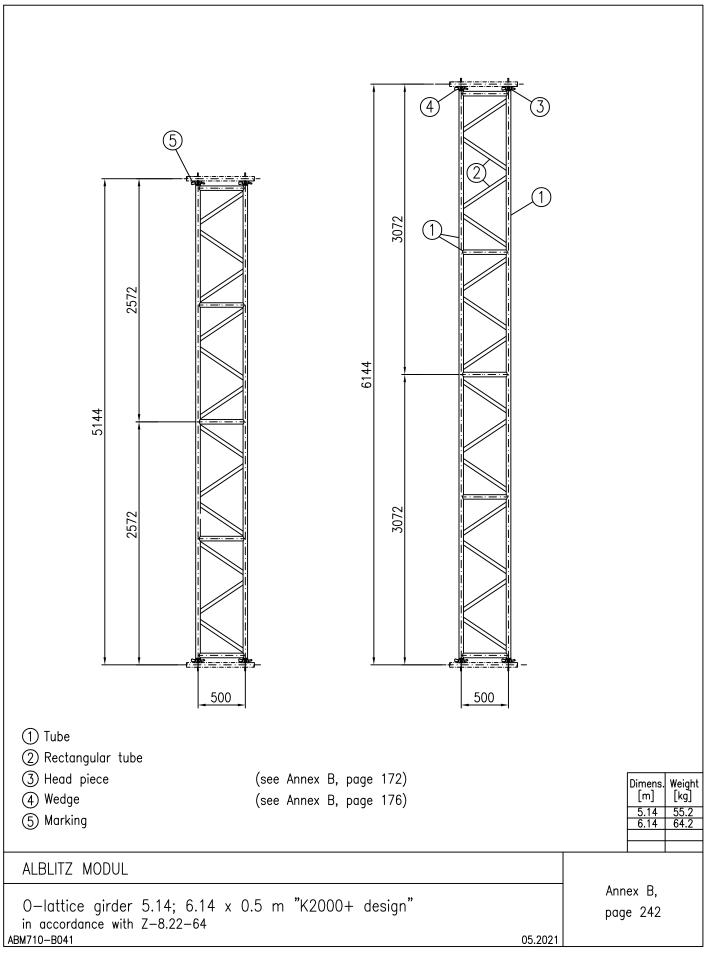
swivel, riveted

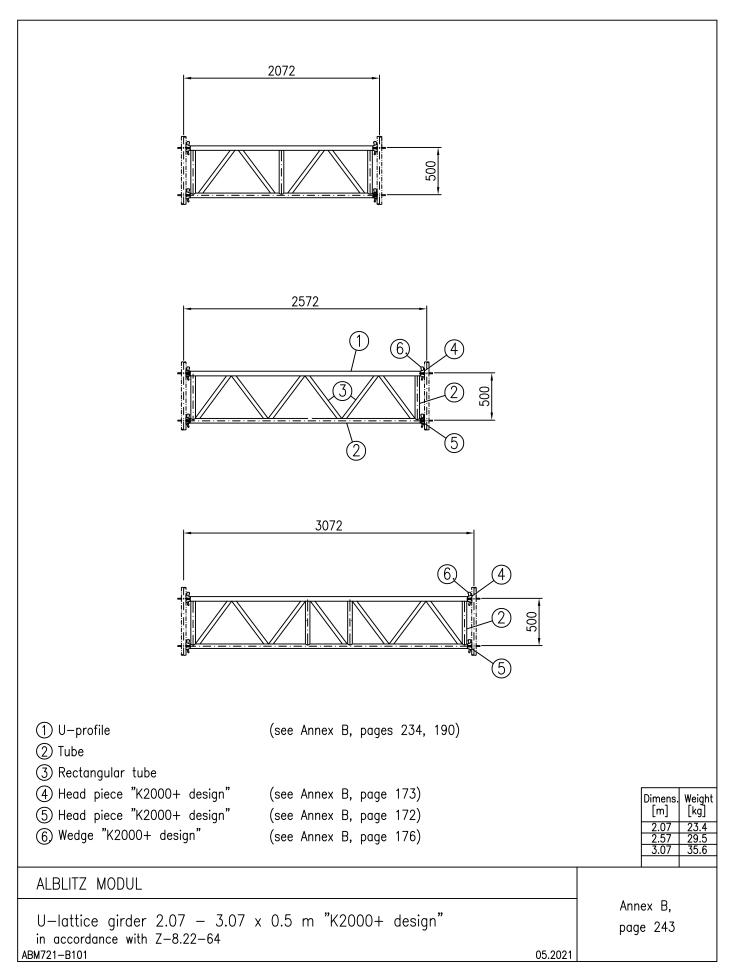
Weight [kg]

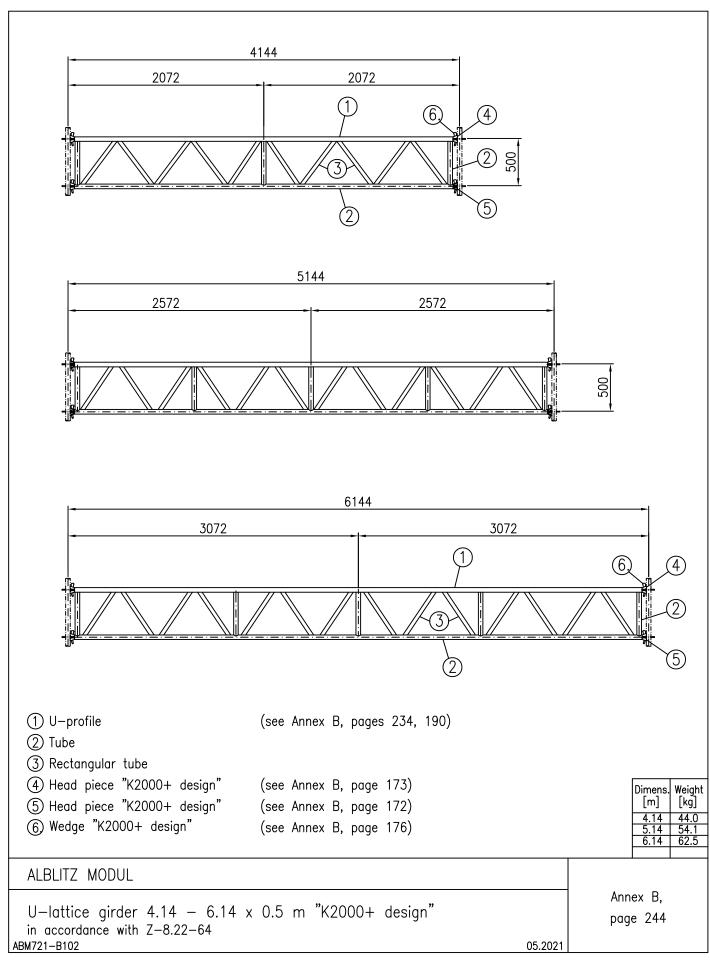
ALBLITZ MODUL

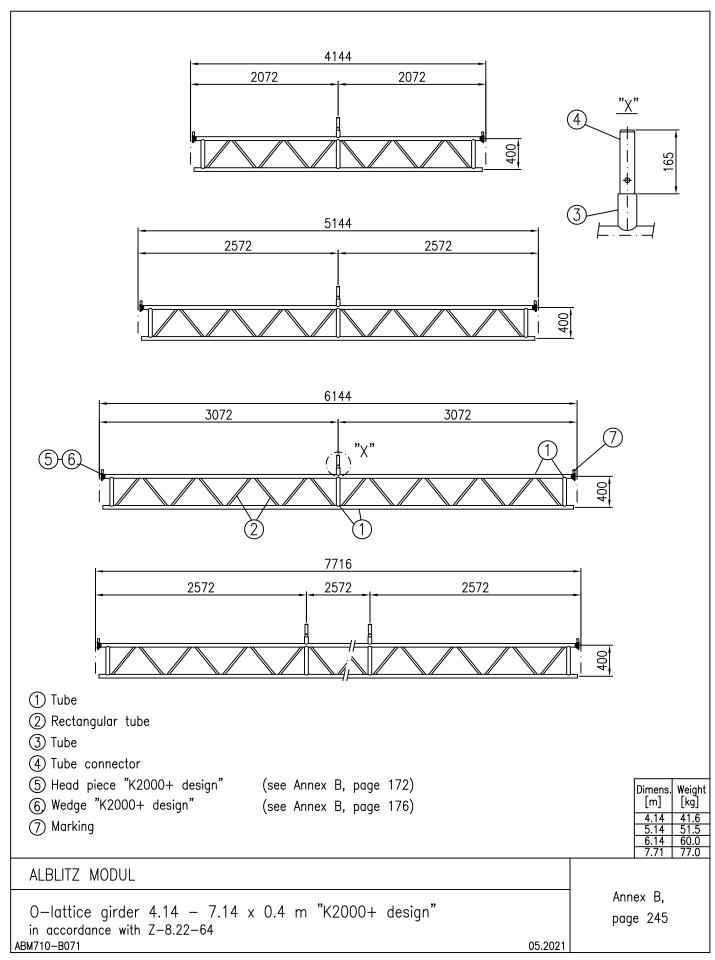
Bracket brace 2.05 m "K2000+ design" in accordance with Z-8.22-64ABM710-B070

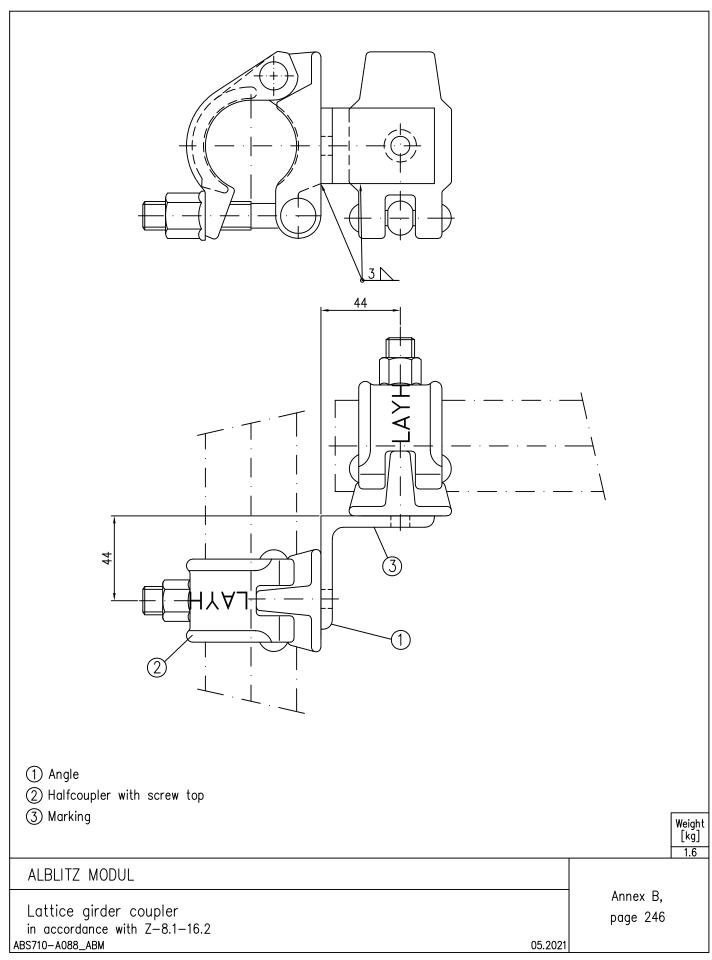
Annex B, page 241

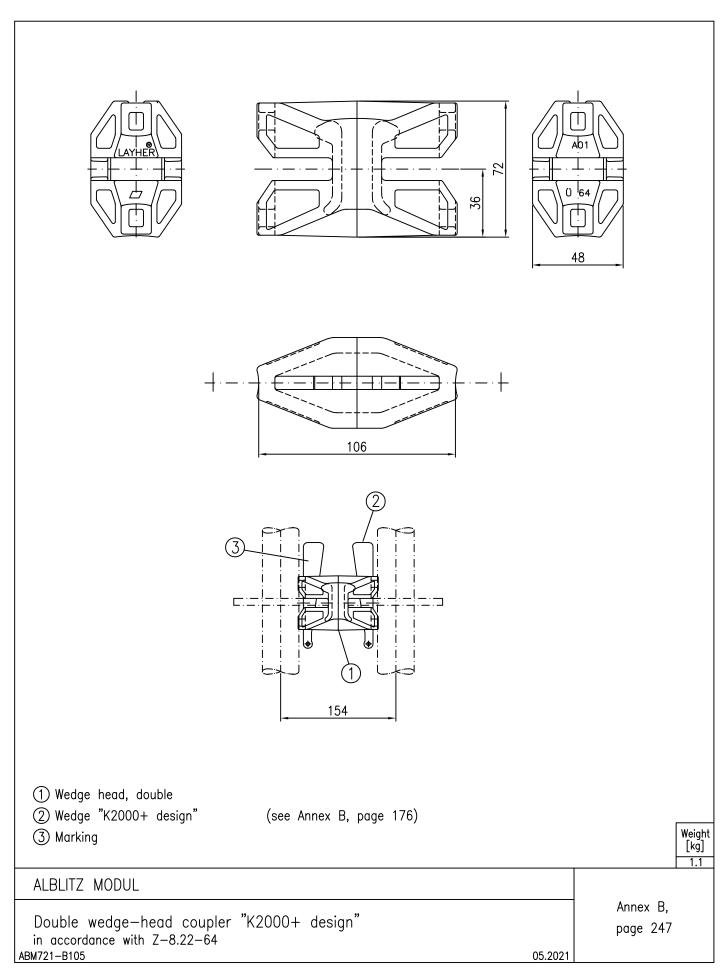


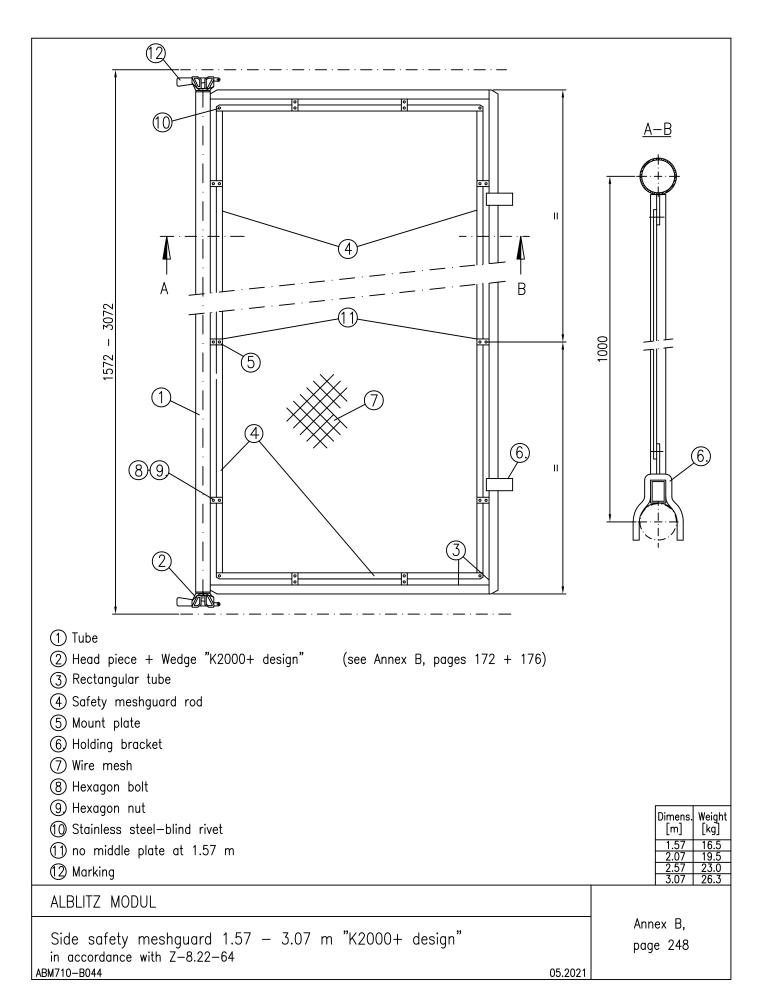


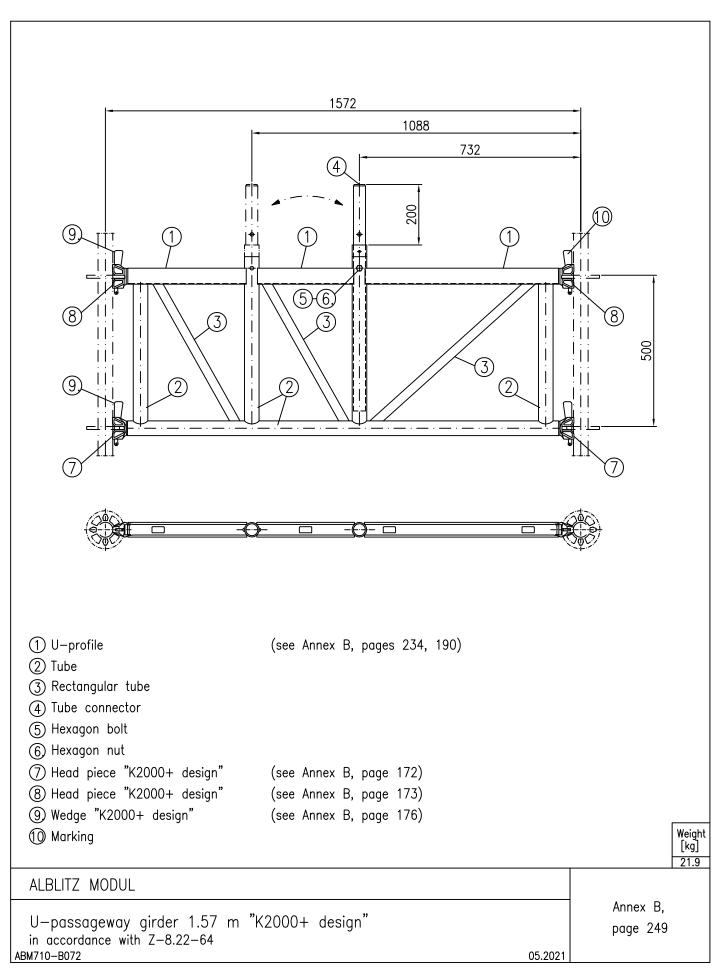


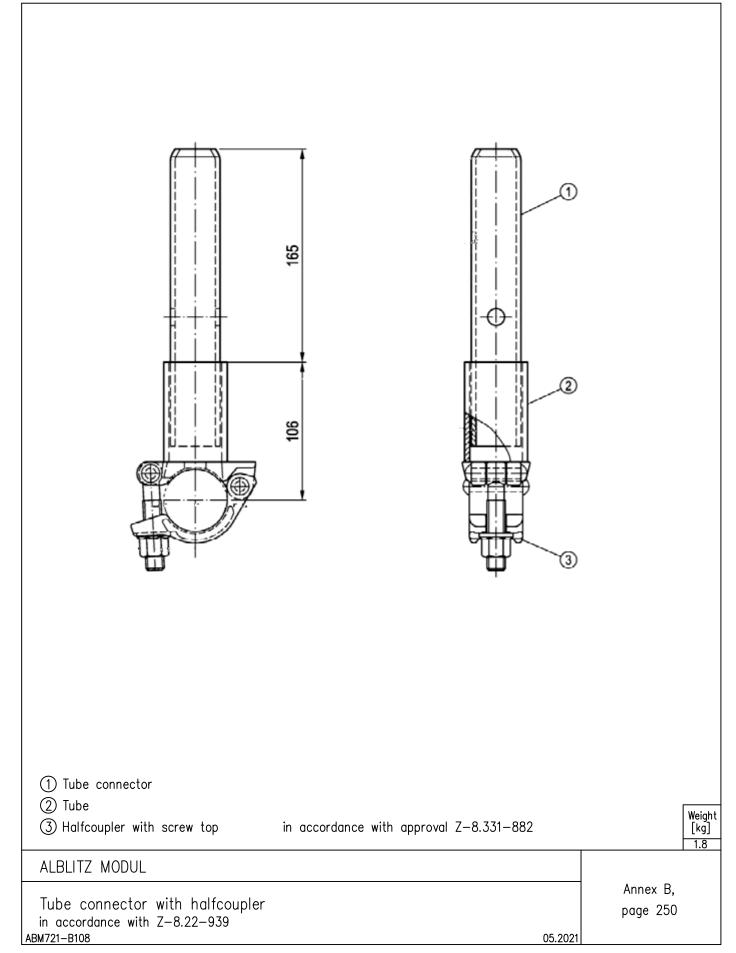


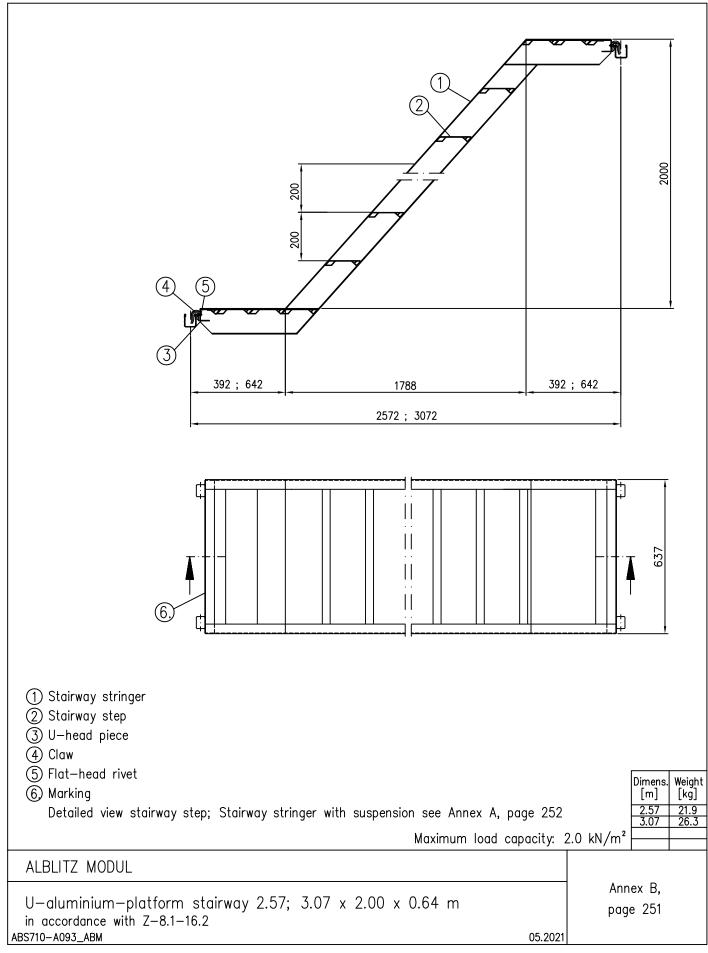


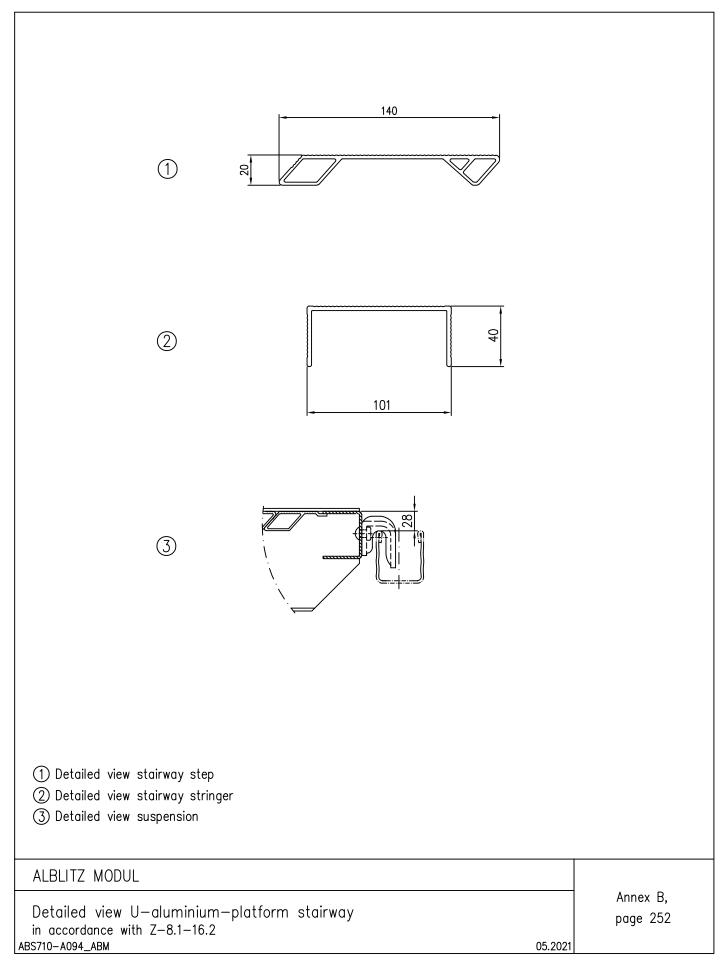


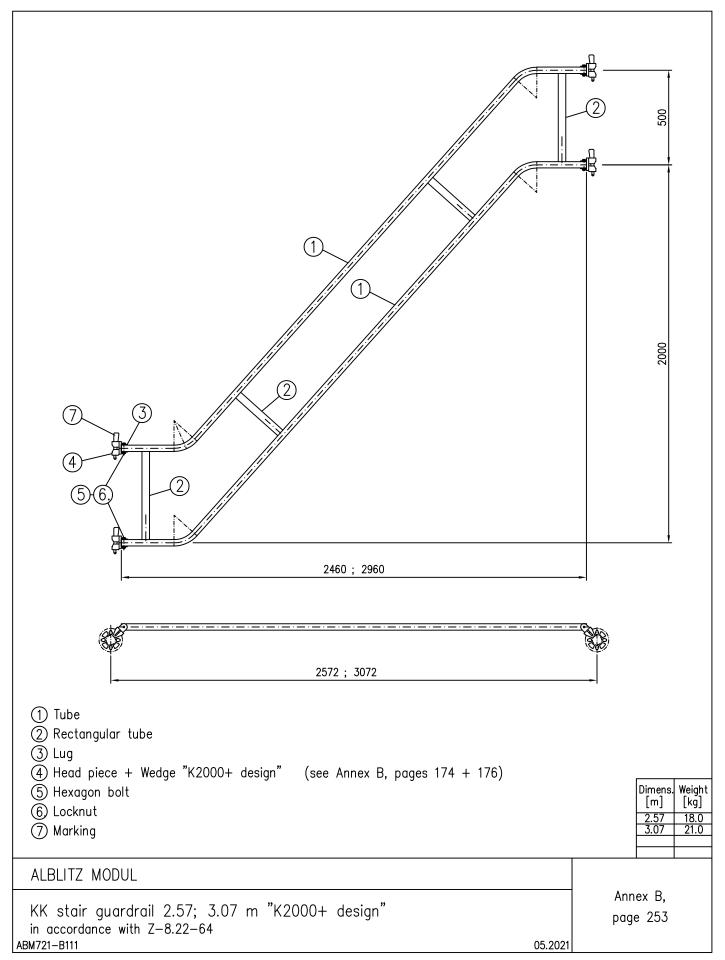


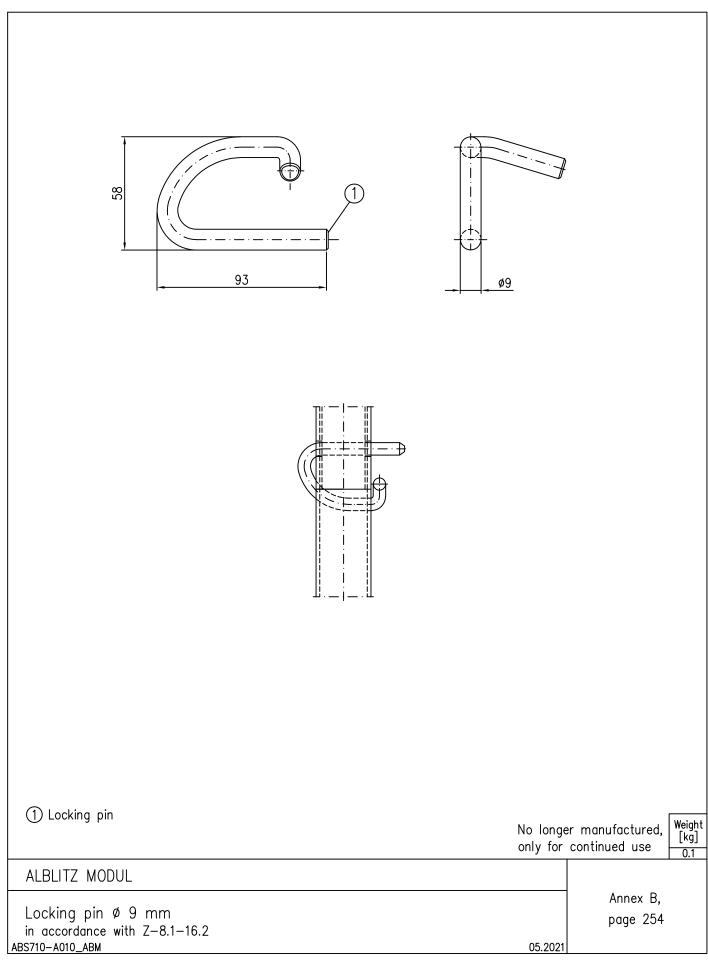


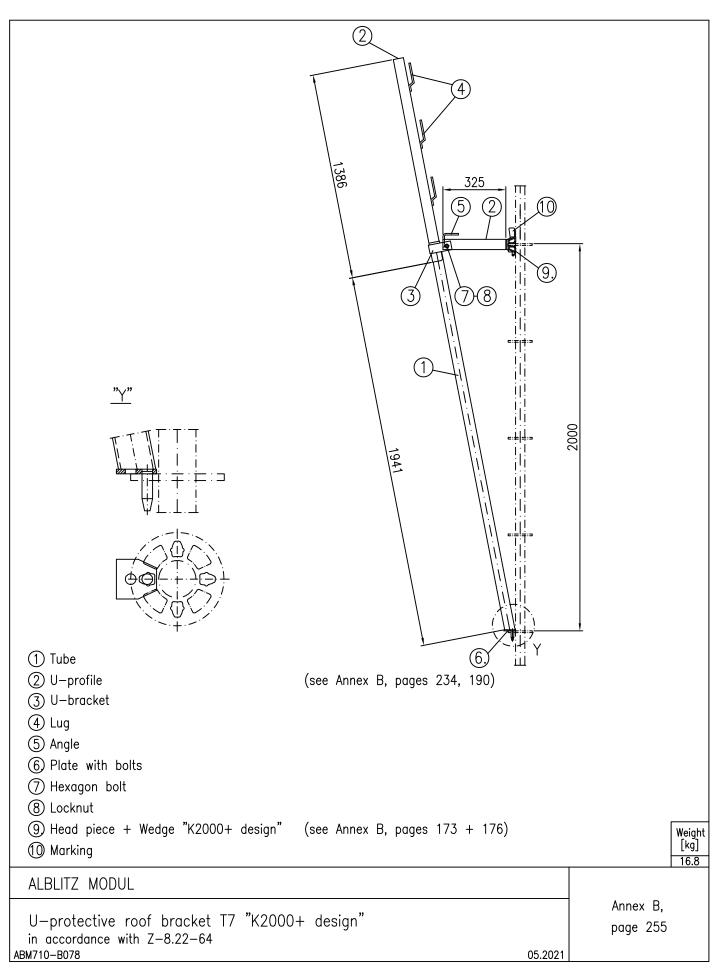


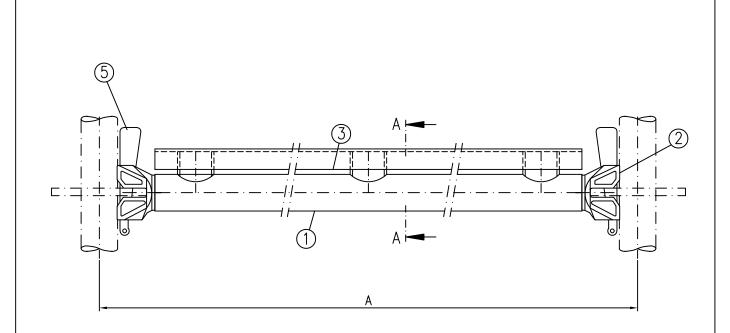






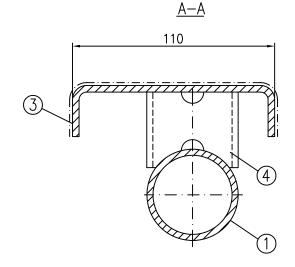






| Dimension A | For use up to | Maximum allowable p *) |
|-------------|---------------|------------------------|
| [mm] | load class | [kN/m²] |
| 732 | | |
| 1088 | | |
| 1286 | _ | 400 |
| 1400 | b | 10.0 |
| 1572 | | |
| 2072 | | |
| 2572 | 5 | 7.5 |
| 3072 | 4 | 5.0 |

*) acting on the entire sheet width

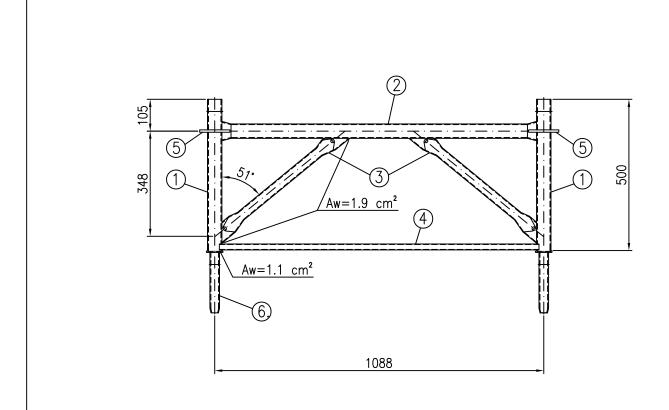


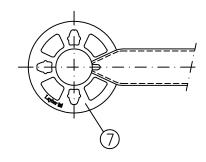
- (1) Tube
- 2 Head piece + Wedge "K2000+ design" (see Annex B, pages 172 + 176)
- 3 Bulb plate
- 4 Spacer tube
- (5) Marking

| Weight [kg] |
|----------------|
| 5.7 |
| 8.3 |
| 9.9 |
| 10.0 |
| 11.9 |
| 15.2 |
| 18.6 |
| |

ALBLITZ MODUL

U-ledger with gap cover 0.73 - 3.07 m "K2000+ design" in accordance with Z-8.1-64ABM721-B114





- 1 Tube
- 2 Tube
- (3) Tube
- (4) Rectangular tube

⑤ Connector disc "K2000+ design" (see Annex B, page 171)

6 Tube connector compressed

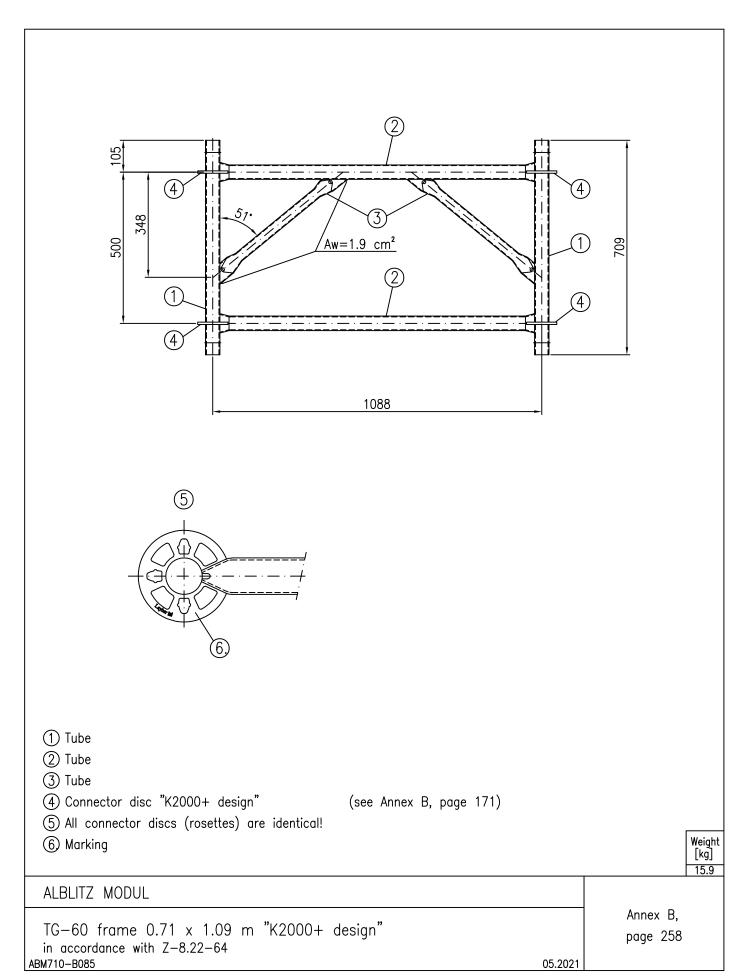
(in accordance with Z-8.1-16.2)

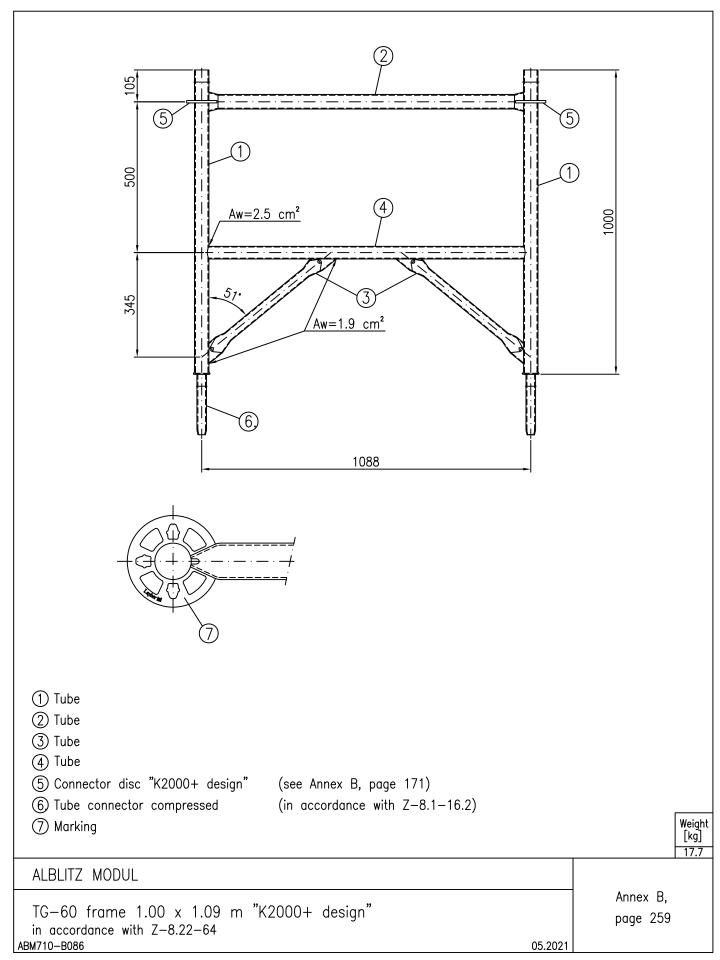
Marking

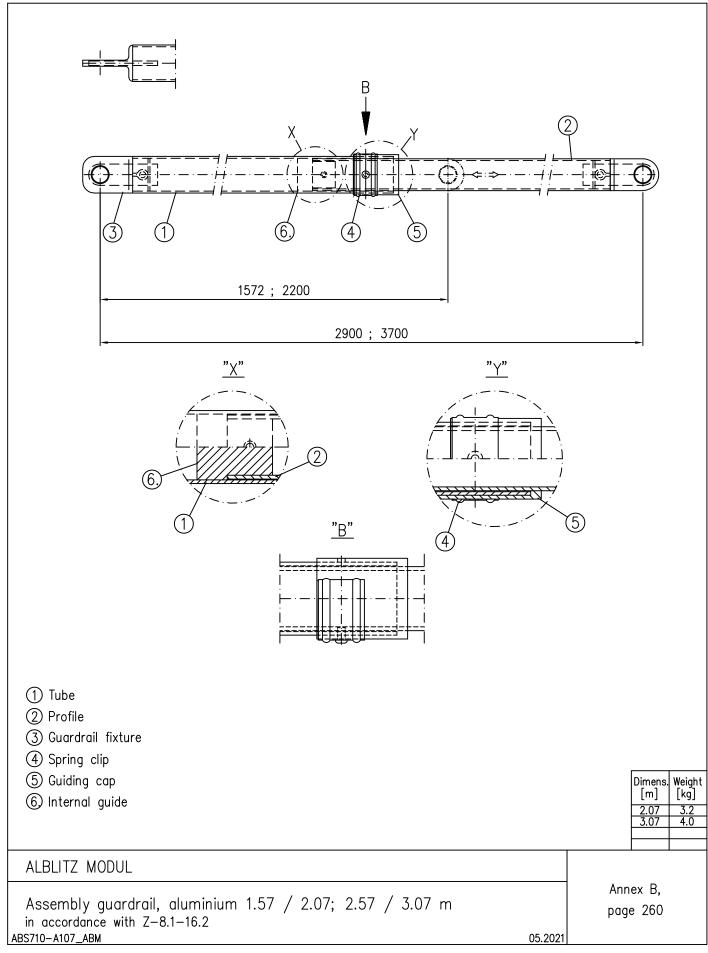
Weight [kg]

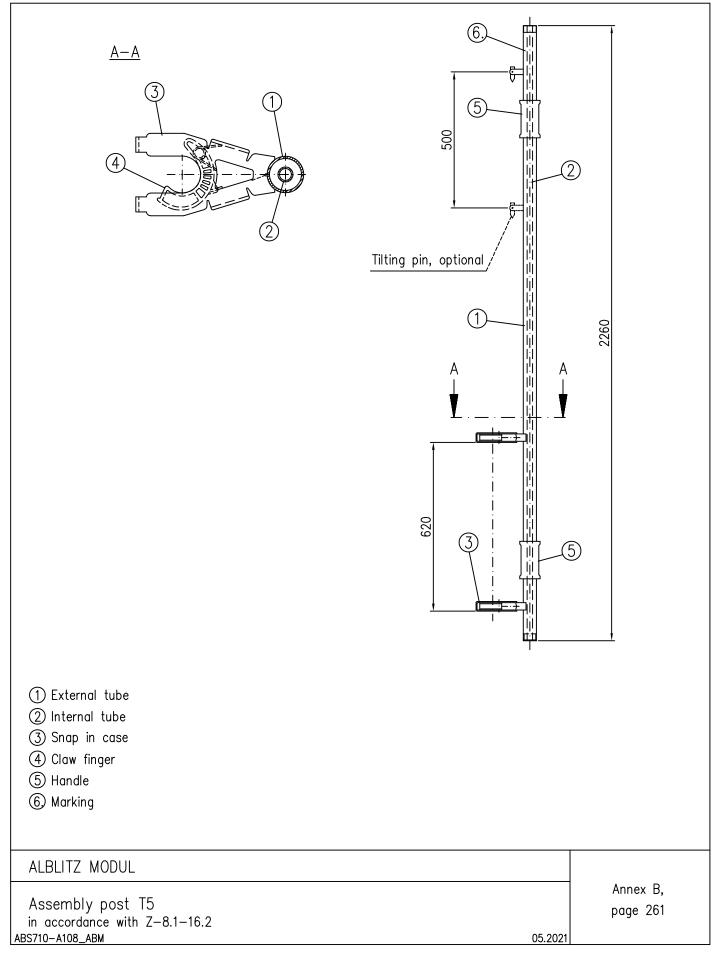
| ALBLITZ MODUL |
|---------------|
|---------------|

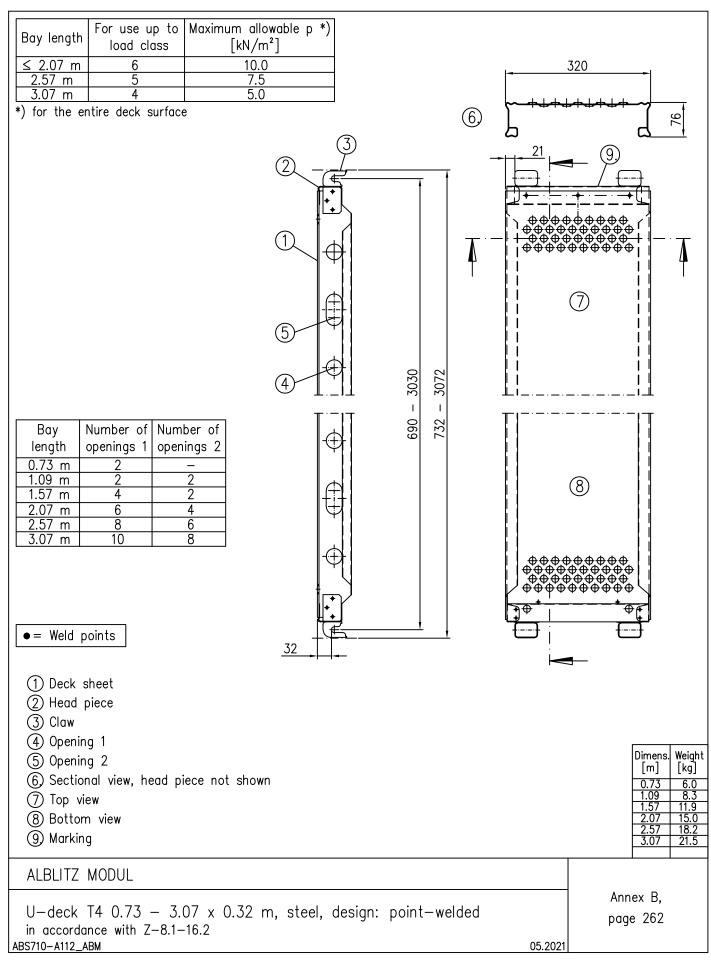
TG-60 frame 0.50 x 1.09 m "K2000+ design" in accordance with Z-8.22-64ABM7<u>10-</u>B084











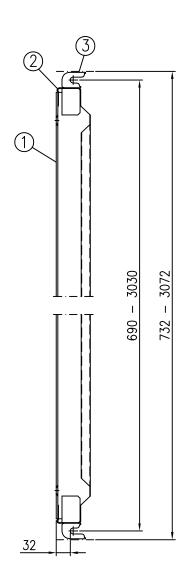
| 4 Opening 1 [m] [kg] 5 Opening 2 0.73 6.0 6 Sectional view, head piece not shown 1.57 11.9 7 Marking 2.07 15.0 | | |
|---|---|---|
| Bay Number of Number of length openings 1 openings 2 0.73 m 2 0.73 m 4 0.22 0.73 m 8 6 4 0.25 m 8 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 | $ \begin{array}{c ccccccccccccccccccccccccccccccccccc$ | ~ + + + + + + + - / , |
| ② Head piece ③ Claw ④ Opening 1 [m] [kg] ⑤ Opening 2 0.73 6.0 ⑥ Sectional view, head piece not shown 1.57 11.9 ⑦ Marking 2.07 15.0 ALBLITZ MODUL Annex B, | Bay Number of Number of length openings 1 openings 2 0.73 m 2 - 1.09 m 2 2 1.57 m 4 2 2.07 m 6 4 2.57 m 8 6 3.07 m 10 8 | 232 - 3030 732 - 3072 |
| Annex B, | ② Head piece ③ Claw ④ Opening 1 ⑤ Opening 2 ⑥ Sectional view, head piece not shown ⑦ Marking | 0.73 6.0 1.09 8.3 1.57 11.9 2.07 15.0 2.57 18.2 |
| | | oign, hand wolded |

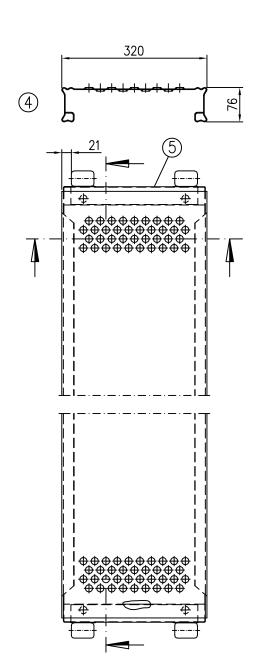
ABS710-A113_ABM

| Bay length For use up to Maximum allowable p *) | |
|--|---|
| ② Head piece ③ Claw ④ Sectional view, head piece not shown ⑤ Top view ⑥ Bottom view ⑦ Marking | Dimens. Weight [m] [kg] 0.73 6.1 1.09 8.6 1.57 11.9 2.07 15.4 2.57 18.7 3.07 22.2 |
| ALBLITZ MODUL U-deck 0.73 - 3.07 x 0.32 m, steel, design: point-welded | Annex B, |
| in accordance with Z-8.1-16.2 ABS710-A115_ABM 05.2021 | page 264 |

| Bay length | For use up to load class | Maximum allowable p *) [kN/m²] |
|---------------|--------------------------|--------------------------------|
| \leq 2.07 m | 6 | 10.0 |
| 2.57 m | 5 | 7.5 |
| 3.07 m | 4 | 5.0 |

*) for the entire deck surface



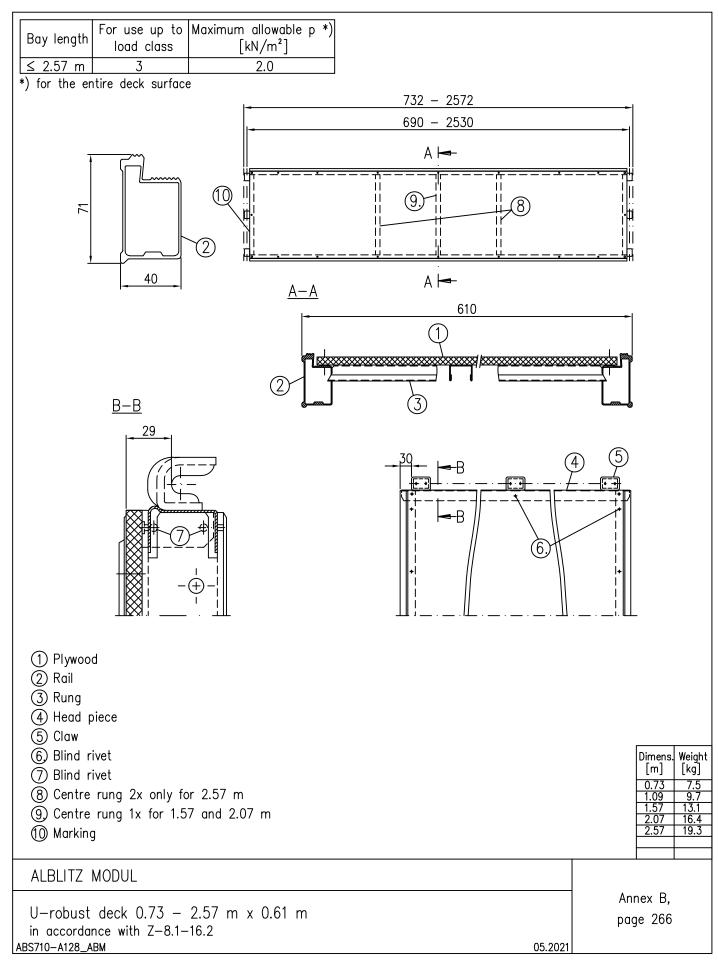


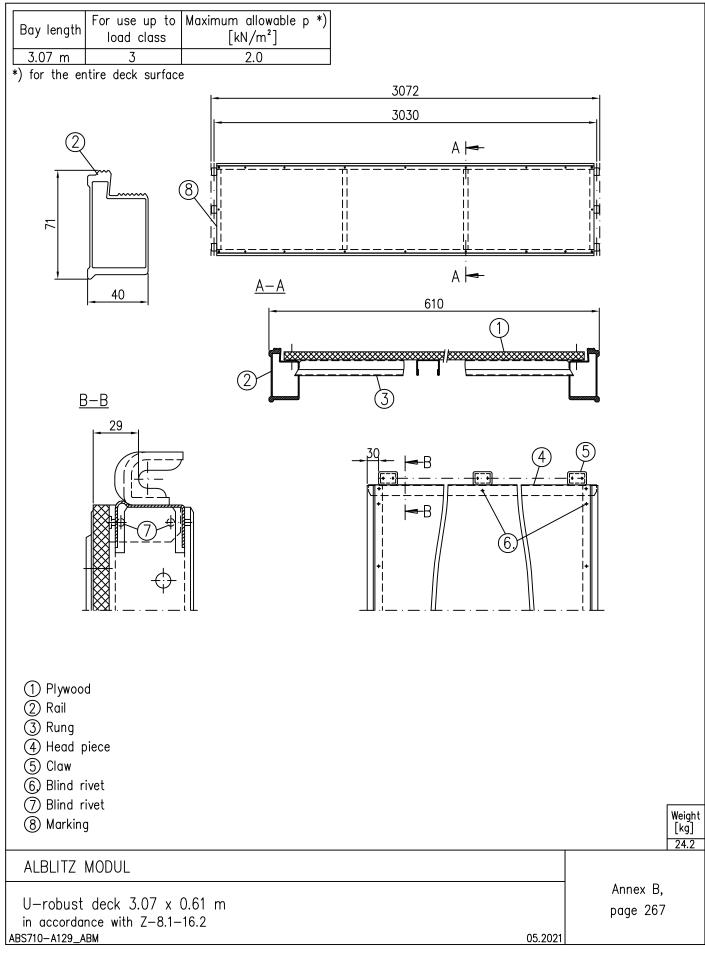
- 1) Deck sheet
- 2 Head piece
- 3 Claw
- (4) Sectional view, head piece not shown
- (5) Marking

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 0.73 | 6.1 |
| 1.09 | 8.6 |
| 1.57 | 11.9 |
| 2.07 | 15.4 |
| 2.57 | 18.7 |
| 3.07 | 22.2 |
| | |

ALBLITZ MODUL

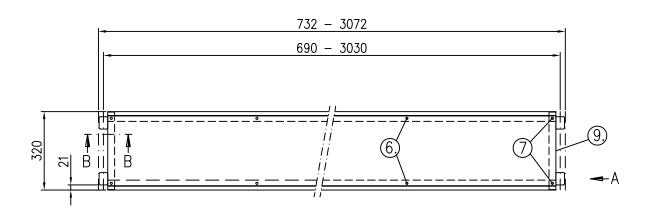
U-deck $0.73-3.07 \times 0.32$ m, steel, design: hand-welded in accordance with Z-8.1-16.2 ABS710-A116_ABM



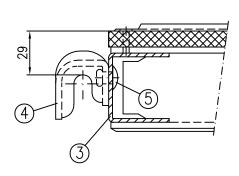


| Bay length | For use up to load class | Maximum allowable p *) [kN/m²] | Bay length | For use up to load class | Maximum allowable p *) [kN/m²] |
|------------|--------------------------|--------------------------------|------------|--------------------------|--------------------------------|
| ≤ 1.57 m | 6 | 10.0 | 2.57 m | 4 | 5.0 |
| 2.07 m | 5 | 7.5 | 3.07 m | 3 | 2.0 |

^{*)} for the entire deck surface



<u>B-B</u>



<u>"A"</u>

- 1 Plywood
- ② Rail
- 3 Head piece
- (4) Claw
- (5) Flat-head rivet
- 6) Blind rivet
- 7 Blind rivet
- (8) Detailed view profile (see Annex B, page 266)
- Marking

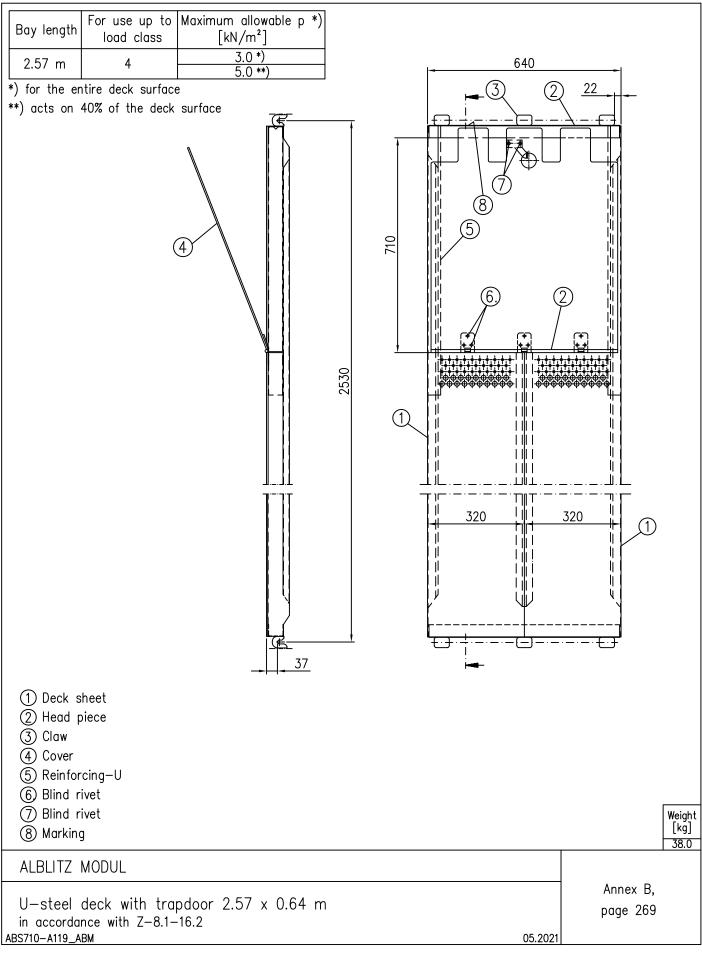
| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 0.73 | 7.4 |
| 1.09 | 8.4 |
| 2.07 | 11.5 |
| 2.57 | 14.7 |
| 3.07 | 16.0 |
| | |

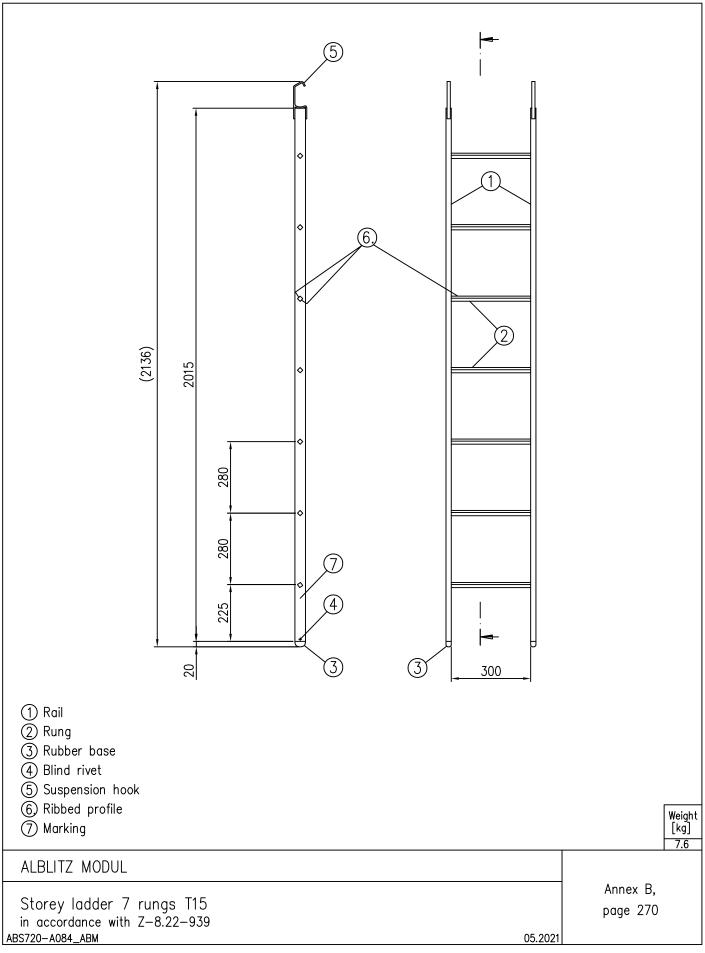
Annex B,

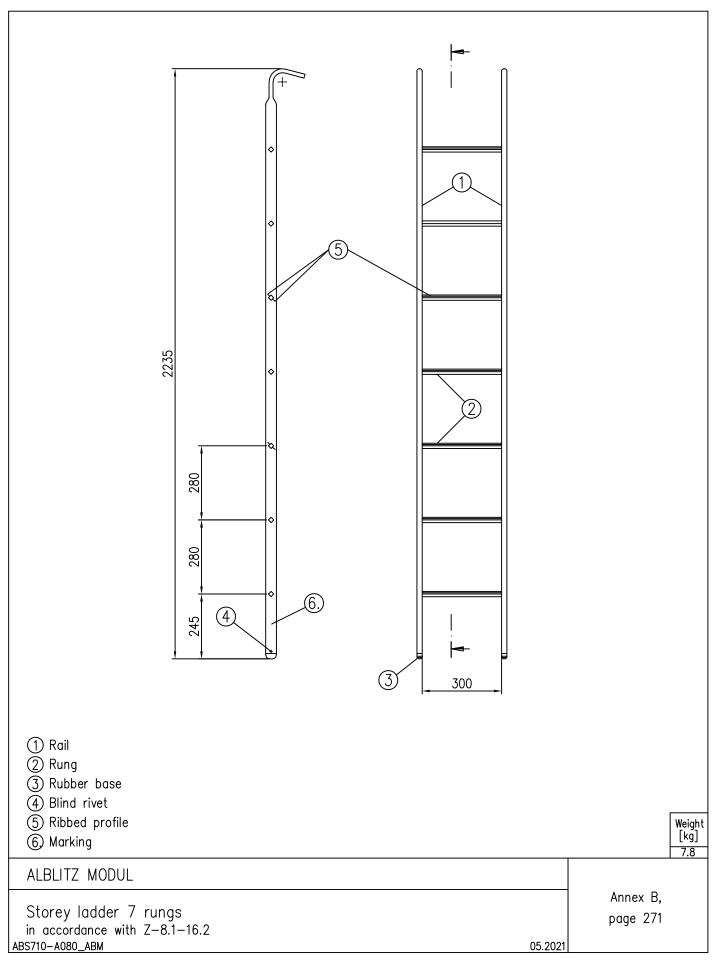
page 268

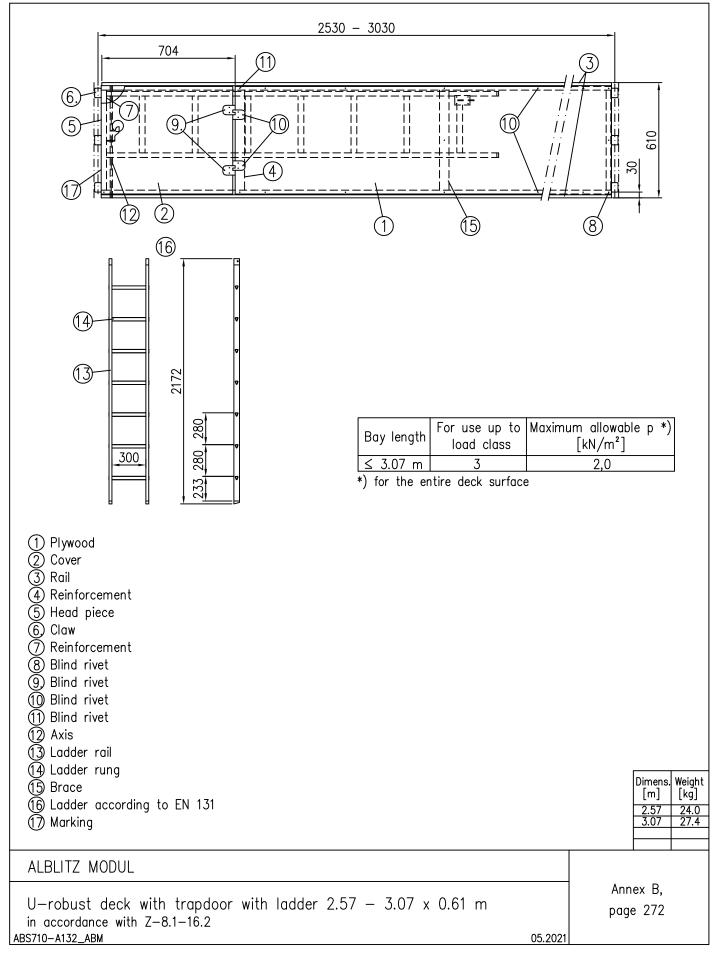
ALBLITZ MODUL

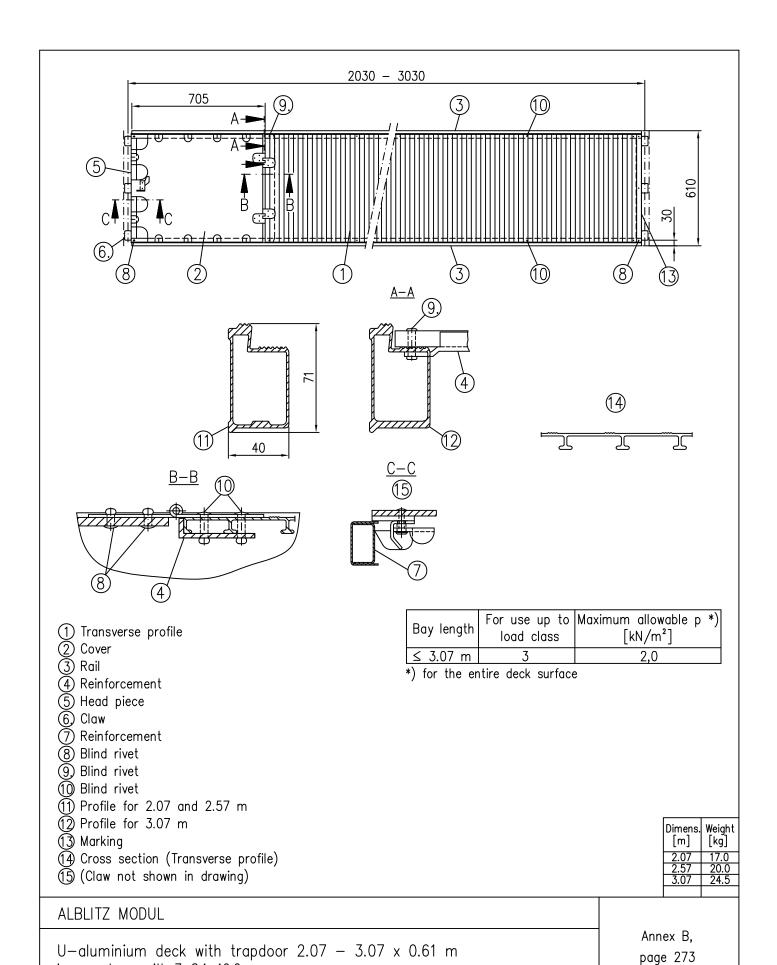
U-robust deck $0.73 - 3.07 \text{ m} \times 0.32 \text{ m}$ in accordance with Z-8.1-16.2ABS710-A130_ABM





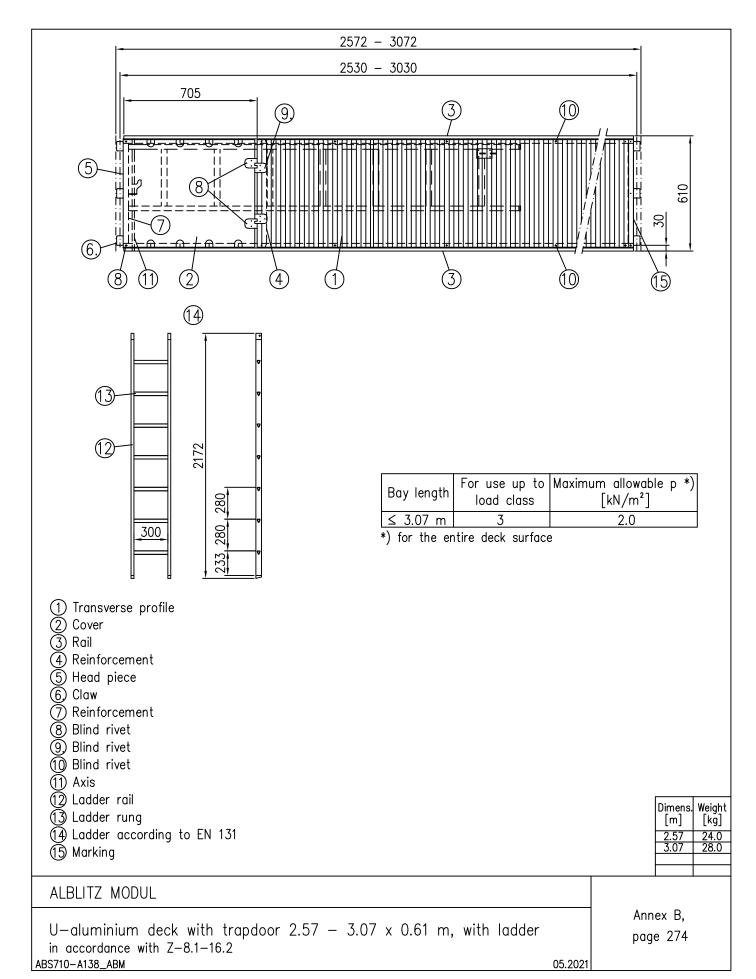


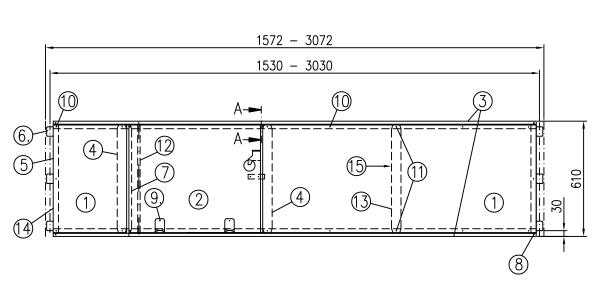


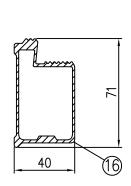


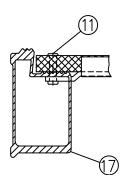
ABS710-A137_ABM

in accordance with Z-8.1-16.2









A-A

| 1 | Plywood |
|---|---------|
| | |

- (2) Cover
- (3) Rail
- Reinforcement
- (5) Head piece
- (6) Claw
- 7 Steel-U
- (8) Blind rivet
- (9) Blind rivet
- 10 Blind rivet
- (11) Blind rivet
- (12) Axis
- (13) Brace
- (14) Marking
- (15) only for 2.57 and 3.07 m
- (16) Profile for 1.57 to 2.57 m
- (17) Profile for 3.07 m

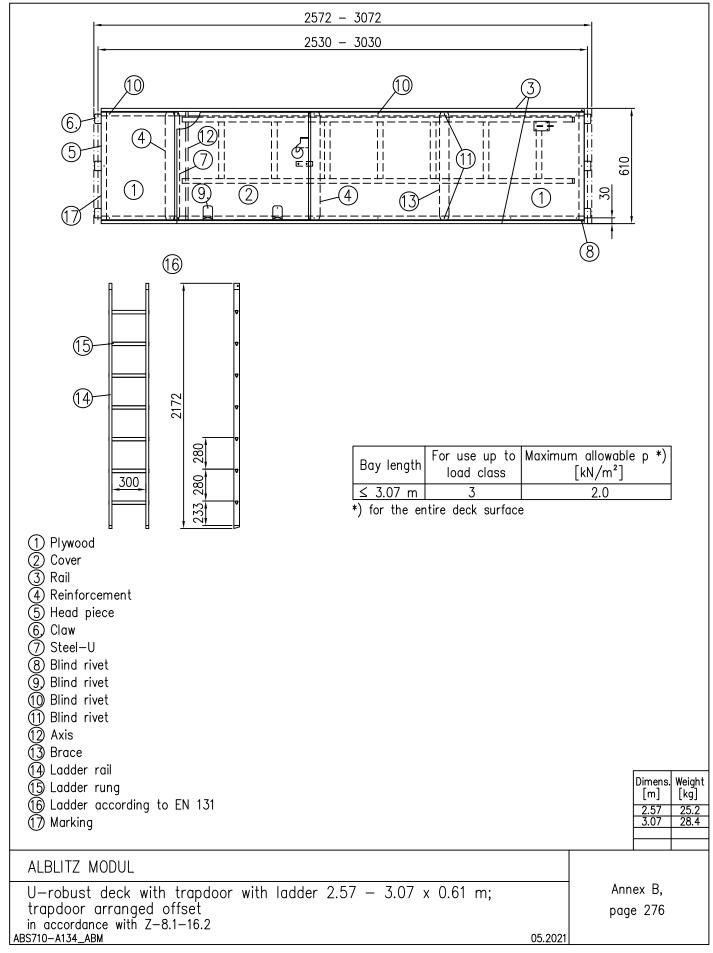
| Bay length | For use up to load class | Maximum allowable p *) [kN/m²] |
|------------|--------------------------|--------------------------------|
| ≤ 3.07 m | 3 | 2.0 |

*) for the entire deck surface

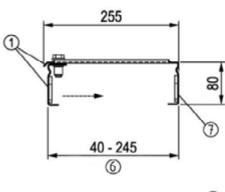
| Dimens. | |
|---------|------|
| [m] | [kg] |
| 1.57 | 14.2 |
| 2.07 | 17.2 |
| 2.57 | 20.5 |
| 3.07 | 24.6 |

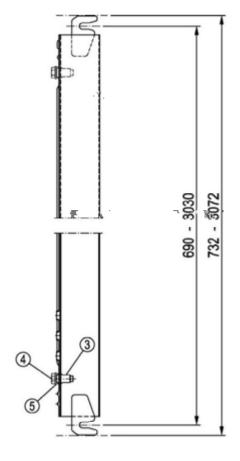
ALBLITZ MODUL

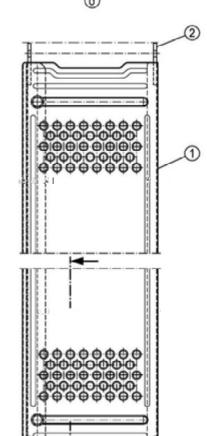
U-robust deck with trapdoor 1.57 - 3.07 x 0.61 m, trapdoor arranged offset in accordance with Z-8.1-16.2ABS710-A133_ABM



| Bay length | For use up to load class | Maximum allowable p *) [kN/m²] |
|--------------------------------|--------------------------|--------------------------------|
| ≤ 2.07 m | 6 | 10.0 |
| 2.57 m | 5 | 7.5 |
| 3.07 m | 4 | 5.0 |
| *) for the entire deck surface | | |







- 1 Deck sheet
- 2 Suspension hook
- 3 Pop rivet nut
- 4 Hexagon bolt
- (5) Disc
- 6 Adjusting range
- Marking

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 0.73 | 5,2 |
| 1.09 | 7.8 |
| 1.57 | 11.4 |
| 2.07 | 14.9 |
| 2.57 | 18.6 |
| 3.07 | 22.3 |
| | |

ALBLITZ MODUL

U-gap deck, telescopic 0.73 - 3.07 m in accordance with Z-8.22-939
ABM721-B143

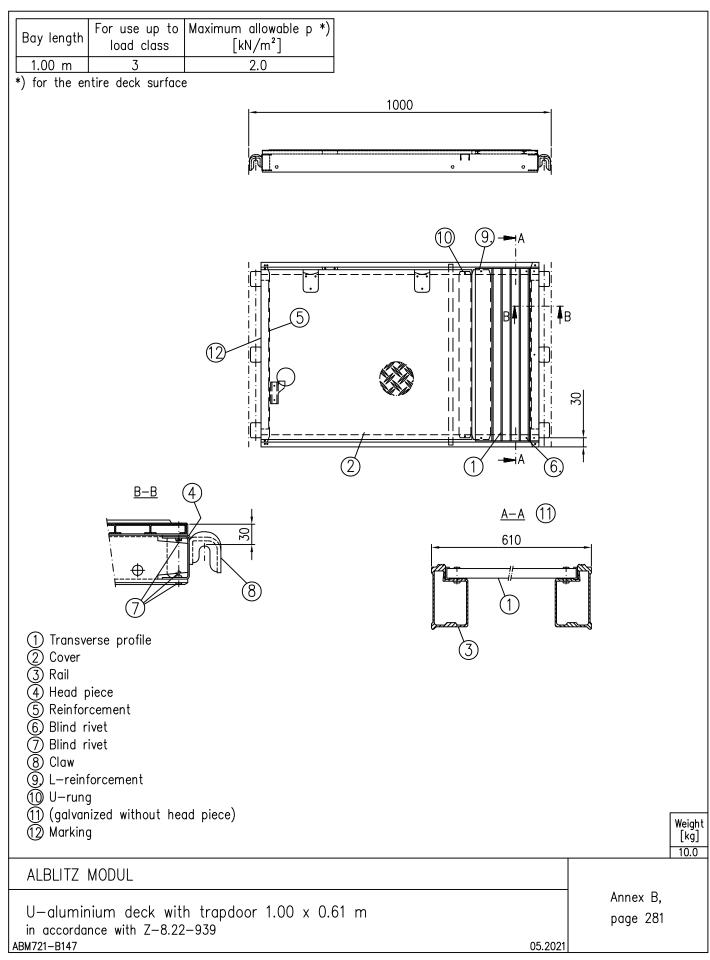
| Bay length | For use up to load class | Maximum allowable p * [kN/m²] | | |
|-------------------------------|---|--|---|---|
| ≤ 3.07 m | 6 | 10.0 | max. span 24 cm in transverse direction (clear gap width appr. 22 cm) | |
| *) for the er | ntire deck surfa | ce | | |
| | | | ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ | |
| | 3 | | 320 | 10 |
| | 4 | 5 | | |
| | | | | |
| | 6 | | | 610 – 2950 |
| | 7 | 2 | | |
| 4 Locking 5 Locking 6 Locking | 3 s to secure the g screw long WS | 19 / 22 ISO 898-1 S 19 / 22 ISO 898-1 | | Dimens. Weigl [kg] 0.73 2.6 1.09 3.8 1.57 4.2 2.07 6.3 2.57 8.5 3.07 12.0 |
| ALBLITZ N | MODUL | | | |
| | nce with Z-8.22 | - 3.07 x 0.32 m -939 | 05.2021 | Annex B, page 278 |

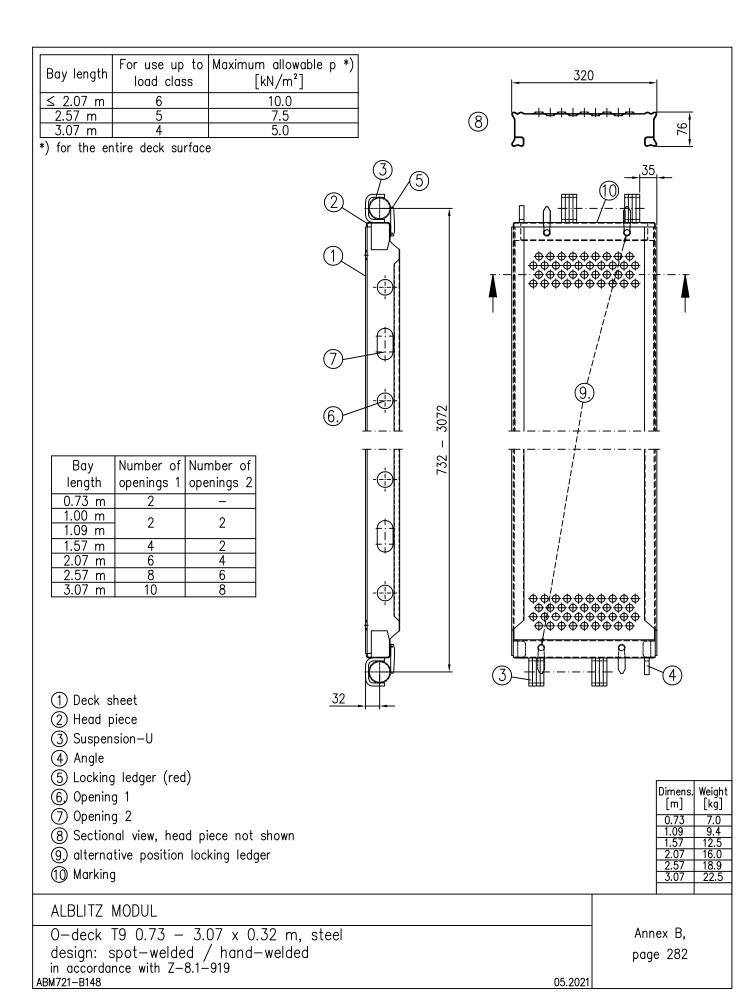
| | | 70 |
|--|--|--|
| 33 200 - 3030 800 - 3030 | 192 192 192 193 194 194 194 195 196 196 196 196 196 196 196 196 | |
| 1 Deck sheet 2 Head piece 3 Claw 4 Marking 5 Top view 6 Bottom view 7 Cross section (drawing does not show suspension) | | Dimens. Weight [m] [kg] 0.73 5.1 1.09 6.4 1.57 8.5 2.07 10.2 2.57 13.2 3.07 15.3 |
| ALBLITZ MODUL U-deck, steel 0.73 - 3.07 x 0.19 m in accordance with Z-8.1-16.2 | 05 2021 | Annex B, page 279 |

ABS710-A117_ABM

| | Maximum allowable p *) [kN/m²] 10.0 7.5 5.0 ce | (5) (1) R | |
|---|---|--|---|
| | 933 | 192 192 194 194 194 194 194 194 194 195 196 196 196 196 196 196 196 196 | |
| Deck sheet Head piece Claw Marking Cross section (drawin) | g does not show suspension) | No longer man only for contin | Dimens. Weight [m] [kg] 0.73 4.5 1.09 6.0 1.57 8.5 2.07 10.2 2.57 13.2 ufactured, 2.57 13.2 ued use |
| ALBLITZ MODUL | | · | |
| U-deck, steel 0.73 - in accordance with Z-8.1 | - 3.07 x 0.19 m (discontinu -16.2 | ed design) | Annex B, page 280 |

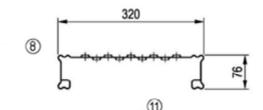
ABS710-A118_ABM

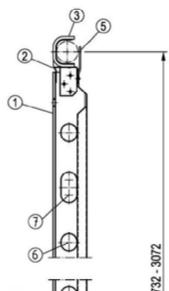


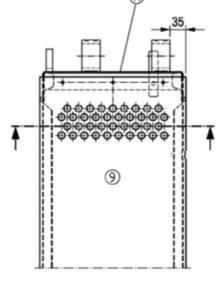


| Bay length | For use up to load class | Maximum allowable p *) [kN/m²] |
|------------|--------------------------|--------------------------------|
| ≤ 2.07 m | 6 | 10.0 |
| 2.57 m | 5 | 7.5 |
| 3.07 m | 4 | 5.0 |
| 4 | | |

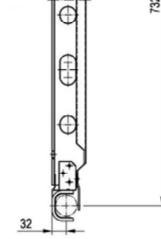
*) for the entire deck surface

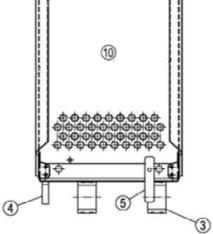






| of |
|-----|
| s 2 |
| |
| |
| |
| |
| |
| |
| |
| |
| |





• = Weld points

- (1) Deck sheet
- (2) Head piece
- (3) Suspension-U
- 4 Angle

Bay

length

0.73 m 1.09 m

1.57 m

2.07 m <u>2.57 m</u>

3.07 m

- (5) Locking plate
- 6 Opening 1
- (7) Opening 2
- (8) Sectional view, head piece not shown
- (9) Top view
- 10 Bottom view
- (11) Marking

No longer manufactured, only for continued use

| 0.73 | 7.0 |
|------|------|
| 1.09 | 9.4 |
| 1.57 | 12.5 |
| 2.07 | 16.0 |
| 2.57 | 18.9 |
| 3.07 | 22.5 |
| | |

Weight

[kg]

Dimens.

[m]

ALBLITZ MODUL

0-deck T4 0.73 - 3.07 x 0.32 m, steel

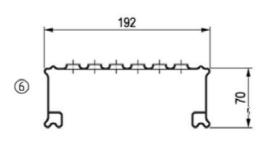
design: spot-welded

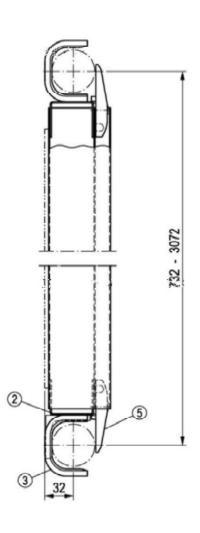
in accordance with Z-8.1-919 ABM721-B149

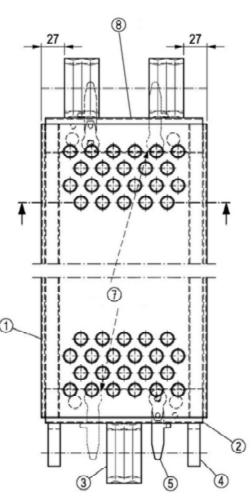
(discontinued design)

| Bay length | For use up to load class | Maximum allowable p *) [kN/m²] |
|------------|--------------------------|--------------------------------|
| ≤ 2.07 m | 6 | 10.0 |
| 2.57 m | 5 | 7.5 |
| 3.07 m | 4 | 5.0 |
| | | |

^{*)} for the entire deck surface







- 1 Deck sheet
- (2) Head piece
- 3 Suspension-U
- (4) Angle
- (5) Locking ledger (red) (positioned diagonally)
- 6) Sectional view, head piece not shown
- (7) alternative position locking ledger
- 8 Marking

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 0.73 | 5.0 |
| 1.09 | 7.0 |
| 1.57 | 10.0 |
| 2.07 | 12.7 |
| 2.57 | 13.0 |
| 3.07 | 18.2 |
| | |

ALBLITZ MODUL

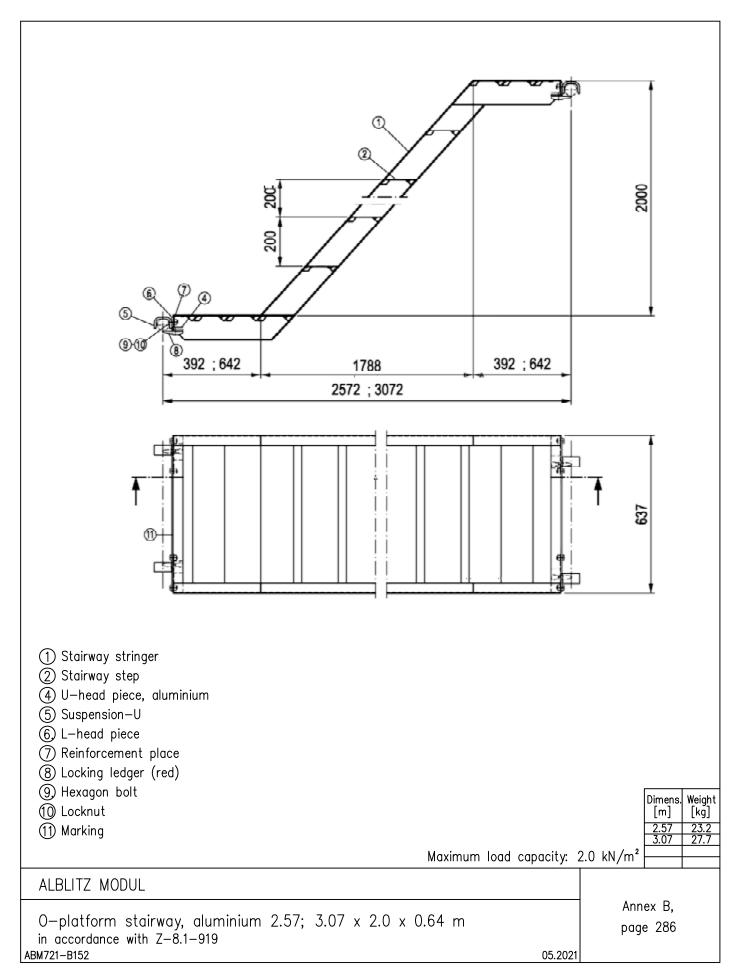
 $O-deck\ T9\ 0.73\ -\ 3.07\ x\ 0.19\ m$, steel

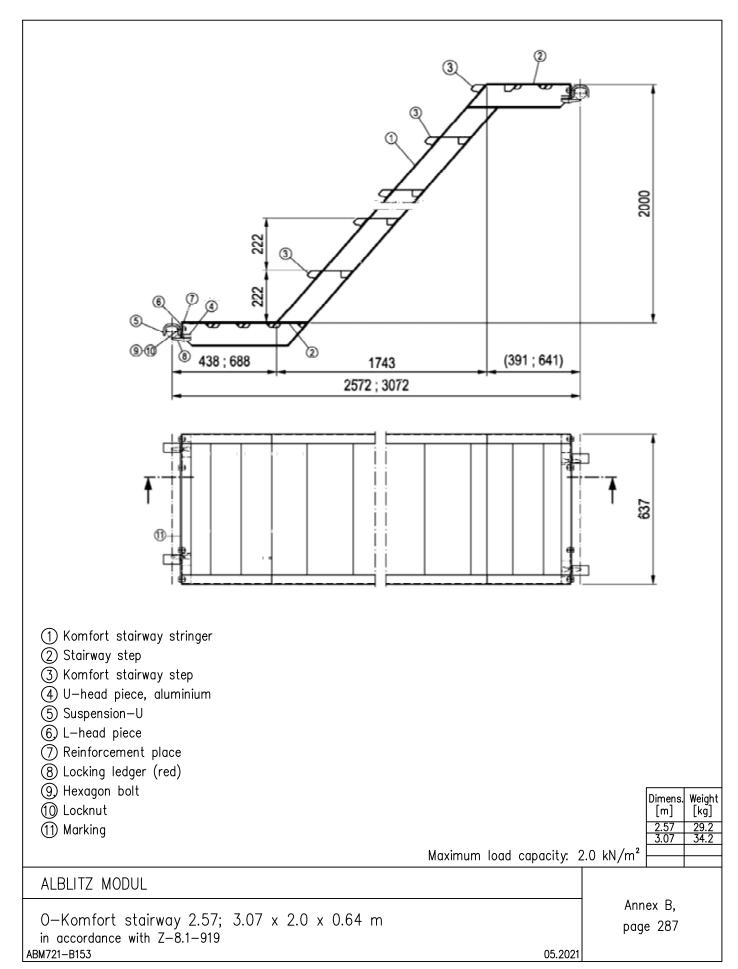
in accordance with Z-8.1-919 ABM721-B150

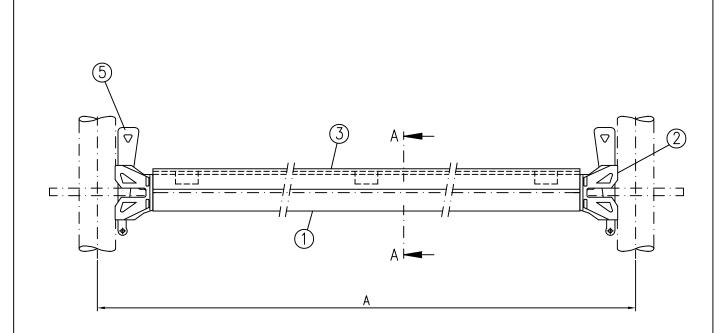
page 284

Annex B,

| $ \begin{array}{ c c c c c c }\hline Bay \ length & For \ use \ up \ to & Maximum \ allowable \ p \ *)\\ \hline & \leq 2.07 \ m & 6 & 10.0\\ \hline & 2.57 \ m & 5 & 7.5\\ \hline & 3.07 \ m & 4 & 5.0\\ \hline *) \ for \ the \ entire \ deck \ surface \\ \hline \end{array} $ | (a) 192 | |
|--|---|--|
| 2.08 - 28L | | |
| ① Deck sheet ② Head piece ③ Suspension—U ④ Angle ⑤ Locking plate ⑥ Sectional view, head piece not shown ⑦ Marking | Dimens. Weight [kg] 0.73 5.0 1.09 7.0 1.57 10.0 2.07 12.7 2.57 13.0 3.07 18.2 | |
| ALBLITZ MODUL O-deck 0.73 - 3.07 x 0.19 m, steel (discontinued design) in accordance with Z-8.1-919 ABM721-B151 O5.2021 | | |







| | For use up to load class | Maximum allowable p *) |
|------|--------------------------|------------------------|
| [mm] | lodd class | [kN/m²] |
| 732 | | |
| 1088 | | |
| 1286 | | |
| 1400 | 6 | 10.0 |
| 1572 | 0 | 10.0 |
| 2072 | | |
| 2572 | | |
| 3072 | | |

*) acting on the entire sheet width

| | <u>A-A</u> | |
|---|------------|---|
| - | 110 | 1 |
| 3 | | |

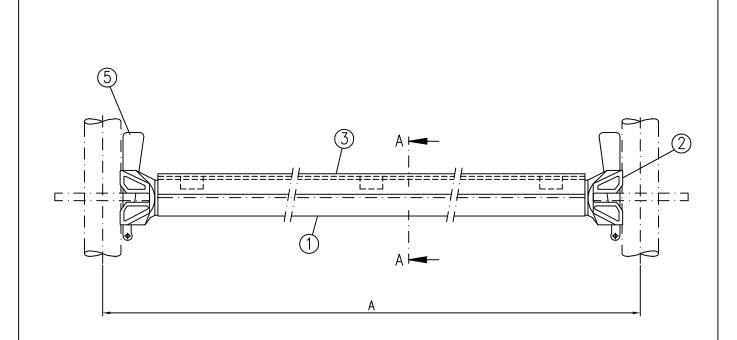
- (1) Tube
- 2 Head piece + Wedge "Lightweight design" (see Annex B, pages 166 + 170)
- 3 Bulb plate
- 4 Spacer bracket
- (5) Marking

| Dimens. [m] | Weight [kg] |
|----------------|----------------|
| 0.73 | 5.2 |
| 1.09 | 7.5 |
| 1.29 | 9.0 |
| 1.40 | 9.4 |
| 1.57 | 10.8 |
| 2.07 | 14.1 |
| 2.57 | 18.1 |

ALBLITZ MODUL

O-ledger with gap cover, lightweight 0.73 - 3.07 m in accordance with Z-8.1-919ABM721-B154

Annex B, page 288



| Dimension A | For use up to | Maximum allowable p *) |
|-------------|---------------|------------------------|
| [mm] | load class | [kN/m²] |
| 732 | | |
| 1088 | | |
| 1286 | و ا | 10.0 |
| 1400 | 0 | 10.0 |
| 1572 | | |
| 2072 | | |
| 2572 | 5 | 7.5 |
| 3072 | 4 5.0 | |

110

A-A

- *) acting on the entire sheet width
- (1) Tube
- 2 Head piece + Wedge "K2000+ design" (see Annex B, pages 172 + 176)

- 3 Bulb plate
- 4 Spacer bracket
- (5) Marking

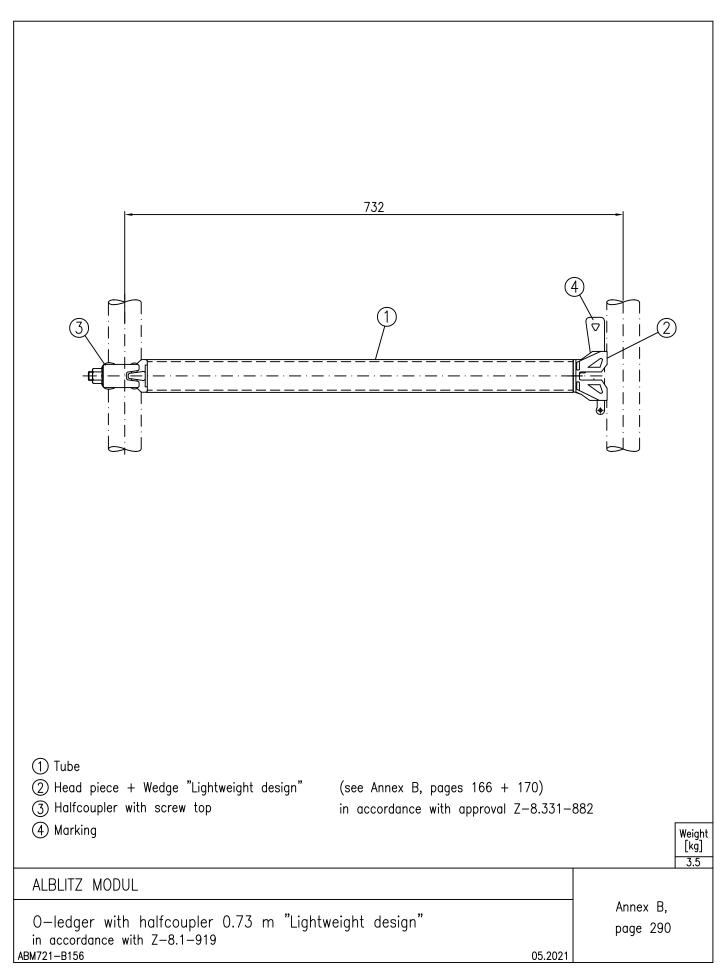
| Dimens. [m] | [kg] |
|----------------|------|
| 0.73 | 5.3 |
| 1.09 | 8.0 |
| 1.29 | 9.6 |
| 1.40 | 10.0 |
| 1.57 | 11.7 |
| 2.07 | 15.0 |
| 2.57 | 19.2 |
| | |

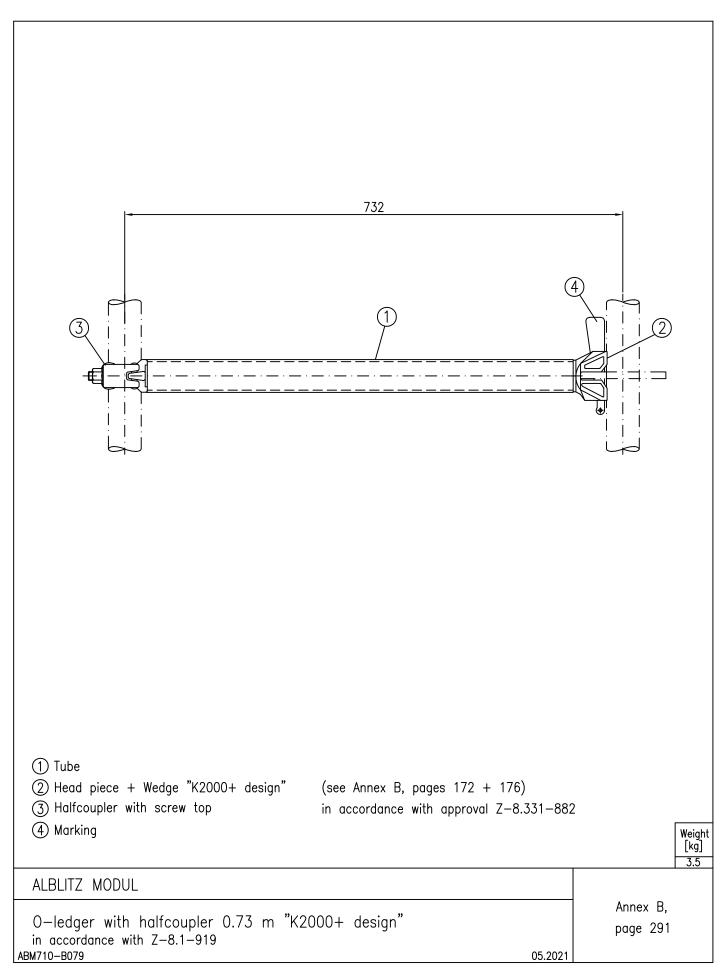
ALBLITZ MODUL

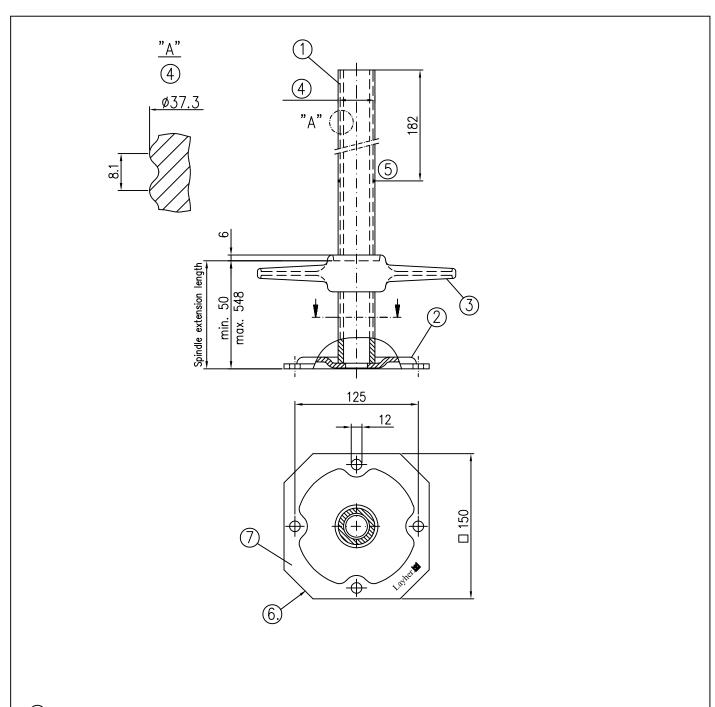
O-ledger with gap cover 0.73 - 3.07 m "K2000+ design" in accordance with Z-8.1-919 ABM721-B155

Annex B, page 289

Z101025.23







- 1 Tube
- ② Base plate
- 3 Spindle nut
- 4) Special thread Ø38 x 8.1

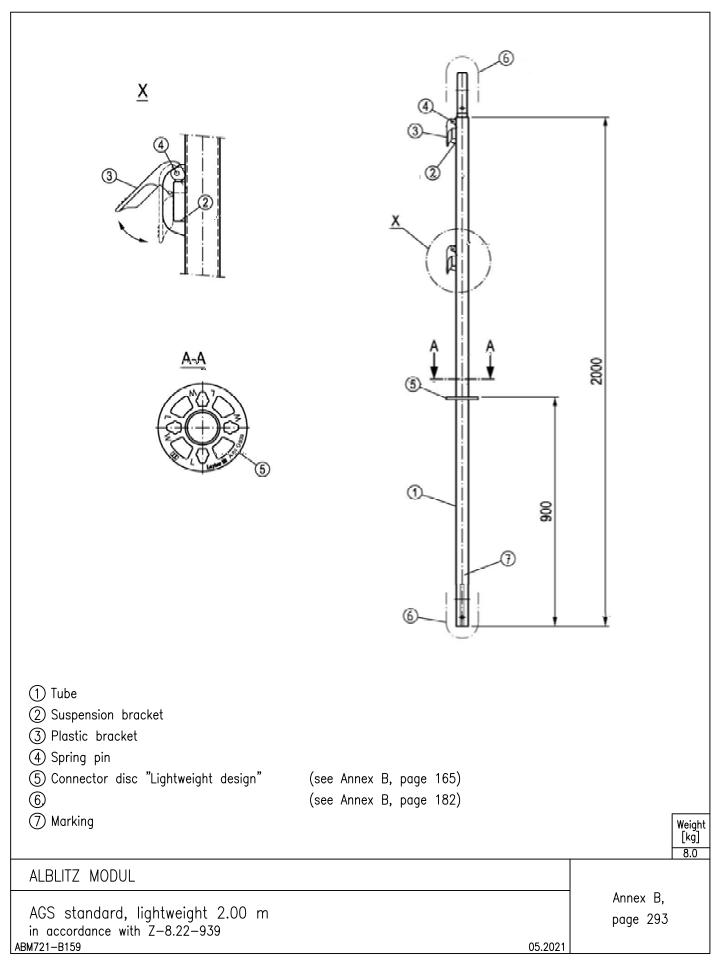
see detail

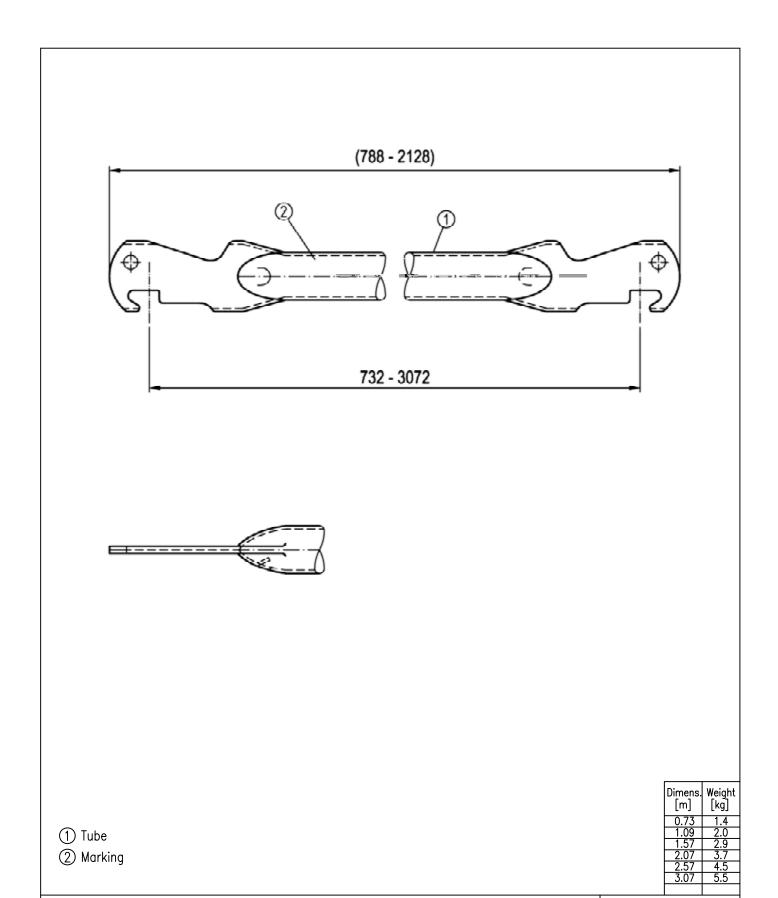
- (5) Thread with notches to limit collar nut travel
- 6) Base plate according to EN 74-3
- Marking

Weight [kg] 4.9

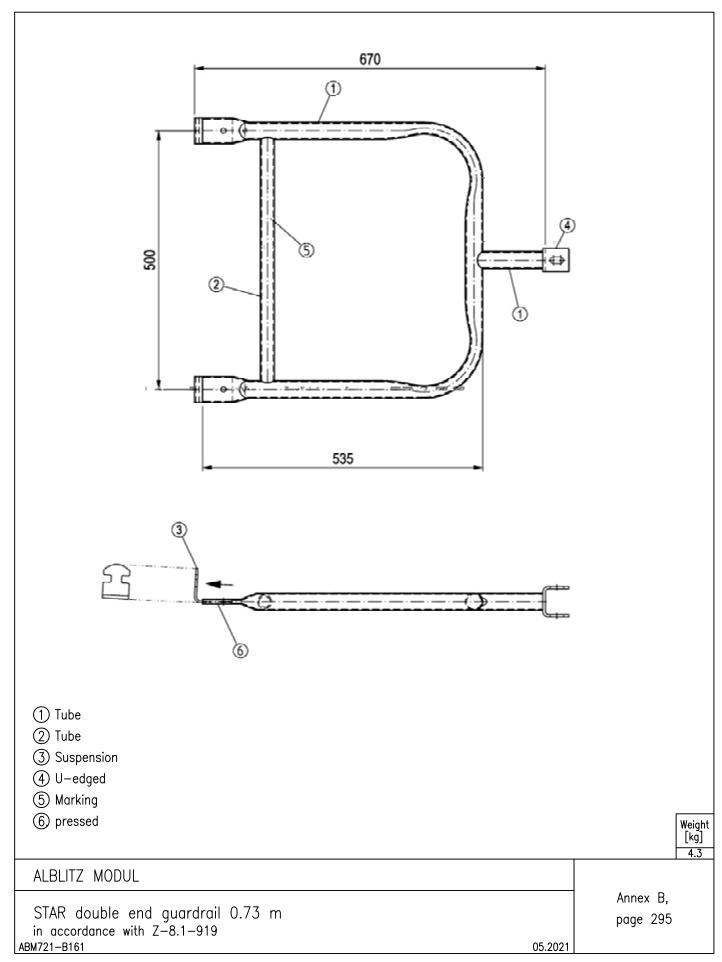
Base jack 80, reinforced in accordance with Z-8.1-16.2 ABS710-A003_ABM

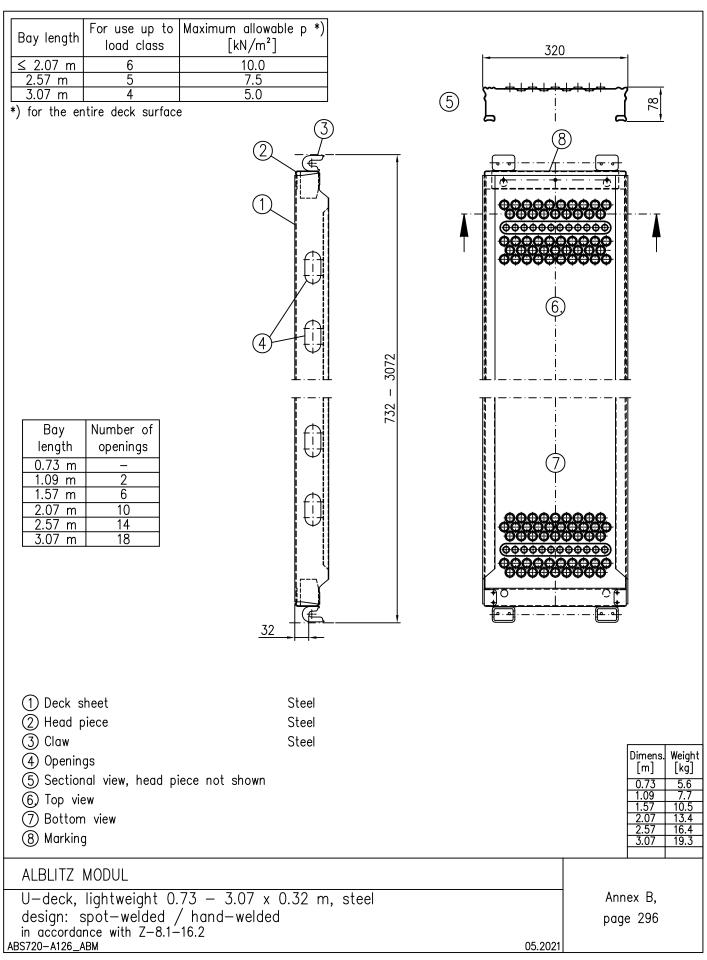
Annex B, page 292

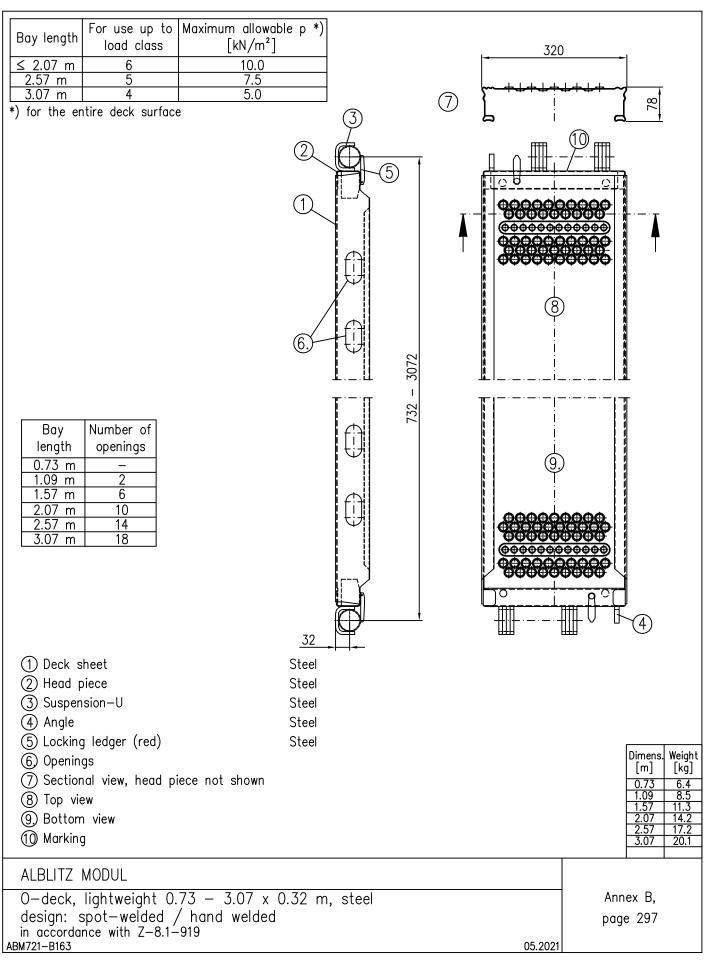


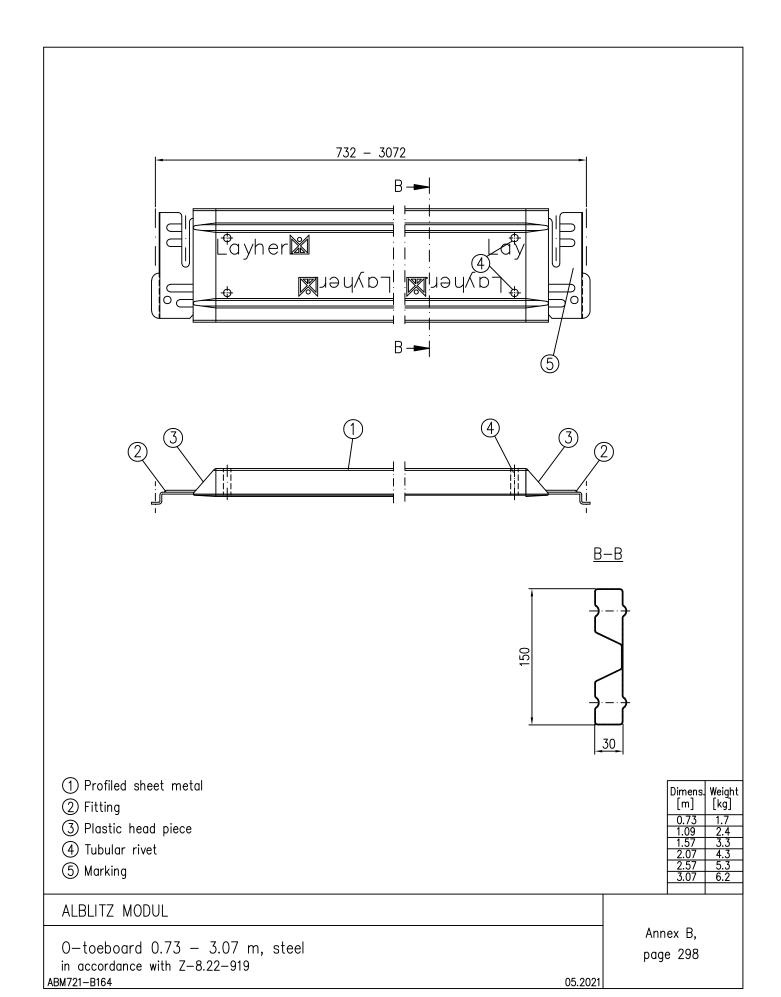


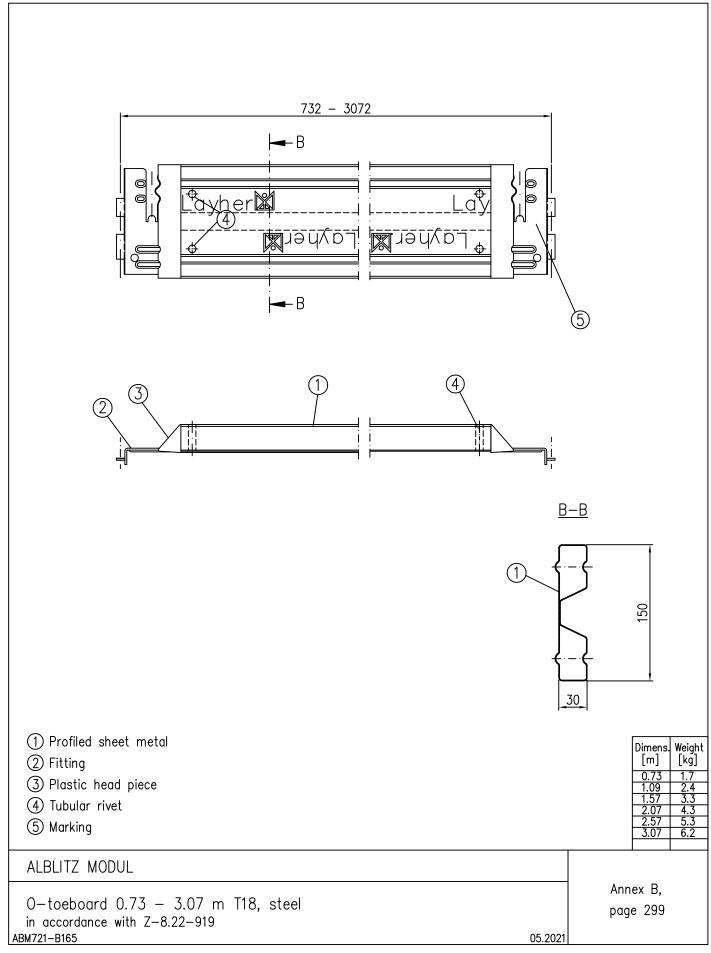
| ALBLITZ MODUL | |
|---|----------------------|
| STAR guardrail 0.73 — 3.07 m T18 in accordance with Z-8.1-919 | Annex B, page 294 |
| ABM721-B160 05 | 5.2021 |

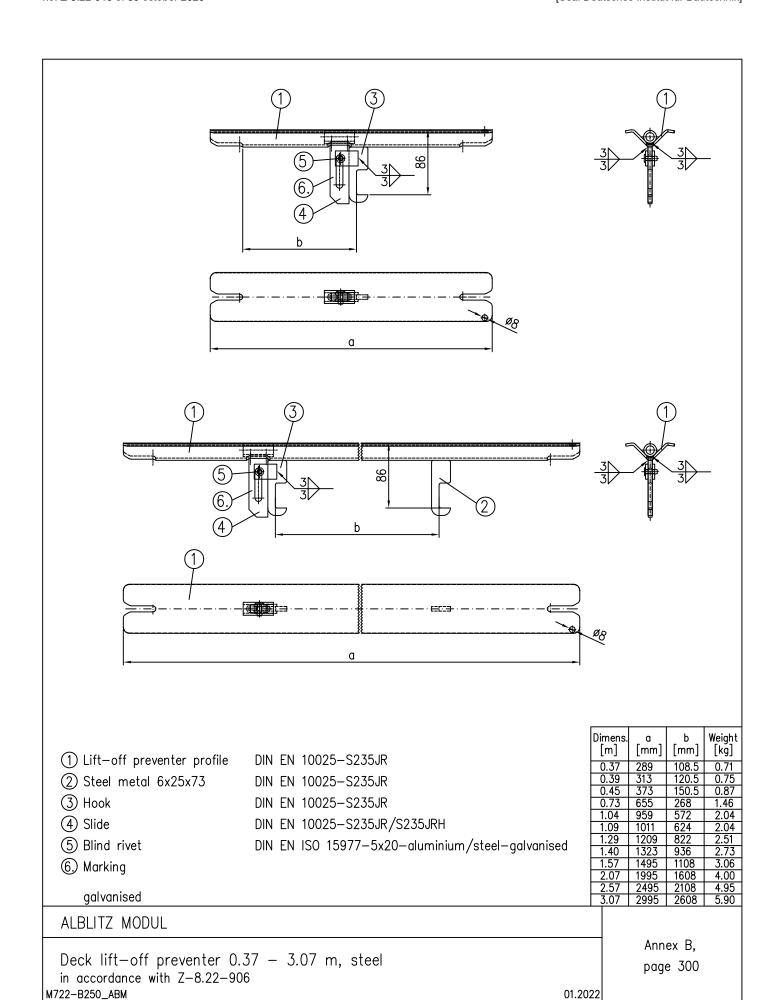


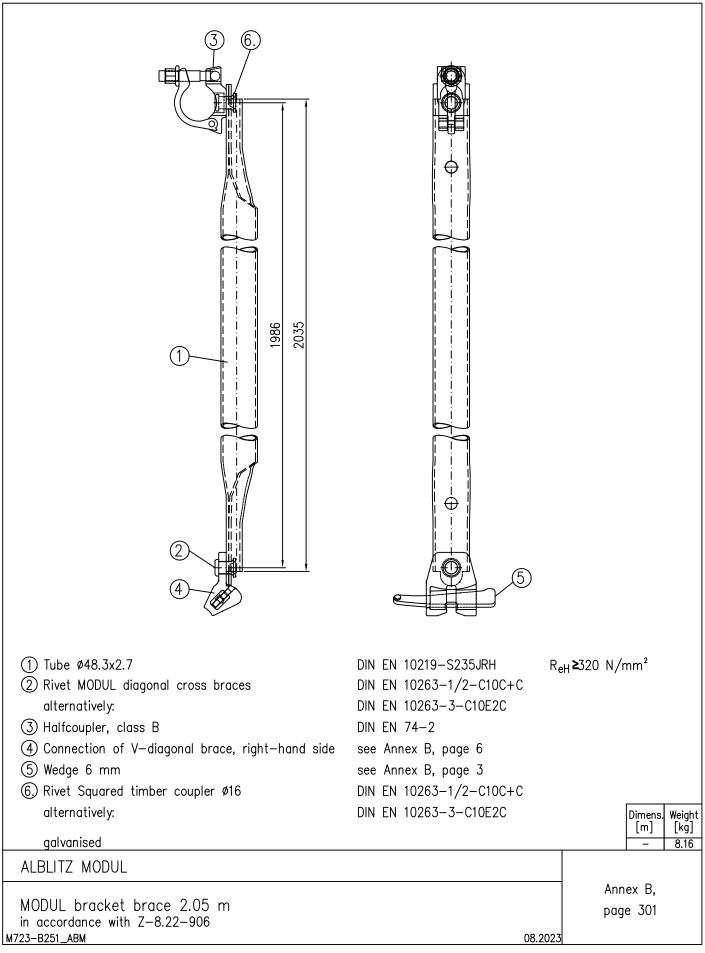


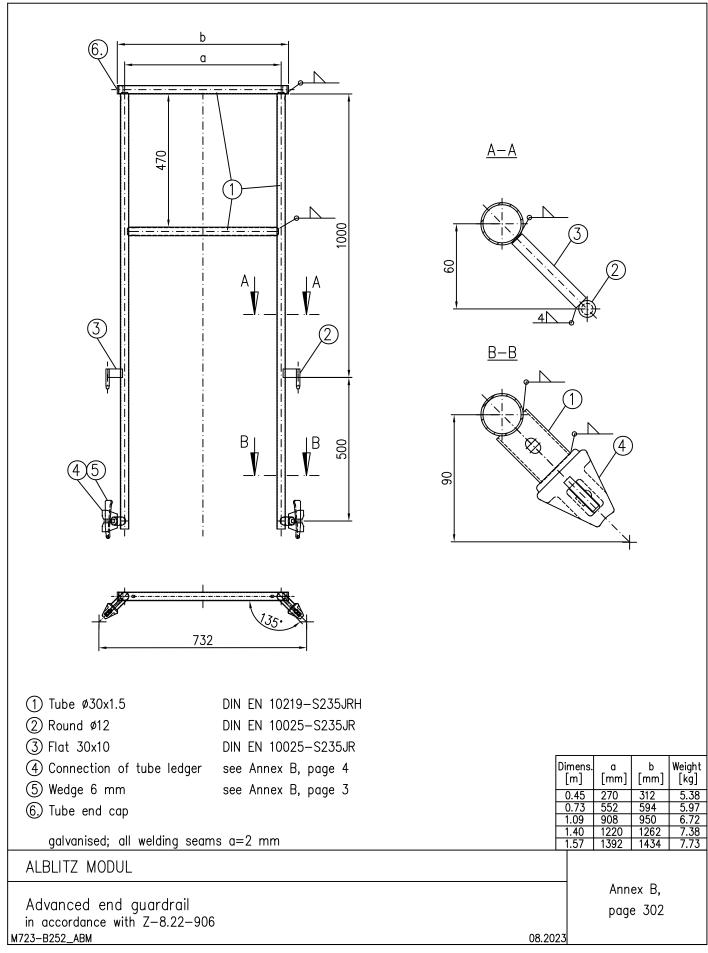


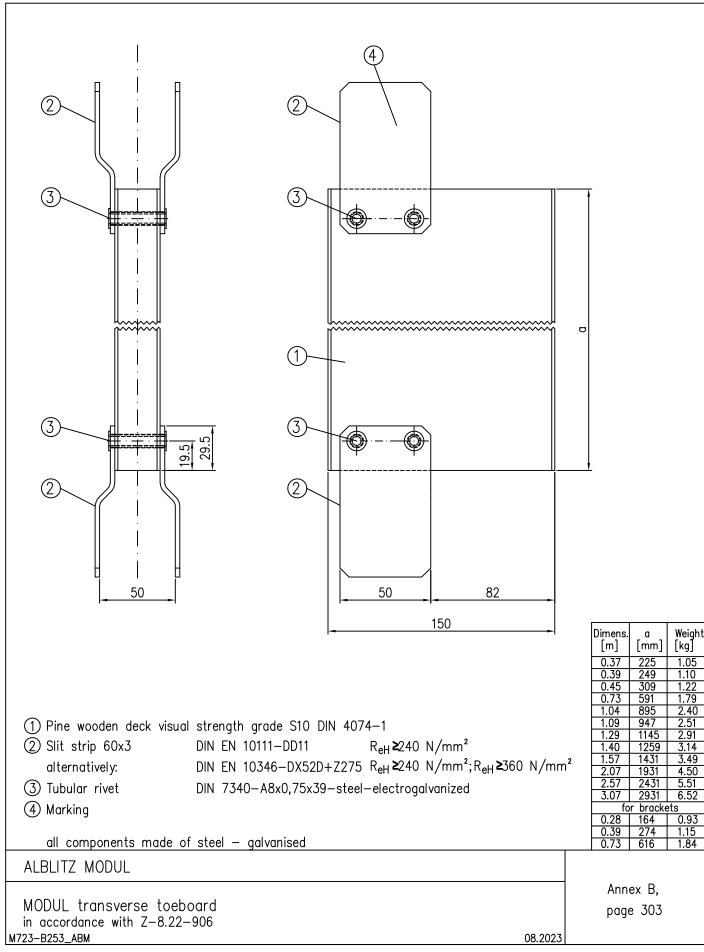


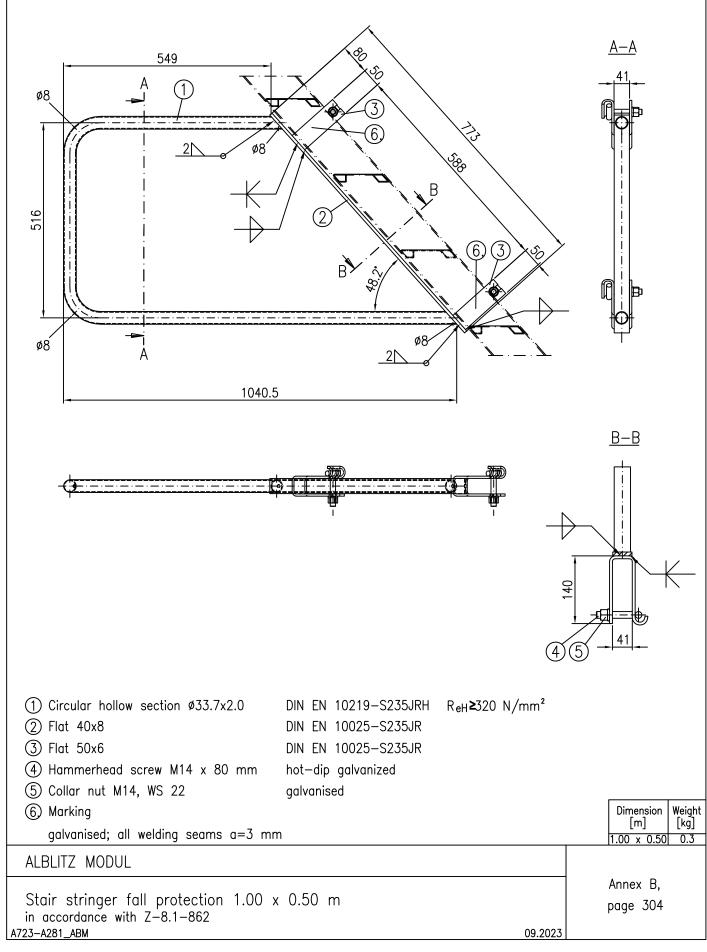


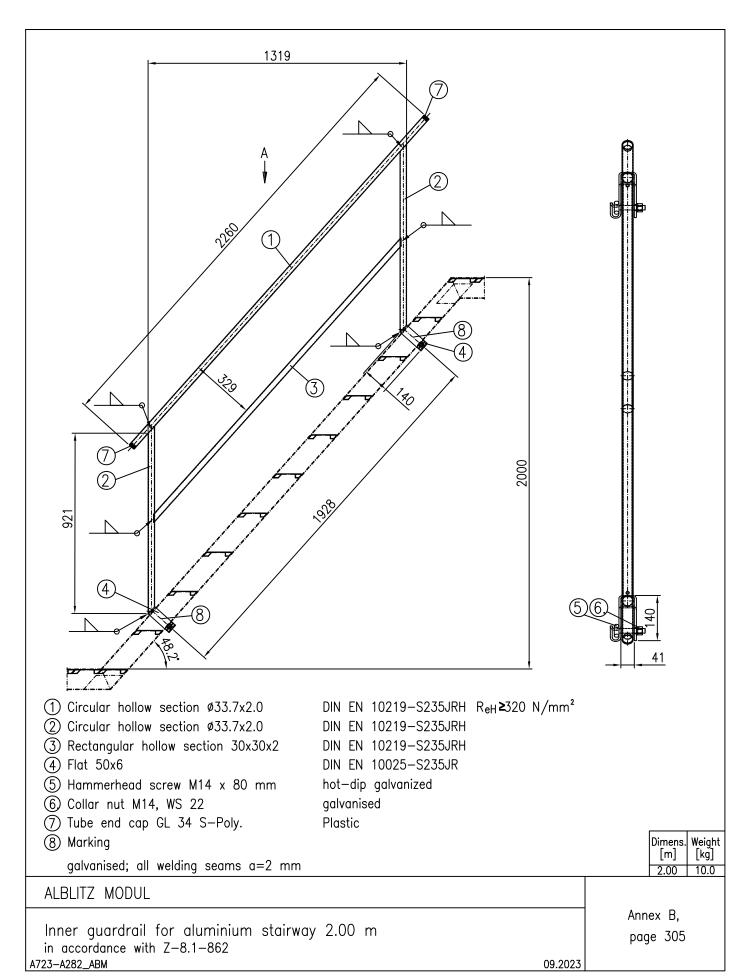


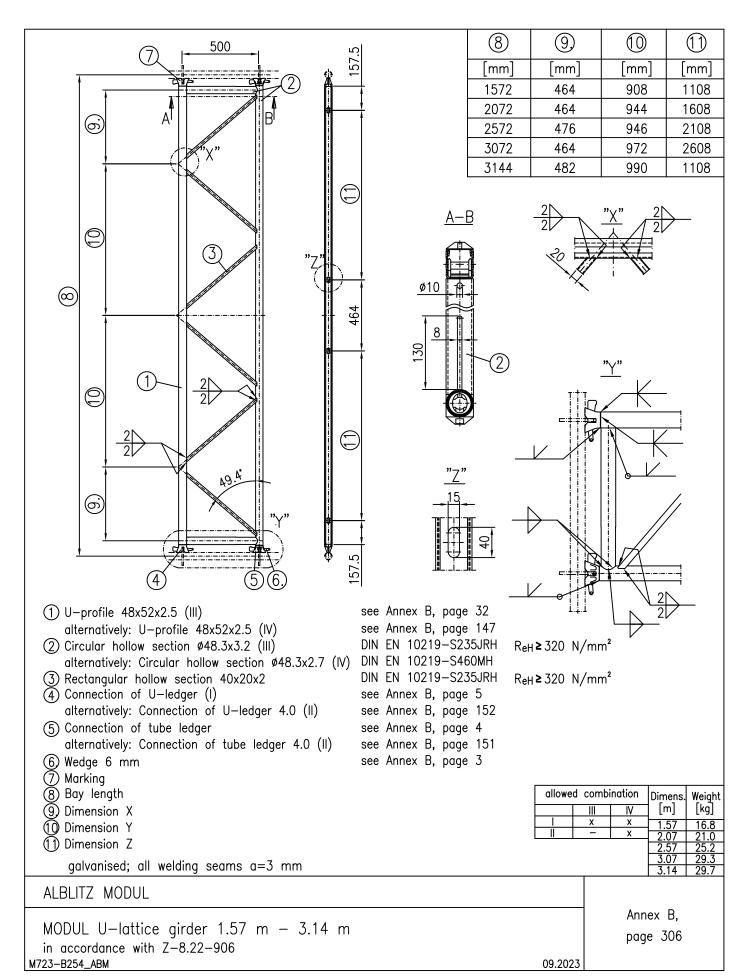












C.1 General provisions

In its standard assembly configuration, the scaffolding system may be used as service scaffold of load classes ≤ 3 with system width b = 0.732 m and bay widths $\ell \leq 3.07$ m in accordance with DIN EN 12811-1:2004-03, as a as roof fall arrest and brick guard scaffold in accordance with DIN 4420-1:2004-03.

The topmost horizontal level (working level) must not exceed 24 m above ground level, not including the spindle extension length. The standard assembly configuration of the scaffolding system is designed for use on a scaffold level in accordance with the regulations of the DIN EN 12811-1:2004-03 standard, Section 6.2.9.2 in front of a "partially open" facade with an open proportion of no more than 60%, and in front of closed facades. When determining the wind load, a service life factor of $\chi=0.7$, assuming a maximum service life of 2 years, has been taken into account. Scaffold cladding with nets or tarpaulins has not been verified in the standard system configuration.

Without any further structural proof, the standard assembly configuration shall only be used if the loads of the bays will carry do not exceed the respective live loads in accordance with DIN EN 12811-1:2004-03. Table 3.

For the standard system configuration of the modular scaffolding system "ALBLITZ MODUL", the following designation in accordance with DIN EN 12810-1:2004-03 must be used:

Scaffolding EN 12810 - 3D - SW06/307 - H2 - A - LA

C.2 Roof fall arrest and brick guard scaffold

In its standard assembly configuration, the scaffolding system may be used as a roof fall arrest and brick guard (service) scaffold with a top fall arresting layer of class FL 1 and as a brick guard (service scaffold) with protective walls of class SWD 1 according to DIN 4420:2004-03. Access decks must not be fitted into brackets.

The protective wall is to be installed in accordance with Annex D, page 7. Use protective nets in accordance with DIN EN 1263-1:2015-03 with a mesh size of no more than 100 mm.

C.3 Components

The components intended for use are listed in Table C.3. In addition, steel tubes \emptyset 48.3 • 3.2 mm and couplers may be used for the horizontal bracing of bridging girders as well as right-angle couplers in accordance with DIN EN 12811-1:2004-03 for the connection of scaffold ties and V-type scaffold ties to the standards.

C.4 Bracing

For the horizontal bracing of the scaffolding, the following components must be continuously installed at vertical intervals of 2 m:

• Tube ledger 0.73 m in the small opening of the connector disc (rosette) and in each case

| - one aluminium frame platform with tube fixture or | in accordance with Annex B, page 51 or 52 | or |
|---|---|----|
| - two steel decks with tube fixture or | in accordance with Annex B, page 64 | or |
| - two steel decks AF with tube fixture 0.32 m | in accordance with Annex B, page 61 | or |

U-ledger 0.73 m in the small opening of the connector disc and

| • 0-ledger 0.75 m in the small opening or | the connector disc and | |
|---|---|----|
| - one aluminium frame platform with plywo | od in accordance with Annex B, page 66 or 67 | or |
| - one aluminium deck with plywood | in accordance with Annex B, page 72, 73, 78 or 79 | or |
| - two steel decks | in accordance with Annex B, page 85 | or |
| - two steel decks AF 0.32 m | in accordance with Annex B, page 84 | or |
| - two U- steel decks T4 | in accordance with Annex B, page 262 or 263 | or |
| - two U- steel decks | in accordance with Annex B, page 264 or 265 | or |
| - one U-robust deck 0.61 m | in accordance with Annex B, page 266 or 267 | |

each. All other decks according to Table C.1 may only be used as bracket deck. When installing ladder access bays, trapdoors according to section C.8 must be used instead of platforms and decks.

| Annex C, page 1 |
|-----------------|
| |

Secure the platforms, decks and hatches against unintentional lift-off.

The outer vertical planes are to be braced by means of tube ledgers used as guardrails (1 m above deck level) continuously for each scaffolding bay.

Vertical starter pieces are to be installed directly above the base jacks (scaffolding spindles) and connected by means of longitudinal ledgers in the inner and outer plane parallel to the facade and by means of transoms at right angles to the facade. Next start fitting the outer planes using 3 m-posts and the inner planes using 2 m- posts.

C.5 Anchoring

Anchoring is to be provided using wall ties in accordance with Annex B, page 120.

Scaffold ties must be installed as anchor pairs at an angle of 90° (V-type scaffold tie) or as "short" scaffold ties only to the inner vertical frame standard using right-angle couplers. Depending on the assembly configuration according to Annex D, the node points that are anchored by means of V-type scaffold ties must be connected on the inner plane parallel to the facade with the adjacent standard section by means of O-ledgers (longitudinal ledgers). When using the protective wall, two (2) V-type scaffold ties must be installed per five (5) scaffold bays in the topmost plane.

The V-type scaffold ties and scaffold ties must be installed in the immediate vicinity of the node points of the tubular standards and transoms below the deck level, see Annex D, pages 5 and 6. V-type scaffold ties may not be fitted at the end sides of the scaffold.

In the configurations according to Annex D, pages 2 and 4, a longitudinal deck ledger must be fitted on the inner face on all working levels.

If a V-type scaffold tie must be arranged adjacent to an inner ladder access, another additional coupler tube (longitudinal ledger) must be fitted to the inner standards in this access bay.

The anchor forces listed in Table C.1 have been determined with the characteristic values of the actions. For the design analysis of the anchorage and the load transfer, the values given must be multiplied by the respective partial safety factor γ_F (generally $\gamma_F = 1.5$).

Each standard section must be anchored at vertical intervals of 8 m; anchoring points of neighbouring vertical frame sections must be arranged with a vertical offset of half the spacing. Standard sections at the edge of a scaffold and frame sections for ladder access bays must be anchored at vertical intervals of 4 m. In the second working level and when using the protective wall in the topmost working level, each standard section must be anchored.

Table C.1: Characteristic anchor forces of the standard system configuration

| Bridging design | Type and direction | without inner brackets | with inner brackets | |
|-------------------------------|-------------------------------|------------------------|---------------------|--|
| | V-type anchor at right angle | 2.4 kN | 3.0 kN | |
| without bridging | V-type anchor parallel | 2.4 KIN | | |
| | Scaffold tie (at right angle) | 3.6 kN | | |
| | V-type anchor at right angle | 2.4 kN | 3.0 kN | |
| with bridging | V-type anchor parallel | 2.4 KIN | 3.0 KIN | |
| Scaffold tie (at right angle) | | 3.6 | kN | |

| Modular scaffolding system "ALBLITZ MODUL" | |
|--|-----------------|
| Standard assembly configuration - General provisions | Annex C, page 2 |

C.6 Foundation loads

Depending on the design version, the foundation loads listed in Table C.2 must be absorbed and transferred in the erection plane (ground) in accordance with figure C.1. The foundation loads are given as characteristic values. For the structural analysis of transfer of loads in the supporting surface, the values given must be multiplied by the partial safety factor γ_F (generally $\gamma_F = 1.5$).

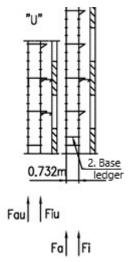


Figure C.1: Representation of the bearing loads

Table C.2: Characteristic bearing loads in the standard system configuration

| | Description | | Characte | eristic bea | aring load | ls in [kN] | |
|------------------|------------------------------|--------|------------|-------------|------------|------------|-------|
| Bridging design | Description | withou | t inner br | ackets | with | inner bra | ckets |
| | Scaffold height | 8 m | 12 m | 24 m | 8 m | 12 m | 24 m |
| without bridging | Outer face Fa | 6.5 | 9.2 | 12.0 | 6.5 | 9.2 | 12.0 |
| without bridging | Inner face F _i | 4.8 | 6.3 | 7.9 | 10.6 | 13.9 | 17.2 |
| | Outer face F _a | 6.5 | 9.2 | 12.0 | 6.5 | 9.2 | 12.0 |
| with bridging | Inner face F _i | 4.8 | 6.3 | 7.9 | 10.6 | 13.9 | 17.2 |
| with bridging | Adjacent to the bridging Fau | 9.8 | 13.8 | 18.0 | 9.8 | 13.8 | 18.0 |
| | Adjacent to the bridgingFau | 7.2 | 9.5 | 11.9 | 15.9 | 20.9 | 27.0 |

C.7 Bridging construction

The bridging girders may be used at a height of 4m to bridge gate entrances or similar openings when the working levels underneath the bridging part are omitted.

The bridging girders must be anchored in the support and centre areas and additionally stiffed by a horizontal bracing of tubes and couplers (cf. Annex D, pages 3, 4 and 8).

C.8 Ladder access

When installing internal ladder access bays, aluminium trapdoor frame platforms (with tube fixture) must be installed when using tube ledgers, or aluminium frame platforms with internal trapdoor, aluminium access frame platforms with ladder or U-robust trapdoor decks with ladders must be installed when using U-ledgers.

C.9 Widening bracket

On the inner face of the scaffolding, brackets with a width of 0.39 m may be installed in all working levels. Fit longitudinal ledgers or gap ledgers between the main and the bracket deck.

| Modular scaffolding system "ALBLITZ MODUL" | |
|--|-----------------|
| Standard assembly configuration - General provisions | Annex C, page 3 |
| | |

| Table C.3: | Components of the | standard assembly | configuration |
|------------|-------------------|-------------------|---------------|
| | | | |

| Designation | Annex B, page |
|---|---------------|
| Vertical starter piece | 10 |
| Standard with tube connector 200 | 11 |
| Vertical starter standard 2.16 m | 15 |
| Base jack | 17 |
| Tube ledger | 25 |
| U-ledger 0.73 m | 32 |
| U-transom lattice girder 0.73 m V | 44 |
| Tube-transom lattice girder 0.73 m V | 45 |
| MODUL lattice girder 6.14 m | 46 |
| MODUL lattice girder 5.14 m | 47 |
| MODUL lattice girder with tube fixture 6.14 m | 48 |
| MODUL lattice girder with tube fixture 5.14 m | 49 |
| MODUL lift-off preventer | 50 |
| Aluminium frame platform with tube fixture 1.57 m; 2.07 m | 51 |
| Aluminium frame platform with tube fixture 2.57 m; 3.07 m | 52 |
| Aluminium access frame platform with tube fixture 3.07 m | 54 |
| Aluminium access frame platform with tube fixture 2.57 m | 55 |
| Steel deck AF with tube fixture 0.32 m; ℓ ≤ 3.07 m | 61 |
| Steel deck with tube fixture | 64 |
| Aluminium frame platform with internal hatch 2.57 m; 3.07 m | 69 |
| Aluminium deck with plywood 2.57 m; 3.07 m | 72 |
| Aluminium deck with plywood 1.57 m; 2.07 m | 73 |
| Aluminium access deck 3.07 m with ladder | 75 |
| Aluminium access deck 2.57 m with ladder | 76 |
| Aluminium deck with plywood 3.07 m | 78 |
| Aluminium deck with plywood 1.57 m; 2.07 m; 2.57 m | 79 |
| Aluminium access deck 3.07 m with ladder | 81 |
| Aluminium access deck 2.57 m with ladder | 82 |
| Steel deck AF 0.32 m; ℓ ≤ 3.07 m | 84 |
| Steel deck | 85 |
| MODUL gap cover; ℓ ≤ 3.07 m | 94 |
| MODUL gap cover with tube fixture; <i>€</i> ≤ 3.07 m | 95 |
| Gap cover; ℓ ≤ 3.07 m | 96 |
| MODUL swinggate | 102 |
| Bracket 0.39 m with tube fixture | 103 |
| MODUL bracket 0.39 m | 104 |
| MODUL toeboard | 107 |
| MODUL toeboard, aluminium | 109 |
| Toeboard, End toeboard | 112 |
| Toeboard, aluminium; End-toeboard, aluminium | 115 |

| Modular scaffolding system "ALBLITZ MODUL" | |
|--|-----------------|
| Standard assembly configuration - General provisions | Annex C, page 4 |

| Table C.S. (Continued) | Table C.3: | (continued) |
|------------------------|------------|-------------|
|------------------------|------------|-------------|

| Designation | Annex B, page |
|---|---------------|
| MODUL guard net system | 116 |
| MODUL double end guardrail | 117 |
| Scaffold retainer / wall tie | 120 |
| Wedge head coupler, swivel base | 122 |
| MODUL U-tube connector | 123 |
| MODUL tube connector | 124 |
| Wedge head coupler, rigid | 125 |
| Locking pin | 132 |
| Standard 4.0 | 153 |
| Vertical starter standard 4.0 | 154 |
| Tube ledger 4.0 | 155 |
| MODUL gap cover, T-shaped | 157 |
| Base jack 60 | 178 |
| Starter piece, lightweight | 179 |
| Standard with integrated tube connector, lightweight | 180 |
| Starter standard 2.21 m, lightweight | 181 |
| O-ledger, lightweight 0.73 – 4.35 m | 185 |
| U-ledger, lightweight 0.73 m T14 | 187 |
| U-toeboard, wood 0.73 – 3.07 m | 192 |
| U-toeboard, steel 0.73 – 3.07 m T17 | 194 |
| U-toeboard, steel 0.73 – 3.07 m | 195 |
| U-bracket, lightweight 0.39 m | 196 |
| U-deck lift-off preventer T8 0.39 – 0.73 m | 202 |
| O-lattice girder, lightweight 5.14; 6.14 x 0.5 m | 205 |
| Tube connector for lattice girder | 209 |
| U-lattice girder ledger, lightweight 0.73 m | 210 |
| Reinforcing post 2.6 m "Lightweight design" | 215 |
| Locking pin, red Ø 11 mm | 223 |
| Starter piece "K2000+ design" | 226 |
| Standard with tube connector "K2000+ design" | 227 |
| O-ledger 0.73 – 4.35 m "K2000+ design" | 229 |
| U-ledger 0.73 m "K2000+ design" | 231 |
| U-bracket 0.39 m "K2000+ design" | 236 |
| O-lattice girder 5.14; 6.14 x 0.5 m "K2000+ design" | 242 |
| Side safety meshguard 1.57 – 3.07 m "K2000+ design" | 248 |
| U-deck T4 0.73 – 3.07 x 0.32 m, steel, design: point-welded | 262 |
| U-deck T4 0.73 – 3.07 x 0.32 m, steel, design: hand-welded | 263 |
| U-deck 0.73 – 3.07 x 0.32 m, steel, design: point-welded | 264 |
| U-deck 0.73 – 3.07 x 0.32 m, steel, design: hand-welded | 265 |
| U-robust deck 0.73 – 2.57 m x 0.61 m | 266 |

| l | Modular scaffolding system "ALBLITZ MODUL" | |
|---|--|-----------------|
| Ì | Standard assembly configuration - General provisions | Annex C, page 5 |

| Table C.3: (continued) |
|------------------------|
|------------------------|

| Designation | Annex B, page |
|--|---------------|
| U-robust deck 3.07 x 0.61 m | 267 |
| U-robust deck 0.73 – 3.07 m x 0.32 m *) | 268 |
| U-robust deck with trapdoor with ladder 2.57 – 3.07 x 0.61 m | 272 |
| Base jack 80, reinforced | 292 |
| *) may only be used as bracket deck | |

Modular scaffolding system "ALBLITZ MODUL"

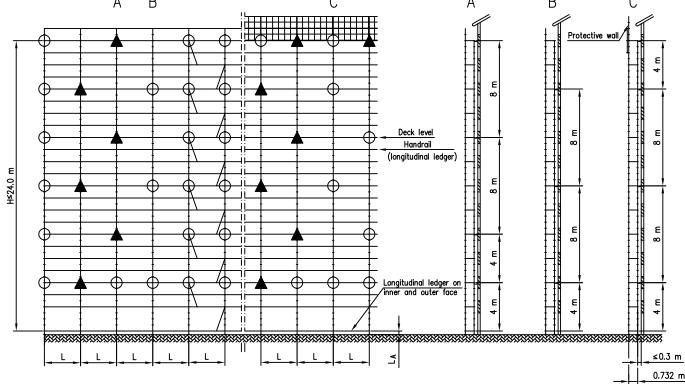
Standard assembly configuration - General provisions

Annex C, page 6

Standard assembly configuration without inner bracket — Bay length L ≤ 3.07 m

① uncladded scaffolding in front of partially open facade
② uncladded scaffolding in front of closed facade

A B C A B



Anchor configuration: — 8 m vertically offset anchor configuration

- at least 1 V-type wall tie per 5 bays

- continuous row of anchorage at H=4.00 m

- Protective wall position: continuous row of anchorage with

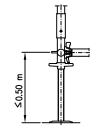
2 V-type wall ties per 5 bays

Spindle extension: $-L_A \le 50$ cm (Distance to foot level)

<u>Bracing:</u> — Handrail as longitudinal ledger on each working level

- Longitudinal ledger on inner and outer face on foot level at H=0.00 m

Supplementary components: - Protective wall (for details see annex D, page 7)



Scaffold retainer/wall tie

V-type wall tie

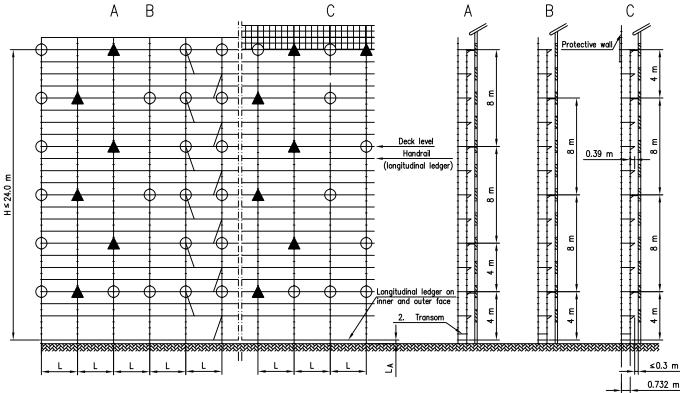
Note:

Side protection components (handrail, knee guardrail, longitudinal ledger) are only shown when they are required for the structural stability.

| ALBLITZ MODUL | |
|---|----------|
| | Annex D, |
| Standard assembly configuration without inner bracket | page 1 |
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Standard assembly configuration with inner bracket — Bay length $L \leq 3.07$ m

- ① uncladded scaffolding in front of partially open facade
- 2 uncladded scaffolding in front of closed facade



Anchor configuration:

- 8 m vertically offset anchor configuration
- at least 1 V-type wall tie per 5 bays
- continuous row of anchorage at H=4.00 m
- Protective wall position: continuous row of anchorage with 2 V-type wall ties per 5 bays

Spindle extension:

 $-L_A \le 50$ cm (Distance to foot level)

Bracing:

- Handrail as longitudinal ledger on each working level
- Longitudinal ledger on inner and outer face on foot level at H=0.00 m
- 2. Transom at a height of 0.50 m
- Longitudinal ledgers on the inner face in each bay and each working level at deck level
- Protective wall position: Knee rails as longitudinal ledger in the access bay at H=2.50 m and 4.50 m



Supplementary components: — Protective wall (for details see annex D, page 7)

Note:

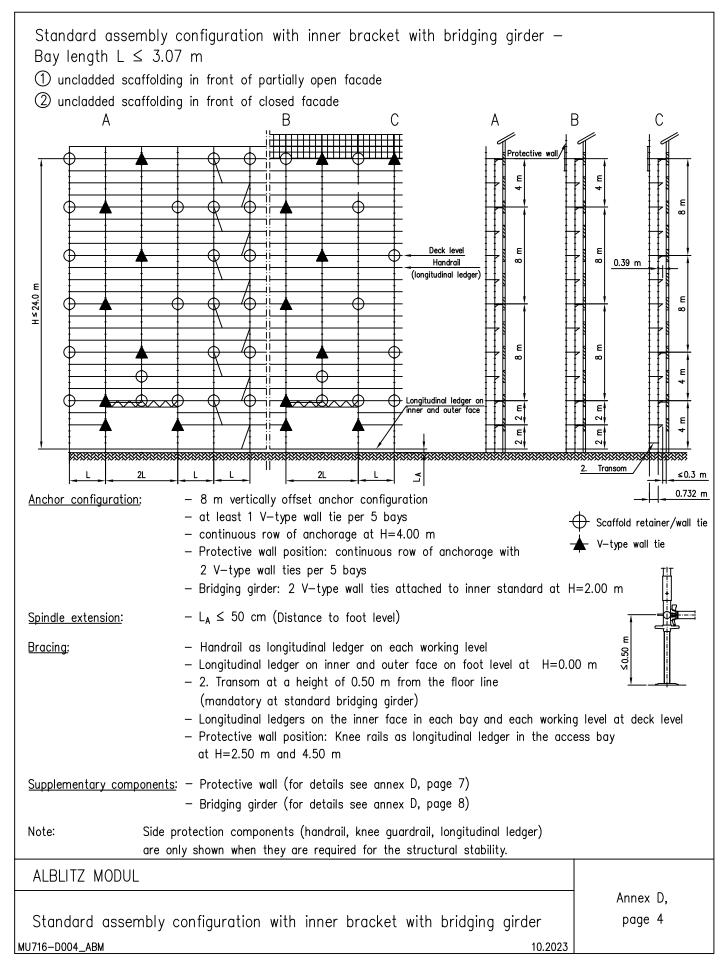
Side protection components (handrail, knee guardrail, longitudinal ledger) are only shown when they are required for the structural stability.

| ALBLITZ MODUL | |
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| | Annex D, |
| Standard assembly configuration with inner bracket | page 2 |
| MU716-D002_ABM 10.2023 | |

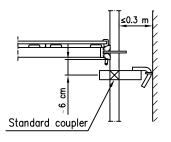
Scaffold retainer/wall tie

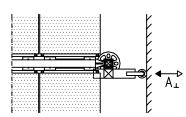
V—type wall tie

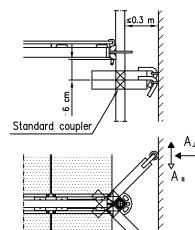
Standard assembly configuration without inner bracket with bridging girder — Bay length $L \leq 3.07$ m ① uncladded scaffolding in front of partially open facade ② uncladded scaffolding in front of closed facade Protective wall Handrail (Ionaitudinal ledger) H < 24.0 m ongitudinal ledger on and outer ≤0.3 m 0.732 m Anchor configuration: - 8 m vertically offset anchor configuration Scaffold retainer/wall tie - at least 1 V-type wall tie per 5 bays V-type wall tie - continuous row of anchorage at H=4.00 m - Protective wall position: continuous row of anchorage with 2 V-type wall ties per 5 bays $-L_A \le 50$ cm (Distance to foot level) Spindle extension: - Handrail as longitudinal ledger on each working level **Bracing**: - Longitudinal ledger on inner and outer face on foot level at H=0.00 m - Protective wall (for details see annex D, page 7) Supplementary components: - Bridging girder (for details see annex D, page 8) Note: Side protection components (handrail, knee guardrail, longitudinal ledger) are only shown when they are required for the structural stability. ALBLITZ MODUL Annex D. Standard assembly configuration without inner bracket with bridging girder page 3 MU716-D003_ABM

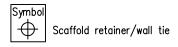


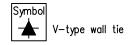
Standard assembly configuration: Detailed view — Anchoring 1 — Scaffolding without inner bracket











Anchor forces $A_{\perp}\, and\, A_{\shortparallel}\, see$ Annex C, Table C.2

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Standard assembly configuration: Detailed view - Anchoring 1 -

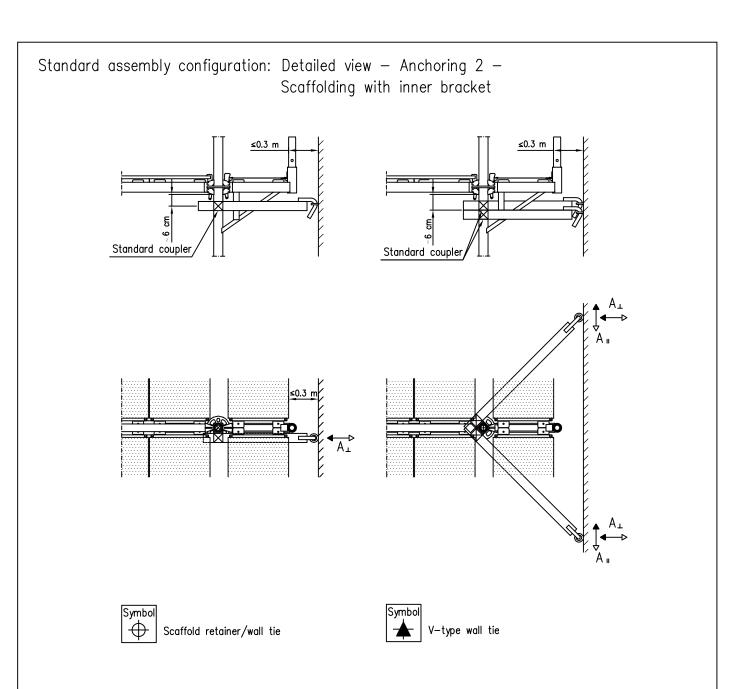
.

Scaffolding without inner bracket

page 5

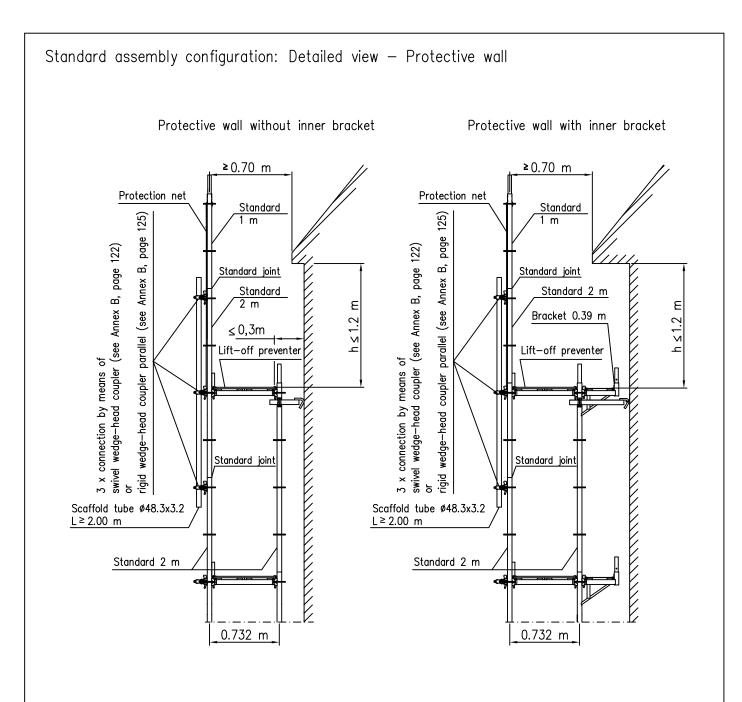
Annex D,

MU716-D005_ABM



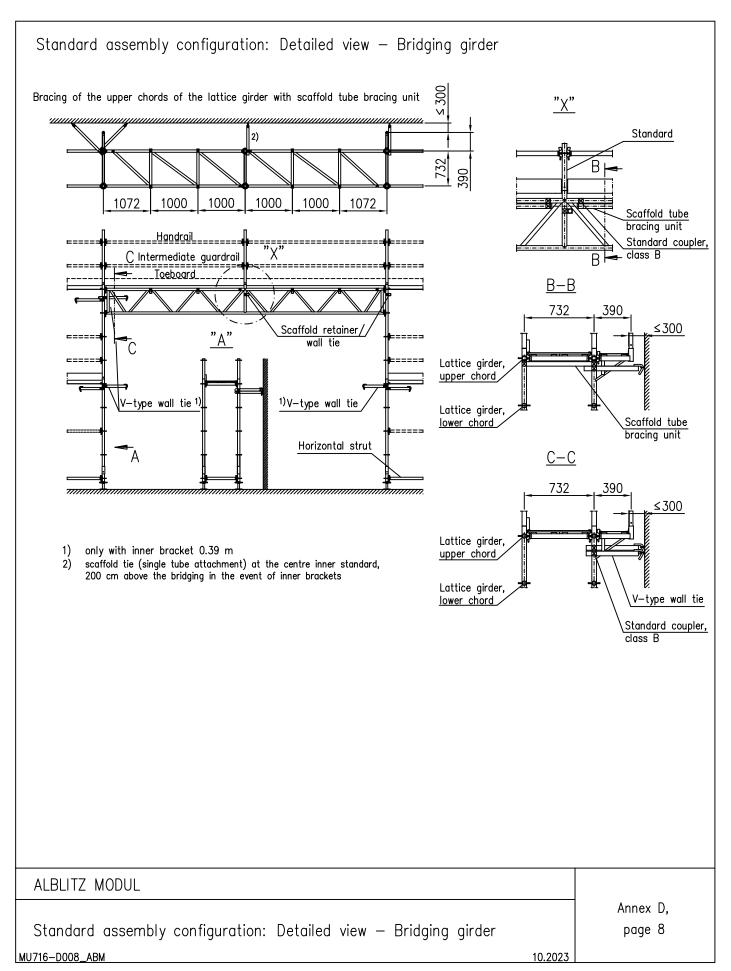
Anchor forces A_{\perp} and A_{\parallel} see Annex C, Table C.2

| ALBLITZ MODUL | |
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| Standard assembly configuration: Detailed view — Anchoring 2 — Scaffolding with inner bracket | Annex D, page 6 |
| MU716-D006_ABM Scarrotating with little bracket | 023 |



Note: Longitudinal ledger on the outer face at deck level for threading the net

| ALBLITZ MODUL | |
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| Standard assembly configuration: Detailed view — Protective wall | Annex D, page 7 |
| MU716-D007_ABM 10 | 0.2023 |



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